



PRELIMINARY HYDROLOGY AND HYDRAULIC REPORT

FREEMAN SITE

Anaheim, California

Prepared For
The Shopoff Realty Investments. L.P.

Prepared By:

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Date Prepared:
January 30, 2017
Revised May 29, 2017
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Job Number: 1293.008.02



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1.0 INTRODUCTION

1.1 GEOGRAPHIC SETTING

The proposed 20.5 +/- acre project site is located on the northerly of South Street and westerly of East Street in the City of Anaheim, CA. The project is bounded by commercial development on the northeast, rail right of way on the southwest, South Street on the southeast and Santa Ana Street on the northwest. The site is currently commercial warehouse spaces and associated parking.

1.2 PROJECT DESCRIPTION

The proposed project consists of residential condominiums units and an apartment development. Private drives and auto courts will serve the condominiums and a parking structure will serve the apartment project. The project main entries are from South Street and Santa Ana Street. Condominium buildings are multi-story, townhome style with multiple units per building. Common use recreation areas are provided for use by the residents and their guests.

1.3 PURPOSE OF THIS REPORT

The purpose of this report is to accomplish the following objectives:

To determine storm water discharges generated within the project area for determination of design feasibility and constructability. (See Appendix 2&3).

To demonstrate that the "storm water" and "flood" protection goals as outlined in Addendum No. 1 to the O.C. Design Manual can be met.

To determine if detention is required for the project.

1.4 REFERENCES

- Orange County Hydrology Manual, 1986
- Orange County Local Drainage Manual, 1986

Project Site Location Map



2.0 EXISTING TOPOGRAPHIC & HYDROLOGIC CONDITIONS

2.1 EXISTING TOPOGRAPHY

The site is generally flat and gently slopes toward South Street from Santa Ana Street.

2.2 EXISTING ON-SITE DRAINAGE PATTERN AND FACILITIES

On site drainage follows the topography of the land, flowing from Santa Ana Street to South Street. A public storm drain system runs through the project starting at a catch basin in Santa Ana Street. In addition, a storm drain line picks up drainage from the adjacent storage yard to the northeast.

3.0 PROPOSED STORM DRAIN FACILITIES

The proposed development will maintain the historic drainage pattern and drain from Santa Ana Street to South Street. All site flows will be collected and directed to the proposed public storm drain system through the site. Catch basins are located at local low points within the site to capture subarea flows. Flow control junction structures will be installed on the storm drain system to meter the water quality flows to Modular Wetland Systems where the low flow will be treated prior to being released from the site. Water Quality is discussed in the project WQMP.

3.1

DETENTION

The proposed residential project consists of increased pervious areas as compared to the existing commercial development resulting in less runoff. The post development

project discharge is less than the existing site discharge for all storm events as shown in Section 8 of this report. Therefore, detention of proposed flows is not required.

3.2 WATER QUALITY

The proposed project water quality treatment facilities will consist of bio-filtration units located near each catch basin. These units will capture and treat the low flows as required by the SWQCB and the County of Orange. As storm flows increase, high flows will bypass these units and flow directly to the storm drain system. The water quality features are detailed in the project WQMP.

4.0 HYDROLOGY STUDY

4.1 STORM FREQUENCY

This study is intended to determine discharges for use in the design of storm drain lines. The 25 year event was chosen as the design criteria as required by the Orange County Hydrology Manual. Overflow paths are also provided due to the need to ensure the site is protected during the 100 year event. The project overflow point is located on South Street at the rail crossing as shown on the proposed hydrology map.

4.2 Hydrologic Soil Group

The project is located within Hydrologic Soil Group B. Soil Group B consists of soils having moderate infiltration rates when thoroughly wetted and consisting chiefly of moderately deep to deep, moderately well to well drained sandy loam soils with moderately fine to moderately coarse textures. These soils have a moderate rate of water transmission.

4.3 METHODOLOGY

This study was prepared in conformance with the Orange County Hydrology Manual.

A.E.S. Computer Software was utilized to compile the hydrologic data and to determine the peak discharges.

Copies of the computer print-outs for hydrologic results are included within this report for the 25 year return frequency storm. (Appendix 2-3)

In addition, the pre and post development discharges were determined for the 2, 10 and 100 year events. The results of the 2 year analyses will be used to determine the need for Hydromodification as required by the County of Orange.

5.0 HYDRAULICS

5.1 HYDROLOGY

The proposed discharge for this analysis was determined by a hydrology study included in this report as Appendix 3

5.2 STORM FREQUENCY

Consistent with the design criteria set forth in Section 4.1 of this report, a storm frequency of 25 year was used.

5.3 METHODOLOGY

The main line storm drain system was sized to convey the proposed condition discharge to the existing 72" line in South Street. As discussed with the City the hydraulic grade line control was set at 1' below flow line of curb and gutter in South Street. This approach was also used previously at the Colony Park project, also located in the City of Anaheim. (See Appendix 7).

The 100 year flow in Santa Ana Street was analyzed to confirm that the flow remains within the street right of way. No changes are proposed to the surface flow in South street. However, the drainage area to South Street adjacent to the project was analyzed as part of this report. Street flow analysis for Santa Ana & South Streets is included in Appendix 8.

6.0 DESIGN CRITERIA

The proposed storm drain system will be designed to be consistent with the following goals and guidelines as presented in the Orange County Hydrology and the Orange County Local Drainage Manuals. Some items listed below may not apply to this project.

All habitable buildings shall be protected from flooding during a 100-year frequency storm.

Offsite design storm frequency, subject to individual review by the controlling authority, should be in accordance with the O.C. Hydrology Manual.

Velocity should not exceed 20 FPS in a standard wall R.C.P.

On local streets, flow should not exceed top of curb, for a 10-year storm event, and in sump conditions, a 25-year storm event shall be used.

Maximum W.S. in CB's for design conditions shall be 0.5' below inlet (FL.) elevation.

Once water is picked up in a storm drain, it should remain in the system.

Pipe size may not be decreased downstream without the City's approval.

Branching of flow is not allowed.

Junction structures should be designed according to the Orange County Local Drainage Manual.

Fill Less Than 20 Feet over structures unless special design is approved.

Normal criteria for storm drain design shall be followed.

7.0 MASTER PLAN OF DRAINAGE

The project area lies within Drainage Area 19 of the City of Anaheim, Barber City Channel, Master Plan of Drainage dated June 2009. The project area is near the up-stream end of the drainage area as shown on the Drainage Area Map included within the Master Plan of Drainage. The existing 72" line in South Street, that the proposed project flows to, is not shown as deficient in capacity and no improvement to it is proposed in the Master Plan. The proposed project runoff is less than existing for all storm events and therefore, the existing 72" will continue perform without improvements as a result of this development. The Master Plan of Drainage for this area is included in Appendix 5.

8.0 RESULTS AND CONCLUSIONS

8.1 SUMMARY OF EXISTING AND PROPOSED FLOWS

Existing Conditions

	2 Year	10 Year	25 Year	100 Year
Discharge (cfs)	37.22	68.64	82.54	106.27
Time of Concentration (min)	14.44	13.88	15.02	14.77

Proposed Conditions

	2 Year	10 Year	25 Year	100 Year
Discharge (cfs)	36.41	68.05	80.92	105.75
Time of Concentration (min)	16.35	15.19	14.90	14.74

8.2 EXISTING DRAINAGE & FLOOD PLAIN STUDIES

The project is located in Flood Zone X per Flood Insurance Rate Map (FIRM) Number 06059C0134J, revised December 3, 2009. The project outside the limits of the 100 year flood plain.

8.3 CONCLUSIONS

The analysis provided in this report provides the following conclusions:

1. The project can be constructed and meet all requirements of the County of Orange with the construction of appropriate storm drain facilities.
2. Overflow protection, flowing to adjacent streets is provided.
3. No improvements are required to the existing storm drain facilities located in adjacent streets.

9.0 APPENDICES

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APPENDIX 1

SOILS MAP

**PROJECT SITE
SOIL TYPE B**



LEGEND

A	B	C	D	HYDROLOGIC SOIL GROUPS
<hr/>				HYDROLOGIC SOIL GROUP BOUNDARY

APPENDIX 2

EXISTING HYDROLOGY STUDIES

RATIONAL METHOD HYDROLOGY COMPUTER PROGRAM PACKAGE
(Reference: 1986 ORANGE COUNTY HYDROLOGY CRITERION)
(c) Copyright 1983-2013 Advanced Engineering Software (aes)
Ver. 20.0 Release Date: 06/01/2013 License ID 1654

Analysis prepared by:

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Irvine, CA 92606
949-474-1960

***** DESCRIPTION OF STUDY *****

* FREEMAN *
* EXISTING CONDITION *
* 2 YEAR ANALYSIS *

FILE NAME: FREX2.DAT
TIME/DATE OF STUDY: 08:58 05/29/2017

=====

USER SPECIFIED HYDROLOGY AND HYDRAULIC MODEL INFORMATION:

=====

--*TIME-OF-CONCENTRATION MODEL*--

USER SPECIFIED STORM EVENT (YEAR) = 2.00
SPECIFIED MINIMUM PIPE SIZE (INCH) = 12.00
SPECIFIED PERCENT OF GRADIENTS (DECIMAL) TO USE FOR FRICTION SLOPE = 0.95
DATA BANK RAINFALL USED
ANTECEDENT MOISTURE CONDITION (AMC) I ASSUMED FOR RATIONAL METHOD

USER-DEFINED STREET-SECTIONS FOR COUPLED PIPEFLOW AND STREETFLOW MODEL

NO.	HALF- WIDTH (FT)	CROWN TO CROSSFALL (FT)	STREET-CROSSFALL: IN- / OUT- / PARK- SIDE / SIDE / WAY	CURB HEIGHT (FT)	GUTTER-GEOMETRIES: WIDTH LIP HIKE (FT) (FT) (FT)	MANNING FACTOR (n)
1	24.0	19.0	0.020/0.020/0.020	0.67	2.00 0.0313 0.167	0.0150

GLOBAL STREET FLOW-DEPTH CONSTRAINTS:

1. Relative Flow-Depth = 0.67 FEET
as (Maximum Allowable Street Flow Depth) - (Top-of-Curb)
 2. (Depth)*(Velocity) Constraint = 6.0 (FT*FT/S)
- *SIZE PIPE WITH A FLOW CAPACITY GREATER THAN
OR EQUAL TO THE UPSTREAM TRIBUTARY PIPE.*
*USER-SPECIFIED MINIMUM TOPOGRAPHIC SLOPE ADJUSTMENT NOT SELECTED

FLOW PROCESS FROM NODE 10.00 TO NODE 32.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<
=====

INITIAL SUBAREA FLOW-LENGTH (FEET) = 265.00
ELEVATION DATA: UPSTREAM (FEET) = 167.50 DOWNSTREAM (FEET) = 166.30

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$

SUBAREA ANALYSIS USED MINIMUM T_c (MIN.) = 8.337

* 2 YEAR RAINFALL INTENSITY (INCH/HR) = 1.688

SUBAREA T_c AND LOSS RATE DATA (AMC I):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	F_p (INCH/HR)	A_p (DECIMAL)	SCS CN	T_c (MIN.)
COMMERCIAL	B	0.15	0.30	0.100	36	8.34

SUBAREA AVERAGE PERVIOUS LOSS RATE, F_p (INCH/HR) = 0.30

SUBAREA AVERAGE PERVIOUS AREA FRACTION, A_p = 0.100

SUBAREA RUNOFF (CFS) = 0.22

TOTAL AREA (ACRES) = 0.15 PEAK FLOW RATE (CFS) = 0.22

FLOW PROCESS FROM NODE 32.00 TO NODE 11.00 IS CODE = 61

>>>>COMPUTE STREET FLOW TRAVEL TIME THRU SUBAREA<<<<<

>>>>(STANDARD CURB SECTION USED)<<<<<
=====

UPSTREAM ELEVATION (FEET) = 166.30 DOWNSTREAM ELEVATION (FEET) = 165.10

STREET LENGTH (FEET) = 210.00 CURB HEIGHT (INCHES) = 6.0

STREET HALFWIDTH (FEET) = 25.00

DISTANCE FROM CROWN TO CROSSFALL GRADEBREAK (FEET) = 10.00

INSIDE STREET CROSSFALL (DECIMAL) = 0.020

OUTSIDE STREET CROSSFALL (DECIMAL) = 0.020

SPECIFIED NUMBER OF HALFSTREETS CARRYING RUNOFF = 1

STREET PARKWAY CROSSFALL (DECIMAL) = 0.020

Manning's FRICTION FACTOR for Streetflow Section (curb-to-curb) = 0.0150

Manning's FRICTION FACTOR for Back-of-Walk Flow Section = 0.0200

**TRAVEL TIME COMPUTED USING ESTIMATED FLOW (CFS) = 0.30

STREETFLOW MODEL RESULTS USING ESTIMATED FLOW:

STREET FLOW DEPTH (FEET) = 0.20

HALFSTREET FLOOD WIDTH (FEET) = 3.58

AVERAGE FLOW VELOCITY (FEET/SEC.) = 1.21

PRODUCT OF DEPTH&VELOCITY (FT*FT/SEC.) = 0.24

STREET FLOW TRAVEL TIME (MIN.) = 2.88 T_c (MIN.) = 11.22

* 2 YEAR RAINFALL INTENSITY (INCH/HR) = 1.423

SUBAREA LOSS RATE DATA (AMC I):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	F_p (INCH/HR)	A_p (DECIMAL)	SCS CN
COMMERCIAL	B	0.12	0.30	0.100	36

SUBAREA AVERAGE PERVIOUS LOSS RATE, F_p (INCH/HR) = 0.30

SUBAREA AVERAGE PERVIOUS AREA FRACTION, A_p = 0.100

SUBAREA AREA (ACRES) = 0.12 SUBAREA RUNOFF (CFS) = 0.15

EFFECTIVE AREA (ACRES) = 0.27 AREA-AVERAGED F_m (INCH/HR) = 0.03

AREA-AVERAGED F_p (INCH/HR) = 0.30 AREA-AVERAGED A_p = 0.10
TOTAL AREA (ACRES) = 0.3 PEAK FLOW RATE (CFS) = 0.34

END OF SUBAREA STREET FLOW HYDRAULICS:

DEPTH (FEET) = 0.21 HALFSTREET FLOOD WIDTH (FEET) = 4.00
FLOW VELOCITY (FEET/SEC.) = 1.22 DEPTH*VELOCITY (FT*FT/SEC.) = 0.25
LONGEST FLOWPATH FROM NODE 10.00 TO NODE 11.00 = 475.00 FEET.

FLOW PROCESS FROM NODE 11.00 TO NODE 11.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

=====

TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION (MIN.) = 11.22
RAINFALL INTENSITY (INCH/HR) = 1.42
AREA-AVERAGED F_m (INCH/HR) = 0.03
AREA-AVERAGED F_p (INCH/HR) = 0.30
AREA-AVERAGED A_p = 0.10
EFFECTIVE STREAM AREA (ACRES) = 0.27
TOTAL STREAM AREA (ACRES) = 0.27
PEAK FLOW RATE (CFS) AT CONFLUENCE = 0.34

FLOW PROCESS FROM NODE 12.00 TO NODE 11.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

=====

INITIAL SUBAREA FLOW-LENGTH (FEET) = 210.00
ELEVATION DATA: UPSTREAM (FEET) = 166.00 DOWNSTREAM (FEET) = 165.10

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$
SUBAREA ANALYSIS USED MINIMUM T_c (MIN.) = 7.680
* 2 YEAR RAINFALL INTENSITY (INCH/HR) = 1.769
SUBAREA T_c AND LOSS RATE DATA (AMC I):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	F_p (INCH/HR)	A_p (DECIMAL)	SCS CN	T_c (MIN.)
COMMERCIAL	B	0.76	0.30	0.100	36	7.68

SUBAREA AVERAGE PERVIOUS LOSS RATE, F_p (INCH/HR) = 0.30
SUBAREA AVERAGE PERVIOUS AREA FRACTION, A_p = 0.100
SUBAREA RUNOFF (CFS) = 1.19
TOTAL AREA (ACRES) = 0.76 PEAK FLOW RATE (CFS) = 1.19

FLOW PROCESS FROM NODE 11.00 TO NODE 11.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<

=====

TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
TIME OF CONCENTRATION (MIN.) = 7.68

RAINFALL INTENSITY (INCH/HR) = 1.77
 AREA-AVERAGED Fm (INCH/HR) = 0.03
 AREA-AVERAGED Fp (INCH/HR) = 0.30
 AREA-AVERAGED Ap = 0.10
 EFFECTIVE STREAM AREA (ACRES) = 0.76
 TOTAL STREAM AREA (ACRES) = 0.76
 PEAK FLOW RATE (CFS) AT CONFLUENCE = 1.19

** CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp (Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	0.34	11.22	1.423	0.30 (0.03)	0.10	0.3	10.00
2	1.19	7.68	1.769	0.30 (0.03)	0.10	0.8	12.00

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
 CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp (Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	1.48	7.68	1.769	0.30 (0.03)	0.10	0.9	12.00
2	1.29	11.22	1.423	0.30 (0.03)	0.10	1.0	10.00

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE (CFS) = 1.48 Tc (MIN.) = 7.68
 EFFECTIVE AREA (ACRES) = 0.94 AREA-AVERAGED Fm (INCH/HR) = 0.03
 AREA-AVERAGED Fp (INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.10
 TOTAL AREA (ACRES) = 1.0
 LONGEST FLOWPATH FROM NODE 10.00 TO NODE 11.00 = 475.00 FEET.

FLOW PROCESS FROM NODE 11.00 TO NODE 13.00 IS CODE = 62

>>>>>COMPUTE STREET FLOW TRAVEL TIME THRU SUBAREA<<<<<<
 >>>>>(STREET TABLE SECTION # 1 USED)<<<<<<

UPSTREAM ELEVATION (FEET) = 165.10 DOWNSTREAM ELEVATION (FEET) = 164.20
 STREET LENGTH (FEET) = 455.00 CURB HEIGHT (INCHES) = 8.0
 STREET HALFWIDTH (FEET) = 24.00

DISTANCE FROM CROWN TO CROSSFALL GRADEBREAK (FEET) = 19.00
 INSIDE STREET CROSSFALL (DECIMAL) = 0.020
 OUTSIDE STREET CROSSFALL (DECIMAL) = 0.020

SPECIFIED NUMBER OF HALFSTREETS CARRYING RUNOFF = 1
 STREET PARKWAY CROSSFALL (DECIMAL) = 0.020
 Manning's FRICTION FACTOR for Streetflow Section (curb-to-curb) = 0.0150
 Manning's FRICTION FACTOR for Back-of-Walk Flow Section = 0.0200

**TRAVEL TIME COMPUTED USING ESTIMATED FLOW (CFS) = 1.67
 STREETFLOW MODEL RESULTS USING ESTIMATED FLOW:
 STREET FLOW DEPTH (FEET) = 0.39
 HALFSTREET FLOOD WIDTH (FEET) = 11.49
 AVERAGE FLOW VELOCITY (FEET/SEC.) = 1.11

PRODUCT OF DEPTH&VELOCITY(FT*FT/SEC.) = 0.43
 STREET FLOW TRAVEL TIME(MIN.) = 6.84 Tc(MIN.) = 14.52
 * 2 YEAR RAINFALL INTENSITY(INCH/HR) = 1.227
 SUBAREA LOSS RATE DATA(AMC I):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
COMMERCIAL	B	0.36	0.30	0.100	36

 SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.30
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.100
 SUBAREA AREA(ACRES) = 0.36 SUBAREA RUNOFF(CFS) = 0.39
 EFFECTIVE AREA(ACRES) = 1.30 AREA-AVERAGED Fm(INCH/HR) = 0.03
 AREA-AVERAGED Fp(INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.10
 TOTAL AREA(ACRES) = 1.4 PEAK FLOW RATE(CFS) = 1.48
 NOTE: PEAK FLOW RATE DEFAULTED TO UPSTREAM VALUE

END OF SUBAREA STREET FLOW HYDRAULICS:
 DEPTH(FEET) = 0.37 HALFSTREET FLOOD WIDTH(FEET) = 10.83
 FLOW VELOCITY(FEET/SEC.) = 1.09 DEPTH*VELOCITY(FT*FT/SEC.) = 0.41
 LONGEST FLOWPATH FROM NODE 10.00 TO NODE 13.00 = 930.00 FEET.

FLOW PROCESS FROM NODE 13.00 TO NODE 13.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

=====
 TOTAL NUMBER OF STREAMS = 2
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
 TIME OF CONCENTRATION(MIN.) = 14.52
 RAINFALL INTENSITY(INCH/HR) = 1.23
 AREA-AVERAGED Fm(INCH/HR) = 0.03
 AREA-AVERAGED Fp(INCH/HR) = 0.30
 AREA-AVERAGED Ap = 0.10
 EFFECTIVE STREAM AREA(ACRES) = 1.30
 TOTAL STREAM AREA(ACRES) = 1.39
 PEAK FLOW RATE(CFS) AT CONFLUENCE = 1.48

FLOW PROCESS FROM NODE 14.00 TO NODE 15.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
 >>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

=====
 INITIAL SUBAREA FLOW-LENGTH(FEET) = 125.00
 ELEVATION DATA: UPSTREAM(FEET) = 164.60 DOWNSTREAM(FEET) = 164.20

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$
 SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) = 6.616
 * 2 YEAR RAINFALL INTENSITY(INCH/HR) = 1.928
 SUBAREA Tc AND LOSS RATE DATA(AMC I):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN	Tc (MIN.)
COMMERCIAL	B	0.15	0.30	0.100	36	6.62

 SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.30
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.100

SUBAREA RUNOFF(CFS) = 0.26
 TOTAL AREA(ACRES) = 0.15 PEAK FLOW RATE(CFS) = 0.26

FLOW PROCESS FROM NODE 15.00 TO NODE 13.00 IS CODE = 41

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
 >>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 159.30 DOWNSTREAM(FEET) = 159.20
 FLOW LENGTH(FEET) = 50.00 MANNING'S N = 0.013
 DEPTH OF FLOW IN 18.0 INCH PIPE IS 2.9 INCHES
 PIPE-FLOW VELOCITY(FEET/SEC.) = 1.41
 GIVEN PIPE DIAMETER(INCH) = 18.00 NUMBER OF PIPES = 1
 PIPE-FLOW(CFS) = 0.26
 PIPE TRAVEL TIME(MIN.) = 0.59 Tc(MIN.) = 7.21
 LONGEST FLOWPATH FROM NODE 14.00 TO NODE 13.00 = 175.00 FEET.

FLOW PROCESS FROM NODE 13.00 TO NODE 13.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
 >>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<

=====

TOTAL NUMBER OF STREAMS = 2
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
 TIME OF CONCENTRATION(MIN.) = 7.21
 RAINFALL INTENSITY(INCH/HR) = 1.83
 AREA-AVERAGED Fm(INCH/HR) = 0.03
 AREA-AVERAGED Fp(INCH/HR) = 0.30
 AREA-AVERAGED Ap = 0.10
 EFFECTIVE STREAM AREA(ACRES) = 0.15
 TOTAL STREAM AREA(ACRES) = 0.15
 PEAK FLOW RATE(CFS) AT CONFLUENCE = 0.26

** CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	1.48	14.52	1.227	0.30 (0.03)	0.10	1.3	12.00
1	1.31	18.29	1.075	0.30 (0.03)	0.10	1.4	10.00
2	0.26	7.21	1.835	0.30 (0.03)	0.10	0.2	14.00

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
 CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	1.36	7.21	1.835	0.30 (0.03)	0.10	0.8	14.00
2	1.65	14.52	1.227	0.30 (0.03)	0.10	1.5	12.00
3	1.46	18.29	1.075	0.30 (0.03)	0.10	1.5	10.00

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
 PEAK FLOW RATE(CFS) = 1.65 Tc(MIN.) = 14.52

EFFECTIVE AREA(ACRES) = 1.45 AREA-AVERAGED Fm(INCH/HR) = 0.03
AREA-AVERAGED Fp(INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.10
TOTAL AREA(ACRES) = 1.5
LONGEST FLOWPATH FROM NODE 10.00 TO NODE 13.00 = 930.00 FEET.

FLOW PROCESS FROM NODE 13.00 TO NODE 16.00 IS CODE = 41

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 159.20 DOWNSTREAM(FEET) = 158.10
FLOW LENGTH(FEET) = 285.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 18.0 INCH PIPE IS 6.3 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 3.02
GIVEN PIPE DIAMETER(INCH) = 18.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 1.65
PIPE TRAVEL TIME(MIN.) = 1.57 Tc(MIN.) = 16.09
LONGEST FLOWPATH FROM NODE 10.00 TO NODE 16.00 = 1215.00 FEET.

FLOW PROCESS FROM NODE 16.00 TO NODE 16.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

=====

TOTAL NUMBER OF STREAMS = 3
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION(MIN.) = 16.09
RAINFALL INTENSITY(INCH/HR) = 1.16
AREA-AVERAGED Fm(INCH/HR) = 0.03
AREA-AVERAGED Fp(INCH/HR) = 0.30
AREA-AVERAGED Ap = 0.10
EFFECTIVE STREAM AREA(ACRES) = 1.45
TOTAL STREAM AREA(ACRES) = 1.54
PEAK FLOW RATE(CFS) AT CONFLUENCE = 1.65

FLOW PROCESS FROM NODE 17.00 TO NODE 16.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

=====

INITIAL SUBAREA FLOW-LENGTH(FEET) = 310.00
ELEVATION DATA: UPSTREAM(FEET) = 166.00 DOWNSTREAM(FEET) = 164.90

Tc = K*[(LENGTH** 3.00)/(ELEVATION CHANGE)]**0.20
SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) = 9.320
* 2 YEAR RAINFALL INTENSITY(INCH/HR) = 1.583
SUBAREA Tc AND LOSS RATE DATA(AMC I):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN	Tc (MIN.)
COMMERCIAL	B	1.52	0.30	0.100	36	9.32

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.30
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.100

SUBAREA RUNOFF(CFS) = 2.13
TOTAL AREA(ACRES) = 1.52 PEAK FLOW RATE(CFS) = 2.13

FLOW PROCESS FROM NODE 16.00 TO NODE 16.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

=====

TOTAL NUMBER OF STREAMS = 3
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
TIME OF CONCENTRATION(MIN.) = 9.32
RAINFALL INTENSITY(INCH/HR) = 1.58
AREA-AVERAGED Fm(INCH/HR) = 0.03
AREA-AVERAGED Fp(INCH/HR) = 0.30
AREA-AVERAGED Ap = 0.10
EFFECTIVE STREAM AREA(ACRES) = 1.52
TOTAL STREAM AREA(ACRES) = 1.52
PEAK FLOW RATE(CFS) AT CONFLUENCE = 2.13

FLOW PROCESS FROM NODE 18.00 TO NODE 19.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

=====

INITIAL SUBAREA FLOW-LENGTH(FEET) = 320.00
ELEVATION DATA: UPSTREAM(FEET) = 165.90 DOWNSTREAM(FEET) = 165.00

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$
SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) = 9.888
* 2 YEAR RAINFALL INTENSITY(INCH/HR) = 1.530
SUBAREA Tc AND LOSS RATE DATA(AMC I):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN	Tc (MIN.)
COMMERCIAL	B	1.26	0.30	0.100	36	9.89

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.30
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.100
SUBAREA RUNOFF(CFS) = 1.70
TOTAL AREA(ACRES) = 1.26 PEAK FLOW RATE(CFS) = 1.70

FLOW PROCESS FROM NODE 19.00 TO NODE 16.00 IS CODE = 51

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<<
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 165.00 DOWNSTREAM(FEET) = 164.90
CHANNEL LENGTH THRU SUBAREA(FEET) = 265.00 CHANNEL SLOPE = 0.0004
CHANNEL BASE(FEET) = 3.00 "Z" FACTOR = 0.040
MANNING'S FACTOR = 0.015 MAXIMUM DEPTH(FEET) = 1.00
CHANNEL FLOW THRU SUBAREA(CFS) = 1.70
FLOW VELOCITY(FEET/SEC.) = 1.05 FLOW DEPTH(FEET) = 0.54
TRAVEL TIME(MIN.) = 4.23 Tc(MIN.) = 14.11
LONGEST FLOWPATH FROM NODE 18.00 TO NODE 16.00 = 585.00 FEET.

 FLOW PROCESS FROM NODE 16.00 TO NODE 16.00 IS CODE = 1

 >>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
 >>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<
 =====

TOTAL NUMBER OF STREAMS = 3
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 3 ARE:
 TIME OF CONCENTRATION(MIN.) = 14.11
 RAINFALL INTENSITY(INCH/HR) = 1.25
 AREA-AVERAGED Fm(INCH/HR) = 0.03
 AREA-AVERAGED Fp(INCH/HR) = 0.30
 AREA-AVERAGED Ap = 0.10
 EFFECTIVE STREAM AREA(ACRES) = 1.26
 TOTAL STREAM AREA(ACRES) = 1.26
 PEAK FLOW RATE(CFS) AT CONFLUENCE = 1.70

** CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	1.36	8.86	1.630	0.30 (0.03)	0.10	0.8	14.00
1	1.65	16.09	1.157	0.30 (0.03)	0.10	1.5	12.00
1	1.46	19.91	1.024	0.30 (0.03)	0.10	1.5	10.00
2	2.13	9.32	1.583	0.30 (0.03)	0.10	1.5	17.00
3	1.70	14.11	1.248	0.30 (0.03)	0.10	1.3	18.00

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
 CONFLUENCE FORMULA USED FOR 3 STREAMS.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	4.85	8.86	1.630	0.30 (0.03)	0.10	3.0	14.00
2	4.94	9.32	1.583	0.30 (0.03)	0.10	3.2	17.00
3	4.98	14.11	1.248	0.30 (0.03)	0.10	4.1	18.00
4	4.77	16.09	1.157	0.30 (0.03)	0.10	4.2	12.00
5	4.21	19.91	1.024	0.30 (0.03)	0.10	4.3	10.00

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE(CFS) = 4.98 Tc(MIN.) = 14.11
 EFFECTIVE AREA(ACRES) = 4.05 AREA-AVERAGED Fm(INCH/HR) = 0.03
 AREA-AVERAGED Fp(INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.10
 TOTAL AREA(ACRES) = 4.3
 LONGEST FLOWPATH FROM NODE 10.00 TO NODE 16.00 = 1215.00 FEET.

 FLOW PROCESS FROM NODE 16.00 TO NODE 20.00 IS CODE = 41

 >>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
 >>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<
 =====

ELEVATION DATA: UPSTREAM(FEET) = 158.10 DOWNSTREAM(FEET) = 157.60
 FLOW LENGTH(FEET) = 150.00 MANNING'S N = 0.013

DEPTH OF FLOW IN 24.0 INCH PIPE IS 10.4 INCHES
 PIPE-FLOW VELOCITY (FEET/SEC.) = 3.81
 GIVEN PIPE DIAMETER (INCH) = 24.00 NUMBER OF PIPES = 1
 PIPE-FLOW (CFS) = 4.98
 PIPE TRAVEL TIME (MIN.) = 0.66 Tc (MIN.) = 14.77
 LONGEST FLOWPATH FROM NODE 10.00 TO NODE 20.00 = 1365.00 FEET.

FLOW PROCESS FROM NODE 20.00 TO NODE 20.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

=====

TOTAL NUMBER OF STREAMS = 2
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
 TIME OF CONCENTRATION (MIN.) = 14.77
 RAINFALL INTENSITY (INCH/HR) = 1.22
 AREA-AVERAGED Fm (INCH/HR) = 0.03
 AREA-AVERAGED Fp (INCH/HR) = 0.30
 AREA-AVERAGED Ap = 0.10
 EFFECTIVE STREAM AREA (ACRES) = 4.05
 TOTAL STREAM AREA (ACRES) = 4.32
 PEAK FLOW RATE (CFS) AT CONFLUENCE = 4.98

FLOW PROCESS FROM NODE 21.00 TO NODE 22.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
 >>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

=====

INITIAL SUBAREA FLOW-LENGTH (FEET) = 260.00
 ELEVATION DATA: UPSTREAM (FEET) = 166.00 DOWNSTREAM (FEET) = 165.50

Tc = K * [(LENGTH** 3.00) / (ELEVATION CHANGE)]**0.20
 SUBAREA ANALYSIS USED MINIMUM Tc (MIN.) = 9.819
 * 2 YEAR RAINFALL INTENSITY (INCH/HR) = 1.537
 SUBAREA Tc AND LOSS RATE DATA (AMC I):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN	Tc (MIN.)
COMMERCIAL	B	0.64	0.30	0.100	36	9.82

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp (INCH/HR) = 0.30
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.100
 SUBAREA RUNOFF (CFS) = 0.87
 TOTAL AREA (ACRES) = 0.64 PEAK FLOW RATE (CFS) = 0.87

FLOW PROCESS FROM NODE 22.00 TO NODE 20.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
 >>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<

=====

ELEVATION DATA: UPSTREAM (FEET) = 158.20 DOWNSTREAM (FEET) = 157.60
 FLOW LENGTH (FEET) = 20.00 MANNING'S N = 0.013
 ESTIMATED PIPE DIAMETER (INCH) INCREASED TO 12.000
 DEPTH OF FLOW IN 12.0 INCH PIPE IS 3.1 INCHES

PIPE-FLOW VELOCITY (FEET/SEC.) = 5.46
 ESTIMATED PIPE DIAMETER (INCH) = 12.00 NUMBER OF PIPES = 1
 PIPE-FLOW (CFS) = 0.87
 PIPE TRAVEL TIME (MIN.) = 0.06 Tc (MIN.) = 9.88
 LONGEST FLOWPATH FROM NODE 21.00 TO NODE 20.00 = 280.00 FEET.

FLOW PROCESS FROM NODE 20.00 TO NODE 20.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
 >>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<

=====
 TOTAL NUMBER OF STREAMS = 2
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
 TIME OF CONCENTRATION (MIN.) = 9.88
 RAINFALL INTENSITY (INCH/HR) = 1.53
 AREA-AVERAGED Fm (INCH/HR) = 0.03
 AREA-AVERAGED Fp (INCH/HR) = 0.30
 AREA-AVERAGED Ap = 0.10
 EFFECTIVE STREAM AREA (ACRES) = 0.64
 TOTAL STREAM AREA (ACRES) = 0.64
 PEAK FLOW RATE (CFS) AT CONFLUENCE = 0.87

** CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp (Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	4.85	9.53	1.564	0.30 (0.03)	0.10	3.0	14.00
1	4.94	9.98	1.523	0.30 (0.03)	0.10	3.2	17.00
1	4.98	14.77	1.216	0.30 (0.03)	0.10	4.1	18.00
1	4.77	16.76	1.131	0.30 (0.03)	0.10	4.2	12.00
1	4.21	20.60	1.004	0.30 (0.03)	0.10	4.3	10.00
2	0.87	9.88	1.531	0.30 (0.03)	0.10	0.6	21.00

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
 CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp (Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	5.70	9.53	1.564	0.30 (0.03)	0.10	3.7	14.00
2	5.79	9.88	1.531	0.30 (0.03)	0.10	3.8	21.00
3	5.80	9.98	1.523	0.30 (0.03)	0.10	3.8	17.00
4	5.67	14.77	1.216	0.30 (0.03)	0.10	4.7	18.00
5	5.40	16.76	1.131	0.30 (0.03)	0.10	4.9	12.00
6	4.77	20.60	1.004	0.30 (0.03)	0.10	5.0	10.00

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE (CFS) = 5.80 Tc (MIN.) = 9.98
 EFFECTIVE AREA (ACRES) = 3.83 AREA-AVERAGED Fm (INCH/HR) = 0.03
 AREA-AVERAGED Fp (INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.10
 TOTAL AREA (ACRES) = 5.0
 LONGEST FLOWPATH FROM NODE 10.00 TO NODE 20.00 = 1365.00 FEET.

FLOW PROCESS FROM NODE 20.00 TO NODE 23.00 IS CODE = 41

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

ELEVATION DATA: UPSTREAM(FEET) = 157.60 DOWNSTREAM(FEET) = 156.40
FLOW LENGTH(FEET) = 365.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 24.0 INCH PIPE IS 11.4 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 3.94
GIVEN PIPE DIAMETER(INCH) = 24.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 5.80
PIPE TRAVEL TIME(MIN.) = 1.54 Tc(MIN.) = 11.52
LONGEST FLOWPATH FROM NODE 10.00 TO NODE 23.00 = 1730.00 FEET.

FLOW PROCESS FROM NODE 23.00 TO NODE 23.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION(MIN.) = 11.52
RAINFALL INTENSITY(INCH/HR) = 1.40
AREA-AVERAGED Fm(INCH/HR) = 0.03
AREA-AVERAGED Fp(INCH/HR) = 0.30
AREA-AVERAGED Ap = 0.10
EFFECTIVE STREAM AREA(ACRES) = 3.83
TOTAL STREAM AREA(ACRES) = 4.96
PEAK FLOW RATE(CFS) AT CONFLUENCE = 5.80

FLOW PROCESS FROM NODE 24.00 TO NODE 25.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

INITIAL SUBAREA FLOW-LENGTH(FEET) = 320.00
ELEVATION DATA: UPSTREAM(FEET) = 166.00 DOWNSTREAM(FEET) = 165.50

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$
SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) = 11.122
* 2 YEAR RAINFALL INTENSITY(INCH/HR) = 1.431
SUBAREA Tc AND LOSS RATE DATA(AMC I):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN	Tc (MIN.)
COMMERCIAL	B	1.40	0.30	0.100	36	11.12

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.30
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.100
SUBAREA RUNOFF(CFS) = 1.76
TOTAL AREA(ACRES) = 1.40 PEAK FLOW RATE(CFS) = 1.76

FLOW PROCESS FROM NODE 25.00 TO NODE 26.00 IS CODE = 51

>>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<<
>>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 165.50 DOWNSTREAM(FEET) = 161.10
CHANNEL LENGTH THRU SUBAREA(FEET) = 330.00 CHANNEL SLOPE = 0.0133
CHANNEL BASE(FEET) = 20.00 "Z" FACTOR = 0.020
MANNING'S FACTOR = 0.015 MAXIMUM DEPTH(FEET) = 2.00
* 2 YEAR RAINFALL INTENSITY(INCH/HR) = 1.275
SUBAREA LOSS RATE DATA(AMC I):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
COMMERCIAL	B	3.87	0.30	0.100	36

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.30
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.100
TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 3.95
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) = 2.23
AVERAGE FLOW DEPTH(FEET) = 0.09 TRAVEL TIME(MIN.) = 2.47
Tc(MIN.) = 13.59
SUBAREA AREA(ACRES) = 3.87 SUBAREA RUNOFF(CFS) = 4.34
EFFECTIVE AREA(ACRES) = 5.27 AREA-AVERAGED Fm(INCH/HR) = 0.03
AREA-AVERAGED Fp(INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.10
TOTAL AREA(ACRES) = 5.3 PEAK FLOW RATE(CFS) = 5.90

END OF SUBAREA CHANNEL FLOW HYDRAULICS:

DEPTH(FEET) = 0.11 FLOW VELOCITY(FEET/SEC.) = 2.65
LONGEST FLOWPATH FROM NODE 24.00 TO NODE 26.00 = 650.00 FEET.

FLOW PROCESS FROM NODE 26.00 TO NODE 23.00 IS CODE = 41

>>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 158.00 DOWNSTREAM(FEET) = 156.40
FLOW LENGTH(FEET) = 315.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 24.0 INCH PIPE IS 10.2 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 4.65
GIVEN PIPE DIAMETER(INCH) = 24.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 5.90
PIPE TRAVEL TIME(MIN.) = 1.13 Tc(MIN.) = 14.72
LONGEST FLOWPATH FROM NODE 24.00 TO NODE 23.00 = 965.00 FEET.

FLOW PROCESS FROM NODE 23.00 TO NODE 23.00 IS CODE = 1

>>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
>>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<

=====

TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
TIME OF CONCENTRATION(MIN.) = 14.72
RAINFALL INTENSITY(INCH/HR) = 1.22
AREA-AVERAGED Fm(INCH/HR) = 0.03
AREA-AVERAGED Fp(INCH/HR) = 0.30

AREA-AVERAGED $A_p = 0.10$
 EFFECTIVE STREAM AREA (ACRES) = 5.27
 TOTAL STREAM AREA (ACRES) = 5.27
 PEAK FLOW RATE (CFS) AT CONFLUENCE = 5.90

** CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp (Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	5.70	11.08	1.434	0.30 (0.03)	0.10	3.7	14.00
1	5.79	11.43	1.409	0.30 (0.03)	0.10	3.8	21.00
1	5.80	11.52	1.402	0.30 (0.03)	0.10	3.8	17.00
1	5.67	16.32	1.148	0.30 (0.03)	0.10	4.7	18.00
1	5.40	18.33	1.074	0.30 (0.03)	0.10	4.9	12.00
1	4.77	22.23	0.961	0.30 (0.03)	0.10	5.0	10.00
2	5.90	14.72	1.218	0.30 (0.03)	0.10	5.3	24.00

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
 CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp (Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	10.95	11.08	1.434	0.30 (0.03)	0.10	7.6	14.00
2	11.11	11.43	1.409	0.30 (0.03)	0.10	7.9	21.00
3	11.14	11.52	1.402	0.30 (0.03)	0.10	8.0	17.00
4	11.62	14.72	1.218	0.30 (0.03)	0.10	9.7	24.00
5	11.23	16.32	1.148	0.30 (0.03)	0.10	10.0	18.00
6	10.59	18.33	1.074	0.30 (0.03)	0.10	10.1	12.00
7	9.40	22.23	0.961	0.30 (0.03)	0.10	10.2	10.00

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE (CFS) = 11.62 Tc (MIN.) = 14.72
 EFFECTIVE AREA (ACRES) = 9.68 AREA-AVERAGED Fm (INCH/HR) = 0.03
 AREA-AVERAGED Fp (INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.10
 TOTAL AREA (ACRES) = 10.2
 LONGEST FLOWPATH FROM NODE 10.00 TO NODE 23.00 = 1730.00 FEET.

 FLOW PROCESS FROM NODE 23.00 TO NODE 27.00 IS CODE = 41

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
 >>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM (FEET) = 156.40 DOWNSTREAM (FEET) = 155.40
 FLOW LENGTH (FEET) = 235.00 MANNING'S N = 0.013
 DEPTH OF FLOW IN 30.0 INCH PIPE IS 14.0 INCHES
 PIPE-FLOW VELOCITY (FEET/SEC.) = 5.16
 GIVEN PIPE DIAMETER (INCH) = 30.00 NUMBER OF PIPES = 1
 PIPE-FLOW (CFS) = 11.62
 PIPE TRAVEL TIME (MIN.) = 0.76 Tc (MIN.) = 15.48
 LONGEST FLOWPATH FROM NODE 10.00 TO NODE 27.00 = 1965.00 FEET.

 FLOW PROCESS FROM NODE 27.00 TO NODE 27.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

=====

TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION(MIN.) = 15.48
RAINFALL INTENSITY(INCH/HR) = 1.18
AREA-AVERAGED Fm(INCH/HR) = 0.03
AREA-AVERAGED Fp(INCH/HR) = 0.30
AREA-AVERAGED Ap = 0.10
EFFECTIVE STREAM AREA(ACRES) = 9.68
TOTAL STREAM AREA(ACRES) = 10.23
PEAK FLOW RATE(CFS) AT CONFLUENCE = 11.62

FLOW PROCESS FROM NODE 28.00 TO NODE 29.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

=====

INITIAL SUBAREA FLOW-LENGTH(FEET) = 225.00
ELEVATION DATA: UPSTREAM(FEET) = 170.00 DOWNSTREAM(FEET) = 166.50

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$

SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) = 6.101

* 2 YEAR RAINFALL INTENSITY(INCH/HR) = 2.019

SUBAREA Tc AND LOSS RATE DATA(AMC I):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN	Tc (MIN.)
COMMERCIAL	B	1.32	0.30	0.100	36	6.10

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.30

SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.100

SUBAREA RUNOFF(CFS) = 2.36

TOTAL AREA(ACRES) = 1.32 PEAK FLOW RATE(CFS) = 2.36

FLOW PROCESS FROM NODE 29.00 TO NODE 30.00 IS CODE = 51

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<<
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 166.50 DOWNSTREAM(FEET) = 164.20

CHANNEL LENGTH THRU SUBAREA(FEET) = 300.00 CHANNEL SLOPE = 0.0077

CHANNEL BASE(FEET) = 5.00 "Z" FACTOR = 1.500

MANNING'S FACTOR = 0.015 MAXIMUM DEPTH(FEET) = 2.00

* 2 YEAR RAINFALL INTENSITY(INCH/HR) = 1.777

SUBAREA LOSS RATE DATA(AMC I):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
COMMERCIAL	B	2.72	0.30	0.100	36

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.30

SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.100

TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 4.51

TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) = 3.29

AVERAGE FLOW DEPTH (FEET) = 0.25 TRAVEL TIME (MIN.) = 1.52
Tc (MIN.) = 7.62
SUBAREA AREA (ACRES) = 2.72 SUBAREA RUNOFF (CFS) = 4.28
EFFECTIVE AREA (ACRES) = 4.04 AREA-AVERAGED Fm (INCH/HR) = 0.03
AREA-AVERAGED Fp (INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.10
TOTAL AREA (ACRES) = 4.0 PEAK FLOW RATE (CFS) = 6.35

END OF SUBAREA CHANNEL FLOW HYDRAULICS:

DEPTH (FEET) = 0.32 FLOW VELOCITY (FEET/SEC.) = 3.68
LONGEST FLOWPATH FROM NODE 28.00 TO NODE 30.00 = 525.00 FEET.

FLOW PROCESS FROM NODE 30.00 TO NODE 30.00 IS CODE = 82

>>>>ADD SUBAREA RUNOFF TO MAINLINE, AT MAINLINE Tc,<<<<<
>>>>(AND COMPUTE INITIAL SUBAREA RUNOFF)<<<<<

=====

INITIAL SUBAREA FLOW-LENGTH (FEET) = 300.00
ELEVATION DATA: UPSTREAM (FEET) = 167.40 DOWNSTREAM (FEET) = 164.20

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$

SUBAREA ANALYSIS USED MINIMUM Tc (MIN.) = 7.381

* 2 YEAR RAINFALL INTENSITY (INCH/HR) = 1.810

SUBAREA Tc AND LOSS RATE DATA (AMC I):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN	Tc (MIN.)
COMMERCIAL	B	2.23	0.30	0.100	36	7.38

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp (INCH/HR) = 0.30

SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.100

SUBAREA AREA (ACRES) = 2.23 INITIAL SUBAREA RUNOFF (CFS) = 3.57

** ADD SUBAREA RUNOFF TO MAINLINE AT MAINLINE Tc:

MAINLINE Tc (MIN.) = 7.62

* 2 YEAR RAINFALL INTENSITY (INCH/HR) = 1.777

SUBAREA AREA (ACRES) = 2.23 SUBAREA RUNOFF (CFS) = 3.51

EFFECTIVE AREA (ACRES) = 6.27 AREA-AVERAGED Fm (INCH/HR) = 0.03

AREA-AVERAGED Fp (INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.10

TOTAL AREA (ACRES) = 6.3 PEAK FLOW RATE (CFS) = 9.86

FLOW PROCESS FROM NODE 30.00 TO NODE 27.00 IS CODE = 41

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM (FEET) = 158.00 DOWNSTREAM (FEET) = 155.40

FLOW LENGTH (FEET) = 160.00 MANNING'S N = 0.013

DEPTH OF FLOW IN 24.0 INCH PIPE IS 9.8 INCHES

PIPE-FLOW VELOCITY (FEET/SEC.) = 8.16

GIVEN PIPE DIAMETER (INCH) = 24.00 NUMBER OF PIPES = 1

PIPE-FLOW (CFS) = 9.86

PIPE TRAVEL TIME (MIN.) = 0.33 Tc (MIN.) = 7.95

LONGEST FLOWPATH FROM NODE 28.00 TO NODE 27.00 = 685.00 FEET.

FLOW PROCESS FROM NODE 27.00 TO NODE 27.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<

TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
TIME OF CONCENTRATION(MIN.) = 7.95
RAINFALL INTENSITY(INCH/HR) = 1.73
AREA-AVERAGED Fm(INCH/HR) = 0.03
AREA-AVERAGED Fp(INCH/HR) = 0.30
AREA-AVERAGED Ap = 0.10
EFFECTIVE STREAM AREA(ACRES) = 6.27
TOTAL STREAM AREA(ACRES) = 6.27
PEAK FLOW RATE(CFS) AT CONFLUENCE = 9.86

** CONFLUENCE DATA **

Table with 8 columns: STREAM NUMBER, Q (CFS), Tc (MIN.), Intensity (INCH/HR), Fp(Fm) (INCH/HR), Ap, Ae (ACRES), HEADWATER NODE. Contains 8 rows of data.

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **

Table with 8 columns: STREAM NUMBER, Q (CFS), Tc (MIN.), Intensity (INCH/HR), Fp(Fm) (INCH/HR), Ap, Ae (ACRES), HEADWATER NODE. Contains 8 rows of data.

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE(CFS) = 19.14 Tc(MIN.) = 7.95
EFFECTIVE AREA(ACRES) = 11.38 AREA-AVERAGED Fm(INCH/HR) = 0.03
AREA-AVERAGED Fp(INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.10
TOTAL AREA(ACRES) = 16.5
LONGEST FLOWPATH FROM NODE 10.00 TO NODE 27.00 = 1965.00 FEET.

FLOW PROCESS FROM NODE 27.00 TO NODE 31.00 IS CODE = 41

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 155.40 DOWNSTREAM(FEET) = 155.00
FLOW LENGTH(FEET) = 105.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 36.0 INCH PIPE IS 17.5 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 5.61
GIVEN PIPE DIAMETER(INCH) = 36.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 19.14
PIPE TRAVEL TIME(MIN.) = 0.31 Tc(MIN.) = 8.26
LONGEST FLOWPATH FROM NODE 10.00 TO NODE 31.00 = 2070.00 FEET.

FLOW PROCESS FROM NODE 31.00 TO NODE 31.00 IS CODE = 10

>>>>MAIN-STREAM MEMORY COPIED ONTO MEMORY BANK # 2 <<<<<

FLOW PROCESS FROM NODE 32.00 TO NODE 33.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

=====

INITIAL SUBAREA FLOW-LENGTH(FEET) = 300.00
ELEVATION DATA: UPSTREAM(FEET) = 166.30 DOWNSTREAM(FEET) = 166.00

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$

SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) = 11.850

* 2 YEAR RAINFALL INTENSITY(INCH/HR) = 1.379

SUBAREA Tc AND LOSS RATE DATA(AMC I):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN	Tc (MIN.)
COMMERCIAL	B	2.62	0.30	0.100	36	11.85

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.30

SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.100

SUBAREA RUNOFF(CFS) = 3.18

TOTAL AREA(ACRES) = 2.62 PEAK FLOW RATE(CFS) = 3.18

FLOW PROCESS FROM NODE 33.00 TO NODE 34.00 IS CODE = 61

>>>>COMPUTE STREET FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>(STANDARD CURB SECTION USED)<<<<<

=====

UPSTREAM ELEVATION(FEET) = 166.00 DOWNSTREAM ELEVATION(FEET) = 165.60
STREET LENGTH(FEET) = 485.00 CURB HEIGHT(INCHES) = 6.0
STREET HALFWIDTH(FEET) = 25.00

DISTANCE FROM CROWN TO CROSSFALL GRADEBREAK(FEET) = 10.00

INSIDE STREET CROSSFALL(DECIMAL) = 0.020

OUTSIDE STREET CROSSFALL(DECIMAL) = 0.020

SPECIFIED NUMBER OF HALFSTREETS CARRYING RUNOFF = 2

STREET PARKWAY CROSSFALL(DECIMAL) = 0.020
Manning's FRICTION FACTOR for Streetflow Section(curbs-to-curbs) = 0.0150
Manning's FRICTION FACTOR for Back-of-Walk Flow Section = 0.0200

**TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 5.32
STREETFLOW MODEL RESULTS USING ESTIMATED FLOW:
STREET FLOW DEPTH(FEET) = 0.47
HALFSTREET FLOOD WIDTH(FEET) = 17.07
AVERAGE FLOW VELOCITY(FEET/SEC.) = 0.88
PRODUCT OF DEPTH&VELOCITY(FT*FT/SEC.) = 0.41
STREET FLOW TRAVEL TIME(MIN.) = 9.21 Tc(MIN.) = 21.06
* 2 YEAR RAINFALL INTENSITY(INCH/HR) = 0.992

SUBAREA LOSS RATE DATA(AMC I):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
COMMERCIAL	B	4.89	0.30	0.100	36

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.30
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.100
SUBAREA AREA(ACRES) = 4.89 SUBAREA RUNOFF(CFS) = 4.23
EFFECTIVE AREA(ACRES) = 7.51 AREA-AVERAGED Fm(INCH/HR) = 0.03
AREA-AVERAGED Fp(INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.10
TOTAL AREA(ACRES) = 7.5 PEAK FLOW RATE(CFS) = 6.50

END OF SUBAREA STREET FLOW HYDRAULICS:
DEPTH(FEET) = 0.49 HALFSTREET FLOOD WIDTH(FEET) = 18.40
FLOW VELOCITY(FEET/SEC.) = 0.93 DEPTH*VELOCITY(FT*FT/SEC.) = 0.46
LONGEST FLOWPATH FROM NODE 32.00 TO NODE 34.00 = 785.00 FEET.

FLOW PROCESS FROM NODE 34.00 TO NODE 35.00 IS CODE = 51

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<<
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<<

=====
ELEVATION DATA: UPSTREAM(FEET) = 165.60 DOWNSTREAM(FEET) = 165.40
CHANNEL LENGTH THRU SUBAREA(FEET) = 180.00 CHANNEL SLOPE = 0.0011
CHANNEL BASE(FEET) = 5.00 "Z" FACTOR = 0.020
MANNING'S FACTOR = 0.015 MAXIMUM DEPTH(FEET) = 2.00
CHANNEL FLOW THRU SUBAREA(CFS) = 6.50
FLOW VELOCITY(FEET/SEC.) = 2.08 FLOW DEPTH(FEET) = 0.62
TRAVEL TIME(MIN.) = 1.44 Tc(MIN.) = 22.50
LONGEST FLOWPATH FROM NODE 32.00 TO NODE 35.00 = 965.00 FEET.

FLOW PROCESS FROM NODE 35.00 TO NODE 35.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

=====
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION(MIN.) = 22.50
RAINFALL INTENSITY(INCH/HR) = 0.95
AREA-AVERAGED Fm(INCH/HR) = 0.03
AREA-AVERAGED Fp(INCH/HR) = 0.30

AREA-AVERAGED $A_p = 0.10$
 EFFECTIVE STREAM AREA (ACRES) = 7.51
 TOTAL STREAM AREA (ACRES) = 7.51
 PEAK FLOW RATE (CFS) AT CONFLUENCE = 6.50

 FLOW PROCESS FROM NODE 35.00 TO NODE 35.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
 >>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

=====

INITIAL SUBAREA FLOW-LENGTH (FEET) = 665.00
 ELEVATION DATA: UPSTREAM (FEET) = 167.20 DOWNSTREAM (FEET) = 165.40

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$
 SUBAREA ANALYSIS USED MINIMUM T_c (MIN.) = 13.351
 * 2 YEAR RAINFALL INTENSITY (INCH/HR) = 1.288

SUBAREA T_c AND LOSS RATE DATA (AMC I):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	F_p (INCH/HR)	A_p (DECIMAL)	SCS CN	T_c (MIN.)
COMMERCIAL	B	6.31	0.30	0.100	36	13.35

SUBAREA AVERAGE PERVIOUS LOSS RATE, F_p (INCH/HR) = 0.30
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, A_p = 0.100
 SUBAREA RUNOFF (CFS) = 7.15
 TOTAL AREA (ACRES) = 6.31 PEAK FLOW RATE (CFS) = 7.15

 FLOW PROCESS FROM NODE 35.00 TO NODE 35.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
 >>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<

=====

TOTAL NUMBER OF STREAMS = 2
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
 TIME OF CONCENTRATION (MIN.) = 13.35
 RAINFALL INTENSITY (INCH/HR) = 1.29
 AREA-AVERAGED F_m (INCH/HR) = 0.03
 AREA-AVERAGED F_p (INCH/HR) = 0.30
 AREA-AVERAGED A_p = 0.10
 EFFECTIVE STREAM AREA (ACRES) = 6.31
 TOTAL STREAM AREA (ACRES) = 6.31
 PEAK FLOW RATE (CFS) AT CONFLUENCE = 7.15

** CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	T_c (MIN.)	Intensity (INCH/HR)	F_p (F_m) (INCH/HR)	A_p	A_e (ACRES)	HEADWATER NODE
1	6.50	22.50	0.955	0.30 (0.03)	0.10	7.5	32.00
2	7.15	13.35	1.288	0.30 (0.03)	0.10	6.3	35.00

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
 CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **

STREAM	Q	T_c	Intensity	F_p (F_m)	A_p	A_e	HEADWATER
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NUMBER	(CFS)	(MIN.)	(INCH/HR)	(INCH/HR)		(ACRES)	NODE
1	12.39	13.35	1.288	0.30 (0.03)	0.10	10.8	35.00
2	11.75	22.50	0.955	0.30 (0.03)	0.10	13.8	32.00

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE (CFS) = 12.39 Tc (MIN.) = 13.35
EFFECTIVE AREA (ACRES) = 10.77 AREA-AVERAGED Fm (INCH/HR) = 0.03
AREA-AVERAGED Fp (INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.10
TOTAL AREA (ACRES) = 13.8
LONGEST FLOWPATH FROM NODE 32.00 TO NODE 35.00 = 965.00 FEET.

FLOW PROCESS FROM NODE 35.00 TO NODE 31.00 IS CODE = 41

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM (FEET) = 159.00 DOWNSTREAM (FEET) = 155.00
FLOW LENGTH (FEET) = 50.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 18.0 INCH PIPE IS 8.2 INCHES
PIPE-FLOW VELOCITY (FEET/SEC.) = 15.75
GIVEN PIPE DIAMETER (INCH) = 18.00 NUMBER OF PIPES = 1
PIPE-FLOW (CFS) = 12.39
PIPE TRAVEL TIME (MIN.) = 0.05 Tc (MIN.) = 13.40
LONGEST FLOWPATH FROM NODE 32.00 TO NODE 31.00 = 1015.00 FEET.

FLOW PROCESS FROM NODE 31.00 TO NODE 31.00 IS CODE = 11

>>>>CONFLUENCE MEMORY BANK # 2 WITH THE MAIN-STREAM MEMORY<<<<<

** MAIN STREAM CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp (Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	12.39	13.40	1.285	0.30 (0.03)	0.10	10.8	35.00
2	11.75	22.56	0.953	0.30 (0.03)	0.10	13.8	32.00
LONGEST FLOWPATH FROM NODE			32.00 TO NODE		31.00 =	1015.00 FEET.	

** MEMORY BANK # 2 CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp (Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	19.14	8.26	1.697	0.30 (0.03)	0.10	11.4	28.00
2	18.76	12.16	1.359	0.30 (0.03)	0.10	13.9	14.00
3	18.78	12.51	1.337	0.30 (0.03)	0.10	14.2	21.00
4	18.78	12.60	1.331	0.30 (0.03)	0.10	14.2	17.00
5	18.29	15.80	1.170	0.30 (0.03)	0.10	15.9	24.00
6	17.52	17.41	1.106	0.30 (0.03)	0.10	16.2	18.00
7	16.48	19.43	1.038	0.30 (0.03)	0.10	16.4	12.00
8	14.67	23.36	0.934	0.30 (0.03)	0.10	16.5	10.00
LONGEST FLOWPATH FROM NODE			10.00 TO NODE		31.00 =	2070.00 FEET.	

** PEAK FLOW RATE TABLE **

STREAM	Q	Tc	Intensity	Fp (Fm)	Ap	Ae	HEADWATER
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NUMBER	(CFS)	(MIN.)	(INCH/HR)	(INCH/HR)		(ACRES)	NODE
1	29.28	8.26	1.697	0.30 (0.03)	0.10	18.0	28.00
2	30.66	12.16	1.359	0.30 (0.03)	0.10	23.7	14.00
3	30.82	12.51	1.337	0.30 (0.03)	0.10	24.2	21.00
4	30.86	12.60	1.331	0.30 (0.03)	0.10	24.3	17.00
5	31.05	13.40	1.285	0.30 (0.03)	0.10	25.4	35.00
6	30.51	15.80	1.170	0.30 (0.03)	0.10	27.5	24.00
7	29.63	17.41	1.106	0.30 (0.03)	0.10	28.3	18.00
8	28.45	19.43	1.038	0.30 (0.03)	0.10	29.2	12.00
9	26.79	22.56	0.953	0.30 (0.03)	0.10	30.3	32.00
10	26.18	23.36	0.934	0.30 (0.03)	0.10	30.3	10.00
TOTAL AREA (ACRES) =			30.3				

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE (CFS) = 31.05 Tc (MIN.) = 13.404
EFFECTIVE AREA (ACRES) = 25.42 AREA-AVERAGED Fm (INCH/HR) = 0.03
AREA-AVERAGED Fp (INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.10
TOTAL AREA (ACRES) = 30.3
LONGEST FLOWPATH FROM NODE 10.00 TO NODE 31.00 = 2070.00 FEET.

FLOW PROCESS FROM NODE 31.00 TO NODE 36.00 IS CODE = 41

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM (FEET) = 155.00 DOWNSTREAM (FEET) = 153.80
FLOW LENGTH (FEET) = 265.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 42.0 INCH PIPE IS 20.3 INCHES
PIPE-FLOW VELOCITY (FEET/SEC.) = 6.75
GIVEN PIPE DIAMETER (INCH) = 42.00 NUMBER OF PIPES = 1
PIPE-FLOW (CFS) = 31.05
PIPE TRAVEL TIME (MIN.) = 0.65 Tc (MIN.) = 14.06
LONGEST FLOWPATH FROM NODE 10.00 TO NODE 36.00 = 2335.00 FEET.

FLOW PROCESS FROM NODE 36.00 TO NODE 36.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

=====

TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION (MIN.) = 14.06
RAINFALL INTENSITY (INCH/HR) = 1.25
AREA-AVERAGED Fm (INCH/HR) = 0.03
AREA-AVERAGED Fp (INCH/HR) = 0.30
AREA-AVERAGED Ap = 0.10
EFFECTIVE STREAM AREA (ACRES) = 25.42
TOTAL STREAM AREA (ACRES) = 30.32
PEAK FLOW RATE (CFS) AT CONFLUENCE = 31.05

FLOW PROCESS FROM NODE 37.00 TO NODE 38.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

=====

INITIAL SUBAREA FLOW-LENGTH (FEET) = 325.00
ELEVATION DATA: UPSTREAM (FEET) = 169.50 DOWNSTREAM (FEET) = 168.00

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$
SUBAREA ANALYSIS USED MINIMUM T_c (MIN.) = 9.011
* 2 YEAR RAINFALL INTENSITY (INCH/HR) = 1.614

SUBAREA T_c AND LOSS RATE DATA (AMC I):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	F_p (INCH/HR)	A_p (DECIMAL)	SCS CN	T_c (MIN.)
COMMERCIAL	B	1.65	0.30	0.100	36	9.01

SUBAREA AVERAGE PERVIOUS LOSS RATE, F_p (INCH/HR) = 0.30

SUBAREA AVERAGE PERVIOUS AREA FRACTION, A_p = 0.100

SUBAREA RUNOFF (CFS) = 2.35

TOTAL AREA (ACRES) = 1.65 PEAK FLOW RATE (CFS) = 2.35

FLOW PROCESS FROM NODE 38.00 TO NODE 36.00 IS CODE = 41

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM (FEET) = 159.00 DOWNSTREAM (FEET) = 153.80
FLOW LENGTH (FEET) = 70.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 18.0 INCH PIPE IS 3.5 INCHES
PIPE-FLOW VELOCITY (FEET/SEC.) = 9.61
GIVEN PIPE DIAMETER (INCH) = 18.00 NUMBER OF PIPES = 1
PIPE-FLOW (CFS) = 2.35
PIPE TRAVEL TIME (MIN.) = 0.12 T_c (MIN.) = 9.13
LONGEST FLOWPATH FROM NODE 37.00 TO NODE 36.00 = 395.00 FEET.

FLOW PROCESS FROM NODE 36.00 TO NODE 36.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<

=====

TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
TIME OF CONCENTRATION (MIN.) = 9.13
RAINFALL INTENSITY (INCH/HR) = 1.60
AREA-AVERAGED F_m (INCH/HR) = 0.03
AREA-AVERAGED F_p (INCH/HR) = 0.30
AREA-AVERAGED A_p = 0.10
EFFECTIVE STREAM AREA (ACRES) = 1.65
TOTAL STREAM AREA (ACRES) = 1.65
PEAK FLOW RATE (CFS) AT CONFLUENCE = 2.35

** CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	T_c (MIN.)	Intensity (INCH/HR)	F_p (F_m) (INCH/HR)	A_p	A_e (ACRES)	HEADWATER NODE
1	29.28	8.92	1.623	0.30 (0.03)	0.10	18.0	28.00

1	30.66	12.82	1.319	0.30 (0.03)	0.10	23.7	14.00
1	30.82	13.16	1.299	0.30 (0.03)	0.10	24.2	21.00
1	30.86	13.26	1.293	0.30 (0.03)	0.10	24.3	17.00
1	31.05	14.06	1.251	0.30 (0.03)	0.10	25.4	35.00
1	30.51	16.45	1.143	0.30 (0.03)	0.10	27.5	24.00
1	29.63	18.07	1.083	0.30 (0.03)	0.10	28.3	18.00
1	28.45	20.10	1.018	0.30 (0.03)	0.10	29.2	12.00
1	26.79	23.24	0.937	0.30 (0.03)	0.10	30.3	32.00
1	26.18	24.05	0.919	0.30 (0.03)	0.10	30.3	10.00
2	2.35	9.13	1.602	0.30 (0.03)	0.10	1.6	37.00

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	31.61	8.92	1.623	0.30 (0.03)	0.10	19.6	28.00
2	31.71	9.13	1.602	0.30 (0.03)	0.10	20.0	37.00
3	32.59	12.82	1.319	0.30 (0.03)	0.10	25.3	14.00
4	32.72	13.16	1.299	0.30 (0.03)	0.10	25.9	21.00
5	32.75	13.26	1.293	0.30 (0.03)	0.10	26.0	17.00
6	32.87	14.06	1.251	0.30 (0.03)	0.10	27.1	35.00
7	32.18	16.45	1.143	0.30 (0.03)	0.10	29.2	24.00
8	31.21	18.07	1.083	0.30 (0.03)	0.10	30.0	18.00
9	29.93	20.10	1.018	0.30 (0.03)	0.10	30.8	12.00
10	28.15	23.24	0.937	0.30 (0.03)	0.10	32.0	32.00
11	27.51	24.05	0.919	0.30 (0.03)	0.10	32.0	10.00

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE (CFS) = 32.87 Tc (MIN.) = 14.06
EFFECTIVE AREA (ACRES) = 27.07 AREA-AVERAGED Fm (INCH/HR) = 0.03
AREA-AVERAGED Fp (INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.10
TOTAL AREA (ACRES) = 32.0
LONGEST FLOWPATH FROM NODE 10.00 TO NODE 36.00 = 2335.00 FEET.

FLOW PROCESS FROM NODE 36.00 TO NODE 39.00 IS CODE = 41

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM (FEET) = 153.80 DOWNSTREAM (FEET) = 152.80
FLOW LENGTH (FEET) = 230.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 42.0 INCH PIPE IS 21.2 INCHES
PIPE-FLOW VELOCITY (FEET/SEC.) = 6.75
GIVEN PIPE DIAMETER (INCH) = 42.00 NUMBER OF PIPES = 1
PIPE-FLOW (CFS) = 32.87
PIPE TRAVEL TIME (MIN.) = 0.57 Tc (MIN.) = 14.63
LONGEST FLOWPATH FROM NODE 10.00 TO NODE 39.00 = 2565.00 FEET.

FLOW PROCESS FROM NODE 39.00 TO NODE 39.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

=====

TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION(MIN.) = 14.63
RAINFALL INTENSITY(INCH/HR) = 1.22
AREA-AVERAGED Fm(INCH/HR) = 0.03
AREA-AVERAGED Fp(INCH/HR) = 0.30
AREA-AVERAGED Ap = 0.10
EFFECTIVE STREAM AREA(ACRES) = 27.07
TOTAL STREAM AREA(ACRES) = 31.97
PEAK FLOW RATE(CFS) AT CONFLUENCE = 32.87

FLOW PROCESS FROM NODE 40.00 TO NODE 39.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

=====

INITIAL SUBAREA FLOW-LENGTH(FEET) = 320.00
ELEVATION DATA: UPSTREAM(FEET) = 167.40 DOWNSTREAM(FEET) = 167.20

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$

SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) = 13.359

* 2 YEAR RAINFALL INTENSITY(INCH/HR) = 1.288

SUBAREA Tc AND LOSS RATE DATA(AMC I):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN	Tc (MIN.)
COMMERCIAL	B	0.48	0.30	0.100	36	13.36

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.30

SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.100

SUBAREA RUNOFF(CFS) = 0.54

TOTAL AREA(ACRES) = 0.48 PEAK FLOW RATE(CFS) = 0.54

FLOW PROCESS FROM NODE 39.00 TO NODE 39.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<

=====

TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
TIME OF CONCENTRATION(MIN.) = 13.36
RAINFALL INTENSITY(INCH/HR) = 1.29
AREA-AVERAGED Fm(INCH/HR) = 0.03
AREA-AVERAGED Fp(INCH/HR) = 0.30
AREA-AVERAGED Ap = 0.10
EFFECTIVE STREAM AREA(ACRES) = 0.48
TOTAL STREAM AREA(ACRES) = 0.48
PEAK FLOW RATE(CFS) AT CONFLUENCE = 0.54

** CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
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1	31.61	9.50	1.566	0.30 (0.03)	0.10	19.6	28.00
1	31.71	9.71	1.547	0.30 (0.03)	0.10	20.0	37.00
1	32.59	13.39	1.286	0.30 (0.03)	0.10	25.3	14.00
1	32.72	13.73	1.268	0.30 (0.03)	0.10	25.9	21.00
1	32.75	13.83	1.263	0.30 (0.03)	0.10	26.0	17.00
1	32.87	14.63	1.223	0.30 (0.03)	0.10	27.1	35.00
1	32.18	17.03	1.120	0.30 (0.03)	0.10	29.2	24.00
1	31.21	18.65	1.063	0.30 (0.03)	0.10	30.0	18.00
1	29.93	20.69	1.002	0.30 (0.03)	0.10	30.8	12.00
1	28.15	23.83	0.924	0.30 (0.03)	0.10	32.0	32.00
1	27.51	24.64	0.906	0.30 (0.03)	0.10	32.0	10.00
2	0.54	13.36	1.288	0.30 (0.03)	0.10	0.5	40.00

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	32.09	9.50	1.566	0.30 (0.03)	0.10	20.0	28.00
2	32.19	9.71	1.547	0.30 (0.03)	0.10	20.3	37.00
3	33.13	13.36	1.288	0.30 (0.03)	0.10	25.7	40.00
4	33.13	13.39	1.286	0.30 (0.03)	0.10	25.8	14.00
5	33.26	13.73	1.268	0.30 (0.03)	0.10	26.3	21.00
6	33.28	13.83	1.263	0.30 (0.03)	0.10	26.5	17.00
7	33.39	14.63	1.223	0.30 (0.03)	0.10	27.6	35.00
8	32.65	17.03	1.120	0.30 (0.03)	0.10	29.6	24.00
9	31.65	18.65	1.063	0.30 (0.03)	0.10	30.5	18.00
10	30.35	20.69	1.002	0.30 (0.03)	0.10	31.3	12.00
11	28.54	23.83	0.924	0.30 (0.03)	0.10	32.4	32.00
12	27.89	24.64	0.906	0.30 (0.03)	0.10	32.5	10.00

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE (CFS) = 33.39 Tc (MIN.) = 14.63
EFFECTIVE AREA (ACRES) = 27.55 AREA-AVERAGED Fm (INCH/HR) = 0.03
AREA-AVERAGED Fp (INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.10
TOTAL AREA (ACRES) = 32.5
LONGEST FLOWPATH FROM NODE 10.00 TO NODE 39.00 = 2565.00 FEET.

FLOW PROCESS FROM NODE 39.00 TO NODE 41.00 IS CODE = 41

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM (FEET) = 152.80 DOWNSTREAM (FEET) = 152.00
FLOW LENGTH (FEET) = 210.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 48.0 INCH PIPE IS 20.7 INCHES
PIPE-FLOW VELOCITY (FEET/SEC.) = 6.43
GIVEN PIPE DIAMETER (INCH) = 48.00 NUMBER OF PIPES = 1
PIPE-FLOW (CFS) = 33.39
PIPE TRAVEL TIME (MIN.) = 0.54 Tc (MIN.) = 15.17
LONGEST FLOWPATH FROM NODE 10.00 TO NODE 41.00 = 2775.00 FEET.

FLOW PROCESS FROM NODE 41.00 TO NODE 41.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION(MIN.) = 15.17
RAINFALL INTENSITY(INCH/HR) = 1.20
AREA-AVERAGED Fm(INCH/HR) = 0.03
AREA-AVERAGED Fp(INCH/HR) = 0.30
AREA-AVERAGED Ap = 0.10
EFFECTIVE STREAM AREA(ACRES) = 27.55
TOTAL STREAM AREA(ACRES) = 32.45
PEAK FLOW RATE(CFS) AT CONFLUENCE = 33.39

FLOW PROCESS FROM NODE 42.00 TO NODE 41.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

INITIAL SUBAREA FLOW-LENGTH(FEET) = 230.00
ELEVATION DATA: UPSTREAM(FEET) = 166.00 DOWNSTREAM(FEET) = 163.20

Tc = K*[(LENGTH** 3.00)/(ELEVATION CHANGE)]**0.20

SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) = 6.464

* 2 YEAR RAINFALL INTENSITY(INCH/HR) = 1.953

SUBAREA Tc AND LOSS RATE DATA(AMC I):

Table with 8 columns: DEVELOPMENT TYPE/LAND USE, SCS SOIL GROUP, AREA (ACRES), Fp (INCH/HR), Ap (DECIMAL), SCS CN, Tc (MIN.). Row 1: COMMERCIAL, B, 0.66, 0.30, 0.100, 36, 6.46

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.30

SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.100

SUBAREA RUNOFF(CFS) = 1.14

TOTAL AREA(ACRES) = 0.66 PEAK FLOW RATE(CFS) = 1.14

FLOW PROCESS FROM NODE 41.00 TO NODE 41.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<

TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
TIME OF CONCENTRATION(MIN.) = 6.46
RAINFALL INTENSITY(INCH/HR) = 1.95
AREA-AVERAGED Fm(INCH/HR) = 0.03
AREA-AVERAGED Fp(INCH/HR) = 0.30
AREA-AVERAGED Ap = 0.10
EFFECTIVE STREAM AREA(ACRES) = 0.66
TOTAL STREAM AREA(ACRES) = 0.66
PEAK FLOW RATE(CFS) AT CONFLUENCE = 1.14

** CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	32.09	10.05	1.517	0.30 (0.03)	0.10	20.0	28.00
1	32.19	10.26	1.499	0.30 (0.03)	0.10	20.3	37.00
1	33.13	13.90	1.259	0.30 (0.03)	0.10	25.7	40.00
1	33.13	13.93	1.257	0.30 (0.03)	0.10	25.8	14.00
1	33.26	14.28	1.240	0.30 (0.03)	0.10	26.3	21.00
1	33.28	14.37	1.235	0.30 (0.03)	0.10	26.5	17.00
1	33.39	15.17	1.197	0.30 (0.03)	0.10	27.6	35.00
1	32.65	17.57	1.100	0.30 (0.03)	0.10	29.6	24.00
1	31.65	19.20	1.046	0.30 (0.03)	0.10	30.5	18.00
1	30.35	21.24	0.987	0.30 (0.03)	0.10	31.3	12.00
1	28.54	24.39	0.911	0.30 (0.03)	0.10	32.4	32.00
1	27.89	25.21	0.894	0.30 (0.03)	0.10	32.5	10.00
2	1.14	6.46	1.953	0.30 (0.03)	0.10	0.7	42.00

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	27.85	6.46	1.953	0.30 (0.03)	0.10	13.5	42.00
2	32.97	10.05	1.517	0.30 (0.03)	0.10	20.6	28.00
3	33.06	10.26	1.499	0.30 (0.03)	0.10	21.0	37.00
4	33.86	13.90	1.259	0.30 (0.03)	0.10	26.4	40.00
5	33.86	13.93	1.257	0.30 (0.03)	0.10	26.4	14.00
6	33.97	14.28	1.240	0.30 (0.03)	0.10	27.0	21.00
7	34.00	14.37	1.235	0.30 (0.03)	0.10	27.1	17.00
8	34.08	15.17	1.197	0.30 (0.03)	0.10	28.2	35.00
9	33.28	17.57	1.100	0.30 (0.03)	0.10	30.3	24.00
10	32.26	19.20	1.046	0.30 (0.03)	0.10	31.1	18.00
11	30.92	21.24	0.987	0.30 (0.03)	0.10	32.0	12.00
12	29.06	24.39	0.911	0.30 (0.03)	0.10	33.1	32.00
13	28.40	25.21	0.894	0.30 (0.03)	0.10	33.1	10.00

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE (CFS) = 34.08 Tc (MIN.) = 15.17
 EFFECTIVE AREA (ACRES) = 28.21 AREA-AVERAGED Fm (INCH/HR) = 0.03
 AREA-AVERAGED Fp (INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.10
 TOTAL AREA (ACRES) = 33.1
 LONGEST FLOWPATH FROM NODE 10.00 TO NODE 41.00 = 2775.00 FEET.

 FLOW PROCESS FROM NODE 41.00 TO NODE 43.00 IS CODE = 41

>>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
 >>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM (FEET) = 152.00 DOWNSTREAM (FEET) = 151.80
 FLOW LENGTH (FEET) = 30.00 MANNING'S N = 0.013
 DEPTH OF FLOW IN 48.0 INCH PIPE IS 18.0 INCHES
 PIPE-FLOW VELOCITY (FEET/SEC.) = 7.94

GIVEN PIPE DIAMETER(INCH) = 48.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 34.08
PIPE TRAVEL TIME(MIN.) = 0.06 Tc(MIN.) = 15.23
LONGEST FLOWPATH FROM NODE 10.00 TO NODE 43.00 = 2805.00 FEET.

FLOW PROCESS FROM NODE 43.00 TO NODE 43.00 IS CODE = 10

>>>>MAIN-STREAM MEMORY COPIED ONTO MEMORY BANK # 1 <<<<<
=====

FLOW PROCESS FROM NODE 44.00 TO NODE 45.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<
=====

INITIAL SUBAREA FLOW-LENGTH(FEET) = 350.00
ELEVATION DATA: UPSTREAM(FEET) = 164.00 DOWNSTREAM(FEET) = 162.00

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$

SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) = 8.894

* 2 YEAR RAINFALL INTENSITY(INCH/HR) = 1.626

SUBAREA Tc AND LOSS RATE DATA(AMC I):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN	Tc (MIN.)
COMMERCIAL	B	1.55	0.30	0.100	36	8.89

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.30

SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.100

SUBAREA RUNOFF(CFS) = 2.23

TOTAL AREA(ACRES) = 1.55 PEAK FLOW RATE(CFS) = 2.23

FLOW PROCESS FROM NODE 45.00 TO NODE 46.00 IS CODE = 51

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<<
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<<
=====

ELEVATION DATA: UPSTREAM(FEET) = 162.00 DOWNSTREAM(FEET) = 161.80

CHANNEL LENGTH THRU SUBAREA(FEET) = 165.00 CHANNEL SLOPE = 0.0012

CHANNEL BASE(FEET) = 3.00 "Z" FACTOR = 0.040

MANNING'S FACTOR = 0.015 MAXIMUM DEPTH(FEET) = 1.00

CHANNEL FLOW THRU SUBAREA(CFS) = 2.23

FLOW VELOCITY(FEET/SEC.) = 1.69 FLOW DEPTH(FEET) = 0.44

TRAVEL TIME(MIN.) = 1.63 Tc(MIN.) = 10.53

LONGEST FLOWPATH FROM NODE 44.00 TO NODE 46.00 = 515.00 FEET.

FLOW PROCESS FROM NODE 46.00 TO NODE 46.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
=====

TOTAL NUMBER OF STREAMS = 2

CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:

TIME OF CONCENTRATION(MIN.) = 10.53
 RAINFALL INTENSITY(INCH/HR) = 1.48
 AREA-AVERAGED Fm(INCH/HR) = 0.03
 AREA-AVERAGED Fp(INCH/HR) = 0.30
 AREA-AVERAGED Ap = 0.10
 EFFECTIVE STREAM AREA(ACRES) = 1.55
 TOTAL STREAM AREA(ACRES) = 1.55
 PEAK FLOW RATE(CFS) AT CONFLUENCE = 2.23

 FLOW PROCESS FROM NODE 47.00 TO NODE 46.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
 >>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

INITIAL SUBAREA FLOW-LENGTH(FEET) = 285.00
 ELEVATION DATA: UPSTREAM(FEET) = 165.00 DOWNSTREAM(FEET) = 161.80

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$
 SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) = 7.157
 * 2 YEAR RAINFALL INTENSITY(INCH/HR) = 1.842

SUBAREA Tc AND LOSS RATE DATA(AMC I):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN	Tc (MIN.)
COMMERCIAL	B	1.23	0.30	0.100	36	7.16

 SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.30
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.100
 SUBAREA RUNOFF(CFS) = 2.01
 TOTAL AREA(ACRES) = 1.23 PEAK FLOW RATE(CFS) = 2.01

 FLOW PROCESS FROM NODE 46.00 TO NODE 46.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
 >>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<

TOTAL NUMBER OF STREAMS = 2
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
 TIME OF CONCENTRATION(MIN.) = 7.16
 RAINFALL INTENSITY(INCH/HR) = 1.84
 AREA-AVERAGED Fm(INCH/HR) = 0.03
 AREA-AVERAGED Fp(INCH/HR) = 0.30
 AREA-AVERAGED Ap = 0.10
 EFFECTIVE STREAM AREA(ACRES) = 1.23
 TOTAL STREAM AREA(ACRES) = 1.23
 PEAK FLOW RATE(CFS) AT CONFLUENCE = 2.01

** CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	2.23	10.53	1.477	0.30 (0.03)	0.10	1.5	44.00
2	2.01	7.16	1.842	0.30 (0.03)	0.10	1.2	47.00

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO

CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	3.90	7.16	1.842	0.30(0.03)	0.10	2.3	47.00
2	3.83	10.53	1.477	0.30(0.03)	0.10	2.8	44.00

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE(CFS) = 3.90 Tc(MIN.) = 7.16
EFFECTIVE AREA(ACRES) = 2.28 AREA-AVERAGED Fm(INCH/HR) = 0.03
AREA-AVERAGED Fp(INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.10
TOTAL AREA(ACRES) = 2.8
LONGEST FLOWPATH FROM NODE 44.00 TO NODE 46.00 = 515.00 FEET.

FLOW PROCESS FROM NODE 46.00 TO NODE 43.00 IS CODE = 41

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 157.00 DOWNSTREAM(FEET) = 151.80
FLOW LENGTH(FEET) = 115.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 18.0 INCH PIPE IS 5.2 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 9.33
GIVEN PIPE DIAMETER(INCH) = 18.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 3.90
PIPE TRAVEL TIME(MIN.) = 0.21 Tc(MIN.) = 7.36
LONGEST FLOWPATH FROM NODE 44.00 TO NODE 43.00 = 630.00 FEET.

FLOW PROCESS FROM NODE 43.00 TO NODE 43.00 IS CODE = 11

>>>>CONFLUENCE MEMORY BANK # 1 WITH THE MAIN-STREAM MEMORY<<<<<

** MAIN STREAM CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	3.90	7.36	1.813	0.30(0.03)	0.10	2.3	47.00
2	3.83	10.73	1.460	0.30(0.03)	0.10	2.8	44.00

LONGEST FLOWPATH FROM NODE 44.00 TO NODE 43.00 = 630.00 FEET.

** MEMORY BANK # 1 CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	27.85	6.53	1.942	0.30(0.03)	0.10	13.5	42.00
2	32.97	10.11	1.511	0.30(0.03)	0.10	20.6	28.00
3	33.06	10.32	1.494	0.30(0.03)	0.10	21.0	37.00
4	33.86	13.97	1.255	0.30(0.03)	0.10	26.4	40.00
5	33.86	14.00	1.254	0.30(0.03)	0.10	26.4	14.00
6	33.97	14.34	1.237	0.30(0.03)	0.10	27.0	21.00
7	34.00	14.44	1.232	0.30(0.03)	0.10	27.1	17.00
8	34.08	15.23	1.194	0.30(0.03)	0.10	28.2	35.00

9	33.28	17.64	1.098	0.30 (0.03)	0.10	30.3	24.00
10	32.26	19.26	1.044	0.30 (0.03)	0.10	31.1	18.00
11	30.92	21.31	0.985	0.30 (0.03)	0.10	32.0	12.00
12	29.06	24.46	0.910	0.30 (0.03)	0.10	33.1	32.00
13	28.40	25.28	0.893	0.30 (0.03)	0.10	33.1	10.00

LONGEST FLOWPATH FROM NODE 10.00 TO NODE 43.00 = 2805.00 FEET.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	31.57	6.53	1.942	0.30 (0.03)	0.10	15.5	42.00
2	32.95	7.36	1.813	0.30 (0.03)	0.10	17.4	47.00
3	36.81	10.11	1.511	0.30 (0.03)	0.10	23.3	28.00
4	36.90	10.32	1.494	0.30 (0.03)	0.10	23.7	37.00
5	36.98	10.73	1.460	0.30 (0.03)	0.10	24.4	44.00
6	37.14	13.97	1.255	0.30 (0.03)	0.10	29.2	40.00
7	37.14	14.00	1.254	0.30 (0.03)	0.10	29.2	14.00
8	37.20	14.34	1.237	0.30 (0.03)	0.10	29.8	21.00
9	37.22	14.44	1.232	0.30 (0.03)	0.10	29.9	17.00
10	37.20	15.23	1.194	0.30 (0.03)	0.10	31.0	35.00
11	36.14	17.64	1.098	0.30 (0.03)	0.10	33.1	24.00
12	34.97	19.26	1.044	0.30 (0.03)	0.10	33.9	18.00
13	33.47	21.31	0.985	0.30 (0.03)	0.10	34.8	12.00
14	31.42	24.46	0.910	0.30 (0.03)	0.10	35.9	32.00
15	30.71	25.28	0.893	0.30 (0.03)	0.10	35.9	10.00

TOTAL AREA (ACRES) = 35.9

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE (CFS) = 37.22 Tc (MIN.) = 14.435
EFFECTIVE AREA (ACRES) = 29.92 AREA-AVERAGED Fm (INCH/HR) = 0.03
AREA-AVERAGED Fp (INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.10
TOTAL AREA (ACRES) = 35.9
LONGEST FLOWPATH FROM NODE 10.00 TO NODE 43.00 = 2805.00 FEET.

=====

END OF STUDY SUMMARY:

TOTAL AREA (ACRES) = 35.9 TC (MIN.) = 14.44
EFFECTIVE AREA (ACRES) = 29.92 AREA-AVERAGED Fm (INCH/HR) = 0.03
AREA-AVERAGED Fp (INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.100
PEAK FLOW RATE (CFS) = 37.22

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	31.57	6.53	1.942	0.30 (0.03)	0.10	15.5	42.00
2	32.95	7.36	1.813	0.30 (0.03)	0.10	17.4	47.00
3	36.81	10.11	1.511	0.30 (0.03)	0.10	23.3	28.00
4	36.90	10.32	1.494	0.30 (0.03)	0.10	23.7	37.00
5	36.98	10.73	1.460	0.30 (0.03)	0.10	24.4	44.00
6	37.14	13.97	1.255	0.30 (0.03)	0.10	29.2	40.00
7	37.14	14.00	1.254	0.30 (0.03)	0.10	29.2	14.00
8	37.20	14.34	1.237	0.30 (0.03)	0.10	29.8	21.00
9	37.22	14.44	1.232	0.30 (0.03)	0.10	29.9	17.00
10	37.20	15.23	1.194	0.30 (0.03)	0.10	31.0	35.00
11	36.14	17.64	1.098	0.30 (0.03)	0.10	33.1	24.00

12	34.97	19.26	1.044	0.30 (0.03)	0.10	33.9	18.00
13	33.47	21.31	0.985	0.30 (0.03)	0.10	34.8	12.00
14	31.42	24.46	0.910	0.30 (0.03)	0.10	35.9	32.00
15	30.71	25.28	0.893	0.30 (0.03)	0.10	35.9	10.00

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END OF RATIONAL METHOD ANALYSIS

RATIONAL METHOD HYDROLOGY COMPUTER PROGRAM PACKAGE
(Reference: 1986 ORANGE COUNTY HYDROLOGY CRITERION)
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Analysis prepared by:

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***** DESCRIPTION OF STUDY *****

* FREEMAN *
* EXISTING CONDITION *
* 10 YEAR ANALYSIS *

FILE NAME: FREX10.DAT
TIME/DATE OF STUDY: 09:01 05/29/2017

=====

USER SPECIFIED HYDROLOGY AND HYDRAULIC MODEL INFORMATION:

=====

--*TIME-OF-CONCENTRATION MODEL*--

USER SPECIFIED STORM EVENT (YEAR) = 10.00
SPECIFIED MINIMUM PIPE SIZE (INCH) = 12.00
SPECIFIED PERCENT OF GRADIENTS (DECIMAL) TO USE FOR FRICTION SLOPE = 0.95
DATA BANK RAINFALL USED
ANTECEDENT MOISTURE CONDITION (AMC) II ASSUMED FOR RATIONAL METHOD

USER-DEFINED STREET-SECTIONS FOR COUPLED PIPEFLOW AND STREETFLOW MODEL

NO.	HALF- WIDTH (FT)	CROWN TO CROSSFALL (FT)	STREET-CROSSFALL: IN- / OUT- / PARK- SIDE / SIDE / WAY	CURB HEIGHT (FT)	GUTTER-GEOMETRIES: WIDTH LIP HIKE (FT) (FT) (FT)	MANNING FACTOR (n)
1	24.0	19.0	0.020/0.020/0.020	0.67	2.00 0.0313 0.167	0.0150

GLOBAL STREET FLOW-DEPTH CONSTRAINTS:

1. Relative Flow-Depth = 0.67 FEET
as (Maximum Allowable Street Flow Depth) - (Top-of-Curb)
 2. (Depth)*(Velocity) Constraint = 6.0 (FT*FT/S)
- *SIZE PIPE WITH A FLOW CAPACITY GREATER THAN
OR EQUAL TO THE UPSTREAM TRIBUTARY PIPE.*
*USER-SPECIFIED MINIMUM TOPOGRAPHIC SLOPE ADJUSTMENT NOT SELECTED

FLOW PROCESS FROM NODE 10.00 TO NODE 32.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<
=====

INITIAL SUBAREA FLOW-LENGTH (FEET) = 265.00
ELEVATION DATA: UPSTREAM (FEET) = 167.50 DOWNSTREAM (FEET) = 166.30

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$

SUBAREA ANALYSIS USED MINIMUM T_c (MIN.) = 8.337

* 10 YEAR RAINFALL INTENSITY (INCH/HR) = 3.029

SUBAREA T_c AND LOSS RATE DATA (AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	F_p (INCH/HR)	A_p (DECIMAL)	SCS CN	T_c (MIN.)
COMMERCIAL	B	0.15	0.30	0.100	56	8.34

SUBAREA AVERAGE PERVIOUS LOSS RATE, F_p (INCH/HR) = 0.30

SUBAREA AVERAGE PERVIOUS AREA FRACTION, A_p = 0.100

SUBAREA RUNOFF (CFS) = 0.40

TOTAL AREA (ACRES) = 0.15 PEAK FLOW RATE (CFS) = 0.40

FLOW PROCESS FROM NODE 32.00 TO NODE 11.00 IS CODE = 61

>>>>COMPUTE STREET FLOW TRAVEL TIME THRU SUBAREA<<<<<

>>>>(STANDARD CURB SECTION USED)<<<<<
=====

UPSTREAM ELEVATION (FEET) = 166.30 DOWNSTREAM ELEVATION (FEET) = 165.10

STREET LENGTH (FEET) = 210.00 CURB HEIGHT (INCHES) = 6.0

STREET HALFWIDTH (FEET) = 25.00

DISTANCE FROM CROWN TO CROSSFALL GRADEBREAK (FEET) = 10.00

INSIDE STREET CROSSFALL (DECIMAL) = 0.020

OUTSIDE STREET CROSSFALL (DECIMAL) = 0.020

SPECIFIED NUMBER OF HALFSTREETS CARRYING RUNOFF = 1

STREET PARKWAY CROSSFALL (DECIMAL) = 0.020

Manning's FRICTION FACTOR for Streetflow Section (curb-to-curb) = 0.0150

Manning's FRICTION FACTOR for Back-of-Walk Flow Section = 0.0200

**TRAVEL TIME COMPUTED USING ESTIMATED FLOW (CFS) = 0.54

STREETFLOW MODEL RESULTS USING ESTIMATED FLOW:

STREET FLOW DEPTH (FEET) = 0.24

HALFSTREET FLOOD WIDTH (FEET) = 5.48

AVERAGE FLOW VELOCITY (FEET/SEC.) = 1.30

PRODUCT OF DEPTH&VELOCITY (FT*FT/SEC.) = 0.31

STREET FLOW TRAVEL TIME (MIN.) = 2.70 T_c (MIN.) = 11.04

* 10 YEAR RAINFALL INTENSITY (INCH/HR) = 2.579

SUBAREA LOSS RATE DATA (AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	F_p (INCH/HR)	A_p (DECIMAL)	SCS CN
COMMERCIAL	B	0.12	0.30	0.100	56

SUBAREA AVERAGE PERVIOUS LOSS RATE, F_p (INCH/HR) = 0.30

SUBAREA AVERAGE PERVIOUS AREA FRACTION, A_p = 0.100

SUBAREA AREA (ACRES) = 0.12 SUBAREA RUNOFF (CFS) = 0.28

EFFECTIVE AREA (ACRES) = 0.27 AREA-AVERAGED F_m (INCH/HR) = 0.03

AREA-AVERAGED F_p (INCH/HR) = 0.30 AREA-AVERAGED A_p = 0.10
TOTAL AREA (ACRES) = 0.3 PEAK FLOW RATE (CFS) = 0.62

END OF SUBAREA STREET FLOW HYDRAULICS:

DEPTH (FEET) = 0.24 HALFSTREET FLOOD WIDTH (FEET) = 5.90
FLOW VELOCITY (FEET/SEC.) = 1.33 DEPTH*VELOCITY (FT*FT/SEC.) = 0.32
LONGEST FLOWPATH FROM NODE 10.00 TO NODE 11.00 = 475.00 FEET.

FLOW PROCESS FROM NODE 11.00 TO NODE 11.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

=====

TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION (MIN.) = 11.04
RAINFALL INTENSITY (INCH/HR) = 2.58
AREA-AVERAGED F_m (INCH/HR) = 0.03
AREA-AVERAGED F_p (INCH/HR) = 0.30
AREA-AVERAGED A_p = 0.10
EFFECTIVE STREAM AREA (ACRES) = 0.27
TOTAL STREAM AREA (ACRES) = 0.27
PEAK FLOW RATE (CFS) AT CONFLUENCE = 0.62

FLOW PROCESS FROM NODE 12.00 TO NODE 11.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

=====

INITIAL SUBAREA FLOW-LENGTH (FEET) = 210.00
ELEVATION DATA: UPSTREAM (FEET) = 166.00 DOWNSTREAM (FEET) = 165.10

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$
SUBAREA ANALYSIS USED MINIMUM T_c (MIN.) = 7.680
* 10 YEAR RAINFALL INTENSITY (INCH/HR) = 3.174
SUBAREA T_c AND LOSS RATE DATA (AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	F_p (INCH/HR)	A_p (DECIMAL)	SCS CN	T_c (MIN.)
COMMERCIAL	B	0.76	0.30	0.100	56	7.68

SUBAREA AVERAGE PERVIOUS LOSS RATE, F_p (INCH/HR) = 0.30
SUBAREA AVERAGE PERVIOUS AREA FRACTION, A_p = 0.100
SUBAREA RUNOFF (CFS) = 2.15
TOTAL AREA (ACRES) = 0.76 PEAK FLOW RATE (CFS) = 2.15

FLOW PROCESS FROM NODE 11.00 TO NODE 11.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<

=====

TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
TIME OF CONCENTRATION (MIN.) = 7.68

RAINFALL INTENSITY (INCH/HR) = 3.17
 AREA-AVERAGED Fm (INCH/HR) = 0.03
 AREA-AVERAGED Fp (INCH/HR) = 0.30
 AREA-AVERAGED Ap = 0.10
 EFFECTIVE STREAM AREA (ACRES) = 0.76
 TOTAL STREAM AREA (ACRES) = 0.76
 PEAK FLOW RATE (CFS) AT CONFLUENCE = 2.15

** CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp (Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	0.62	11.04	2.579	0.30 (0.03)	0.10	0.3	10.00
2	2.15	7.68	3.174	0.30 (0.03)	0.10	0.8	12.00

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
 CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp (Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	2.68	7.68	3.174	0.30 (0.03)	0.10	0.9	12.00
2	2.36	11.04	2.579	0.30 (0.03)	0.10	1.0	10.00

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE (CFS) = 2.68 Tc (MIN.) = 7.68
 EFFECTIVE AREA (ACRES) = 0.95 AREA-AVERAGED Fm (INCH/HR) = 0.03
 AREA-AVERAGED Fp (INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.10
 TOTAL AREA (ACRES) = 1.0
 LONGEST FLOWPATH FROM NODE 10.00 TO NODE 11.00 = 475.00 FEET.

FLOW PROCESS FROM NODE 11.00 TO NODE 13.00 IS CODE = 62

>>>>COMPUTE STREET FLOW TRAVEL TIME THRU SUBAREA<<<<<
 >>>>(STREET TABLE SECTION # 1 USED)<<<<<

UPSTREAM ELEVATION (FEET) = 165.10 DOWNSTREAM ELEVATION (FEET) = 164.20
 STREET LENGTH (FEET) = 455.00 CURB HEIGHT (INCHES) = 8.0
 STREET HALFWIDTH (FEET) = 24.00

DISTANCE FROM CROWN TO CROSSFALL GRADEBREAK (FEET) = 19.00
 INSIDE STREET CROSSFALL (DECIMAL) = 0.020
 OUTSIDE STREET CROSSFALL (DECIMAL) = 0.020

SPECIFIED NUMBER OF HALFSTREETS CARRYING RUNOFF = 1
 STREET PARKWAY CROSSFALL (DECIMAL) = 0.020
 Manning's FRICTION FACTOR for Streetflow Section (curb-to-curb) = 0.0150
 Manning's FRICTION FACTOR for Back-of-Walk Flow Section = 0.0200

**TRAVEL TIME COMPUTED USING ESTIMATED FLOW (CFS) = 3.05
 STREETFLOW MODEL RESULTS USING ESTIMATED FLOW:
 STREET FLOW DEPTH (FEET) = 0.45
 HALFSTREET FLOOD WIDTH (FEET) = 14.83
 AVERAGE FLOW VELOCITY (FEET/SEC.) = 1.28

PRODUCT OF DEPTH&VELOCITY (FT*FT/SEC.) = 0.58
 STREET FLOW TRAVEL TIME (MIN.) = 5.94 Tc (MIN.) = 13.62
 * 10 YEAR RAINFALL INTENSITY (INCH/HR) = 2.286
 SUBAREA LOSS RATE DATA (AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
COMMERCIAL	B	0.36	0.30	0.100	56

 SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp (INCH/HR) = 0.30
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.100
 SUBAREA AREA (ACRES) = 0.36 SUBAREA RUNOFF (CFS) = 0.73
 EFFECTIVE AREA (ACRES) = 1.31 AREA-AVERAGED Fm (INCH/HR) = 0.03
 AREA-AVERAGED Fp (INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.10
 TOTAL AREA (ACRES) = 1.4 PEAK FLOW RATE (CFS) = 2.68
 NOTE: PEAK FLOW RATE DEFAULTED TO UPSTREAM VALUE

END OF SUBAREA STREET FLOW HYDRAULICS:
 DEPTH (FEET) = 0.44 HALFSTREET FLOOD WIDTH (FEET) = 14.09
 FLOW VELOCITY (FEET/SEC.) = 1.23 DEPTH*VELOCITY (FT*FT/SEC.) = 0.54
 LONGEST FLOWPATH FROM NODE 10.00 TO NODE 13.00 = 930.00 FEET.

FLOW PROCESS FROM NODE 13.00 TO NODE 13.00 IS CODE = 1

 >>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

=====

TOTAL NUMBER OF STREAMS = 2
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
 TIME OF CONCENTRATION (MIN.) = 13.62
 RAINFALL INTENSITY (INCH/HR) = 2.29
 AREA-AVERAGED Fm (INCH/HR) = 0.03
 AREA-AVERAGED Fp (INCH/HR) = 0.30
 AREA-AVERAGED Ap = 0.10
 EFFECTIVE STREAM AREA (ACRES) = 1.31
 TOTAL STREAM AREA (ACRES) = 1.39
 PEAK FLOW RATE (CFS) AT CONFLUENCE = 2.68

FLOW PROCESS FROM NODE 14.00 TO NODE 15.00 IS CODE = 21

 >>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
 >>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

=====

INITIAL SUBAREA FLOW-LENGTH (FEET) = 125.00
 ELEVATION DATA: UPSTREAM (FEET) = 164.60 DOWNSTREAM (FEET) = 164.20

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$
 SUBAREA ANALYSIS USED MINIMUM Tc (MIN.) = 6.616
 * 10 YEAR RAINFALL INTENSITY (INCH/HR) = 3.458
 SUBAREA Tc AND LOSS RATE DATA (AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN	Tc (MIN.)
COMMERCIAL	B	0.15	0.30	0.100	56	6.62

 SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp (INCH/HR) = 0.30
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.100

SUBAREA RUNOFF(CFS) = 0.46
 TOTAL AREA(ACRES) = 0.15 PEAK FLOW RATE(CFS) = 0.46

FLOW PROCESS FROM NODE 15.00 TO NODE 13.00 IS CODE = 41

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
 >>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 159.30 DOWNSTREAM(FEET) = 159.20
 FLOW LENGTH(FEET) = 50.00 MANNING'S N = 0.013
 DEPTH OF FLOW IN 18.0 INCH PIPE IS 3.9 INCHES
 PIPE-FLOW VELOCITY(FEET/SEC.) = 1.66
 GIVEN PIPE DIAMETER(INCH) = 18.00 NUMBER OF PIPES = 1
 PIPE-FLOW(CFS) = 0.46
 PIPE TRAVEL TIME(MIN.) = 0.50 Tc(MIN.) = 7.12
 LONGEST FLOWPATH FROM NODE 14.00 TO NODE 13.00 = 175.00 FEET.

FLOW PROCESS FROM NODE 13.00 TO NODE 13.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
 >>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<

=====

TOTAL NUMBER OF STREAMS = 2
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
 TIME OF CONCENTRATION(MIN.) = 7.12
 RAINFALL INTENSITY(INCH/HR) = 3.32
 AREA-AVERAGED Fm(INCH/HR) = 0.03
 AREA-AVERAGED Fp(INCH/HR) = 0.30
 AREA-AVERAGED Ap = 0.10
 EFFECTIVE STREAM AREA(ACRES) = 0.15
 TOTAL STREAM AREA(ACRES) = 0.15
 PEAK FLOW RATE(CFS) AT CONFLUENCE = 0.46

** CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	2.68	13.62	2.286	0.30 (0.03)	0.10	1.3	12.00
1	2.47	17.18	2.001	0.30 (0.03)	0.10	1.4	10.00
2	0.46	7.12	3.316	0.30 (0.03)	0.10	0.2	14.00

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
 CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	2.50	7.12	3.316	0.30 (0.03)	0.10	0.8	14.00
2	3.00	13.62	2.286	0.30 (0.03)	0.10	1.5	12.00
3	2.74	17.18	2.001	0.30 (0.03)	0.10	1.5	10.00

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
 PEAK FLOW RATE(CFS) = 3.00 Tc(MIN.) = 13.62

EFFECTIVE AREA (ACRES) = 1.46 AREA-AVERAGED Fm (INCH/HR) = 0.03
AREA-AVERAGED Fp (INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.10
TOTAL AREA (ACRES) = 1.5
LONGEST FLOWPATH FROM NODE 10.00 TO NODE 13.00 = 930.00 FEET.

FLOW PROCESS FROM NODE 13.00 TO NODE 16.00 IS CODE = 41

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM (FEET) = 159.20 DOWNSTREAM (FEET) = 158.10
FLOW LENGTH (FEET) = 285.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 18.0 INCH PIPE IS 8.7 INCHES
PIPE-FLOW VELOCITY (FEET/SEC.) = 3.55
GIVEN PIPE DIAMETER (INCH) = 18.00 NUMBER OF PIPES = 1
PIPE-FLOW (CFS) = 3.00
PIPE TRAVEL TIME (MIN.) = 1.34 Tc (MIN.) = 14.96
LONGEST FLOWPATH FROM NODE 10.00 TO NODE 16.00 = 1215.00 FEET.

FLOW PROCESS FROM NODE 16.00 TO NODE 16.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

=====

TOTAL NUMBER OF STREAMS = 3
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION (MIN.) = 14.96
RAINFALL INTENSITY (INCH/HR) = 2.17
AREA-AVERAGED Fm (INCH/HR) = 0.03
AREA-AVERAGED Fp (INCH/HR) = 0.30
AREA-AVERAGED Ap = 0.10
EFFECTIVE STREAM AREA (ACRES) = 1.46
TOTAL STREAM AREA (ACRES) = 1.54
PEAK FLOW RATE (CFS) AT CONFLUENCE = 3.00

FLOW PROCESS FROM NODE 17.00 TO NODE 16.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

=====

INITIAL SUBAREA FLOW-LENGTH (FEET) = 310.00
ELEVATION DATA: UPSTREAM (FEET) = 166.00 DOWNSTREAM (FEET) = 164.90

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$
SUBAREA ANALYSIS USED MINIMUM Tc (MIN.) = 9.320
* 10 YEAR RAINFALL INTENSITY (INCH/HR) = 2.841
SUBAREA Tc AND LOSS RATE DATA (AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN	Tc (MIN.)
COMMERCIAL	B	1.52	0.30	0.100	56	9.32

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp (INCH/HR) = 0.30
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.100

SUBAREA RUNOFF(CFS) = 3.85
TOTAL AREA(ACRES) = 1.52 PEAK FLOW RATE(CFS) = 3.85

FLOW PROCESS FROM NODE 16.00 TO NODE 16.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

=====

TOTAL NUMBER OF STREAMS = 3
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
TIME OF CONCENTRATION(MIN.) = 9.32
RAINFALL INTENSITY(INCH/HR) = 2.84
AREA-AVERAGED Fm(INCH/HR) = 0.03
AREA-AVERAGED Fp(INCH/HR) = 0.30
AREA-AVERAGED Ap = 0.10
EFFECTIVE STREAM AREA(ACRES) = 1.52
TOTAL STREAM AREA(ACRES) = 1.52
PEAK FLOW RATE(CFS) AT CONFLUENCE = 3.85

FLOW PROCESS FROM NODE 18.00 TO NODE 19.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

=====

INITIAL SUBAREA FLOW-LENGTH(FEET) = 320.00
ELEVATION DATA: UPSTREAM(FEET) = 165.90 DOWNSTREAM(FEET) = 165.00

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$
SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) = 9.888
* 10 YEAR RAINFALL INTENSITY(INCH/HR) = 2.747
SUBAREA Tc AND LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN	Tc (MIN.)
COMMERCIAL	B	1.26	0.30	0.100	56	9.89

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.30
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.100
SUBAREA RUNOFF(CFS) = 3.08
TOTAL AREA(ACRES) = 1.26 PEAK FLOW RATE(CFS) = 3.08

FLOW PROCESS FROM NODE 19.00 TO NODE 16.00 IS CODE = 51

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<<
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 165.00 DOWNSTREAM(FEET) = 164.90
CHANNEL LENGTH THRU SUBAREA(FEET) = 265.00 CHANNEL SLOPE = 0.0004
CHANNEL BASE(FEET) = 3.00 "Z" FACTOR = 0.040
MANNING'S FACTOR = 0.015 MAXIMUM DEPTH(FEET) = 1.00
CHANNEL FLOW THRU SUBAREA(CFS) = 3.08
FLOW VELOCITY(FEET/SEC.) = 1.26 FLOW DEPTH(FEET) = 0.81
TRAVEL TIME(MIN.) = 3.50 Tc(MIN.) = 13.39
LONGEST FLOWPATH FROM NODE 18.00 TO NODE 16.00 = 585.00 FEET.

 FLOW PROCESS FROM NODE 16.00 TO NODE 16.00 IS CODE = 1

 >>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
 >>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<
 =====

TOTAL NUMBER OF STREAMS = 3
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 3 ARE:
 TIME OF CONCENTRATION(MIN.) = 13.39
 RAINFALL INTENSITY(INCH/HR) = 2.31
 AREA-AVERAGED Fm(INCH/HR) = 0.03
 AREA-AVERAGED Fp(INCH/HR) = 0.30
 AREA-AVERAGED Ap = 0.10
 EFFECTIVE STREAM AREA(ACRES) = 1.26
 TOTAL STREAM AREA(ACRES) = 1.26
 PEAK FLOW RATE(CFS) AT CONFLUENCE = 3.08

** CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	2.50	8.52	2.991	0.30 (0.03)	0.10	0.8	14.00
1	3.00	14.96	2.166	0.30 (0.03)	0.10	1.5	12.00
1	2.74	18.55	1.915	0.30 (0.03)	0.10	1.5	10.00
2	3.85	9.32	2.841	0.30 (0.03)	0.10	1.5	17.00
3	3.08	13.39	2.308	0.30 (0.03)	0.10	1.3	18.00

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
 CONFLUENCE FORMULA USED FOR 3 STREAMS.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	8.75	8.52	2.991	0.30 (0.03)	0.10	3.0	14.00
2	9.06	9.32	2.841	0.30 (0.03)	0.10	3.3	17.00
3	9.14	13.39	2.308	0.30 (0.03)	0.10	4.1	18.00
4	8.81	14.96	2.166	0.30 (0.03)	0.10	4.2	12.00
5	7.87	18.55	1.915	0.30 (0.03)	0.10	4.3	10.00

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE(CFS) = 9.14 Tc(MIN.) = 13.39
 EFFECTIVE AREA(ACRES) = 4.09 AREA-AVERAGED Fm(INCH/HR) = 0.03
 AREA-AVERAGED Fp(INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.10
 TOTAL AREA(ACRES) = 4.3
 LONGEST FLOWPATH FROM NODE 10.00 TO NODE 16.00 = 1215.00 FEET.

 FLOW PROCESS FROM NODE 16.00 TO NODE 20.00 IS CODE = 41

 >>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
 >>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<
 =====

ELEVATION DATA: UPSTREAM(FEET) = 158.10 DOWNSTREAM(FEET) = 157.60
 FLOW LENGTH(FEET) = 150.00 MANNING'S N = 0.013

DEPTH OF FLOW IN 24.0 INCH PIPE IS 15.0 INCHES
 PIPE-FLOW VELOCITY (FEET/SEC.) = 4.41
 GIVEN PIPE DIAMETER (INCH) = 24.00 NUMBER OF PIPES = 1
 PIPE-FLOW (CFS) = 9.14
 PIPE TRAVEL TIME (MIN.) = 0.57 Tc (MIN.) = 13.96
 LONGEST FLOWPATH FROM NODE 10.00 TO NODE 20.00 = 1365.00 FEET.

FLOW PROCESS FROM NODE 20.00 TO NODE 20.00 IS CODE = 1

 >>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

=====

TOTAL NUMBER OF STREAMS = 2
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
 TIME OF CONCENTRATION (MIN.) = 13.96
 RAINFALL INTENSITY (INCH/HR) = 2.25
 AREA-AVERAGED Fm (INCH/HR) = 0.03
 AREA-AVERAGED Fp (INCH/HR) = 0.30
 AREA-AVERAGED Ap = 0.10
 EFFECTIVE STREAM AREA (ACRES) = 4.09
 TOTAL STREAM AREA (ACRES) = 4.32
 PEAK FLOW RATE (CFS) AT CONFLUENCE = 9.14

FLOW PROCESS FROM NODE 21.00 TO NODE 22.00 IS CODE = 21

 >>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
 >>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

=====

INITIAL SUBAREA FLOW-LENGTH (FEET) = 260.00
 ELEVATION DATA: UPSTREAM (FEET) = 166.00 DOWNSTREAM (FEET) = 165.50

Tc = K * [(LENGTH** 3.00) / (ELEVATION CHANGE)]**0.20
 SUBAREA ANALYSIS USED MINIMUM Tc (MIN.) = 9.819
 * 10 YEAR RAINFALL INTENSITY (INCH/HR) = 2.758
 SUBAREA Tc AND LOSS RATE DATA (AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN	Tc (MIN.)
COMMERCIAL	B	0.64	0.30	0.100	56	9.82

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp (INCH/HR) = 0.30
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.100
 SUBAREA RUNOFF (CFS) = 1.57
 TOTAL AREA (ACRES) = 0.64 PEAK FLOW RATE (CFS) = 1.57

FLOW PROCESS FROM NODE 22.00 TO NODE 20.00 IS CODE = 31

 >>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
 >>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<

=====

ELEVATION DATA: UPSTREAM (FEET) = 158.20 DOWNSTREAM (FEET) = 157.60
 FLOW LENGTH (FEET) = 20.00 MANNING'S N = 0.013
 ESTIMATED PIPE DIAMETER (INCH) INCREASED TO 12.000
 DEPTH OF FLOW IN 12.0 INCH PIPE IS 4.2 INCHES

PIPE-FLOW VELOCITY (FEET/SEC.) = 6.45
 ESTIMATED PIPE DIAMETER (INCH) = 12.00 NUMBER OF PIPES = 1
 PIPE-FLOW (CFS) = 1.57
 PIPE TRAVEL TIME (MIN.) = 0.05 Tc (MIN.) = 9.87
 LONGEST FLOWPATH FROM NODE 21.00 TO NODE 20.00 = 280.00 FEET.

FLOW PROCESS FROM NODE 20.00 TO NODE 20.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
 >>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<

=====
 TOTAL NUMBER OF STREAMS = 2
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
 TIME OF CONCENTRATION (MIN.) = 9.87
 RAINFALL INTENSITY (INCH/HR) = 2.75
 AREA-AVERAGED Fm (INCH/HR) = 0.03
 AREA-AVERAGED Fp (INCH/HR) = 0.30
 AREA-AVERAGED Ap = 0.10
 EFFECTIVE STREAM AREA (ACRES) = 0.64
 TOTAL STREAM AREA (ACRES) = 0.64
 PEAK FLOW RATE (CFS) AT CONFLUENCE = 1.57

** CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp (Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	8.75	9.09	2.882	0.30 (0.03)	0.10	3.0	14.00
1	9.06	9.89	2.747	0.30 (0.03)	0.10	3.3	17.00
1	9.14	13.96	2.254	0.30 (0.03)	0.10	4.1	18.00
1	8.81	15.53	2.120	0.30 (0.03)	0.10	4.2	12.00
1	7.87	19.14	1.881	0.30 (0.03)	0.10	4.3	10.00
2	1.57	9.87	2.749	0.30 (0.03)	0.10	0.6	21.00

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
 CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp (Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	10.27	9.09	2.882	0.30 (0.03)	0.10	3.6	14.00
2	10.62	9.87	2.749	0.30 (0.03)	0.10	3.9	21.00
3	10.63	9.89	2.747	0.30 (0.03)	0.10	3.9	17.00
4	10.42	13.96	2.254	0.30 (0.03)	0.10	4.7	18.00
5	10.02	15.53	2.120	0.30 (0.03)	0.10	4.9	12.00
6	8.94	19.14	1.881	0.30 (0.03)	0.10	5.0	10.00

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE (CFS) = 10.63 Tc (MIN.) = 9.89
 EFFECTIVE AREA (ACRES) = 3.95 AREA-AVERAGED Fm (INCH/HR) = 0.03
 AREA-AVERAGED Fp (INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.10
 TOTAL AREA (ACRES) = 5.0
 LONGEST FLOWPATH FROM NODE 10.00 TO NODE 20.00 = 1365.00 FEET.

FLOW PROCESS FROM NODE 20.00 TO NODE 23.00 IS CODE = 41

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

ELEVATION DATA: UPSTREAM(FEET) = 157.60 DOWNSTREAM(FEET) = 156.40
FLOW LENGTH(FEET) = 365.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 24.0 INCH PIPE IS 16.8 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 4.51
GIVEN PIPE DIAMETER(INCH) = 24.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 10.63
PIPE TRAVEL TIME(MIN.) = 1.35 Tc(MIN.) = 11.24
LONGEST FLOWPATH FROM NODE 10.00 TO NODE 23.00 = 1730.00 FEET.

FLOW PROCESS FROM NODE 23.00 TO NODE 23.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION(MIN.) = 11.24
RAINFALL INTENSITY(INCH/HR) = 2.55
AREA-AVERAGED Fm(INCH/HR) = 0.03
AREA-AVERAGED Fp(INCH/HR) = 0.30
AREA-AVERAGED Ap = 0.10
EFFECTIVE STREAM AREA(ACRES) = 3.95
TOTAL STREAM AREA(ACRES) = 4.96
PEAK FLOW RATE(CFS) AT CONFLUENCE = 10.63

FLOW PROCESS FROM NODE 24.00 TO NODE 25.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

INITIAL SUBAREA FLOW-LENGTH(FEET) = 320.00
ELEVATION DATA: UPSTREAM(FEET) = 166.00 DOWNSTREAM(FEET) = 165.50

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$

SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) = 11.122

* 10 YEAR RAINFALL INTENSITY(INCH/HR) = 2.568

SUBAREA Tc AND LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN	Tc (MIN.)
COMMERCIAL	B	1.40	0.30	0.100	56	11.12

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.30

SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.100

SUBAREA RUNOFF(CFS) = 3.20

TOTAL AREA(ACRES) = 1.40 PEAK FLOW RATE(CFS) = 3.20

FLOW PROCESS FROM NODE 25.00 TO NODE 26.00 IS CODE = 51

>>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<<
>>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 165.50 DOWNSTREAM(FEET) = 161.10
CHANNEL LENGTH THRU SUBAREA(FEET) = 330.00 CHANNEL SLOPE = 0.0133
CHANNEL BASE(FEET) = 20.00 "Z" FACTOR = 0.020
MANNING'S FACTOR = 0.015 MAXIMUM DEPTH(FEET) = 2.00
* 10 YEAR RAINFALL INTENSITY(INCH/HR) = 2.343
SUBAREA LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
COMMERCIAL	B	3.87	0.30	0.100	56

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.30
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.100
TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 7.23
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) = 2.85
AVERAGE FLOW DEPTH(FEET) = 0.13 TRAVEL TIME(MIN.) = 1.93
Tc(MIN.) = 13.05
SUBAREA AREA(ACRES) = 3.87 SUBAREA RUNOFF(CFS) = 8.05
EFFECTIVE AREA(ACRES) = 5.27 AREA-AVERAGED Fm(INCH/HR) = 0.03
AREA-AVERAGED Fp(INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.10
TOTAL AREA(ACRES) = 5.3 PEAK FLOW RATE(CFS) = 10.97

END OF SUBAREA CHANNEL FLOW HYDRAULICS:

DEPTH(FEET) = 0.16 FLOW VELOCITY(FEET/SEC.) = 3.36
LONGEST FLOWPATH FROM NODE 24.00 TO NODE 26.00 = 650.00 FEET.

FLOW PROCESS FROM NODE 26.00 TO NODE 23.00 IS CODE = 41

>>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 158.00 DOWNSTREAM(FEET) = 156.40
FLOW LENGTH(FEET) = 315.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 24.0 INCH PIPE IS 14.8 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 5.41
GIVEN PIPE DIAMETER(INCH) = 24.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 10.97
PIPE TRAVEL TIME(MIN.) = 0.97 Tc(MIN.) = 14.02
LONGEST FLOWPATH FROM NODE 24.00 TO NODE 23.00 = 965.00 FEET.

FLOW PROCESS FROM NODE 23.00 TO NODE 23.00 IS CODE = 1

>>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
>>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<

=====

TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
TIME OF CONCENTRATION(MIN.) = 14.02
RAINFALL INTENSITY(INCH/HR) = 2.25
AREA-AVERAGED Fm(INCH/HR) = 0.03
AREA-AVERAGED Fp(INCH/HR) = 0.30

AREA-AVERAGED $A_p = 0.10$
 EFFECTIVE STREAM AREA (ACRES) = 5.27
 TOTAL STREAM AREA (ACRES) = 5.27
 PEAK FLOW RATE (CFS) AT CONFLUENCE = 10.97

** CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp (Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	10.27	10.45	2.661	0.30 (0.03)	0.10	3.6	14.00
1	10.62	11.22	2.555	0.30 (0.03)	0.10	3.9	21.00
1	10.63	11.24	2.552	0.30 (0.03)	0.10	3.9	17.00
1	10.42	15.31	2.138	0.30 (0.03)	0.10	4.7	18.00
1	10.02	16.90	2.020	0.30 (0.03)	0.10	4.9	12.00
1	8.94	20.53	1.807	0.30 (0.03)	0.10	5.0	10.00
2	10.97	14.02	2.248	0.30 (0.03)	0.10	5.3	24.00

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
 CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp (Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	19.97	10.45	2.661	0.30 (0.03)	0.10	7.5	14.00
2	20.61	11.22	2.555	0.30 (0.03)	0.10	8.2	21.00
3	20.62	11.24	2.552	0.30 (0.03)	0.10	8.2	17.00
4	21.46	14.02	2.248	0.30 (0.03)	0.10	9.7	24.00
5	20.85	15.31	2.138	0.30 (0.03)	0.10	10.0	18.00
6	19.86	16.90	2.020	0.30 (0.03)	0.10	10.1	12.00
7	17.73	20.53	1.807	0.30 (0.03)	0.10	10.2	10.00

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE (CFS) = 21.46 Tc (MIN.) = 14.02
 EFFECTIVE AREA (ACRES) = 9.75 AREA-AVERAGED Fm (INCH/HR) = 0.03
 AREA-AVERAGED Fp (INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.10
 TOTAL AREA (ACRES) = 10.2
 LONGEST FLOWPATH FROM NODE 10.00 TO NODE 23.00 = 1730.00 FEET.

 FLOW PROCESS FROM NODE 23.00 TO NODE 27.00 IS CODE = 41

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
 >>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM (FEET) = 156.40 DOWNSTREAM (FEET) = 155.40
 FLOW LENGTH (FEET) = 235.00 MANNING'S N = 0.013
 DEPTH OF FLOW IN 30.0 INCH PIPE IS 20.7 INCHES
 PIPE-FLOW VELOCITY (FEET/SEC.) = 5.93
 GIVEN PIPE DIAMETER (INCH) = 30.00 NUMBER OF PIPES = 1
 PIPE-FLOW (CFS) = 21.46
 PIPE TRAVEL TIME (MIN.) = 0.66 Tc (MIN.) = 14.68
 LONGEST FLOWPATH FROM NODE 10.00 TO NODE 27.00 = 1965.00 FEET.

 FLOW PROCESS FROM NODE 27.00 TO NODE 27.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

=====

TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION(MIN.) = 14.68
RAINFALL INTENSITY(INCH/HR) = 2.19
AREA-AVERAGED Fm(INCH/HR) = 0.03
AREA-AVERAGED Fp(INCH/HR) = 0.30
AREA-AVERAGED Ap = 0.10
EFFECTIVE STREAM AREA(ACRES) = 9.75
TOTAL STREAM AREA(ACRES) = 10.23
PEAK FLOW RATE(CFS) AT CONFLUENCE = 21.46

FLOW PROCESS FROM NODE 28.00 TO NODE 29.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

=====

INITIAL SUBAREA FLOW-LENGTH(FEET) = 225.00
ELEVATION DATA: UPSTREAM(FEET) = 170.00 DOWNSTREAM(FEET) = 166.50

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$

SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) = 6.101

* 10 YEAR RAINFALL INTENSITY(INCH/HR) = 3.622

SUBAREA Tc AND LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN	Tc (MIN.)
COMMERCIAL	B	1.32	0.30	0.100	56	6.10

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.30

SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.100

SUBAREA RUNOFF(CFS) = 4.27

TOTAL AREA(ACRES) = 1.32 PEAK FLOW RATE(CFS) = 4.27

FLOW PROCESS FROM NODE 29.00 TO NODE 30.00 IS CODE = 51

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<<
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 166.50 DOWNSTREAM(FEET) = 164.20

CHANNEL LENGTH THRU SUBAREA(FEET) = 300.00 CHANNEL SLOPE = 0.0077

CHANNEL BASE(FEET) = 5.00 "Z" FACTOR = 1.500

MANNING'S FACTOR = 0.015 MAXIMUM DEPTH(FEET) = 2.00

* 10 YEAR RAINFALL INTENSITY(INCH/HR) = 3.262

SUBAREA LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
COMMERCIAL	B	2.72	0.30	0.100	56

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.30

SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.100

TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 8.23

TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) = 4.08

AVERAGE FLOW DEPTH (FEET) = 0.36 TRAVEL TIME (MIN.) = 1.22
Tc (MIN.) = 7.32
SUBAREA AREA (ACRES) = 2.72 SUBAREA RUNOFF (CFS) = 7.91
EFFECTIVE AREA (ACRES) = 4.04 AREA-AVERAGED Fm (INCH/HR) = 0.03
AREA-AVERAGED Fp (INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.10
TOTAL AREA (ACRES) = 4.0 PEAK FLOW RATE (CFS) = 11.75

END OF SUBAREA CHANNEL FLOW HYDRAULICS:

DEPTH (FEET) = 0.45 FLOW VELOCITY (FEET/SEC.) = 4.57
LONGEST FLOWPATH FROM NODE 28.00 TO NODE 30.00 = 525.00 FEET.

FLOW PROCESS FROM NODE 30.00 TO NODE 30.00 IS CODE = 82

>>>>ADD SUBAREA RUNOFF TO MAINLINE, AT MAINLINE Tc,<<<<<
>>>>(AND COMPUTE INITIAL SUBAREA RUNOFF)<<<<<

=====

INITIAL SUBAREA FLOW-LENGTH (FEET) = 300.00
ELEVATION DATA: UPSTREAM (FEET) = 167.40 DOWNSTREAM (FEET) = 164.20

$$Tc = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$$

SUBAREA ANALYSIS USED MINIMUM Tc (MIN.) = 7.381

* 10 YEAR RAINFALL INTENSITY (INCH/HR) = 3.248

SUBAREA Tc AND LOSS RATE DATA (AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN	Tc (MIN.)
COMMERCIAL	B	2.23	0.30	0.100	56	7.38

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp (INCH/HR) = 0.30

SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.100

SUBAREA AREA (ACRES) = 2.23 INITIAL SUBAREA RUNOFF (CFS) = 6.46

** ADD SUBAREA RUNOFF TO MAINLINE AT MAINLINE Tc:

MAINLINE Tc (MIN.) = 7.32

* 10 YEAR RAINFALL INTENSITY (INCH/HR) = 3.262

SUBAREA AREA (ACRES) = 2.23 SUBAREA RUNOFF (CFS) = 6.49

EFFECTIVE AREA (ACRES) = 6.27 AREA-AVERAGED Fm (INCH/HR) = 0.03

AREA-AVERAGED Fp (INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.10

TOTAL AREA (ACRES) = 6.3 PEAK FLOW RATE (CFS) = 18.24

FLOW PROCESS FROM NODE 30.00 TO NODE 27.00 IS CODE = 41

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM (FEET) = 158.00 DOWNSTREAM (FEET) = 155.40

FLOW LENGTH (FEET) = 160.00 MANNING'S N = 0.013

DEPTH OF FLOW IN 24.0 INCH PIPE IS 14.1 INCHES

PIPE-FLOW VELOCITY (FEET/SEC.) = 9.52

GIVEN PIPE DIAMETER (INCH) = 24.00 NUMBER OF PIPES = 1

PIPE-FLOW (CFS) = 18.24

PIPE TRAVEL TIME (MIN.) = 0.28 Tc (MIN.) = 7.60

LONGEST FLOWPATH FROM NODE 28.00 TO NODE 27.00 = 685.00 FEET.

FLOW PROCESS FROM NODE 27.00 TO NODE 27.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<

TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
TIME OF CONCENTRATION(MIN.) = 7.60
RAINFALL INTENSITY(INCH/HR) = 3.19
AREA-AVERAGED Fm(INCH/HR) = 0.03
AREA-AVERAGED Fp(INCH/HR) = 0.30
AREA-AVERAGED Ap = 0.10
EFFECTIVE STREAM AREA(ACRES) = 6.27
TOTAL STREAM AREA(ACRES) = 6.27
PEAK FLOW RATE(CFS) AT CONFLUENCE = 18.24

** CONFLUENCE DATA **

Table with 8 columns: STREAM NUMBER, Q (CFS), Tc (MIN.), Intensity (INCH/HR), Fp(Fm) (INCH/HR), Ap, Ae (ACRES), HEADWATER NODE. Contains 8 rows of data.

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **

Table with 8 columns: STREAM NUMBER, Q (CFS), Tc (MIN.), Intensity (INCH/HR), Fp(Fm) (INCH/HR), Ap, Ae (ACRES), HEADWATER NODE. Contains 8 rows of data.

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE(CFS) = 35.25 Tc(MIN.) = 7.60
EFFECTIVE AREA(ACRES) = 11.43 AREA-AVERAGED Fm(INCH/HR) = 0.03
AREA-AVERAGED Fp(INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.10
TOTAL AREA(ACRES) = 16.5
LONGEST FLOWPATH FROM NODE 10.00 TO NODE 27.00 = 1965.00 FEET.

FLOW PROCESS FROM NODE 27.00 TO NODE 31.00 IS CODE = 41

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 155.40 DOWNSTREAM(FEET) = 155.00
FLOW LENGTH(FEET) = 105.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 36.0 INCH PIPE IS 26.2 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 6.40
GIVEN PIPE DIAMETER(INCH) = 36.00 NUMBER OF PIPES = 1
PIPE-FLOW (CFS) = 35.25
PIPE TRAVEL TIME(MIN.) = 0.27 Tc(MIN.) = 7.88
LONGEST FLOWPATH FROM NODE 10.00 TO NODE 31.00 = 2070.00 FEET.

FLOW PROCESS FROM NODE 31.00 TO NODE 31.00 IS CODE = 10

>>>>MAIN-STREAM MEMORY COPIED ONTO MEMORY BANK # 2 <<<<<

FLOW PROCESS FROM NODE 32.00 TO NODE 33.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

=====

INITIAL SUBAREA FLOW-LENGTH(FEET) = 300.00
ELEVATION DATA: UPSTREAM(FEET) = 166.30 DOWNSTREAM(FEET) = 166.00

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$
SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) = 11.850
* 10 YEAR RAINFALL INTENSITY(INCH/HR) = 2.476

SUBAREA Tc AND LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN	Tc (MIN.)
COMMERCIAL	B	2.62	0.30	0.100	56	11.85

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.30

SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.100

SUBAREA RUNOFF(CFS) = 5.77

TOTAL AREA(ACRES) = 2.62 PEAK FLOW RATE(CFS) = 5.77

FLOW PROCESS FROM NODE 33.00 TO NODE 34.00 IS CODE = 61

>>>>COMPUTE STREET FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>(STANDARD CURB SECTION USED)<<<<<

=====

UPSTREAM ELEVATION(FEET) = 166.00 DOWNSTREAM ELEVATION(FEET) = 165.60
STREET LENGTH(FEET) = 485.00 CURB HEIGHT(INCHES) = 6.0
STREET HALFWIDTH(FEET) = 25.00

DISTANCE FROM CROWN TO CROSSFALL GRADEBREAK(FEET) = 10.00

INSIDE STREET CROSSFALL(DECIMAL) = 0.020

OUTSIDE STREET CROSSFALL(DECIMAL) = 0.020

SPECIFIED NUMBER OF HALFSTREETS CARRYING RUNOFF = 2

STREET PARKWAY CROSSFALL(DECIMAL) = 0.020
Manning's FRICTION FACTOR for Streetflow Section(curbs-to-curbs) = 0.0150
Manning's FRICTION FACTOR for Back-of-Walk Flow Section = 0.0200

**TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 9.78
STREETFLOW MODEL RESULTS USING ESTIMATED FLOW:
STREET FLOW DEPTH(FEET) = 0.56
HALFSTREET FLOOD WIDTH(FEET) = 24.36
AVERAGE FLOW VELOCITY(FEET/SEC.) = 1.01
PRODUCT OF DEPTH&VELOCITY(FT*FT/SEC.) = 0.56
STREET FLOW TRAVEL TIME(MIN.) = 7.99 Tc(MIN.) = 19.84
* 10 YEAR RAINFALL INTENSITY(INCH/HR) = 1.843

SUBAREA LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
COMMERCIAL	B	4.89	0.30	0.100	56

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.30
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.100
SUBAREA AREA(ACRES) = 4.89 SUBAREA RUNOFF(CFS) = 7.98
EFFECTIVE AREA(ACRES) = 7.51 AREA-AVERAGED Fm(INCH/HR) = 0.03
AREA-AVERAGED Fp(INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.10
TOTAL AREA(ACRES) = 7.5 PEAK FLOW RATE(CFS) = 12.25

END OF SUBAREA STREET FLOW HYDRAULICS:
DEPTH(FEET) = 0.60 HALFSTREET FLOOD WIDTH(FEET) = 28.27
FLOW VELOCITY(FEET/SEC.) = 1.05 DEPTH*VELOCITY(FT*FT/SEC.) = 0.62
*NOTE: INITIAL SUBAREA NOMOGRAPH WITH SUBAREA PARAMETERS,
AND L = 485.0 FT WITH ELEVATION-DROP = 0.4 FT, IS 9.4 CFS,
WHICH EXCEEDS THE TOP-OF-CURB STREET CAPACITY AT NODE 34.00
LONGEST FLOWPATH FROM NODE 32.00 TO NODE 34.00 = 785.00 FEET.

FLOW PROCESS FROM NODE 34.00 TO NODE 35.00 IS CODE = 51

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<<
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) =	165.60	DOWNSTREAM(FEET) =	165.40
CHANNEL LENGTH THRU SUBAREA(FEET) =	180.00	CHANNEL SLOPE =	0.0011
CHANNEL BASE(FEET) =	5.00	"Z" FACTOR =	0.020
MANNING'S FACTOR =	0.015	MAXIMUM DEPTH(FEET) =	2.00
CHANNEL FLOW THRU SUBAREA(CFS) =	12.25		
FLOW VELOCITY(FEET/SEC.) =	2.57	FLOW DEPTH(FEET) =	0.95
TRAVEL TIME(MIN.) =	1.17	Tc(MIN.) =	21.00
LONGEST FLOWPATH FROM NODE	32.00	TO NODE	35.00 = 965.00 FEET.

FLOW PROCESS FROM NODE 35.00 TO NODE 35.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

=====

TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION(MIN.) = 21.00

RAINFALL INTENSITY (INCH/HR) = 1.78
 AREA-AVERAGED Fm (INCH/HR) = 0.03
 AREA-AVERAGED Fp (INCH/HR) = 0.30
 AREA-AVERAGED Ap = 0.10
 EFFECTIVE STREAM AREA (ACRES) = 7.51
 TOTAL STREAM AREA (ACRES) = 7.51
 PEAK FLOW RATE (CFS) AT CONFLUENCE = 12.25

 FLOW PROCESS FROM NODE 35.00 TO NODE 35.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
 >>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

INITIAL SUBAREA FLOW-LENGTH (FEET) = 665.00
 ELEVATION DATA: UPSTREAM (FEET) = 167.20 DOWNSTREAM (FEET) = 165.40

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$
 SUBAREA ANALYSIS USED MINIMUM Tc (MIN.) = 13.351
 * 10 YEAR RAINFALL INTENSITY (INCH/HR) = 2.312

SUBAREA Tc AND LOSS RATE DATA (AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN	Tc (MIN.)
COMMERCIAL	B	6.31	0.30	0.100	56	13.35

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp (INCH/HR) = 0.30

SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.100

SUBAREA RUNOFF (CFS) = 12.96

TOTAL AREA (ACRES) = 6.31 PEAK FLOW RATE (CFS) = 12.96

 FLOW PROCESS FROM NODE 35.00 TO NODE 35.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
 >>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<

TOTAL NUMBER OF STREAMS = 2
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
 TIME OF CONCENTRATION (MIN.) = 13.35
 RAINFALL INTENSITY (INCH/HR) = 2.31
 AREA-AVERAGED Fm (INCH/HR) = 0.03
 AREA-AVERAGED Fp (INCH/HR) = 0.30
 AREA-AVERAGED Ap = 0.10
 EFFECTIVE STREAM AREA (ACRES) = 6.31
 TOTAL STREAM AREA (ACRES) = 6.31
 PEAK FLOW RATE (CFS) AT CONFLUENCE = 12.96

** CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp (Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	12.25	21.00	1.784	0.30 (0.03)	0.10	7.5	32.00
2	12.96	13.35	2.312	0.30 (0.03)	0.10	6.3	35.00

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
 CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	23.10	13.35	2.312	0.30 (0.03)	0.10	11.1	35.00
2	22.21	21.00	1.784	0.30 (0.03)	0.10	13.8	32.00

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE (CFS) = 23.10 Tc (MIN.) = 13.35
 EFFECTIVE AREA (ACRES) = 11.08 AREA-AVERAGED Fm (INCH/HR) = 0.03
 AREA-AVERAGED Fp (INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.10
 TOTAL AREA (ACRES) = 13.8
 LONGEST FLOWPATH FROM NODE 32.00 TO NODE 35.00 = 965.00 FEET.

FLOW PROCESS FROM NODE 35.00 TO NODE 31.00 IS CODE = 41

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
 >>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM (FEET) = 159.00 DOWNSTREAM (FEET) = 155.00
 FLOW LENGTH (FEET) = 50.00 MANNING'S N = 0.013
 DEPTH OF FLOW IN 18.0 INCH PIPE IS 12.2 INCHES
 PIPE-FLOW VELOCITY (FEET/SEC.) = 18.19
 GIVEN PIPE DIAMETER (INCH) = 18.00 NUMBER OF PIPES = 1
 PIPE-FLOW (CFS) = 23.10
 PIPE TRAVEL TIME (MIN.) = 0.05 Tc (MIN.) = 13.40
 LONGEST FLOWPATH FROM NODE 32.00 TO NODE 31.00 = 1015.00 FEET.

FLOW PROCESS FROM NODE 31.00 TO NODE 31.00 IS CODE = 11

>>>>CONFLUENCE MEMORY BANK # 2 WITH THE MAIN-STREAM MEMORY<<<<<

** MAIN STREAM CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	23.10	13.40	2.308	0.30 (0.03)	0.10	11.1	35.00
2	22.21	21.05	1.781	0.30 (0.03)	0.10	13.8	32.00

LONGEST FLOWPATH FROM NODE 32.00 TO NODE 31.00 = 1015.00 FEET.

** MEMORY BANK # 2 CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	35.25	7.88	3.128	0.30 (0.03)	0.10	11.4	28.00
2	34.60	11.39	2.532	0.30 (0.03)	0.10	13.8	14.00
3	34.69	12.16	2.440	0.30 (0.03)	0.10	14.4	21.00
4	34.69	12.18	2.438	0.30 (0.03)	0.10	14.4	17.00
5	33.91	14.96	2.167	0.30 (0.03)	0.10	16.0	24.00
6	32.71	16.25	2.066	0.30 (0.03)	0.10	16.3	18.00
7	31.08	17.85	1.958	0.30 (0.03)	0.10	16.4	12.00
8	27.78	21.50	1.760	0.30 (0.03)	0.10	16.5	10.00

LONGEST FLOWPATH FROM NODE 10.00 TO NODE 31.00 = 2070.00 FEET.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	53.73	7.88	3.128	0.30 (0.03)	0.10	17.9	28.00
2	56.18	11.39	2.532	0.30 (0.03)	0.10	23.2	14.00
3	56.87	12.16	2.440	0.30 (0.03)	0.10	24.5	21.00
4	56.88	12.18	2.438	0.30 (0.03)	0.10	24.5	17.00
5	57.45	13.40	2.308	0.30 (0.03)	0.10	26.2	35.00
6	56.83	14.96	2.167	0.30 (0.03)	0.10	27.7	24.00
7	55.47	16.25	2.066	0.30 (0.03)	0.10	28.4	18.00
8	53.67	17.85	1.958	0.30 (0.03)	0.10	29.1	12.00
9	50.40	21.05	1.781	0.30 (0.03)	0.10	30.3	32.00
10	49.72	21.50	1.760	0.30 (0.03)	0.10	30.3	10.00
TOTAL AREA (ACRES) =			30.3				

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE (CFS) = 57.45 Tc (MIN.) = 13.397
 EFFECTIVE AREA (ACRES) = 26.22 AREA-AVERAGED Fm (INCH/HR) = 0.03
 AREA-AVERAGED Fp (INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.10
 TOTAL AREA (ACRES) = 30.3
 LONGEST FLOWPATH FROM NODE 10.00 TO NODE 31.00 = 2070.00 FEET.

FLOW PROCESS FROM NODE 31.00 TO NODE 36.00 IS CODE = 41

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
 >>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM (FEET) = 155.00 DOWNSTREAM (FEET) = 153.80
 FLOW LENGTH (FEET) = 265.00 MANNING'S N = 0.013
 DEPTH OF FLOW IN 42.0 INCH PIPE IS 30.3 INCHES
 PIPE-FLOW VELOCITY (FEET/SEC.) = 7.73
 GIVEN PIPE DIAMETER (INCH) = 42.00 NUMBER OF PIPES = 1
 PIPE-FLOW (CFS) = 57.45
 PIPE TRAVEL TIME (MIN.) = 0.57 Tc (MIN.) = 13.97
 LONGEST FLOWPATH FROM NODE 10.00 TO NODE 36.00 = 2335.00 FEET.

FLOW PROCESS FROM NODE 36.00 TO NODE 36.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

=====

TOTAL NUMBER OF STREAMS = 2
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
 TIME OF CONCENTRATION (MIN.) = 13.97
 RAINFALL INTENSITY (INCH/HR) = 2.25
 AREA-AVERAGED Fm (INCH/HR) = 0.03
 AREA-AVERAGED Fp (INCH/HR) = 0.30
 AREA-AVERAGED Ap = 0.10
 EFFECTIVE STREAM AREA (ACRES) = 26.22
 TOTAL STREAM AREA (ACRES) = 30.32
 PEAK FLOW RATE (CFS) AT CONFLUENCE = 57.45

FLOW PROCESS FROM NODE 37.00 TO NODE 38.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

INITIAL SUBAREA FLOW-LENGTH (FEET) = 325.00
ELEVATION DATA: UPSTREAM (FEET) = 169.50 DOWNSTREAM (FEET) = 168.00

Tc = K * [(LENGTH** 3.00) / (ELEVATION CHANGE)]**0.20
SUBAREA ANALYSIS USED MINIMUM Tc (MIN.) = 9.011
* 10 YEAR RAINFALL INTENSITY (INCH/HR) = 2.897

SUBAREA Tc AND LOSS RATE DATA (AMC II):

DEVELOPMENT TYPE/ SCS SOIL AREA Fp Ap SCS Tc
LAND USE GROUP (ACRES) (INCH/HR) (DECIMAL) CN (MIN.)
COMMERCIAL B 1.65 0.30 0.100 56 9.01

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp (INCH/HR) = 0.30

SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.100

SUBAREA RUNOFF (CFS) = 4.26

TOTAL AREA (ACRES) = 1.65 PEAK FLOW RATE (CFS) = 4.26

FLOW PROCESS FROM NODE 38.00 TO NODE 36.00 IS CODE = 41

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

ELEVATION DATA: UPSTREAM (FEET) = 159.00 DOWNSTREAM (FEET) = 153.80
FLOW LENGTH (FEET) = 70.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 18.0 INCH PIPE IS 4.8 INCHES
PIPE-FLOW VELOCITY (FEET/SEC.) = 11.42
GIVEN PIPE DIAMETER (INCH) = 18.00 NUMBER OF PIPES = 1
PIPE-FLOW (CFS) = 4.26
PIPE TRAVEL TIME (MIN.) = 0.10 Tc (MIN.) = 9.11
LONGEST FLOWPATH FROM NODE 37.00 TO NODE 36.00 = 395.00 FEET.

FLOW PROCESS FROM NODE 36.00 TO NODE 36.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<

TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
TIME OF CONCENTRATION (MIN.) = 9.11
RAINFALL INTENSITY (INCH/HR) = 2.88
AREA-AVERAGED Fm (INCH/HR) = 0.03
AREA-AVERAGED Fp (INCH/HR) = 0.30
AREA-AVERAGED Ap = 0.10
EFFECTIVE STREAM AREA (ACRES) = 1.65
TOTAL STREAM AREA (ACRES) = 1.65
PEAK FLOW RATE (CFS) AT CONFLUENCE = 4.26

** CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	53.73	8.46	3.004	0.30 (0.03)	0.10	17.9	28.00
1	56.18	11.97	2.462	0.30 (0.03)	0.10	23.2	14.00
1	56.87	12.73	2.376	0.30 (0.03)	0.10	24.5	21.00
1	56.88	12.75	2.374	0.30 (0.03)	0.10	24.5	17.00
1	57.45	13.97	2.253	0.30 (0.03)	0.10	26.2	35.00
1	56.83	15.53	2.120	0.30 (0.03)	0.10	27.7	24.00
1	55.47	16.83	2.025	0.30 (0.03)	0.10	28.4	18.00
1	53.67	18.42	1.923	0.30 (0.03)	0.10	29.1	12.00
1	50.40	21.63	1.754	0.30 (0.03)	0.10	30.3	32.00
1	49.72	22.09	1.733	0.30 (0.03)	0.10	30.3	10.00
2	4.26	9.11	2.878	0.30 (0.03)	0.10	1.6	37.00

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	57.86	8.46	3.004	0.30 (0.03)	0.10	19.5	28.00
2	58.45	9.11	2.878	0.30 (0.03)	0.10	20.6	37.00
3	59.82	11.97	2.462	0.30 (0.03)	0.10	24.9	14.00
4	60.38	12.73	2.376	0.30 (0.03)	0.10	26.1	21.00
5	60.39	12.75	2.374	0.30 (0.03)	0.10	26.2	17.00
6	60.77	13.97	2.253	0.30 (0.03)	0.10	27.9	35.00
7	59.95	15.53	2.120	0.30 (0.03)	0.10	29.3	24.00
8	58.46	16.83	2.025	0.30 (0.03)	0.10	30.0	18.00
9	56.50	18.42	1.923	0.30 (0.03)	0.10	30.7	12.00
10	52.98	21.63	1.754	0.30 (0.03)	0.10	32.0	32.00
11	52.26	22.09	1.733	0.30 (0.03)	0.10	32.0	10.00

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE (CFS) = 60.77 Tc (MIN.) = 13.97
EFFECTIVE AREA (ACRES) = 27.87 AREA-AVERAGED Fm (INCH/HR) = 0.03
AREA-AVERAGED Fp (INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.10
TOTAL AREA (ACRES) = 32.0
LONGEST FLOWPATH FROM NODE 10.00 TO NODE 36.00 = 2335.00 FEET.

FLOW PROCESS FROM NODE 36.00 TO NODE 39.00 IS CODE = 41

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM (FEET) = 153.80 DOWNSTREAM (FEET) = 152.80
FLOW LENGTH (FEET) = 230.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 42.0 INCH PIPE IS 32.4 INCHES
PIPE-FLOW VELOCITY (FEET/SEC.) = 7.64
GIVEN PIPE DIAMETER (INCH) = 42.00 NUMBER OF PIPES = 1
PIPE-FLOW (CFS) = 60.77
PIPE TRAVEL TIME (MIN.) = 0.50 Tc (MIN.) = 14.47
LONGEST FLOWPATH FROM NODE 10.00 TO NODE 39.00 = 2565.00 FEET.

FLOW PROCESS FROM NODE 39.00 TO NODE 39.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION(MIN.) = 14.47
RAINFALL INTENSITY(INCH/HR) = 2.21
AREA-AVERAGED Fm(INCH/HR) = 0.03
AREA-AVERAGED Fp(INCH/HR) = 0.30
AREA-AVERAGED Ap = 0.10
EFFECTIVE STREAM AREA(ACRES) = 27.87
TOTAL STREAM AREA(ACRES) = 31.97
PEAK FLOW RATE(CFS) AT CONFLUENCE = 60.77

FLOW PROCESS FROM NODE 40.00 TO NODE 39.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

INITIAL SUBAREA FLOW-LENGTH(FEET) = 320.00
ELEVATION DATA: UPSTREAM(FEET) = 167.40 DOWNSTREAM(FEET) = 167.20

Tc = K*[(LENGTH** 3.00)/(ELEVATION CHANGE)]**0.20
SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) = 13.359
* 10 YEAR RAINFALL INTENSITY(INCH/HR) = 2.312
SUBAREA Tc AND LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ SCS SOIL AREA Fp Ap SCS Tc
LAND USE GROUP (ACRES) (INCH/HR) (DECIMAL) CN (MIN.)
COMMERCIAL B 0.48 0.30 0.100 56 13.36
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.30
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.100
SUBAREA RUNOFF(CFS) = 0.99
TOTAL AREA(ACRES) = 0.48 PEAK FLOW RATE(CFS) = 0.99

FLOW PROCESS FROM NODE 39.00 TO NODE 39.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<

TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
TIME OF CONCENTRATION(MIN.) = 13.36
RAINFALL INTENSITY(INCH/HR) = 2.31
AREA-AVERAGED Fm(INCH/HR) = 0.03
AREA-AVERAGED Fp(INCH/HR) = 0.30
AREA-AVERAGED Ap = 0.10
EFFECTIVE STREAM AREA(ACRES) = 0.48
TOTAL STREAM AREA(ACRES) = 0.48
PEAK FLOW RATE(CFS) AT CONFLUENCE = 0.99

** CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	57.86	8.96	2.906	0.30 (0.03)	0.10	19.5	28.00
1	58.45	9.62	2.791	0.30 (0.03)	0.10	20.6	37.00
1	59.82	12.47	2.405	0.30 (0.03)	0.10	24.9	14.00
1	60.38	13.23	2.324	0.30 (0.03)	0.10	26.1	21.00
1	60.39	13.25	2.322	0.30 (0.03)	0.10	26.2	17.00
1	60.77	14.47	2.208	0.30 (0.03)	0.10	27.9	35.00
1	59.95	16.03	2.082	0.30 (0.03)	0.10	29.3	24.00
1	58.46	17.33	1.991	0.30 (0.03)	0.10	30.0	18.00
1	56.50	18.93	1.893	0.30 (0.03)	0.10	30.7	12.00
1	52.98	22.15	1.730	0.30 (0.03)	0.10	32.0	32.00
1	52.26	22.60	1.710	0.30 (0.03)	0.10	32.0	10.00
2	0.99	13.36	2.312	0.30 (0.03)	0.10	0.5	40.00

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	58.69	8.96	2.906	0.30 (0.03)	0.10	19.8	28.00
2	59.31	9.62	2.791	0.30 (0.03)	0.10	20.9	37.00
3	60.78	12.47	2.405	0.30 (0.03)	0.10	25.3	14.00
4	61.36	13.23	2.324	0.30 (0.03)	0.10	26.6	21.00
5	61.37	13.25	2.322	0.30 (0.03)	0.10	26.6	17.00
6	61.41	13.36	2.312	0.30 (0.03)	0.10	26.8	40.00
7	61.71	14.47	2.208	0.30 (0.03)	0.10	28.3	35.00
8	60.84	16.03	2.082	0.30 (0.03)	0.10	29.8	24.00
9	59.30	17.33	1.991	0.30 (0.03)	0.10	30.5	18.00
10	57.30	18.93	1.893	0.30 (0.03)	0.10	31.2	12.00
11	53.71	22.15	1.730	0.30 (0.03)	0.10	32.4	32.00
12	52.99	22.60	1.710	0.30 (0.03)	0.10	32.5	10.00

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE (CFS) = 61.71 Tc (MIN.) = 14.47
 EFFECTIVE AREA (ACRES) = 28.35 AREA-AVERAGED Fm (INCH/HR) = 0.03
 AREA-AVERAGED Fp (INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.10
 TOTAL AREA (ACRES) = 32.5
 LONGEST FLOWPATH FROM NODE 10.00 TO NODE 39.00 = 2565.00 FEET.

 FLOW PROCESS FROM NODE 39.00 TO NODE 41.00 IS CODE = 41

>>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
 >>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM (FEET) = 152.80 DOWNSTREAM (FEET) = 152.00
 FLOW LENGTH (FEET) = 210.00 MANNING'S N = 0.013
 DEPTH OF FLOW IN 48.0 INCH PIPE IS 30.0 INCHES
 PIPE-FLOW VELOCITY (FEET/SEC.) = 7.47
 GIVEN PIPE DIAMETER (INCH) = 48.00 NUMBER OF PIPES = 1
 PIPE-FLOW (CFS) = 61.71

PIPE TRAVEL TIME(MIN.) = 0.47 Tc(MIN.) = 14.94
LONGEST FLOWPATH FROM NODE 10.00 TO NODE 41.00 = 2775.00 FEET.

FLOW PROCESS FROM NODE 41.00 TO NODE 41.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

=====

TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION(MIN.) = 14.94
RAINFALL INTENSITY(INCH/HR) = 2.17
AREA-AVERAGED Fm(INCH/HR) = 0.03
AREA-AVERAGED Fp(INCH/HR) = 0.30
AREA-AVERAGED Ap = 0.10
EFFECTIVE STREAM AREA(ACRES) = 28.35
TOTAL STREAM AREA(ACRES) = 32.45
PEAK FLOW RATE(CFS) AT CONFLUENCE = 61.71

FLOW PROCESS FROM NODE 42.00 TO NODE 41.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

=====

INITIAL SUBAREA FLOW-LENGTH(FEET) = 230.00
ELEVATION DATA: UPSTREAM(FEET) = 166.00 DOWNSTREAM(FEET) = 163.20

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$
SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) = 6.464
* 10 YEAR RAINFALL INTENSITY(INCH/HR) = 3.504
SUBAREA Tc AND LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN	Tc (MIN.)
COMMERCIAL	B	0.66	0.30	0.100	56	6.46

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.30
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.100
SUBAREA RUNOFF(CFS) = 2.06
TOTAL AREA(ACRES) = 0.66 PEAK FLOW RATE(CFS) = 2.06

FLOW PROCESS FROM NODE 41.00 TO NODE 41.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<

=====

TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
TIME OF CONCENTRATION(MIN.) = 6.46
RAINFALL INTENSITY(INCH/HR) = 3.50
AREA-AVERAGED Fm(INCH/HR) = 0.03
AREA-AVERAGED Fp(INCH/HR) = 0.30
AREA-AVERAGED Ap = 0.10
EFFECTIVE STREAM AREA(ACRES) = 0.66

TOTAL STREAM AREA(ACRES) = 0.66
 PEAK FLOW RATE(CFS) AT CONFLUENCE = 2.06

** CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	58.69	9.43	2.822	0.30 (0.03)	0.10	19.8	28.00
1	59.31	10.09	2.715	0.30 (0.03)	0.10	20.9	37.00
1	60.78	12.94	2.354	0.30 (0.03)	0.10	25.3	14.00
1	61.36	13.70	2.278	0.30 (0.03)	0.10	26.6	21.00
1	61.37	13.72	2.277	0.30 (0.03)	0.10	26.6	17.00
1	61.41	13.83	2.266	0.30 (0.03)	0.10	26.8	40.00
1	61.71	14.94	2.168	0.30 (0.03)	0.10	28.3	35.00
1	60.84	16.50	2.048	0.30 (0.03)	0.10	29.8	24.00
1	59.30	17.80	1.961	0.30 (0.03)	0.10	30.5	18.00
1	57.30	19.41	1.866	0.30 (0.03)	0.10	31.2	12.00
1	53.71	22.63	1.709	0.30 (0.03)	0.10	32.4	32.00
1	52.99	23.09	1.690	0.30 (0.03)	0.10	32.5	10.00
2	2.06	6.46	3.504	0.30 (0.03)	0.10	0.7	42.00

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
 CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	52.11	6.46	3.504	0.30 (0.03)	0.10	14.2	42.00
2	60.35	9.43	2.822	0.30 (0.03)	0.10	20.5	28.00
3	60.90	10.09	2.715	0.30 (0.03)	0.10	21.6	37.00
4	62.16	12.94	2.354	0.30 (0.03)	0.10	26.0	14.00
5	62.69	13.70	2.278	0.30 (0.03)	0.10	27.3	21.00
6	62.70	13.72	2.277	0.30 (0.03)	0.10	27.3	17.00
7	62.73	13.83	2.266	0.30 (0.03)	0.10	27.5	40.00
8	62.98	14.94	2.168	0.30 (0.03)	0.10	29.0	35.00
9	62.04	16.50	2.048	0.30 (0.03)	0.10	30.5	24.00
10	60.45	17.80	1.961	0.30 (0.03)	0.10	31.2	18.00
11	58.39	19.41	1.866	0.30 (0.03)	0.10	31.9	12.00
12	54.71	22.63	1.709	0.30 (0.03)	0.10	33.1	32.00
13	53.97	23.09	1.690	0.30 (0.03)	0.10	33.1	10.00

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE(CFS) = 62.98 Tc(MIN.) = 14.94
 EFFECTIVE AREA(ACRES) = 29.01 AREA-AVERAGED Fm(INCH/HR) = 0.03
 AREA-AVERAGED Fp(INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.10
 TOTAL AREA(ACRES) = 33.1
 LONGEST FLOWPATH FROM NODE 10.00 TO NODE 41.00 = 2775.00 FEET.

FLOW PROCESS FROM NODE 41.00 TO NODE 43.00 IS CODE = 41

>>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
 >>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 152.00 DOWNSTREAM(FEET) = 151.80

FLOW LENGTH(FEET) = 30.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 48.0 INCH PIPE IS 25.4 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 9.32
GIVEN PIPE DIAMETER(INCH) = 48.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 62.98
PIPE TRAVEL TIME(MIN.) = 0.05 Tc(MIN.) = 14.99
LONGEST FLOWPATH FROM NODE 10.00 TO NODE 43.00 = 2805.00 FEET.

FLOW PROCESS FROM NODE 43.00 TO NODE 43.00 IS CODE = 10

>>>>MAIN-STREAM MEMORY COPIED ONTO MEMORY BANK # 1 <<<<<

FLOW PROCESS FROM NODE 44.00 TO NODE 45.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

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INITIAL SUBAREA FLOW-LENGTH(FEET) = 350.00
ELEVATION DATA: UPSTREAM(FEET) = 164.00 DOWNSTREAM(FEET) = 162.00

$$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$$

SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) = 8.894

* 10 YEAR RAINFALL INTENSITY(INCH/HR) = 2.918

SUBAREA Tc AND LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN	Tc (MIN.)
COMMERCIAL	B	1.55	0.30	0.100	56	8.89

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.30

SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.100

SUBAREA RUNOFF(CFS) = 4.03

TOTAL AREA(ACRES) = 1.55 PEAK FLOW RATE(CFS) = 4.03

FLOW PROCESS FROM NODE 45.00 TO NODE 46.00 IS CODE = 51

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<<
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<<

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ELEVATION DATA: UPSTREAM(FEET) = 162.00 DOWNSTREAM(FEET) = 161.80
CHANNEL LENGTH THRU SUBAREA(FEET) = 165.00 CHANNEL SLOPE = 0.0012
CHANNEL BASE(FEET) = 3.00 "Z" FACTOR = 0.040
MANNING'S FACTOR = 0.015 MAXIMUM DEPTH(FEET) = 1.00
CHANNEL FLOW THRU SUBAREA(CFS) = 4.03
FLOW VELOCITY(FEET/SEC.) = 2.04 FLOW DEPTH(FEET) = 0.65
TRAVEL TIME(MIN.) = 1.35 Tc(MIN.) = 10.24
LONGEST FLOWPATH FROM NODE 44.00 TO NODE 46.00 = 515.00 FEET.

FLOW PROCESS FROM NODE 46.00 TO NODE 46.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

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TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION(MIN.) = 10.24
RAINFALL INTENSITY(INCH/HR) = 2.69
AREA-AVERAGED Fm(INCH/HR) = 0.03
AREA-AVERAGED Fp(INCH/HR) = 0.30
AREA-AVERAGED Ap = 0.10
EFFECTIVE STREAM AREA(ACRES) = 1.55
TOTAL STREAM AREA(ACRES) = 1.55
PEAK FLOW RATE(CFS) AT CONFLUENCE = 4.03

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FLOW PROCESS FROM NODE 47.00 TO NODE 46.00 IS CODE = 21
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>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<
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INITIAL SUBAREA FLOW-LENGTH(FEET) = 285.00
ELEVATION DATA: UPSTREAM(FEET) = 165.00 DOWNSTREAM(FEET) = 161.80

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Tc = K*[(LENGTH** 3.00)/(ELEVATION CHANGE)]**0.20
SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) = 7.157
* 10 YEAR RAINFALL INTENSITY(INCH/HR) = 3.305
SUBAREA Tc AND LOSS RATE DATA(AMC II):
DEVELOPMENT TYPE/      SCS SOIL  AREA      Fp      Ap      SCS  Tc
LAND USE              GROUP   (ACRES) (INCH/HR) (DECIMAL) CN (MIN.)
COMMERCIAL            B       1.23    0.30    0.100    56   7.16
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.30
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.100
SUBAREA RUNOFF(CFS) = 3.63
TOTAL AREA(ACRES) = 1.23 PEAK FLOW RATE(CFS) = 3.63

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*****
FLOW PROCESS FROM NODE 46.00 TO NODE 46.00 IS CODE = 1
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>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<
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TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
TIME OF CONCENTRATION(MIN.) = 7.16
RAINFALL INTENSITY(INCH/HR) = 3.31
AREA-AVERAGED Fm(INCH/HR) = 0.03
AREA-AVERAGED Fp(INCH/HR) = 0.30
AREA-AVERAGED Ap = 0.10
EFFECTIVE STREAM AREA(ACRES) = 1.23
TOTAL STREAM AREA(ACRES) = 1.23
PEAK FLOW RATE(CFS) AT CONFLUENCE = 3.63

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** CONFLUENCE DATA **

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STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	4.03	10.24	2.692	0.30 (0.03)	0.10	1.5	44.00

2 3.63 7.16 3.305 0.30(0.03) 0.10 1.2 47.00

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	7.09	7.16	3.305	0.30(0.03)	0.10	2.3	47.00
2	6.98	10.24	2.692	0.30(0.03)	0.10	2.8	44.00

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE(CFS) = 7.09 Tc(MIN.) = 7.16
EFFECTIVE AREA(ACRES) = 2.31 AREA-AVERAGED Fm(INCH/HR) = 0.03
AREA-AVERAGED Fp(INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.10
TOTAL AREA(ACRES) = 2.8
LONGEST FLOWPATH FROM NODE 44.00 TO NODE 46.00 = 515.00 FEET.

FLOW PROCESS FROM NODE 46.00 TO NODE 43.00 IS CODE = 41

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 157.00 DOWNSTREAM(FEET) = 151.80
FLOW LENGTH(FEET) = 115.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 18.0 INCH PIPE IS 7.1 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 11.01
GIVEN PIPE DIAMETER(INCH) = 18.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 7.09
PIPE TRAVEL TIME(MIN.) = 0.17 Tc(MIN.) = 7.33
LONGEST FLOWPATH FROM NODE 44.00 TO NODE 43.00 = 630.00 FEET.

FLOW PROCESS FROM NODE 43.00 TO NODE 43.00 IS CODE = 11

>>>>CONFLUENCE MEMORY BANK # 1 WITH THE MAIN-STREAM MEMORY<<<<<

** MAIN STREAM CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	7.09	7.33	3.260	0.30(0.03)	0.10	2.3	47.00
2	6.98	10.41	2.666	0.30(0.03)	0.10	2.8	44.00

LONGEST FLOWPATH FROM NODE 44.00 TO NODE 43.00 = 630.00 FEET.

** MEMORY BANK # 1 CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	52.11	6.52	3.487	0.30(0.03)	0.10	14.2	42.00
2	60.35	9.49	2.812	0.30(0.03)	0.10	20.5	28.00
3	60.90	10.14	2.707	0.30(0.03)	0.10	21.6	37.00
4	62.16	12.99	2.349	0.30(0.03)	0.10	26.0	14.00
5	62.69	13.76	2.273	0.30(0.03)	0.10	27.3	21.00

6	62.70	13.77	2.272	0.30 (0.03)	0.10	27.3	17.00
7	62.73	13.88	2.261	0.30 (0.03)	0.10	27.5	40.00
8	62.98	14.99	2.164	0.30 (0.03)	0.10	29.0	35.00
9	62.04	16.56	2.044	0.30 (0.03)	0.10	30.5	24.00
10	60.45	17.86	1.957	0.30 (0.03)	0.10	31.2	18.00
11	58.39	19.46	1.863	0.30 (0.03)	0.10	31.9	12.00
12	54.71	22.68	1.707	0.30 (0.03)	0.10	33.1	32.00
13	53.97	23.14	1.687	0.30 (0.03)	0.10	33.1	10.00

LONGEST FLOWPATH FROM NODE 10.00 TO NODE 43.00 = 2805.00 FEET.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp (Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	58.86	6.52	3.487	0.30 (0.03)	0.10	16.3	42.00
2	61.45	7.33	3.260	0.30 (0.03)	0.10	18.2	47.00
3	67.36	9.49	2.812	0.30 (0.03)	0.10	23.1	28.00
4	67.89	10.14	2.707	0.30 (0.03)	0.10	24.3	37.00
5	68.00	10.41	2.666	0.30 (0.03)	0.10	24.8	44.00
6	68.29	12.99	2.349	0.30 (0.03)	0.10	28.8	14.00
7	68.63	13.76	2.273	0.30 (0.03)	0.10	30.1	21.00
8	68.64	13.77	2.272	0.30 (0.03)	0.10	30.1	17.00
9	68.64	13.88	2.261	0.30 (0.03)	0.10	30.2	40.00
10	68.63	14.99	2.164	0.30 (0.03)	0.10	31.8	35.00
11	67.37	16.56	2.044	0.30 (0.03)	0.10	33.2	24.00
12	65.55	17.86	1.957	0.30 (0.03)	0.10	33.9	18.00
13	63.24	19.46	1.863	0.30 (0.03)	0.10	34.7	12.00
14	59.15	22.68	1.707	0.30 (0.03)	0.10	35.9	32.00
15	58.36	23.14	1.687	0.30 (0.03)	0.10	35.9	10.00

TOTAL AREA (ACRES) = 35.9

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE (CFS) = 68.64 Tc (MIN.) = 13.881
EFFECTIVE AREA (ACRES) = 30.24 AREA-AVERAGED Fm (INCH/HR) = 0.03
AREA-AVERAGED Fp (INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.10
TOTAL AREA (ACRES) = 35.9
LONGEST FLOWPATH FROM NODE 10.00 TO NODE 43.00 = 2805.00 FEET.

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END OF STUDY SUMMARY:

TOTAL AREA (ACRES) = 35.9 TC (MIN.) = 13.88
EFFECTIVE AREA (ACRES) = 30.24 AREA-AVERAGED Fm (INCH/HR) = 0.03
AREA-AVERAGED Fp (INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.100
PEAK FLOW RATE (CFS) = 68.64

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp (Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	58.86	6.52	3.487	0.30 (0.03)	0.10	16.3	42.00
2	61.45	7.33	3.260	0.30 (0.03)	0.10	18.2	47.00
3	67.36	9.49	2.812	0.30 (0.03)	0.10	23.1	28.00
4	67.89	10.14	2.707	0.30 (0.03)	0.10	24.3	37.00
5	68.00	10.41	2.666	0.30 (0.03)	0.10	24.8	44.00
6	68.29	12.99	2.349	0.30 (0.03)	0.10	28.8	14.00
7	68.63	13.76	2.273	0.30 (0.03)	0.10	30.1	21.00
8	68.64	13.77	2.272	0.30 (0.03)	0.10	30.1	17.00

9	68.64	13.88	2.261	0.30 (0.03)	0.10	30.2	40.00
10	68.63	14.99	2.164	0.30 (0.03)	0.10	31.8	35.00
11	67.37	16.56	2.044	0.30 (0.03)	0.10	33.2	24.00
12	65.55	17.86	1.957	0.30 (0.03)	0.10	33.9	18.00
13	63.24	19.46	1.863	0.30 (0.03)	0.10	34.7	12.00
14	59.15	22.68	1.707	0.30 (0.03)	0.10	35.9	32.00
15	58.36	23.14	1.687	0.30 (0.03)	0.10	35.9	10.00

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END OF RATIONAL METHOD ANALYSIS

RATIONAL METHOD HYDROLOGY COMPUTER PROGRAM PACKAGE
(Reference: 1986 ORANGE COUNTY HYDROLOGY CRITERION)
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Ver. 20.0 Release Date: 06/01/2013 License ID 1654

Analysis prepared by:

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949-474-1960

***** DESCRIPTION OF STUDY *****
* FREEMAN *
* EXISTING CONDITION *
* 25 YEAR ANALYSIS *

FILE NAME: FREX25.DAT
TIME/DATE OF STUDY: 09:33 05/27/2017

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USER SPECIFIED HYDROLOGY AND HYDRAULIC MODEL INFORMATION:

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--*TIME-OF-CONCENTRATION MODEL*--

USER SPECIFIED STORM EVENT (YEAR) = 25.00
SPECIFIED MINIMUM PIPE SIZE (INCH) = 12.00
SPECIFIED PERCENT OF GRADIENTS (DECIMAL) TO USE FOR FRICTION SLOPE = 0.95
DATA BANK RAINFALL USED
ANTECEDENT MOISTURE CONDITION (AMC) II ASSUMED FOR RATIONAL METHOD

USER-DEFINED STREET-SECTIONS FOR COUPLED PIPEFLOW AND STREETFLOW MODEL

NO.	HALF- WIDTH (FT)	CROWN TO CROSSFALL (FT)	STREET-CROSSFALL: IN- / OUT- / PARK- SIDE / SIDE / WAY	CURB HEIGHT (FT)	GUTTER-GEOMETRIES: WIDTH LIP HIKE (FT) (FT) (FT)	MANNING FACTOR (n)
1	24.0	19.0	0.020/0.020/0.020	0.67	2.00 0.0312 0.167	0.0150

GLOBAL STREET FLOW-DEPTH CONSTRAINTS:
1. Relative Flow-Depth = 0.67 FEET
as (Maximum Allowable Street Flow Depth) - (Top-of-Curb)
2. (Depth)*(Velocity) Constraint = 6.0 (FT*FT/S)
*SIZE PIPE WITH A FLOW CAPACITY GREATER THAN
OR EQUAL TO THE UPSTREAM TRIBUTARY PIPE.*
*USER-SPECIFIED MINIMUM TOPOGRAPHIC SLOPE ADJUSTMENT NOT SELECTED

FLOW PROCESS FROM NODE 10.00 TO NODE 32.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

INITIAL SUBAREA FLOW-LENGTH (FEET) = 265.00
ELEVATION DATA: UPSTREAM (FEET) = 167.50 DOWNSTREAM (FEET) = 166.30

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$
SUBAREA ANALYSIS USED MINIMUM T_c (MIN.) = 8.337
* 25 YEAR RAINFALL INTENSITY (INCH/HR) = 3.612

SUBAREA T_c AND LOSS RATE DATA (AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	F_p (INCH/HR)	A_p (DECIMAL)	SCS CN	T_c (MIN.)
COMMERCIAL	B	0.15	0.30	0.100	56	8.34

SUBAREA AVERAGE PERVIOUS LOSS RATE, F_p (INCH/HR) = 0.30

SUBAREA AVERAGE PERVIOUS AREA FRACTION, A_p = 0.100

SUBAREA RUNOFF (CFS) = 0.48

TOTAL AREA (ACRES) = 0.15 PEAK FLOW RATE (CFS) = 0.48

FLOW PROCESS FROM NODE 32.00 TO NODE 11.00 IS CODE = 61

>>>>COMPUTE STREET FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>(STANDARD CURB SECTION USED)<<<<<

UPSTREAM ELEVATION (FEET) = 166.30 DOWNSTREAM ELEVATION (FEET) = 165.10
STREET LENGTH (FEET) = 210.00 CURB HEIGHT (INCHES) = 6.0
STREET HALFWIDTH (FEET) = 25.00

DISTANCE FROM CROWN TO CROSSFALL GRADEBREAK (FEET) = 10.00

INSIDE STREET CROSSFALL (DECIMAL) = 0.020

OUTSIDE STREET CROSSFALL (DECIMAL) = 0.020

SPECIFIED NUMBER OF HALFSTREETS CARRYING RUNOFF = 1

STREET PARKWAY CROSSFALL (DECIMAL) = 0.020

Manning's FRICTION FACTOR for Streetflow Section (curb-to-curb) = 0.0150

Manning's FRICTION FACTOR for Back-of-Walk Flow Section = 0.0200

**TRAVEL TIME COMPUTED USING ESTIMATED FLOW (CFS) = 0.65

STREETFLOW MODEL RESULTS USING ESTIMATED FLOW:

STREET FLOW DEPTH (FEET) = 0.25

HALFSTREET FLOOD WIDTH (FEET) = 6.01

AVERAGE FLOW VELOCITY (FEET/SEC.) = 1.36

PRODUCT OF DEPTH&VELOCITY (FT*FT/SEC.) = 0.33

STREET FLOW TRAVEL TIME (MIN.) = 2.58 T_c (MIN.) = 10.92

* 25 YEAR RAINFALL INTENSITY (INCH/HR) = 3.100

SUBAREA LOSS RATE DATA (AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	F_p (INCH/HR)	A_p (DECIMAL)	SCS CN
COMMERCIAL	B	0.12	0.30	0.100	56

SUBAREA AVERAGE PERVIOUS LOSS RATE, F_p (INCH/HR) = 0.30

SUBAREA AVERAGE PERVIOUS AREA FRACTION, A_p = 0.100

SUBAREA AREA (ACRES) = 0.12 SUBAREA RUNOFF (CFS) = 0.33

EFFECTIVE AREA (ACRES) = 0.27 AREA-AVERAGED Fm (INCH/HR) = 0.03
AREA-AVERAGED Fp (INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.10
TOTAL AREA (ACRES) = 0.3 PEAK FLOW RATE (CFS) = 0.75

END OF SUBAREA STREET FLOW HYDRAULICS:

DEPTH (FEET) = 0.26 HALFSTREET FLOOD WIDTH (FEET) = 6.48
FLOW VELOCITY (FEET/SEC.) = 1.39 DEPTH*VELOCITY (FT*FT/SEC.) = 0.35
LONGEST FLOWPATH FROM NODE 10.00 TO NODE 11.00 = 475.00 FEET.

FLOW PROCESS FROM NODE 11.00 TO NODE 11.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

=====

TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION (MIN.) = 10.92
RAINFALL INTENSITY (INCH/HR) = 3.10
AREA-AVERAGED Fm (INCH/HR) = 0.03
AREA-AVERAGED Fp (INCH/HR) = 0.30
AREA-AVERAGED Ap = 0.10
EFFECTIVE STREAM AREA (ACRES) = 0.27
TOTAL STREAM AREA (ACRES) = 0.27
PEAK FLOW RATE (CFS) AT CONFLUENCE = 0.75

FLOW PROCESS FROM NODE 12.00 TO NODE 11.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<

>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

=====

INITIAL SUBAREA FLOW-LENGTH (FEET) = 210.00
ELEVATION DATA: UPSTREAM (FEET) = 166.00 DOWNSTREAM (FEET) = 165.10

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$

SUBAREA ANALYSIS USED MINIMUM Tc (MIN.) = 7.680

* 25 YEAR RAINFALL INTENSITY (INCH/HR) = 3.783

SUBAREA Tc AND LOSS RATE DATA (AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN	Tc (MIN.)
COMMERCIAL	B	0.76	0.30	0.100	56	7.68

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp (INCH/HR) = 0.30

SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.100

SUBAREA RUNOFF (CFS) = 2.57

TOTAL AREA (ACRES) = 0.76 PEAK FLOW RATE (CFS) = 2.57

FLOW PROCESS FROM NODE 11.00 TO NODE 11.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<

=====

TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:

TIME OF CONCENTRATION(MIN.) = 7.68
 RAINFALL INTENSITY(INCH/HR) = 3.78
 AREA-AVERAGED Fm(INCH/HR) = 0.03
 AREA-AVERAGED Fp(INCH/HR) = 0.30
 AREA-AVERAGED Ap = 0.10
 EFFECTIVE STREAM AREA(ACRES) = 0.76
 TOTAL STREAM AREA(ACRES) = 0.76
 PEAK FLOW RATE(CFS) AT CONFLUENCE = 2.57

** CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	0.75	10.92	3.100	0.30(0.03)	0.10	0.3	10.00
2	2.57	7.68	3.783	0.30(0.03)	0.10	0.8	12.00

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
 CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	3.21	7.68	3.783	0.30(0.03)	0.10	0.9	12.00
2	2.85	10.92	3.100	0.30(0.03)	0.10	1.0	10.00

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE(CFS) = 3.21 Tc(MIN.) = 7.68
 EFFECTIVE AREA(ACRES) = 0.95 AREA-AVERAGED Fm(INCH/HR) = 0.03
 AREA-AVERAGED Fp(INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.10
 TOTAL AREA(ACRES) = 1.0
 LONGEST FLOWPATH FROM NODE 10.00 TO NODE 11.00 = 475.00 FEET.

FLOW PROCESS FROM NODE 11.00 TO NODE 13.00 IS CODE = 62

>>>>COMPUTE STREET FLOW TRAVEL TIME THRU SUBAREA<<<<<
 >>>>(STREET TABLE SECTION # 1 USED)<<<<<

UPSTREAM ELEVATION(FEET) = 165.10 DOWNSTREAM ELEVATION(FEET) = 164.20
 STREET LENGTH(FEET) = 455.00 CURB HEIGHT(INCHES) = 8.0
 STREET HALFWIDTH(FEET) = 24.00

DISTANCE FROM CROWN TO CROSSFALL GRADEBREAK(FEET) = 19.00
 INSIDE STREET CROSSFALL(DECIMAL) = 0.020
 OUTSIDE STREET CROSSFALL(DECIMAL) = 0.020

SPECIFIED NUMBER OF HALFSTREETS CARRYING RUNOFF = 1
 STREET PARKWAY CROSSFALL(DECIMAL) = 0.020
 Manning's FRICTION FACTOR for Streetflow Section(curbs-to-curbs) = 0.0150
 Manning's FRICTION FACTOR for Back-of-Walk Flow Section = 0.0200

**TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 3.65
 STREETFLOW MODEL RESULTS USING ESTIMATED FLOW:
 STREET FLOW DEPTH(FEET) = 0.48
 HALFSTREET FLOOD WIDTH(FEET) = 15.95

AVERAGE FLOW VELOCITY (FEET/SEC.) = 1.34
 PRODUCT OF DEPTH&VELOCITY (FT*FT/SEC.) = 0.64
 STREET FLOW TRAVEL TIME (MIN.) = 5.67 Tc (MIN.) = 13.35
 * 25 YEAR RAINFALL INTENSITY (INCH/HR) = 2.767
 SUBAREA LOSS RATE DATA (AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
COMMERCIAL	B	0.36	0.30	0.100	56

 SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp (INCH/HR) = 0.30
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.100
 SUBAREA AREA (ACRES) = 0.36 SUBAREA RUNOFF (CFS) = 0.89
 EFFECTIVE AREA (ACRES) = 1.31 AREA-AVERAGED Fm (INCH/HR) = 0.03
 AREA-AVERAGED Fp (INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.10
 TOTAL AREA (ACRES) = 1.4 PEAK FLOW RATE (CFS) = 3.23

END OF SUBAREA STREET FLOW HYDRAULICS:
 DEPTH (FEET) = 0.46 HALFSTREET FLOOD WIDTH (FEET) = 15.21
 FLOW VELOCITY (FEET/SEC.) = 1.29 DEPTH*VELOCITY (FT*FT/SEC.) = 0.60
 LONGEST FLOWPATH FROM NODE 10.00 TO NODE 13.00 = 930.00 FEET.

FLOW PROCESS FROM NODE 13.00 TO NODE 13.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

=====
 TOTAL NUMBER OF STREAMS = 2
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
 TIME OF CONCENTRATION (MIN.) = 13.35
 RAINFALL INTENSITY (INCH/HR) = 2.77
 AREA-AVERAGED Fm (INCH/HR) = 0.03
 AREA-AVERAGED Fp (INCH/HR) = 0.30
 AREA-AVERAGED Ap = 0.10
 EFFECTIVE STREAM AREA (ACRES) = 1.31
 TOTAL STREAM AREA (ACRES) = 1.39
 PEAK FLOW RATE (CFS) AT CONFLUENCE = 3.23

FLOW PROCESS FROM NODE 14.00 TO NODE 15.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
 >>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

=====
 INITIAL SUBAREA FLOW-LENGTH (FEET) = 125.00
 ELEVATION DATA: UPSTREAM (FEET) = 164.60 DOWNSTREAM (FEET) = 164.20

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$
 SUBAREA ANALYSIS USED MINIMUM Tc (MIN.) = 6.616
 * 25 YEAR RAINFALL INTENSITY (INCH/HR) = 4.117
 SUBAREA Tc AND LOSS RATE DATA (AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN	Tc (MIN.)
COMMERCIAL	B	0.15	0.30	0.100	56	6.62

 SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp (INCH/HR) = 0.30
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.100

SUBAREA RUNOFF(CFS) = 0.55
 TOTAL AREA(ACRES) = 0.15 PEAK FLOW RATE(CFS) = 0.55

FLOW PROCESS FROM NODE 15.00 TO NODE 13.00 IS CODE = 41

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
 >>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 159.30 DOWNSTREAM(FEET) = 159.20
 FLOW LENGTH(FEET) = 50.00 MANNING'S N = 0.013
 DEPTH OF FLOW IN 18.0 INCH PIPE IS 4.2 INCHES
 PIPE-FLOW VELOCITY(FEET/SEC.) = 1.76
 GIVEN PIPE DIAMETER(INCH) = 18.00 NUMBER OF PIPES = 1
 PIPE-FLOW(CFS) = 0.55
 PIPE TRAVEL TIME(MIN.) = 0.47 Tc(MIN.) = 7.09
 LONGEST FLOWPATH FROM NODE 14.00 TO NODE 13.00 = 175.00 FEET.

FLOW PROCESS FROM NODE 13.00 TO NODE 13.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
 >>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<

=====

TOTAL NUMBER OF STREAMS = 2
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
 TIME OF CONCENTRATION(MIN.) = 7.09
 RAINFALL INTENSITY(INCH/HR) = 3.96
 AREA-AVERAGED Fm(INCH/HR) = 0.03
 AREA-AVERAGED Fp(INCH/HR) = 0.30
 AREA-AVERAGED Ap = 0.10
 EFFECTIVE STREAM AREA(ACRES) = 0.15
 TOTAL STREAM AREA(ACRES) = 0.15
 PEAK FLOW RATE(CFS) AT CONFLUENCE = 0.55

** CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	3.23	13.35	2.767	0.30 (0.03)	0.10	1.3	12.00
1	3.00	16.78	2.431	0.30 (0.03)	0.10	1.4	10.00
2	0.55	7.09	3.959	0.30 (0.03)	0.10	0.2	14.00

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
 CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	3.01	7.09	3.959	0.30 (0.03)	0.10	0.8	14.00
2	3.61	13.35	2.767	0.30 (0.03)	0.10	1.5	12.00
3	3.34	16.78	2.431	0.30 (0.03)	0.10	1.5	10.00

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
 PEAK FLOW RATE(CFS) = 3.61 Tc(MIN.) = 13.35

EFFECTIVE AREA (ACRES) = 1.46 AREA-AVERAGED Fm (INCH/HR) = 0.03
AREA-AVERAGED Fp (INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.10
TOTAL AREA (ACRES) = 1.5
LONGEST FLOWPATH FROM NODE 10.00 TO NODE 13.00 = 930.00 FEET.

FLOW PROCESS FROM NODE 13.00 TO NODE 16.00 IS CODE = 41

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM (FEET) = 159.20 DOWNSTREAM (FEET) = 158.10
FLOW LENGTH (FEET) = 285.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 18.0 INCH PIPE IS 9.7 INCHES
PIPE-FLOW VELOCITY (FEET/SEC.) = 3.71
GIVEN PIPE DIAMETER (INCH) = 18.00 NUMBER OF PIPES = 1
PIPE-FLOW (CFS) = 3.61
PIPE TRAVEL TIME (MIN.) = 1.28 Tc (MIN.) = 14.63
LONGEST FLOWPATH FROM NODE 10.00 TO NODE 16.00 = 1215.00 FEET.

FLOW PROCESS FROM NODE 16.00 TO NODE 16.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

=====

TOTAL NUMBER OF STREAMS = 3
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION (MIN.) = 14.63
RAINFALL INTENSITY (INCH/HR) = 2.63
AREA-AVERAGED Fm (INCH/HR) = 0.03
AREA-AVERAGED Fp (INCH/HR) = 0.30
AREA-AVERAGED Ap = 0.10
EFFECTIVE STREAM AREA (ACRES) = 1.46
TOTAL STREAM AREA (ACRES) = 1.54
PEAK FLOW RATE (CFS) AT CONFLUENCE = 3.61

FLOW PROCESS FROM NODE 17.00 TO NODE 16.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

=====

INITIAL SUBAREA FLOW-LENGTH (FEET) = 310.00
ELEVATION DATA: UPSTREAM (FEET) = 166.00 DOWNSTREAM (FEET) = 164.90

Tc = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20
SUBAREA ANALYSIS USED MINIMUM Tc (MIN.) = 9.320
* 25 YEAR RAINFALL INTENSITY (INCH/HR) = 3.391
SUBAREA Tc AND LOSS RATE DATA (AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN	Tc (MIN.)
COMMERCIAL	B	1.52	0.30	0.100	56	9.32

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp (INCH/HR) = 0.30
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.100

SUBAREA RUNOFF(CFS) = 4.60
TOTAL AREA(ACRES) = 1.52 PEAK FLOW RATE(CFS) = 4.60

FLOW PROCESS FROM NODE 16.00 TO NODE 16.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

=====

TOTAL NUMBER OF STREAMS = 3
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
TIME OF CONCENTRATION(MIN.) = 9.32
RAINFALL INTENSITY(INCH/HR) = 3.39
AREA-AVERAGED Fm(INCH/HR) = 0.03
AREA-AVERAGED Fp(INCH/HR) = 0.30
AREA-AVERAGED Ap = 0.10
EFFECTIVE STREAM AREA(ACRES) = 1.52
TOTAL STREAM AREA(ACRES) = 1.52
PEAK FLOW RATE(CFS) AT CONFLUENCE = 4.60

FLOW PROCESS FROM NODE 18.00 TO NODE 19.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

=====

INITIAL SUBAREA FLOW-LENGTH(FEET) = 320.00
ELEVATION DATA: UPSTREAM(FEET) = 165.90 DOWNSTREAM(FEET) = 165.00

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$
SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) = 9.888
* 25 YEAR RAINFALL INTENSITY(INCH/HR) = 3.279
SUBAREA Tc AND LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN	Tc (MIN.)
COMMERCIAL	B	1.26	0.30	0.100	56	9.89

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.30
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.100
SUBAREA RUNOFF(CFS) = 3.68
TOTAL AREA(ACRES) = 1.26 PEAK FLOW RATE(CFS) = 3.68

FLOW PROCESS FROM NODE 19.00 TO NODE 16.00 IS CODE = 51

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<<
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 165.00 DOWNSTREAM(FEET) = 164.90
CHANNEL LENGTH THRU SUBAREA(FEET) = 265.00 CHANNEL SLOPE = 0.0004
CHANNEL BASE(FEET) = 3.00 "Z" FACTOR = 0.040
MANNING'S FACTOR = 0.015 MAXIMUM DEPTH(FEET) = 1.00
CHANNEL FLOW THRU SUBAREA(CFS) = 3.68
FLOW VELOCITY(FEET/SEC.) = 1.33 FLOW DEPTH(FEET) = 0.91
TRAVEL TIME(MIN.) = 3.32 Tc(MIN.) = 13.21
LONGEST FLOWPATH FROM NODE 18.00 TO NODE 16.00 = 585.00 FEET.

 FLOW PROCESS FROM NODE 16.00 TO NODE 16.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
 >>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<

=====

TOTAL NUMBER OF STREAMS = 3
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 3 ARE:
 TIME OF CONCENTRATION(MIN.) = 13.21
 RAINFALL INTENSITY(INCH/HR) = 2.78
 AREA-AVERAGED Fm(INCH/HR) = 0.03
 AREA-AVERAGED Fp(INCH/HR) = 0.30
 AREA-AVERAGED Ap = 0.10
 EFFECTIVE STREAM AREA(ACRES) = 1.26
 TOTAL STREAM AREA(ACRES) = 1.26
 PEAK FLOW RATE(CFS) AT CONFLUENCE = 3.68

** CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	3.01	8.43	3.590	0.30 (0.03)	0.10	0.8	14.00
1	3.61	14.63	2.627	0.30 (0.03)	0.10	1.5	12.00
1	3.34	18.09	2.330	0.30 (0.03)	0.10	1.5	10.00
2	4.60	9.32	3.391	0.30 (0.03)	0.10	1.5	17.00
3	3.68	13.21	2.783	0.30 (0.03)	0.10	1.3	18.00

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
 CONFLUENCE FORMULA USED FOR 3 STREAMS.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	10.45	8.43	3.590	0.30 (0.03)	0.10	3.0	14.00
2	10.87	9.32	3.391	0.30 (0.03)	0.10	3.3	17.00
3	10.99	13.21	2.783	0.30 (0.03)	0.10	4.1	18.00
4	10.64	14.63	2.627	0.30 (0.03)	0.10	4.2	12.00
5	9.56	18.09	2.330	0.30 (0.03)	0.10	4.3	10.00

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE(CFS) = 10.99 Tc(MIN.) = 13.21
 EFFECTIVE AREA(ACRES) = 4.10 AREA-AVERAGED Fm(INCH/HR) = 0.03
 AREA-AVERAGED Fp(INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.10
 TOTAL AREA(ACRES) = 4.3
 LONGEST FLOWPATH FROM NODE 10.00 TO NODE 16.00 = 1215.00 FEET.

 FLOW PROCESS FROM NODE 16.00 TO NODE 20.00 IS CODE = 41

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
 >>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 158.10 DOWNSTREAM(FEET) = 157.60
 FLOW LENGTH(FEET) = 150.00 MANNING'S N = 0.013

DEPTH OF FLOW IN 24.0 INCH PIPE IS 17.2 INCHES
PIPE-FLOW VELOCITY (FEET/SEC.) = 4.56
GIVEN PIPE DIAMETER (INCH) = 24.00 NUMBER OF PIPES = 1
PIPE-FLOW (CFS) = 10.99
PIPE TRAVEL TIME (MIN.) = 0.55 Tc (MIN.) = 13.76
LONGEST FLOWPATH FROM NODE 10.00 TO NODE 20.00 = 1365.00 FEET.

FLOW PROCESS FROM NODE 20.00 TO NODE 20.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

=====

TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION (MIN.) = 13.76
RAINFALL INTENSITY (INCH/HR) = 2.72
AREA-AVERAGED Fm (INCH/HR) = 0.03
AREA-AVERAGED Fp (INCH/HR) = 0.30
AREA-AVERAGED Ap = 0.10
EFFECTIVE STREAM AREA (ACRES) = 4.10
TOTAL STREAM AREA (ACRES) = 4.32
PEAK FLOW RATE (CFS) AT CONFLUENCE = 10.99

FLOW PROCESS FROM NODE 21.00 TO NODE 22.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<

>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

=====

INITIAL SUBAREA FLOW-LENGTH (FEET) = 260.00
ELEVATION DATA: UPSTREAM (FEET) = 166.00 DOWNSTREAM (FEET) = 165.50

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$

SUBAREA ANALYSIS USED MINIMUM Tc (MIN.) = 9.819

* 25 YEAR RAINFALL INTENSITY (INCH/HR) = 3.292

SUBAREA Tc AND LOSS RATE DATA (AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN	Tc (MIN.)
COMMERCIAL	B	0.64	0.30	0.100	56	9.82

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp (INCH/HR) = 0.30

SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.100

SUBAREA RUNOFF (CFS) = 1.88

TOTAL AREA (ACRES) = 0.64 PEAK FLOW RATE (CFS) = 1.88

FLOW PROCESS FROM NODE 22.00 TO NODE 20.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<

>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<

=====

ELEVATION DATA: UPSTREAM (FEET) = 158.20 DOWNSTREAM (FEET) = 157.60

FLOW LENGTH (FEET) = 20.00 MANNING'S N = 0.013

ESTIMATED PIPE DIAMETER (INCH) INCREASED TO 12.000

DEPTH OF FLOW IN 12.0 INCH PIPE IS 4.6 INCHES

PIPE-FLOW VELOCITY (FEET/SEC.) = 6.78
 ESTIMATED PIPE DIAMETER (INCH) = 12.00 NUMBER OF PIPES = 1
 PIPE-FLOW (CFS) = 1.88
 PIPE TRAVEL TIME (MIN.) = 0.05 Tc (MIN.) = 9.87
 LONGEST FLOWPATH FROM NODE 21.00 TO NODE 20.00 = 280.00 FEET.

FLOW PROCESS FROM NODE 20.00 TO NODE 20.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
 >>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<

=====
 TOTAL NUMBER OF STREAMS = 2
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
 TIME OF CONCENTRATION (MIN.) = 9.87
 RAINFALL INTENSITY (INCH/HR) = 3.28
 AREA-AVERAGED Fm (INCH/HR) = 0.03
 AREA-AVERAGED Fp (INCH/HR) = 0.30
 AREA-AVERAGED Ap = 0.10
 EFFECTIVE STREAM AREA (ACRES) = 0.64
 TOTAL STREAM AREA (ACRES) = 0.64
 PEAK FLOW RATE (CFS) AT CONFLUENCE = 1.88

** CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp (Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	10.45	8.98	3.463	0.30 (0.03)	0.10	3.0	14.00
1	10.87	9.87	3.283	0.30 (0.03)	0.10	3.3	17.00
1	10.99	13.76	2.720	0.30 (0.03)	0.10	4.1	18.00
1	10.64	15.18	2.573	0.30 (0.03)	0.10	4.2	12.00
1	9.56	18.65	2.290	0.30 (0.03)	0.10	4.3	10.00
2	1.88	9.87	3.283	0.30 (0.03)	0.10	0.6	21.00

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
 CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp (Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	12.26	8.98	3.463	0.30 (0.03)	0.10	3.6	14.00
2	12.75	9.87	3.283	0.30 (0.03)	0.10	4.0	21.00
3	12.75	9.87	3.283	0.30 (0.03)	0.10	4.0	17.00
4	12.54	13.76	2.720	0.30 (0.03)	0.10	4.7	18.00
5	12.11	15.18	2.573	0.30 (0.03)	0.10	4.9	12.00
6	10.87	18.65	2.290	0.30 (0.03)	0.10	5.0	10.00

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE (CFS) = 12.75 Tc (MIN.) = 9.87
 EFFECTIVE AREA (ACRES) = 3.98 AREA-AVERAGED Fm (INCH/HR) = 0.03
 AREA-AVERAGED Fp (INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.10
 TOTAL AREA (ACRES) = 5.0
 LONGEST FLOWPATH FROM NODE 10.00 TO NODE 20.00 = 1365.00 FEET.

FLOW PROCESS FROM NODE 20.00 TO NODE 23.00 IS CODE = 41

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

ELEVATION DATA: UPSTREAM(FEET) = 157.60 DOWNSTREAM(FEET) = 156.40
FLOW LENGTH(FEET) = 365.00 MANNING'S N = 0.013
ASSUME FULL-FLOWING PIPELINE
PIPE-FLOW VELOCITY(FEET/SEC.) = 4.06
PIPE FLOW VELOCITY = (TOTAL FLOW)/(PIPE CROSS SECTION AREA)
GIVEN PIPE DIAMETER(INCH) = 24.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 12.75
PIPE TRAVEL TIME(MIN.) = 1.50 Tc(MIN.) = 11.37
LONGEST FLOWPATH FROM NODE 10.00 TO NODE 23.00 = 1730.00 FEET.

FLOW PROCESS FROM NODE 23.00 TO NODE 23.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION(MIN.) = 11.37
RAINFALL INTENSITY(INCH/HR) = 3.03
AREA-AVERAGED Fm(INCH/HR) = 0.03
AREA-AVERAGED Fp(INCH/HR) = 0.30
AREA-AVERAGED Ap = 0.10
EFFECTIVE STREAM AREA(ACRES) = 3.98
TOTAL STREAM AREA(ACRES) = 4.96
PEAK FLOW RATE(CFS) AT CONFLUENCE = 12.75

FLOW PROCESS FROM NODE 24.00 TO NODE 25.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

INITIAL SUBAREA FLOW-LENGTH(FEET) = 320.00
ELEVATION DATA: UPSTREAM(FEET) = 166.00 DOWNSTREAM(FEET) = 165.50

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$
SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) = 11.122
* 25 YEAR RAINFALL INTENSITY(INCH/HR) = 3.068
SUBAREA Tc AND LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN	Tc (MIN.)
COMMERCIAL	B	1.40	0.30	0.100	56	11.12

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.30
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.100
SUBAREA RUNOFF(CFS) = 3.83
TOTAL AREA(ACRES) = 1.40 PEAK FLOW RATE(CFS) = 3.83

FLOW PROCESS FROM NODE 25.00 TO NODE 26.00 IS CODE = 51

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<<
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<<
=====

ELEVATION DATA: UPSTREAM(FEET) = 165.50 DOWNSTREAM(FEET) = 161.10
CHANNEL LENGTH THRU SUBAREA(FEET) = 330.00 CHANNEL SLOPE = 0.0133
CHANNEL BASE(FEET) = 20.00 "Z" FACTOR = 0.020
MANNING'S FACTOR = 0.015 MAXIMUM DEPTH(FEET) = 2.00
* 25 YEAR RAINFALL INTENSITY(INCH/HR) = 2.818
SUBAREA LOSS RATE DATA(AMC II):
DEVELOPMENT TYPE/ SCS SOIL AREA Fp Ap SCS
LAND USE GROUP (ACRES) (INCH/HR) (DECIMAL) CN
COMMERCIAL B 3.87 0.30 0.100 56
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.30
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.100
TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 8.69
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) = 3.06
AVERAGE FLOW DEPTH(FEET) = 0.14 TRAVEL TIME(MIN.) = 1.80
Tc(MIN.) = 12.92
SUBAREA AREA(ACRES) = 3.87 SUBAREA RUNOFF(CFS) = 9.71
EFFECTIVE AREA(ACRES) = 5.27 AREA-AVERAGED Fm(INCH/HR) = 0.03
AREA-AVERAGED Fp(INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.10
TOTAL AREA(ACRES) = 5.3 PEAK FLOW RATE(CFS) = 13.23

END OF SUBAREA CHANNEL FLOW HYDRAULICS:
DEPTH(FEET) = 0.18 FLOW VELOCITY(FEET/SEC.) = 3.67
LONGEST FLOWPATH FROM NODE 24.00 TO NODE 26.00 = 650.00 FEET.

FLOW PROCESS FROM NODE 26.00 TO NODE 23.00 IS CODE = 41

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<
=====

ELEVATION DATA: UPSTREAM(FEET) = 158.00 DOWNSTREAM(FEET) = 156.40
FLOW LENGTH(FEET) = 315.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 24.0 INCH PIPE IS 16.9 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 5.61
GIVEN PIPE DIAMETER(INCH) = 24.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 13.23
PIPE TRAVEL TIME(MIN.) = 0.94 Tc(MIN.) = 13.86
LONGEST FLOWPATH FROM NODE 24.00 TO NODE 23.00 = 965.00 FEET.

FLOW PROCESS FROM NODE 23.00 TO NODE 23.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<
=====

TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
TIME OF CONCENTRATION(MIN.) = 13.86
RAINFALL INTENSITY(INCH/HR) = 2.71
AREA-AVERAGED Fm(INCH/HR) = 0.03

AREA-AVERAGED F_p (INCH/HR) = 0.30
 AREA-AVERAGED A_p = 0.10
 EFFECTIVE STREAM AREA (ACRES) = 5.27
 TOTAL STREAM AREA (ACRES) = 5.27
 PEAK FLOW RATE (CFS) AT CONFLUENCE = 13.23

** CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp (Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	12.26	10.31	3.203	0.30 (0.03)	0.10	3.6	14.00
1	12.75	11.37	3.030	0.30 (0.03)	0.10	4.0	21.00
1	12.75	11.37	3.030	0.30 (0.03)	0.10	4.0	17.00
1	12.54	15.08	2.582	0.30 (0.03)	0.10	4.7	18.00
1	12.11	16.51	2.453	0.30 (0.03)	0.10	4.9	12.00
1	10.87	19.99	2.201	0.30 (0.03)	0.10	5.0	10.00
2	13.23	13.86	2.709	0.30 (0.03)	0.10	5.3	24.00

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
 CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp (Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	23.91	10.31	3.203	0.30 (0.03)	0.10	7.5	14.00
2	24.90	11.37	3.030	0.30 (0.03)	0.10	8.3	21.00
3	24.90	11.37	3.030	0.30 (0.03)	0.10	8.3	17.00
4	25.84	13.86	2.709	0.30 (0.03)	0.10	9.8	24.00
5	25.14	15.08	2.582	0.30 (0.03)	0.10	10.0	18.00
6	24.07	16.51	2.453	0.30 (0.03)	0.10	10.1	12.00
7	21.59	19.99	2.201	0.30 (0.03)	0.10	10.2	10.00

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE (CFS) = 25.84 Tc (MIN.) = 13.86
 EFFECTIVE AREA (ACRES) = 9.76 AREA-AVERAGED F_m (INCH/HR) = 0.03
 AREA-AVERAGED F_p (INCH/HR) = 0.30 AREA-AVERAGED A_p = 0.10
 TOTAL AREA (ACRES) = 10.2
 LONGEST FLOWPATH FROM NODE 10.00 TO NODE 23.00 = 1730.00 FEET.

FLOW PROCESS FROM NODE 23.00 TO NODE 27.00 IS CODE = 41

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
 >>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

ELEVATION DATA: UPSTREAM (FEET) = 156.40 DOWNSTREAM (FEET) = 155.40
 FLOW LENGTH (FEET) = 235.00 MANNING'S N = 0.013
 DEPTH OF FLOW IN 30.0 INCH PIPE IS 24.3 INCHES
 PIPE-FLOW VELOCITY (FEET/SEC.) = 6.06
 GIVEN PIPE DIAMETER (INCH) = 30.00 NUMBER OF PIPES = 1
 PIPE-FLOW (CFS) = 25.84
 PIPE TRAVEL TIME (MIN.) = 0.65 Tc (MIN.) = 14.50
 LONGEST FLOWPATH FROM NODE 10.00 TO NODE 27.00 = 1965.00 FEET.

FLOW PROCESS FROM NODE 27.00 TO NODE 27.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION(MIN.) = 14.50
RAINFALL INTENSITY(INCH/HR) = 2.64
AREA-AVERAGED Fm(INCH/HR) = 0.03
AREA-AVERAGED Fp(INCH/HR) = 0.30
AREA-AVERAGED Ap = 0.10
EFFECTIVE STREAM AREA(ACRES) = 9.76
TOTAL STREAM AREA(ACRES) = 10.23
PEAK FLOW RATE(CFS) AT CONFLUENCE = 25.84

FLOW PROCESS FROM NODE 28.00 TO NODE 29.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

INITIAL SUBAREA FLOW-LENGTH(FEET) = 225.00
ELEVATION DATA: UPSTREAM(FEET) = 170.00 DOWNSTREAM(FEET) = 166.50

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$

SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) = 6.101

* 25 YEAR RAINFALL INTENSITY(INCH/HR) = 4.310

SUBAREA Tc AND LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN	Tc (MIN.)
COMMERCIAL	B	1.32	0.30	0.100	56	6.10

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.30

SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.100

SUBAREA RUNOFF(CFS) = 5.08

TOTAL AREA(ACRES) = 1.32 PEAK FLOW RATE(CFS) = 5.08

FLOW PROCESS FROM NODE 29.00 TO NODE 30.00 IS CODE = 51

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<<
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<<

ELEVATION DATA: UPSTREAM(FEET) = 166.50 DOWNSTREAM(FEET) = 164.20

CHANNEL LENGTH THRU SUBAREA(FEET) = 300.00 CHANNEL SLOPE = 0.0077

CHANNEL BASE(FEET) = 5.00 "Z" FACTOR = 1.500

MANNING'S FACTOR = 0.015 MAXIMUM DEPTH(FEET) = 2.00

* 25 YEAR RAINFALL INTENSITY(INCH/HR) = 3.905

SUBAREA LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
COMMERCIAL	B	2.72	0.30	0.100	56

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.30

SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.100

TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 9.84

TRAVEL TIME THRU SUBAREA BASED ON VELOCITY (FEET/SEC.) = 4.31
 AVERAGE FLOW DEPTH (FEET) = 0.41 TRAVEL TIME (MIN.) = 1.16
 Tc (MIN.) = 7.26
 SUBAREA AREA (ACRES) = 2.72 SUBAREA RUNOFF (CFS) = 9.49
 EFFECTIVE AREA (ACRES) = 4.04 AREA-AVERAGED Fm (INCH/HR) = 0.03
 AREA-AVERAGED Fp (INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.10
 TOTAL AREA (ACRES) = 4.0 PEAK FLOW RATE (CFS) = 14.09

END OF SUBAREA CHANNEL FLOW HYDRAULICS:

DEPTH (FEET) = 0.50 FLOW VELOCITY (FEET/SEC.) = 4.89
 LONGEST FLOWPATH FROM NODE 28.00 TO NODE 30.00 = 525.00 FEET.

FLOW PROCESS FROM NODE 30.00 TO NODE 30.00 IS CODE = 82

>>>>ADD SUBAREA RUNOFF TO MAINLINE, AT MAINLINE Tc,<<<<<
 >>>>(AND COMPUTE INITIAL SUBAREA RUNOFF)<<<<<

=====

INITIAL SUBAREA FLOW-LENGTH (FEET) = 300.00
 ELEVATION DATA: UPSTREAM (FEET) = 167.40 DOWNSTREAM (FEET) = 164.20

Tc = K * [(LENGTH** 3.00) / (ELEVATION CHANGE)] ** 0.20
 SUBAREA ANALYSIS USED MINIMUM Tc (MIN.) = 7.381
 * 25 YEAR RAINFALL INTENSITY (INCH/HR) = 3.869

SUBAREA Tc AND LOSS RATE DATA (AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN	Tc (MIN.)
COMMERCIAL	B	2.23	0.30	0.100	56	7.38

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp (INCH/HR) = 0.30
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.100
 SUBAREA AREA (ACRES) = 2.23 INITIAL SUBAREA RUNOFF (CFS) = 7.71

** ADD SUBAREA RUNOFF TO MAINLINE AT MAINLINE Tc:

MAINLINE Tc (MIN.) = 7.26
 * 25 YEAR RAINFALL INTENSITY (INCH/HR) = 3.905
 SUBAREA AREA (ACRES) = 2.23 SUBAREA RUNOFF (CFS) = 7.78
 EFFECTIVE AREA (ACRES) = 6.27 AREA-AVERAGED Fm (INCH/HR) = 0.03
 AREA-AVERAGED Fp (INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.10
 TOTAL AREA (ACRES) = 6.3 PEAK FLOW RATE (CFS) = 21.87

FLOW PROCESS FROM NODE 30.00 TO NODE 27.00 IS CODE = 41

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
 >>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM (FEET) = 158.00 DOWNSTREAM (FEET) = 155.40
 FLOW LENGTH (FEET) = 160.00 MANNING'S N = 0.013
 DEPTH OF FLOW IN 24.0 INCH PIPE IS 15.9 INCHES
 PIPE-FLOW VELOCITY (FEET/SEC.) = 9.89
 GIVEN PIPE DIAMETER (INCH) = 24.00 NUMBER OF PIPES = 1
 PIPE-FLOW (CFS) = 21.87
 PIPE TRAVEL TIME (MIN.) = 0.27 Tc (MIN.) = 7.53
 LONGEST FLOWPATH FROM NODE 28.00 TO NODE 27.00 = 685.00 FEET.

 FLOW PROCESS FROM NODE 27.00 TO NODE 27.00 IS CODE = 1

 >>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
 >>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<
 =====

TOTAL NUMBER OF STREAMS = 2
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
 TIME OF CONCENTRATION(MIN.) = 7.53
 RAINFALL INTENSITY(INCH/HR) = 3.83
 AREA-AVERAGED Fm(INCH/HR) = 0.03
 AREA-AVERAGED Fp(INCH/HR) = 0.30
 AREA-AVERAGED Ap = 0.10
 EFFECTIVE STREAM AREA(ACRES) = 6.27
 TOTAL STREAM AREA(ACRES) = 6.27
 PEAK FLOW RATE(CFS) AT CONFLUENCE = 21.87

** CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	23.91	10.96	3.094	0.30 (0.03)	0.10	7.5	14.00
1	24.90	12.02	2.937	0.30 (0.03)	0.10	8.3	21.00
1	24.90	12.02	2.937	0.30 (0.03)	0.10	8.3	17.00
1	25.84	14.50	2.640	0.30 (0.03)	0.10	9.8	24.00
1	25.14	15.73	2.521	0.30 (0.03)	0.10	10.0	18.00
1	24.07	17.16	2.400	0.30 (0.03)	0.10	10.1	12.00
1	21.59	20.65	2.161	0.30 (0.03)	0.10	10.2	10.00
2	21.87	7.53	3.825	0.30 (0.03)	0.10	6.3	28.00

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
 CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	42.22	7.53	3.825	0.30 (0.03)	0.10	11.4	28.00
2	41.56	10.96	3.094	0.30 (0.03)	0.10	13.8	14.00
3	41.64	12.02	2.937	0.30 (0.03)	0.10	14.6	21.00
4	41.64	12.02	2.937	0.30 (0.03)	0.10	14.6	17.00
5	40.87	14.50	2.640	0.30 (0.03)	0.10	16.0	24.00
6	39.50	15.73	2.521	0.30 (0.03)	0.10	16.3	18.00
7	37.73	17.16	2.400	0.30 (0.03)	0.10	16.4	12.00
8	33.87	20.65	2.161	0.30 (0.03)	0.10	16.5	10.00

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE(CFS) = 42.22 Tc(MIN.) = 7.53
 EFFECTIVE AREA(ACRES) = 11.44 AREA-AVERAGED Fm(INCH/HR) = 0.03
 AREA-AVERAGED Fp(INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.10
 TOTAL AREA(ACRES) = 16.5
 LONGEST FLOWPATH FROM NODE 10.00 TO NODE 27.00 = 1965.00 FEET.

 FLOW PROCESS FROM NODE 27.00 TO NODE 31.00 IS CODE = 41


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>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 155.40 DOWNSTREAM(FEET) = 155.00
FLOW LENGTH(FEET) = 105.00 MANNING'S N = 0.013
ASSUME FULL-FLOWING PIPELINE
PIPE-FLOW VELOCITY(FEET/SEC.) = 5.97
PIPE FLOW VELOCITY = (TOTAL FLOW)/(PIPE CROSS SECTION AREA)
GIVEN PIPE DIAMETER(INCH) = 36.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 42.22
PIPE TRAVEL TIME(MIN.) = 0.29 Tc(MIN.) = 7.82
LONGEST FLOWPATH FROM NODE 10.00 TO NODE 31.00 = 2070.00 FEET.

*****
FLOW PROCESS FROM NODE 31.00 TO NODE 31.00 IS CODE = 10
-----
>>>>MAIN-STREAM MEMORY COPIED ONTO MEMORY BANK # 2 <<<<<
=====
*****
FLOW PROCESS FROM NODE 32.00 TO NODE 33.00 IS CODE = 21
-----
>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<
=====
INITIAL SUBAREA FLOW-LENGTH(FEET) = 300.00
ELEVATION DATA: UPSTREAM(FEET) = 166.30 DOWNSTREAM(FEET) = 166.00

Tc = K*[(LENGTH** 3.00)/(ELEVATION CHANGE)]**0.20
SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) = 11.850
* 25 YEAR RAINFALL INTENSITY(INCH/HR) = 2.960
SUBAREA Tc AND LOSS RATE DATA(AMC II):
DEVELOPMENT TYPE/ SCS SOIL AREA Fp Ap SCS Tc
LAND USE GROUP (ACRES) (INCH/HR) (DECIMAL) CN (MIN.)
COMMERCIAL B 2.62 0.30 0.100 56 11.85
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.30
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.100
SUBAREA RUNOFF(CFS) = 6.91
TOTAL AREA(ACRES) = 2.62 PEAK FLOW RATE(CFS) = 6.91

*****
FLOW PROCESS FROM NODE 33.00 TO NODE 34.00 IS CODE = 61
-----
>>>>COMPUTE STREET FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>(STANDARD CURB SECTION USED)<<<<<
=====
UPSTREAM ELEVATION(FEET) = 166.00 DOWNSTREAM ELEVATION(FEET) = 165.60
STREET LENGTH(FEET) = 485.00 CURB HEIGHT(INCHES) = 6.0
STREET HALFWIDTH(FEET) = 25.00

DISTANCE FROM CROWN TO CROSSFALL GRADEBREAK(FEET) = 10.00
INSIDE STREET CROSSFALL(DECIMAL) = 0.020
OUTSIDE STREET CROSSFALL(DECIMAL) = 0.020

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SPECIFIED NUMBER OF HALFSTREETS CARRYING RUNOFF = 2
 STREET PARKWAY CROSSFALL(DECIMAL) = 0.020
 Manning's FRICTION FACTOR for Streetflow Section(curbs-to-curbs) = 0.0150
 Manning's FRICTION FACTOR for Back-of-Walk Flow Section = 0.0200

**TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 11.77
 STREETFLOW MODEL RESULTS USING ESTIMATED FLOW:
 STREET FLOW DEPTH(FEET) = 0.59
 HALFSTREET FLOOD WIDTH(FEET) = 27.48
 AVERAGE FLOW VELOCITY(FEET/SEC.) = 1.04
 PRODUCT OF DEPTH&VELOCITY(FT*FT/SEC.) = 0.61
 STREET FLOW TRAVEL TIME(MIN.) = 7.75 Tc(MIN.) = 19.60

* 25 YEAR RAINFALL INTENSITY(INCH/HR) = 2.226

SUBAREA LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
COMMERCIAL	B	4.89	0.30	0.100	56

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.30
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.100
 SUBAREA AREA(ACRES) = 4.89 SUBAREA RUNOFF(CFS) = 9.67
 EFFECTIVE AREA(ACRES) = 7.51 AREA-AVERAGED Fm(INCH/HR) = 0.03
 AREA-AVERAGED Fp(INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.10
 TOTAL AREA(ACRES) = 7.5 PEAK FLOW RATE(CFS) = 14.84

END OF SUBAREA STREET FLOW HYDRAULICS:

DEPTH(FEET) = 0.63 HALFSTREET FLOOD WIDTH(FEET) = 31.40
 FLOW VELOCITY(FEET/SEC.) = 1.09 DEPTH*VELOCITY(FT*FT/SEC.) = 0.68
 *NOTE: INITIAL SUBAREA NOMOGRAPH WITH SUBAREA PARAMETERS,
 AND L = 485.0 FT WITH ELEVATION-DROP = 0.4 FT, IS 11.3 CFS,
 WHICH EXCEEDS THE TOP-OF-CURB STREET CAPACITY AT NODE 34.00
 LONGEST FLOWPATH FROM NODE 32.00 TO NODE 34.00 = 785.00 FEET.

 FLOW PROCESS FROM NODE 34.00 TO NODE 35.00 IS CODE = 51

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<<
 >>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 165.60 DOWNSTREAM(FEET) = 165.40
 CHANNEL LENGTH THRU SUBAREA(FEET) = 180.00 CHANNEL SLOPE = 0.0011
 CHANNEL BASE(FEET) = 5.00 "Z" FACTOR = 0.020
 MANNING'S FACTOR = 0.015 MAXIMUM DEPTH(FEET) = 2.00
 CHANNEL FLOW THRU SUBAREA(CFS) = 14.84
 FLOW VELOCITY(FEET/SEC.) = 2.74 FLOW DEPTH(FEET) = 1.08
 TRAVEL TIME(MIN.) = 1.09 Tc(MIN.) = 20.70
 LONGEST FLOWPATH FROM NODE 32.00 TO NODE 35.00 = 965.00 FEET.

 FLOW PROCESS FROM NODE 35.00 TO NODE 35.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

=====

TOTAL NUMBER OF STREAMS = 2

CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:

TIME OF CONCENTRATION(MIN.) = 20.70
RAINFALL INTENSITY(INCH/HR) = 2.16
AREA-AVERAGED Fm(INCH/HR) = 0.03
AREA-AVERAGED Fp(INCH/HR) = 0.30
AREA-AVERAGED Ap = 0.10
EFFECTIVE STREAM AREA(ACRES) = 7.51
TOTAL STREAM AREA(ACRES) = 7.51
PEAK FLOW RATE(CFS) AT CONFLUENCE = 14.84

FLOW PROCESS FROM NODE 35.00 TO NODE 35.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

INITIAL SUBAREA FLOW-LENGTH(FEET) = 665.00
ELEVATION DATA: UPSTREAM(FEET) = 167.20 DOWNSTREAM(FEET) = 165.40

Tc = K*[(LENGTH** 3.00)/(ELEVATION CHANGE)]**0.20
SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) = 13.351
* 25 YEAR RAINFALL INTENSITY(INCH/HR) = 2.767

SUBAREA Tc AND LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN	Tc (MIN.)
COMMERCIAL	B	6.31	0.30	0.100	56	13.35

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.30
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.100
SUBAREA RUNOFF(CFS) = 15.54
TOTAL AREA(ACRES) = 6.31 PEAK FLOW RATE(CFS) = 15.54

FLOW PROCESS FROM NODE 35.00 TO NODE 35.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<

TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
TIME OF CONCENTRATION(MIN.) = 13.35
RAINFALL INTENSITY(INCH/HR) = 2.77
AREA-AVERAGED Fm(INCH/HR) = 0.03
AREA-AVERAGED Fp(INCH/HR) = 0.30
AREA-AVERAGED Ap = 0.10
EFFECTIVE STREAM AREA(ACRES) = 6.31
TOTAL STREAM AREA(ACRES) = 6.31
PEAK FLOW RATE(CFS) AT CONFLUENCE = 15.54

** CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	14.84	20.70	2.159	0.30(0.03)	0.10	7.5	32.00
2	15.54	13.35	2.767	0.30(0.03)	0.10	6.3	35.00

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
 CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	27.85	13.35	2.767	0.30 (0.03)	0.10	11.2	35.00
2	26.93	20.70	2.159	0.30 (0.03)	0.10	13.8	32.00

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE (CFS) = 27.85 Tc (MIN.) = 13.35
 EFFECTIVE AREA (ACRES) = 11.15 AREA-AVERAGED Fm (INCH/HR) = 0.03
 AREA-AVERAGED Fp (INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.10
 TOTAL AREA (ACRES) = 13.8
 LONGEST FLOWPATH FROM NODE 32.00 TO NODE 35.00 = 965.00 FEET.

FLOW PROCESS FROM NODE 35.00 TO NODE 31.00 IS CODE = 41

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
 >>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM (FEET) = 159.00 DOWNSTREAM (FEET) = 155.00
 FLOW LENGTH (FEET) = 50.00 MANNING'S N = 0.013
 DEPTH OF FLOW IN 18.0 INCH PIPE IS 14.2 INCHES
 PIPE-FLOW VELOCITY (FEET/SEC.) = 18.67
 GIVEN PIPE DIAMETER (INCH) = 18.00 NUMBER OF PIPES = 1
 PIPE-FLOW (CFS) = 27.85
 PIPE TRAVEL TIME (MIN.) = 0.04 Tc (MIN.) = 13.40
 LONGEST FLOWPATH FROM NODE 32.00 TO NODE 31.00 = 1015.00 FEET.

FLOW PROCESS FROM NODE 31.00 TO NODE 31.00 IS CODE = 11

>>>>CONFLUENCE MEMORY BANK # 2 WITH THE MAIN-STREAM MEMORY<<<<<

** MAIN STREAM CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	27.85	13.40	2.761	0.30 (0.03)	0.10	11.2	35.00
2	26.93	20.74	2.156	0.30 (0.03)	0.10	13.8	32.00

LONGEST FLOWPATH FROM NODE 32.00 TO NODE 31.00 = 1015.00 FEET.

** MEMORY BANK # 2 CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	42.22	7.82	3.744	0.30 (0.03)	0.10	11.4	28.00
2	41.56	11.25	3.048	0.30 (0.03)	0.10	13.8	14.00
3	41.64	12.31	2.896	0.30 (0.03)	0.10	14.6	21.00
4	41.64	12.31	2.896	0.30 (0.03)	0.10	14.6	17.00
5	40.87	14.81	2.609	0.30 (0.03)	0.10	16.0	24.00
6	39.50	16.00	2.497	0.30 (0.03)	0.10	16.3	18.00
7	37.73	17.43	2.379	0.30 (0.03)	0.10	16.4	12.00

8 33.87 20.93 2.145 0.30(0.03) 0.10 16.5 10.00
 LONGEST FLOWPATH FROM NODE 10.00 TO NODE 31.00 = 2070.00 FEET.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	64.34	7.82	3.744	0.30(0.03)	0.10	18.0	28.00
2	67.41	11.25	3.048	0.30(0.03)	0.10	23.2	14.00
3	68.51	12.31	2.896	0.30(0.03)	0.10	24.8	21.00
4	68.51	12.31	2.896	0.30(0.03)	0.10	24.8	17.00
5	69.16	13.40	2.761	0.30(0.03)	0.10	26.4	35.00
6	68.55	14.81	2.609	0.30(0.03)	0.10	27.7	24.00
7	67.02	16.00	2.497	0.30(0.03)	0.10	28.4	18.00
8	65.07	17.43	2.379	0.30(0.03)	0.10	29.0	12.00
9	61.01	20.74	2.156	0.30(0.03)	0.10	30.3	32.00
10	60.66	20.93	2.145	0.30(0.03)	0.10	30.3	10.00
TOTAL AREA(ACRES) =			30.3				

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE(CFS) = 69.16 Tc(MIN.) = 13.396
 EFFECTIVE AREA(ACRES) = 26.36 AREA-AVERAGED Fm(INCH/HR) = 0.03
 AREA-AVERAGED Fp(INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.10
 TOTAL AREA(ACRES) = 30.3
 LONGEST FLOWPATH FROM NODE 10.00 TO NODE 31.00 = 2070.00 FEET.

FLOW PROCESS FROM NODE 31.00 TO NODE 36.00 IS CODE = 41

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
 >>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 155.00 DOWNSTREAM(FEET) = 153.80
 FLOW LENGTH(FEET) = 265.00 MANNING'S N = 0.013
 ASSUME FULL-FLOWING PIPELINE
 PIPE-FLOW VELOCITY(FEET/SEC.) = 7.19
 PIPE FLOW VELOCITY = (TOTAL FLOW)/(PIPE CROSS SECTION AREA)
 GIVEN PIPE DIAMETER(INCH) = 42.00 NUMBER OF PIPES = 1
 PIPE-FLOW(CFS) = 69.16
 PIPE TRAVEL TIME(MIN.) = 0.61 Tc(MIN.) = 14.01
 LONGEST FLOWPATH FROM NODE 10.00 TO NODE 36.00 = 2335.00 FEET.

FLOW PROCESS FROM NODE 36.00 TO NODE 36.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

=====

TOTAL NUMBER OF STREAMS = 2
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
 TIME OF CONCENTRATION(MIN.) = 14.01
 RAINFALL INTENSITY(INCH/HR) = 2.69
 AREA-AVERAGED Fm(INCH/HR) = 0.03
 AREA-AVERAGED Fp(INCH/HR) = 0.30
 AREA-AVERAGED Ap = 0.10
 EFFECTIVE STREAM AREA(ACRES) = 26.36

TOTAL STREAM AREA (ACRES) = 30.32
PEAK FLOW RATE (CFS) AT CONFLUENCE = 69.16

FLOW PROCESS FROM NODE 37.00 TO NODE 38.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

=====

INITIAL SUBAREA FLOW-LENGTH (FEET) = 325.00
ELEVATION DATA: UPSTREAM (FEET) = 169.50 DOWNSTREAM (FEET) = 168.00

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$
SUBAREA ANALYSIS USED MINIMUM T_c (MIN.) = 9.011
* 25 YEAR RAINFALL INTENSITY (INCH/HR) = 3.456

SUBAREA T_c AND LOSS RATE DATA (AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	F_p (INCH/HR)	A_p (DECIMAL)	SCS CN	T_c (MIN.)
COMMERCIAL	B	1.65	0.30	0.100	56	9.01

SUBAREA AVERAGE PERVIOUS LOSS RATE, F_p (INCH/HR) = 0.30

SUBAREA AVERAGE PERVIOUS AREA FRACTION, A_p = 0.100

SUBAREA RUNOFF (CFS) = 5.09

TOTAL AREA (ACRES) = 1.65 PEAK FLOW RATE (CFS) = 5.09

FLOW PROCESS FROM NODE 38.00 TO NODE 36.00 IS CODE = 41

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM (FEET) = 159.00 DOWNSTREAM (FEET) = 153.80
FLOW LENGTH (FEET) = 70.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 18.0 INCH PIPE IS 5.2 INCHES
PIPE-FLOW VELOCITY (FEET/SEC.) = 12.01
GIVEN PIPE DIAMETER (INCH) = 18.00 NUMBER OF PIPES = 1
PIPE-FLOW (CFS) = 5.09
PIPE TRAVEL TIME (MIN.) = 0.10 T_c (MIN.) = 9.11
LONGEST FLOWPATH FROM NODE 37.00 TO NODE 36.00 = 395.00 FEET.

FLOW PROCESS FROM NODE 36.00 TO NODE 36.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<

=====

TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
TIME OF CONCENTRATION (MIN.) = 9.11
RAINFALL INTENSITY (INCH/HR) = 3.44
AREA-AVERAGED F_m (INCH/HR) = 0.03
AREA-AVERAGED F_p (INCH/HR) = 0.30
AREA-AVERAGED A_p = 0.10
EFFECTIVE STREAM AREA (ACRES) = 1.65
TOTAL STREAM AREA (ACRES) = 1.65

PEAK FLOW RATE (CFS) AT CONFLUENCE = 5.09

** CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp (Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	64.34	8.39	3.599	0.30 (0.03)	0.10	18.0	28.00
1	67.41	11.88	2.955	0.30 (0.03)	0.10	23.2	14.00
1	68.51	12.93	2.817	0.30 (0.03)	0.10	24.8	21.00
1	68.51	12.93	2.817	0.30 (0.03)	0.10	24.8	17.00
1	69.16	14.01	2.692	0.30 (0.03)	0.10	26.4	35.00
1	68.55	15.43	2.549	0.30 (0.03)	0.10	27.7	24.00
1	67.02	16.64	2.443	0.30 (0.03)	0.10	28.4	18.00
1	65.07	18.00	2.337	0.30 (0.03)	0.10	29.0	12.00
1	61.01	21.31	2.123	0.30 (0.03)	0.10	30.3	32.00
1	60.66	21.50	2.113	0.30 (0.03)	0.10	30.3	10.00
2	5.09	9.11	3.435	0.30 (0.03)	0.10	1.6	37.00

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp (Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	69.25	8.39	3.599	0.30 (0.03)	0.10	19.5	28.00
2	70.06	9.11	3.435	0.30 (0.03)	0.10	20.7	37.00
3	71.78	11.88	2.955	0.30 (0.03)	0.10	24.8	14.00
4	72.67	12.93	2.817	0.30 (0.03)	0.10	26.5	21.00
5	72.67	12.93	2.817	0.30 (0.03)	0.10	26.5	17.00
6	73.14	14.01	2.692	0.30 (0.03)	0.10	28.0	35.00
7	72.31	15.43	2.549	0.30 (0.03)	0.10	29.3	24.00
8	70.63	16.64	2.443	0.30 (0.03)	0.10	30.0	18.00
9	68.52	18.00	2.337	0.30 (0.03)	0.10	30.7	12.00
10	64.14	21.31	2.123	0.30 (0.03)	0.10	32.0	32.00
11	63.78	21.50	2.113	0.30 (0.03)	0.10	32.0	10.00

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE (CFS) = 73.14 Tc (MIN.) = 14.01
EFFECTIVE AREA (ACRES) = 28.01 AREA-AVERAGED Fm (INCH/HR) = 0.03
AREA-AVERAGED Fp (INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.10
TOTAL AREA (ACRES) = 32.0
LONGEST FLOWPATH FROM NODE 10.00 TO NODE 36.00 = 2335.00 FEET.

FLOW PROCESS FROM NODE 36.00 TO NODE 39.00 IS CODE = 41

>>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<
=====

ELEVATION DATA: UPSTREAM (FEET) = 153.80 DOWNSTREAM (FEET) = 152.80
FLOW LENGTH (FEET) = 230.00 MANNING'S N = 0.013
ASSUME FULL-FLOWING PIPELINE
PIPE-FLOW VELOCITY (FEET/SEC.) = 7.60
PIPE FLOW VELOCITY = (TOTAL FLOW)/(PIPE CROSS SECTION AREA)
GIVEN PIPE DIAMETER (INCH) = 42.00 NUMBER OF PIPES = 1

PIPE-FLOW (CFS) = 73.14
PIPE TRAVEL TIME (MIN.) = 0.50 Tc (MIN.) = 14.51
LONGEST FLOWPATH FROM NODE 10.00 TO NODE 39.00 = 2565.00 FEET.

FLOW PROCESS FROM NODE 39.00 TO NODE 39.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

=====

TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION (MIN.) = 14.51
RAINFALL INTENSITY (INCH/HR) = 2.64
AREA-AVERAGED Fm (INCH/HR) = 0.03
AREA-AVERAGED Fp (INCH/HR) = 0.30
AREA-AVERAGED Ap = 0.10
EFFECTIVE STREAM AREA (ACRES) = 28.01
TOTAL STREAM AREA (ACRES) = 31.97
PEAK FLOW RATE (CFS) AT CONFLUENCE = 73.14

FLOW PROCESS FROM NODE 40.00 TO NODE 39.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

=====

INITIAL SUBAREA FLOW-LENGTH (FEET) = 320.00
ELEVATION DATA: UPSTREAM (FEET) = 167.40 DOWNSTREAM (FEET) = 167.20

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$
SUBAREA ANALYSIS USED MINIMUM Tc (MIN.) = 13.359
* 25 YEAR RAINFALL INTENSITY (INCH/HR) = 2.766
SUBAREA Tc AND LOSS RATE DATA (AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN	Tc (MIN.)
COMMERCIAL	B	0.48	0.30	0.100	56	13.36

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp (INCH/HR) = 0.30
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.100
SUBAREA RUNOFF (CFS) = 1.18
TOTAL AREA (ACRES) = 0.48 PEAK FLOW RATE (CFS) = 1.18

FLOW PROCESS FROM NODE 39.00 TO NODE 39.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<

=====

TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
TIME OF CONCENTRATION (MIN.) = 13.36
RAINFALL INTENSITY (INCH/HR) = 2.77
AREA-AVERAGED Fm (INCH/HR) = 0.03
AREA-AVERAGED Fp (INCH/HR) = 0.30
AREA-AVERAGED Ap = 0.10

EFFECTIVE STREAM AREA (ACRES) = 0.48
 TOTAL STREAM AREA (ACRES) = 0.48
 PEAK FLOW RATE (CFS) AT CONFLUENCE = 1.18

** CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp (Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	69.25	8.92	3.476	0.30 (0.03)	0.10	19.5	28.00
1	70.06	9.63	3.328	0.30 (0.03)	0.10	20.7	37.00
1	71.78	12.40	2.885	0.30 (0.03)	0.10	24.8	14.00
1	72.67	13.44	2.756	0.30 (0.03)	0.10	26.5	21.00
1	72.67	13.44	2.756	0.30 (0.03)	0.10	26.5	17.00
1	73.14	14.51	2.639	0.30 (0.03)	0.10	28.0	35.00
1	72.31	15.94	2.503	0.30 (0.03)	0.10	29.3	24.00
1	70.63	17.16	2.400	0.30 (0.03)	0.10	30.0	18.00
1	68.52	18.53	2.298	0.30 (0.03)	0.10	30.7	12.00
1	64.14	21.81	2.096	0.30 (0.03)	0.10	32.0	32.00
1	63.78	22.00	2.086	0.30 (0.03)	0.10	32.0	10.00
2	1.18	13.36	2.766	0.30 (0.03)	0.10	0.5	40.00

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
 CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp (Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	70.25	8.92	3.476	0.30 (0.03)	0.10	19.8	28.00
2	71.09	9.63	3.328	0.30 (0.03)	0.10	21.0	37.00
3	72.93	12.40	2.885	0.30 (0.03)	0.10	25.3	14.00
4	73.79	13.36	2.766	0.30 (0.03)	0.10	26.8	40.00
5	73.85	13.44	2.756	0.30 (0.03)	0.10	27.0	21.00
6	73.85	13.44	2.756	0.30 (0.03)	0.10	27.0	17.00
7	74.27	14.51	2.639	0.30 (0.03)	0.10	28.5	35.00
8	73.38	15.94	2.503	0.30 (0.03)	0.10	29.8	24.00
9	71.65	17.16	2.400	0.30 (0.03)	0.10	30.5	18.00
10	69.50	18.53	2.298	0.30 (0.03)	0.10	31.2	12.00
11	65.03	21.81	2.096	0.30 (0.03)	0.10	32.4	32.00
12	64.66	22.00	2.086	0.30 (0.03)	0.10	32.5	10.00

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE (CFS) = 74.27 Tc (MIN.) = 14.51
 EFFECTIVE AREA (ACRES) = 28.49 AREA-AVERAGED Fm (INCH/HR) = 0.03
 AREA-AVERAGED Fp (INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.10
 TOTAL AREA (ACRES) = 32.5
 LONGEST FLOWPATH FROM NODE 10.00 TO NODE 39.00 = 2565.00 FEET.

FLOW PROCESS FROM NODE 39.00 TO NODE 41.00 IS CODE = 41

>>>>> COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA <<<<<
 >>>>> USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT) <<<<<

ELEVATION DATA: UPSTREAM (FEET) = 152.80 DOWNSTREAM (FEET) = 152.00
 FLOW LENGTH (FEET) = 210.00 MANNING'S N = 0.013

DEPTH OF FLOW IN 48.0 INCH PIPE IS 34.3 INCHES
 PIPE-FLOW VELOCITY (FEET/SEC.) = 7.73
 GIVEN PIPE DIAMETER (INCH) = 48.00 NUMBER OF PIPES = 1
 PIPE-FLOW (CFS) = 74.27
 PIPE TRAVEL TIME (MIN.) = 0.45 Tc (MIN.) = 14.97
 LONGEST FLOWPATH FROM NODE 10.00 TO NODE 41.00 = 2775.00 FEET.

FLOW PROCESS FROM NODE 41.00 TO NODE 41.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

=====

TOTAL NUMBER OF STREAMS = 2
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
 TIME OF CONCENTRATION (MIN.) = 14.97
 RAINFALL INTENSITY (INCH/HR) = 2.59
 AREA-AVERAGED Fm (INCH/HR) = 0.03
 AREA-AVERAGED Fp (INCH/HR) = 0.30
 AREA-AVERAGED Ap = 0.10
 EFFECTIVE STREAM AREA (ACRES) = 28.49
 TOTAL STREAM AREA (ACRES) = 32.45
 PEAK FLOW RATE (CFS) AT CONFLUENCE = 74.27

FLOW PROCESS FROM NODE 42.00 TO NODE 41.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<

>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

=====

INITIAL SUBAREA FLOW-LENGTH (FEET) = 230.00
 ELEVATION DATA: UPSTREAM (FEET) = 166.00 DOWNSTREAM (FEET) = 163.20

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$

SUBAREA ANALYSIS USED MINIMUM Tc (MIN.) = 6.464

* 25 YEAR RAINFALL INTENSITY (INCH/HR) = 4.171

SUBAREA Tc AND LOSS RATE DATA (AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN	Tc (MIN.)
COMMERCIAL	B	0.66	0.30	0.100	56	6.46

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp (INCH/HR) = 0.30

SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.100

SUBAREA RUNOFF (CFS) = 2.46

TOTAL AREA (ACRES) = 0.66 PEAK FLOW RATE (CFS) = 2.46

FLOW PROCESS FROM NODE 41.00 TO NODE 41.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<

=====

TOTAL NUMBER OF STREAMS = 2
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
 TIME OF CONCENTRATION (MIN.) = 6.46
 RAINFALL INTENSITY (INCH/HR) = 4.17

AREA-AVERAGED Fm (INCH/HR) = 0.03
 AREA-AVERAGED Fp (INCH/HR) = 0.30
 AREA-AVERAGED Ap = 0.10
 EFFECTIVE STREAM AREA (ACRES) = 0.66
 TOTAL STREAM AREA (ACRES) = 0.66
 PEAK FLOW RATE (CFS) AT CONFLUENCE = 2.46

** CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp (Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	70.25	9.38	3.379	0.30 (0.03)	0.10	19.8	28.00
1	71.09	10.09	3.242	0.30 (0.03)	0.10	21.0	37.00
1	72.93	12.85	2.827	0.30 (0.03)	0.10	25.3	14.00
1	73.79	13.81	2.714	0.30 (0.03)	0.10	26.8	40.00
1	73.85	13.89	2.705	0.30 (0.03)	0.10	27.0	21.00
1	73.85	13.89	2.705	0.30 (0.03)	0.10	27.0	17.00
1	74.27	14.97	2.593	0.30 (0.03)	0.10	28.5	35.00
1	73.38	16.39	2.463	0.30 (0.03)	0.10	29.8	24.00
1	71.65	17.61	2.365	0.30 (0.03)	0.10	30.5	18.00
1	69.50	18.99	2.266	0.30 (0.03)	0.10	31.2	12.00
1	65.03	22.27	2.071	0.30 (0.03)	0.10	32.4	32.00
1	64.66	22.46	2.061	0.30 (0.03)	0.10	32.5	10.00
2	2.46	6.46	4.171	0.30 (0.03)	0.10	0.7	42.00

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
 CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp (Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	62.33	6.46	4.171	0.30 (0.03)	0.10	14.3	42.00
2	72.24	9.38	3.379	0.30 (0.03)	0.10	20.5	28.00
3	73.00	10.09	3.242	0.30 (0.03)	0.10	21.7	37.00
4	74.59	12.85	2.827	0.30 (0.03)	0.10	25.9	14.00
5	75.38	13.81	2.714	0.30 (0.03)	0.10	27.5	40.00
6	75.44	13.89	2.705	0.30 (0.03)	0.10	27.6	21.00
7	75.44	13.89	2.705	0.30 (0.03)	0.10	27.6	17.00
8	75.79	14.97	2.593	0.30 (0.03)	0.10	29.2	35.00
9	74.83	16.39	2.463	0.30 (0.03)	0.10	30.5	24.00
10	73.04	17.61	2.365	0.30 (0.03)	0.10	31.2	18.00
11	70.83	18.99	2.266	0.30 (0.03)	0.10	31.8	12.00
12	66.24	22.27	2.071	0.30 (0.03)	0.10	33.1	32.00
13	65.87	22.46	2.061	0.30 (0.03)	0.10	33.1	10.00

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE (CFS) = 75.79 Tc (MIN.) = 14.97
 EFFECTIVE AREA (ACRES) = 29.15 AREA-AVERAGED Fm (INCH/HR) = 0.03
 AREA-AVERAGED Fp (INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.10
 TOTAL AREA (ACRES) = 33.1
 LONGEST FLOWPATH FROM NODE 10.00 TO NODE 41.00 = 2775.00 FEET.

 FLOW PROCESS FROM NODE 41.00 TO NODE 43.00 IS CODE = 41

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 152.00 DOWNSTREAM(FEET) = 151.80
FLOW LENGTH(FEET) = 30.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 48.0 INCH PIPE IS 28.6 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 9.73
GIVEN PIPE DIAMETER(INCH) = 48.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 75.79
PIPE TRAVEL TIME(MIN.) = 0.05 Tc(MIN.) = 15.02
LONGEST FLOWPATH FROM NODE 10.00 TO NODE 43.00 = 2805.00 FEET.

FLOW PROCESS FROM NODE 43.00 TO NODE 43.00 IS CODE = 10

>>>>MAIN-STREAM MEMORY COPIED ONTO MEMORY BANK # 1 <<<<<

FLOW PROCESS FROM NODE 44.00 TO NODE 45.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

=====

INITIAL SUBAREA FLOW-LENGTH(FEET) = 350.00
ELEVATION DATA: UPSTREAM(FEET) = 164.00 DOWNSTREAM(FEET) = 162.00

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$

SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) = 8.894

* 25 YEAR RAINFALL INTENSITY(INCH/HR) = 3.482

SUBAREA Tc AND LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN	Tc (MIN.)
COMMERCIAL	B	1.55	0.30	0.100	56	8.89

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.30

SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.100

SUBAREA RUNOFF(CFS) = 4.82

TOTAL AREA(ACRES) = 1.55 PEAK FLOW RATE(CFS) = 4.82

FLOW PROCESS FROM NODE 45.00 TO NODE 46.00 IS CODE = 51

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<<
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 162.00 DOWNSTREAM(FEET) = 161.80
CHANNEL LENGTH THRU SUBAREA(FEET) = 165.00 CHANNEL SLOPE = 0.0012
CHANNEL BASE(FEET) = 3.00 "Z" FACTOR = 0.040
MANNING'S FACTOR = 0.015 MAXIMUM DEPTH(FEET) = 1.00
CHANNEL FLOW THRU SUBAREA(CFS) = 4.82
FLOW VELOCITY(FEET/SEC.) = 2.16 FLOW DEPTH(FEET) = 0.74
TRAVEL TIME(MIN.) = 1.27 Tc(MIN.) = 10.17
LONGEST FLOWPATH FROM NODE 44.00 TO NODE 46.00 = 515.00 FEET.

FLOW PROCESS FROM NODE 46.00 TO NODE 46.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION(MIN.) = 10.17
RAINFALL INTENSITY(INCH/HR) = 3.23
AREA-AVERAGED Fm(INCH/HR) = 0.03
AREA-AVERAGED Fp(INCH/HR) = 0.30
AREA-AVERAGED Ap = 0.10
EFFECTIVE STREAM AREA(ACRES) = 1.55
TOTAL STREAM AREA(ACRES) = 1.55
PEAK FLOW RATE(CFS) AT CONFLUENCE = 4.82

FLOW PROCESS FROM NODE 47.00 TO NODE 46.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

INITIAL SUBAREA FLOW-LENGTH(FEET) = 285.00
ELEVATION DATA: UPSTREAM(FEET) = 165.00 DOWNSTREAM(FEET) = 161.80

Tc = K*[(LENGTH** 3.00)/(ELEVATION CHANGE)]**0.20
SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) = 7.157
* 25 YEAR RAINFALL INTENSITY(INCH/HR) = 3.937
SUBAREA Tc AND LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ SCS SOIL AREA Fp Ap SCS Tc
LAND USE GROUP (ACRES) (INCH/HR) (DECIMAL) CN (MIN.)
COMMERCIAL B 1.23 0.30 0.100 56 7.16
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.30
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.100
SUBAREA RUNOFF(CFS) = 4.33
TOTAL AREA(ACRES) = 1.23 PEAK FLOW RATE(CFS) = 4.33

FLOW PROCESS FROM NODE 46.00 TO NODE 46.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<

TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
TIME OF CONCENTRATION(MIN.) = 7.16
RAINFALL INTENSITY(INCH/HR) = 3.94
AREA-AVERAGED Fm(INCH/HR) = 0.03
AREA-AVERAGED Fp(INCH/HR) = 0.30
AREA-AVERAGED Ap = 0.10
EFFECTIVE STREAM AREA(ACRES) = 1.23
TOTAL STREAM AREA(ACRES) = 1.23
PEAK FLOW RATE(CFS) AT CONFLUENCE = 4.33

** CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	4.82	10.17	3.228	0.30 (0.03)	0.10	1.5	44.00
2	4.33	7.16	3.937	0.30 (0.03)	0.10	1.2	47.00

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	8.47	7.16	3.937	0.30 (0.03)	0.10	2.3	47.00
2	8.36	10.17	3.228	0.30 (0.03)	0.10	2.8	44.00

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE (CFS) = 8.47 Tc (MIN.) = 7.16
 EFFECTIVE AREA (ACRES) = 2.32 AREA-AVERAGED Fm (INCH/HR) = 0.03
 AREA-AVERAGED Fp (INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.10
 TOTAL AREA (ACRES) = 2.8
 LONGEST FLOWPATH FROM NODE 44.00 TO NODE 46.00 = 515.00 FEET.

FLOW PROCESS FROM NODE 46.00 TO NODE 43.00 IS CODE = 41

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
 >>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM (FEET) = 157.00 DOWNSTREAM (FEET) = 151.80
 FLOW LENGTH (FEET) = 115.00 MANNING'S N = 0.013
 DEPTH OF FLOW IN 18.0 INCH PIPE IS 7.8 INCHES
 PIPE-FLOW VELOCITY (FEET/SEC.) = 11.55
 GIVEN PIPE DIAMETER (INCH) = 18.00 NUMBER OF PIPES = 1
 PIPE-FLOW (CFS) = 8.47
 PIPE TRAVEL TIME (MIN.) = 0.17 Tc (MIN.) = 7.32
 LONGEST FLOWPATH FROM NODE 44.00 TO NODE 43.00 = 630.00 FEET.

FLOW PROCESS FROM NODE 43.00 TO NODE 43.00 IS CODE = 11

>>>>CONFLUENCE MEMORY BANK # 1 WITH THE MAIN-STREAM MEMORY<<<<<

** MAIN STREAM CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	8.47	7.32	3.887	0.30 (0.03)	0.10	2.3	47.00
2	8.36	10.33	3.199	0.30 (0.03)	0.10	2.8	44.00

LONGEST FLOWPATH FROM NODE 44.00 TO NODE 43.00 = 630.00 FEET.

** MEMORY BANK # 1 CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	62.33	6.52	4.152	0.30 (0.03)	0.10	14.3	42.00

2	72.24	9.43	3.368	0.30 (0.03)	0.10	20.5	28.00
3	73.00	10.14	3.232	0.30 (0.03)	0.10	21.7	37.00
4	74.59	12.90	2.821	0.30 (0.03)	0.10	25.9	14.00
5	75.38	13.86	2.708	0.30 (0.03)	0.10	27.5	40.00
6	75.44	13.94	2.699	0.30 (0.03)	0.10	27.6	21.00
7	75.44	13.95	2.699	0.30 (0.03)	0.10	27.6	17.00
8	75.79	15.02	2.588	0.30 (0.03)	0.10	29.2	35.00
9	74.83	16.44	2.459	0.30 (0.03)	0.10	30.5	24.00
10	73.04	17.67	2.361	0.30 (0.03)	0.10	31.2	18.00
11	70.83	19.04	2.263	0.30 (0.03)	0.10	31.8	12.00
12	66.24	22.33	2.068	0.30 (0.03)	0.10	33.1	32.00
13	65.87	22.51	2.058	0.30 (0.03)	0.10	33.1	10.00

LONGEST FLOWPATH FROM NODE 10.00 TO NODE 43.00 = 2805.00 FEET.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp (Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	70.38	6.52	4.152	0.30 (0.03)	0.10	16.4	42.00
2	73.54	7.32	3.887	0.30 (0.03)	0.10	18.3	47.00
3	80.63	9.43	3.368	0.30 (0.03)	0.10	23.1	28.00
4	81.36	10.14	3.232	0.30 (0.03)	0.10	24.4	37.00
5	81.46	10.33	3.199	0.30 (0.03)	0.10	24.8	44.00
6	81.95	12.90	2.821	0.30 (0.03)	0.10	28.7	14.00
7	82.44	13.86	2.708	0.30 (0.03)	0.10	30.3	40.00
8	82.48	13.94	2.699	0.30 (0.03)	0.10	30.4	21.00
9	82.48	13.95	2.699	0.30 (0.03)	0.10	30.4	17.00
10	82.54	15.02	2.588	0.30 (0.03)	0.10	31.9	35.00
11	81.23	16.44	2.459	0.30 (0.03)	0.10	33.3	24.00
12	79.19	17.67	2.361	0.30 (0.03)	0.10	33.9	18.00
13	76.72	19.04	2.263	0.30 (0.03)	0.10	34.6	12.00
14	71.62	22.33	2.068	0.30 (0.03)	0.10	35.9	32.00
15	71.22	22.51	2.058	0.30 (0.03)	0.10	35.9	10.00

TOTAL AREA (ACRES) = 35.9

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE (CFS) = 82.54 Tc (MIN.) = 15.018
EFFECTIVE AREA (ACRES) = 31.93 AREA-AVERAGED Fm (INCH/HR) = 0.03
AREA-AVERAGED Fp (INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.10
TOTAL AREA (ACRES) = 35.9
LONGEST FLOWPATH FROM NODE 10.00 TO NODE 43.00 = 2805.00 FEET.

=====
END OF STUDY SUMMARY:

TOTAL AREA (ACRES) = 35.9 TC (MIN.) = 15.02
EFFECTIVE AREA (ACRES) = 31.93 AREA-AVERAGED Fm (INCH/HR) = 0.03
AREA-AVERAGED Fp (INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.100
PEAK FLOW RATE (CFS) = 82.54

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp (Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	70.38	6.52	4.152	0.30 (0.03)	0.10	16.4	42.00
2	73.54	7.32	3.887	0.30 (0.03)	0.10	18.3	47.00
3	80.63	9.43	3.368	0.30 (0.03)	0.10	23.1	28.00
4	81.36	10.14	3.232	0.30 (0.03)	0.10	24.4	37.00

5	81.46	10.33	3.199	0.30 (0.03)	0.10	24.8	44.00
6	81.95	12.90	2.821	0.30 (0.03)	0.10	28.7	14.00
7	82.44	13.86	2.708	0.30 (0.03)	0.10	30.3	40.00
8	82.48	13.94	2.699	0.30 (0.03)	0.10	30.4	21.00
9	82.48	13.95	2.699	0.30 (0.03)	0.10	30.4	17.00
10	82.54	15.02	2.588	0.30 (0.03)	0.10	31.9	35.00
11	81.23	16.44	2.459	0.30 (0.03)	0.10	33.3	24.00
12	79.19	17.67	2.361	0.30 (0.03)	0.10	33.9	18.00
13	76.72	19.04	2.263	0.30 (0.03)	0.10	34.6	12.00
14	71.62	22.33	2.068	0.30 (0.03)	0.10	35.9	32.00
15	71.22	22.51	2.058	0.30 (0.03)	0.10	35.9	10.00

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END OF RATIONAL METHOD ANALYSIS

RATIONAL METHOD HYDROLOGY COMPUTER PROGRAM PACKAGE
(Reference: 1986 ORANGE COUNTY HYDROLOGY CRITERION)
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Ver. 20.0 Release Date: 06/01/2013 License ID 1654

Analysis prepared by:

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***** DESCRIPTION OF STUDY *****

* FREEMAN *
* EXISTING CONDITION *
* 100 YEAR ANALYSIS *

FILE NAME: FREX100.DAT
TIME/DATE OF STUDY: 09:04 05/29/2017

=====

USER SPECIFIED HYDROLOGY AND HYDRAULIC MODEL INFORMATION:

=====

--*TIME-OF-CONCENTRATION MODEL*--

USER SPECIFIED STORM EVENT (YEAR) = 100.00
SPECIFIED MINIMUM PIPE SIZE (INCH) = 12.00
SPECIFIED PERCENT OF GRADIENTS (DECIMAL) TO USE FOR FRICTION SLOPE = 0.95
DATA BANK RAINFALL USED
ANTECEDENT MOISTURE CONDITION (AMC) III ASSUMED FOR RATIONAL METHOD

USER-DEFINED STREET-SECTIONS FOR COUPLED PIPEFLOW AND STREETFLOW MODEL

NO.	HALF- WIDTH (FT)	CROWN TO CROSSFALL (FT)	STREET-CROSSFALL: IN- / OUT- / PARK- SIDE / SIDE / WAY	CURB HEIGHT (FT)	GUTTER-GEOMETRIES: WIDTH LIP HIKE (FT) (FT) (FT)	MANNING FACTOR (n)
1	24.0	19.0	0.020/0.020/0.020	0.67	2.00 0.0313 0.167	0.0150

GLOBAL STREET FLOW-DEPTH CONSTRAINTS:

1. Relative Flow-Depth = 0.67 FEET
as (Maximum Allowable Street Flow Depth) - (Top-of-Curb)
 2. (Depth)*(Velocity) Constraint = 6.0 (FT*FT/S)
- *SIZE PIPE WITH A FLOW CAPACITY GREATER THAN
OR EQUAL TO THE UPSTREAM TRIBUTARY PIPE.*
*USER-SPECIFIED MINIMUM TOPOGRAPHIC SLOPE ADJUSTMENT NOT SELECTED

FLOW PROCESS FROM NODE 10.00 TO NODE 32.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<
=====

INITIAL SUBAREA FLOW-LENGTH (FEET) = 265.00
ELEVATION DATA: UPSTREAM (FEET) = 167.50 DOWNSTREAM (FEET) = 166.30

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$

SUBAREA ANALYSIS USED MINIMUM T_c (MIN.) = 8.337

* 100 YEAR RAINFALL INTENSITY (INCH/HR) = 4.616

SUBAREA T_c AND LOSS RATE DATA (AMC III):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	F_p (INCH/HR)	A_p (DECIMAL)	SCS CN	T_c (MIN.)
COMMERCIAL	B	0.15	0.30	0.100	76	8.34

SUBAREA AVERAGE PERVIOUS LOSS RATE, F_p (INCH/HR) = 0.30

SUBAREA AVERAGE PERVIOUS AREA FRACTION, A_p = 0.100

SUBAREA RUNOFF (CFS) = 0.62

TOTAL AREA (ACRES) = 0.15 PEAK FLOW RATE (CFS) = 0.62

FLOW PROCESS FROM NODE 32.00 TO NODE 11.00 IS CODE = 61

>>>>COMPUTE STREET FLOW TRAVEL TIME THRU SUBAREA<<<<<

>>>>(STANDARD CURB SECTION USED)<<<<<
=====

UPSTREAM ELEVATION (FEET) = 166.30 DOWNSTREAM ELEVATION (FEET) = 165.10

STREET LENGTH (FEET) = 210.00 CURB HEIGHT (INCHES) = 6.0

STREET HALFWIDTH (FEET) = 25.00

DISTANCE FROM CROWN TO CROSSFALL GRADEBREAK (FEET) = 10.00

INSIDE STREET CROSSFALL (DECIMAL) = 0.020

OUTSIDE STREET CROSSFALL (DECIMAL) = 0.020

SPECIFIED NUMBER OF HALFSTREETS CARRYING RUNOFF = 1

STREET PARKWAY CROSSFALL (DECIMAL) = 0.020

Manning's FRICTION FACTOR for Streetflow Section (curb-to-curb) = 0.0150

Manning's FRICTION FACTOR for Back-of-Walk Flow Section = 0.0200

**TRAVEL TIME COMPUTED USING ESTIMATED FLOW (CFS) = 0.83

STREETFLOW MODEL RESULTS USING ESTIMATED FLOW:

STREET FLOW DEPTH (FEET) = 0.26

HALFSTREET FLOOD WIDTH (FEET) = 6.85

AVERAGE FLOW VELOCITY (FEET/SEC.) = 1.42

PRODUCT OF DEPTH&VELOCITY (FT*FT/SEC.) = 0.37

STREET FLOW TRAVEL TIME (MIN.) = 2.47 T_c (MIN.) = 10.81

* 100 YEAR RAINFALL INTENSITY (INCH/HR) = 3.978

SUBAREA LOSS RATE DATA (AMC III):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	F_p (INCH/HR)	A_p (DECIMAL)	SCS CN
COMMERCIAL	B	0.12	0.30	0.100	76

SUBAREA AVERAGE PERVIOUS LOSS RATE, F_p (INCH/HR) = 0.30

SUBAREA AVERAGE PERVIOUS AREA FRACTION, A_p = 0.100

SUBAREA AREA (ACRES) = 0.12 SUBAREA RUNOFF (CFS) = 0.43

EFFECTIVE AREA (ACRES) = 0.27 AREA-AVERAGED F_m (INCH/HR) = 0.03

AREA-AVERAGED F_p (INCH/HR) = 0.30 AREA-AVERAGED A_p = 0.10
TOTAL AREA (ACRES) = 0.3 PEAK FLOW RATE (CFS) = 0.96

END OF SUBAREA STREET FLOW HYDRAULICS:

DEPTH (FEET) = 0.27 HALFSTREET FLOOD WIDTH (FEET) = 7.33
FLOW VELOCITY (FEET/SEC.) = 1.46 DEPTH*VELOCITY (FT*FT/SEC.) = 0.40
LONGEST FLOWPATH FROM NODE 10.00 TO NODE 11.00 = 475.00 FEET.

FLOW PROCESS FROM NODE 11.00 TO NODE 11.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

=====

TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION (MIN.) = 10.81
RAINFALL INTENSITY (INCH/HR) = 3.98
AREA-AVERAGED F_m (INCH/HR) = 0.03
AREA-AVERAGED F_p (INCH/HR) = 0.30
AREA-AVERAGED A_p = 0.10
EFFECTIVE STREAM AREA (ACRES) = 0.27
TOTAL STREAM AREA (ACRES) = 0.27
PEAK FLOW RATE (CFS) AT CONFLUENCE = 0.96

FLOW PROCESS FROM NODE 12.00 TO NODE 11.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

=====

INITIAL SUBAREA FLOW-LENGTH (FEET) = 210.00
ELEVATION DATA: UPSTREAM (FEET) = 166.00 DOWNSTREAM (FEET) = 165.10

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$

SUBAREA ANALYSIS USED MINIMUM T_c (MIN.) = 7.680

* 100 YEAR RAINFALL INTENSITY (INCH/HR) = 4.838

SUBAREA T_c AND LOSS RATE DATA (AMC III):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	F_p (INCH/HR)	A_p (DECIMAL)	SCS CN	T_c (MIN.)
COMMERCIAL	B	0.76	0.30	0.100	76	7.68

SUBAREA AVERAGE PERVIOUS LOSS RATE, F_p (INCH/HR) = 0.30

SUBAREA AVERAGE PERVIOUS AREA FRACTION, A_p = 0.100

SUBAREA RUNOFF (CFS) = 3.29

TOTAL AREA (ACRES) = 0.76 PEAK FLOW RATE (CFS) = 3.29

FLOW PROCESS FROM NODE 11.00 TO NODE 11.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<

=====

TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
TIME OF CONCENTRATION (MIN.) = 7.68

RAINFALL INTENSITY (INCH/HR) = 4.84
 AREA-AVERAGED Fm (INCH/HR) = 0.03
 AREA-AVERAGED Fp (INCH/HR) = 0.30
 AREA-AVERAGED Ap = 0.10
 EFFECTIVE STREAM AREA (ACRES) = 0.76
 TOTAL STREAM AREA (ACRES) = 0.76
 PEAK FLOW RATE (CFS) AT CONFLUENCE = 3.29

** CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp (Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	0.96	10.81	3.978	0.30 (0.03)	0.10	0.3	10.00
2	3.29	7.68	4.838	0.30 (0.03)	0.10	0.8	12.00

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
 CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp (Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	4.12	7.68	4.838	0.30 (0.03)	0.10	1.0	12.00
2	3.66	10.81	3.978	0.30 (0.03)	0.10	1.0	10.00

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE (CFS) = 4.12 Tc (MIN.) = 7.68
 EFFECTIVE AREA (ACRES) = 0.95 AREA-AVERAGED Fm (INCH/HR) = 0.03
 AREA-AVERAGED Fp (INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.10
 TOTAL AREA (ACRES) = 1.0
 LONGEST FLOWPATH FROM NODE 10.00 TO NODE 11.00 = 475.00 FEET.

FLOW PROCESS FROM NODE 11.00 TO NODE 13.00 IS CODE = 62

>>>>> COMPUTE STREET FLOW TRAVEL TIME THRU SUBAREA <<<<<<
 >>>>> (STREET TABLE SECTION # 1 USED) <<<<<<

UPSTREAM ELEVATION (FEET) = 165.10 DOWNSTREAM ELEVATION (FEET) = 164.20
 STREET LENGTH (FEET) = 455.00 CURB HEIGHT (INCHES) = 8.0
 STREET HALFWIDTH (FEET) = 24.00

DISTANCE FROM CROWN TO CROSSFALL GRADEBREAK (FEET) = 19.00
 INSIDE STREET CROSSFALL (DECIMAL) = 0.020
 OUTSIDE STREET CROSSFALL (DECIMAL) = 0.020

SPECIFIED NUMBER OF HALFSTREETS CARRYING RUNOFF = 1
 STREET PARKWAY CROSSFALL (DECIMAL) = 0.020
 Manning's FRICTION FACTOR for Streetflow Section (curb-to-curb) = 0.0150
 Manning's FRICTION FACTOR for Back-of-Walk Flow Section = 0.0200

**TRAVEL TIME COMPUTED USING ESTIMATED FLOW (CFS) = 4.69
 STREETFLOW MODEL RESULTS USING ESTIMATED FLOW:
 STREET FLOW DEPTH (FEET) = 0.51
 HALFSTREET FLOOD WIDTH (FEET) = 17.65
 AVERAGE FLOW VELOCITY (FEET/SEC.) = 1.42

PRODUCT OF DEPTH&VELOCITY(FT*FT/SEC.) = 0.73
 STREET FLOW TRAVEL TIME(MIN.) = 5.34 Tc(MIN.) = 13.02
 * 100 YEAR RAINFALL INTENSITY(INCH/HR) = 3.575

SUBAREA LOSS RATE DATA(AMC III):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
COMMERCIAL	B	0.36	0.30	0.100	76

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.30
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.100
 SUBAREA AREA(ACRES) = 0.36 SUBAREA RUNOFF(CFS) = 1.15
 EFFECTIVE AREA(ACRES) = 1.31 AREA-AVERAGED Fm(INCH/HR) = 0.03
 AREA-AVERAGED Fp(INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.10
 TOTAL AREA(ACRES) = 1.4 PEAK FLOW RATE(CFS) = 4.19

END OF SUBAREA STREET FLOW HYDRAULICS:

DEPTH(FEET) = 0.50 HALFSTREET FLOOD WIDTH(FEET) = 16.91
 FLOW VELOCITY(FEET/SEC.) = 1.37 DEPTH*VELOCITY(FT*FT/SEC.) = 0.68
 LONGEST FLOWPATH FROM NODE 10.00 TO NODE 13.00 = 930.00 FEET.

FLOW PROCESS FROM NODE 13.00 TO NODE 13.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

TOTAL NUMBER OF STREAMS = 2
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
 TIME OF CONCENTRATION(MIN.) = 13.02
 RAINFALL INTENSITY(INCH/HR) = 3.58
 AREA-AVERAGED Fm(INCH/HR) = 0.03
 AREA-AVERAGED Fp(INCH/HR) = 0.30
 AREA-AVERAGED Ap = 0.10
 EFFECTIVE STREAM AREA(ACRES) = 1.31
 TOTAL STREAM AREA(ACRES) = 1.39
 PEAK FLOW RATE(CFS) AT CONFLUENCE = 4.19

FLOW PROCESS FROM NODE 14.00 TO NODE 15.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<

>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

INITIAL SUBAREA FLOW-LENGTH(FEET) = 125.00
 ELEVATION DATA: UPSTREAM(FEET) = 164.60 DOWNSTREAM(FEET) = 164.20

Tc = K*[(LENGTH** 3.00)/(ELEVATION CHANGE)]**0.20

SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) = 6.616

* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 5.270

SUBAREA Tc AND LOSS RATE DATA(AMC III):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN	Tc (MIN.)
COMMERCIAL	B	0.15	0.30	0.100	76	6.62

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.30

SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.100

SUBAREA RUNOFF(CFS) = 0.71

TOTAL AREA (ACRES) = 0.15 PEAK FLOW RATE (CFS) = 0.71

FLOW PROCESS FROM NODE 15.00 TO NODE 13.00 IS CODE = 41

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM (FEET) = 159.30 DOWNSTREAM (FEET) = 159.20
FLOW LENGTH (FEET) = 50.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 18.0 INCH PIPE IS 4.8 INCHES
PIPE-FLOW VELOCITY (FEET/SEC.) = 1.88
GIVEN PIPE DIAMETER (INCH) = 18.00 NUMBER OF PIPES = 1
PIPE-FLOW (CFS) = 0.71
PIPE TRAVEL TIME (MIN.) = 0.44 Tc (MIN.) = 7.06
LONGEST FLOWPATH FROM NODE 14.00 TO NODE 13.00 = 175.00 FEET.

FLOW PROCESS FROM NODE 13.00 TO NODE 13.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<

=====

TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
TIME OF CONCENTRATION (MIN.) = 7.06
RAINFALL INTENSITY (INCH/HR) = 5.08
AREA-AVERAGED Fm (INCH/HR) = 0.03
AREA-AVERAGED Fp (INCH/HR) = 0.30
AREA-AVERAGED Ap = 0.10
EFFECTIVE STREAM AREA (ACRES) = 0.15
TOTAL STREAM AREA (ACRES) = 0.15
PEAK FLOW RATE (CFS) AT CONFLUENCE = 0.71

** CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp (Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	4.19	13.02	3.575	0.30 (0.03)	0.10	1.3	12.00
1	3.89	16.31	3.142	0.30 (0.03)	0.10	1.4	10.00
2	0.71	7.06	5.077	0.30 (0.03)	0.10	0.2	14.00

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp (Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	3.94	7.06	5.077	0.30 (0.03)	0.10	0.9	14.00
2	4.68	13.02	3.575	0.30 (0.03)	0.10	1.5	12.00
3	4.33	16.31	3.142	0.30 (0.03)	0.10	1.5	10.00

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE (CFS) = 4.68 Tc (MIN.) = 13.02
EFFECTIVE AREA (ACRES) = 1.46 AREA-AVERAGED Fm (INCH/HR) = 0.03

AREA-AVERAGED F_p (INCH/HR) = 0.30 AREA-AVERAGED A_p = 0.10
TOTAL AREA (ACRES) = 1.5
LONGEST FLOWPATH FROM NODE 10.00 TO NODE 13.00 = 930.00 FEET.

FLOW PROCESS FROM NODE 13.00 TO NODE 16.00 IS CODE = 41

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 159.20 DOWNSTREAM(FEET) = 158.10
FLOW LENGTH(FEET) = 285.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 18.0 INCH PIPE IS 11.5 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 3.93
GIVEN PIPE DIAMETER(INCH) = 18.00 NUMBER OF PIPES = 1
PIPE-FLOW (CFS) = 4.68
PIPE TRAVEL TIME (MIN.) = 1.21 T_c (MIN.) = 14.23
LONGEST FLOWPATH FROM NODE 10.00 TO NODE 16.00 = 1215.00 FEET.

FLOW PROCESS FROM NODE 16.00 TO NODE 16.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

=====

TOTAL NUMBER OF STREAMS = 3
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION (MIN.) = 14.23
RAINFALL INTENSITY (INCH/HR) = 3.40
AREA-AVERAGED F_m (INCH/HR) = 0.03
AREA-AVERAGED F_p (INCH/HR) = 0.30
AREA-AVERAGED A_p = 0.10
EFFECTIVE STREAM AREA (ACRES) = 1.46
TOTAL STREAM AREA (ACRES) = 1.54
PEAK FLOW RATE (CFS) AT CONFLUENCE = 4.68

FLOW PROCESS FROM NODE 17.00 TO NODE 16.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

=====

INITIAL SUBAREA FLOW-LENGTH (FEET) = 310.00
ELEVATION DATA: UPSTREAM (FEET) = 166.00 DOWNSTREAM (FEET) = 164.90

$$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$$

SUBAREA ANALYSIS USED MINIMUM T_c (MIN.) = 9.320

* 100 YEAR RAINFALL INTENSITY (INCH/HR) = 4.330

SUBAREA T_c AND LOSS RATE DATA (AMC III):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	F_p (INCH/HR)	A_p (DECIMAL)	SCS CN	T_c (MIN.)
COMMERCIAL	B	1.52	0.30	0.100	76	9.32

SUBAREA AVERAGE PERVIOUS LOSS RATE, F_p (INCH/HR) = 0.30

SUBAREA AVERAGE PERVIOUS AREA FRACTION, A_p = 0.100

SUBAREA RUNOFF (CFS) = 5.88

TOTAL AREA (ACRES) = 1.52 PEAK FLOW RATE (CFS) = 5.88

FLOW PROCESS FROM NODE 16.00 TO NODE 16.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

=====

TOTAL NUMBER OF STREAMS = 3
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
TIME OF CONCENTRATION (MIN.) = 9.32
RAINFALL INTENSITY (INCH/HR) = 4.33
AREA-AVERAGED F_m (INCH/HR) = 0.03
AREA-AVERAGED F_p (INCH/HR) = 0.30
AREA-AVERAGED A_p = 0.10
EFFECTIVE STREAM AREA (ACRES) = 1.52
TOTAL STREAM AREA (ACRES) = 1.52
PEAK FLOW RATE (CFS) AT CONFLUENCE = 5.88

FLOW PROCESS FROM NODE 18.00 TO NODE 19.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

=====

INITIAL SUBAREA FLOW-LENGTH (FEET) = 320.00
ELEVATION DATA: UPSTREAM (FEET) = 165.90 DOWNSTREAM (FEET) = 165.00

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$
SUBAREA ANALYSIS USED MINIMUM T_c (MIN.) = 9.888
* 100 YEAR RAINFALL INTENSITY (INCH/HR) = 4.186
SUBAREA T_c AND LOSS RATE DATA (AMC III):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	F_p (INCH/HR)	A_p (DECIMAL)	SCS CN	T_c (MIN.)
COMMERCIAL	B	1.26	0.30	0.100	76	9.89

SUBAREA AVERAGE PERVIOUS LOSS RATE, F_p (INCH/HR) = 0.30
SUBAREA AVERAGE PERVIOUS AREA FRACTION, A_p = 0.100
SUBAREA RUNOFF (CFS) = 4.71
TOTAL AREA (ACRES) = 1.26 PEAK FLOW RATE (CFS) = 4.71

FLOW PROCESS FROM NODE 19.00 TO NODE 16.00 IS CODE = 51

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<<
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM (FEET) = 165.00 DOWNSTREAM (FEET) = 164.90
CHANNEL LENGTH THRU SUBAREA (FEET) = 265.00 CHANNEL SLOPE = 0.0004
CHANNEL BASE (FEET) = 3.00 "Z" FACTOR = 0.040
MANNING'S FACTOR = 0.015 MAXIMUM DEPTH (FEET) = 1.00

==>>WARNING: FLOW IN CHANNEL EXCEEDS CHANNEL
CAPACITY (NORMAL DEPTH EQUAL TO SPECIFIED MAXIMUM
ALLOWABLE DEPTH).
AS AN APPROXIMATION, FLOWDEPTH IS SET AT MAXIMUM

ALLOWABLE DEPTH AND IS USED FOR TRAVELTIME CALCULATIONS.

CHANNEL FLOW THRU SUBAREA(CFS) = 4.71
 FLOW VELOCITY(FEET/SEC.) = 1.55 FLOW DEPTH(FEET) = 1.00
 TRAVEL TIME(MIN.) = 2.85 Tc(MIN.) = 12.74
 LONGEST FLOWPATH FROM NODE 18.00 TO NODE 16.00 = 585.00 FEET.

 FLOW PROCESS FROM NODE 16.00 TO NODE 16.00 IS CODE = 1

 >>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
 >>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<
 =====

TOTAL NUMBER OF STREAMS = 3
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 3 ARE:
 TIME OF CONCENTRATION(MIN.) = 12.74
 RAINFALL INTENSITY(INCH/HR) = 3.62
 AREA-AVERAGED Fm(INCH/HR) = 0.03
 AREA-AVERAGED Fp(INCH/HR) = 0.30
 AREA-AVERAGED Ap = 0.10
 EFFECTIVE STREAM AREA(ACRES) = 1.26
 TOTAL STREAM AREA(ACRES) = 1.26
 PEAK FLOW RATE(CFS) AT CONFLUENCE = 4.71

** CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	3.94	8.31	4.624	0.30 (0.03)	0.10	0.9	14.00
1	4.68	14.23	3.398	0.30 (0.03)	0.10	1.5	12.00
1	4.33	17.54	3.014	0.30 (0.03)	0.10	1.5	10.00
2	5.88	9.32	4.330	0.30 (0.03)	0.10	1.5	17.00
3	4.71	12.74	3.621	0.30 (0.03)	0.10	1.3	18.00

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
 CONFLUENCE FORMULA USED FOR 3 STREAMS.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	13.48	8.31	4.624	0.30 (0.03)	0.10	3.0	14.00
2	14.08	9.32	4.330	0.30 (0.03)	0.10	3.4	17.00
3	14.20	12.74	3.621	0.30 (0.03)	0.10	4.1	18.00
4	13.71	14.23	3.398	0.30 (0.03)	0.10	4.2	12.00
5	12.33	17.54	3.014	0.30 (0.03)	0.10	4.3	10.00

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE(CFS) = 14.20 Tc(MIN.) = 12.74
 EFFECTIVE AREA(ACRES) = 4.09 AREA-AVERAGED Fm(INCH/HR) = 0.03
 AREA-AVERAGED Fp(INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.10
 TOTAL AREA(ACRES) = 4.3
 LONGEST FLOWPATH FROM NODE 10.00 TO NODE 16.00 = 1215.00 FEET.

 FLOW PROCESS FROM NODE 16.00 TO NODE 20.00 IS CODE = 41

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-----
>>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 158.10 DOWNSTREAM(FEET) = 157.60
FLOW LENGTH(FEET) = 150.00 MANNING'S N = 0.013
ASSUME FULL-FLOWING PIPELINE
PIPE-FLOW VELOCITY(FEET/SEC.) = 4.52
PIPE FLOW VELOCITY = (TOTAL FLOW)/(PIPE CROSS SECTION AREA)
GIVEN PIPE DIAMETER(INCH) = 24.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 14.20
PIPE TRAVEL TIME(MIN.) = 0.55 Tc(MIN.) = 13.29
LONGEST FLOWPATH FROM NODE 10.00 TO NODE 20.00 = 1365.00 FEET.

*****
FLOW PROCESS FROM NODE 20.00 TO NODE 20.00 IS CODE = 1
-----
>>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
=====
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION(MIN.) = 13.29
RAINFALL INTENSITY(INCH/HR) = 3.53
AREA-AVERAGED Fm(INCH/HR) = 0.03
AREA-AVERAGED Fp(INCH/HR) = 0.30
AREA-AVERAGED Ap = 0.10
EFFECTIVE STREAM AREA(ACRES) = 4.09
TOTAL STREAM AREA(ACRES) = 4.32
PEAK FLOW RATE(CFS) AT CONFLUENCE = 14.20

*****
FLOW PROCESS FROM NODE 21.00 TO NODE 22.00 IS CODE = 21
-----
>>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<
=====
INITIAL SUBAREA FLOW-LENGTH(FEET) = 260.00
ELEVATION DATA: UPSTREAM(FEET) = 166.00 DOWNSTREAM(FEET) = 165.50

Tc = K*[(LENGTH** 3.00)/(ELEVATION CHANGE)]**0.20
SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) = 9.819
* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 4.203
SUBAREA Tc AND LOSS RATE DATA(AMC III):
DEVELOPMENT TYPE/ SCS SOIL AREA Fp Ap SCS Tc
LAND USE GROUP (ACRES) (INCH/HR) (DECIMAL) CN (MIN.)
COMMERCIAL B 0.64 0.30 0.100 76 9.82
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.30
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.100
SUBAREA RUNOFF(CFS) = 2.40
TOTAL AREA(ACRES) = 0.64 PEAK FLOW RATE(CFS) = 2.40

*****
FLOW PROCESS FROM NODE 22.00 TO NODE 20.00 IS CODE = 31
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>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
 >>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 158.20 DOWNSTREAM(FEET) = 157.60
 FLOW LENGTH(FEET) = 20.00 MANNING'S N = 0.013
 ESTIMATED PIPE DIAMETER(INCH) INCREASED TO 12.000
 DEPTH OF FLOW IN 12.0 INCH PIPE IS 5.3 INCHES
 PIPE-FLOW VELOCITY(FEET/SEC.) = 7.22
 ESTIMATED PIPE DIAMETER(INCH) = 12.000 NUMBER OF PIPES = 1
 PIPE-FLOW(CFS) = 2.40
 PIPE TRAVEL TIME(MIN.) = 0.05 Tc(MIN.) = 9.87
 LONGEST FLOWPATH FROM NODE 21.00 TO NODE 20.00 = 280.00 FEET.

 FLOW PROCESS FROM NODE 20.00 TO NODE 20.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
 >>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<

=====

TOTAL NUMBER OF STREAMS = 2
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
 TIME OF CONCENTRATION(MIN.) = 9.87
 RAINFALL INTENSITY(INCH/HR) = 4.19
 AREA-AVERAGED Fm(INCH/HR) = 0.03
 AREA-AVERAGED Fp(INCH/HR) = 0.30
 AREA-AVERAGED Ap = 0.10
 EFFECTIVE STREAM AREA(ACRES) = 0.64
 TOTAL STREAM AREA(ACRES) = 0.64
 PEAK FLOW RATE(CFS) AT CONFLUENCE = 2.40

** CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	13.48	8.90	4.448	0.30 (0.03)	0.10	3.0	14.00
1	14.08	9.88	4.189	0.30 (0.03)	0.10	3.4	17.00
1	14.20	13.29	3.534	0.30 (0.03)	0.10	4.1	18.00
1	13.71	14.80	3.322	0.30 (0.03)	0.10	4.2	12.00
1	12.33	18.08	2.962	0.30 (0.03)	0.10	4.3	10.00
2	2.40	9.87	4.192	0.30 (0.03)	0.10	0.6	21.00

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
 CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	15.78	8.90	4.448	0.30 (0.03)	0.10	3.6	14.00
2	16.47	9.87	4.192	0.30 (0.03)	0.10	4.0	21.00
3	16.48	9.88	4.189	0.30 (0.03)	0.10	4.0	17.00
4	16.23	13.29	3.534	0.30 (0.03)	0.10	4.7	18.00
5	15.61	14.80	3.322	0.30 (0.03)	0.10	4.9	12.00
6	14.02	18.08	2.962	0.30 (0.03)	0.10	5.0	10.00

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE (CFS) = 16.48 Tc (MIN.) = 9.88
 EFFECTIVE AREA (ACRES) = 4.05 AREA-AVERAGED Fm (INCH/HR) = 0.03
 AREA-AVERAGED Fp (INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.10
 TOTAL AREA (ACRES) = 5.0
 LONGEST FLOWPATH FROM NODE 10.00 TO NODE 20.00 = 1365.00 FEET.

FLOW PROCESS FROM NODE 20.00 TO NODE 23.00 IS CODE = 41

 >>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
 >>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM (FEET) = 157.60 DOWNSTREAM (FEET) = 156.40
 FLOW LENGTH (FEET) = 365.00 MANNING'S N = 0.013
 ASSUME FULL-FLOWING PIPELINE
 PIPE-FLOW VELOCITY (FEET/SEC.) = 5.25
 PIPE FLOW VELOCITY = (TOTAL FLOW)/(PIPE CROSS SECTION AREA)
 GIVEN PIPE DIAMETER (INCH) = 24.00 NUMBER OF PIPES = 1
 PIPE-FLOW (CFS) = 16.48
 PIPE TRAVEL TIME (MIN.) = 1.16 Tc (MIN.) = 11.04
 LONGEST FLOWPATH FROM NODE 10.00 TO NODE 23.00 = 1730.00 FEET.

FLOW PROCESS FROM NODE 23.00 TO NODE 23.00 IS CODE = 1

 >>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

=====

TOTAL NUMBER OF STREAMS = 2
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
 TIME OF CONCENTRATION (MIN.) = 11.04
 RAINFALL INTENSITY (INCH/HR) = 3.93
 AREA-AVERAGED Fm (INCH/HR) = 0.03
 AREA-AVERAGED Fp (INCH/HR) = 0.30
 AREA-AVERAGED Ap = 0.10
 EFFECTIVE STREAM AREA (ACRES) = 4.05
 TOTAL STREAM AREA (ACRES) = 4.96
 PEAK FLOW RATE (CFS) AT CONFLUENCE = 16.48

FLOW PROCESS FROM NODE 24.00 TO NODE 25.00 IS CODE = 21

 >>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
 >>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

=====

INITIAL SUBAREA FLOW-LENGTH (FEET) = 320.00
 ELEVATION DATA: UPSTREAM (FEET) = 166.00 DOWNSTREAM (FEET) = 165.50

Tc = K * [(LENGTH** 3.00) / (ELEVATION CHANGE)]**0.20
 SUBAREA ANALYSIS USED MINIMUM Tc (MIN.) = 11.122
 * 100 YEAR RAINFALL INTENSITY (INCH/HR) = 3.913
 SUBAREA Tc AND LOSS RATE DATA (AMC III):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN	Tc (MIN.)
COMMERCIAL	B	1.40	0.30	0.100	76	11.12

SUBAREA AVERAGE PERVIOUS LOSS RATE, F_p (INCH/HR) = 0.30
SUBAREA AVERAGE PERVIOUS AREA FRACTION, A_p = 0.100
SUBAREA RUNOFF (CFS) = 4.89
TOTAL AREA (ACRES) = 1.40 PEAK FLOW RATE (CFS) = 4.89

FLOW PROCESS FROM NODE 25.00 TO NODE 26.00 IS CODE = 51

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<<
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM (FEET) = 165.50 DOWNSTREAM (FEET) = 161.10
CHANNEL LENGTH THRU SUBAREA (FEET) = 330.00 CHANNEL SLOPE = 0.0133
CHANNEL BASE (FEET) = 20.00 "Z" FACTOR = 0.020
MANNING'S FACTOR = 0.015 MAXIMUM DEPTH (FEET) = 2.00
* 100 YEAR RAINFALL INTENSITY (INCH/HR) = 3.619

SUBAREA LOSS RATE DATA (AMC III):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	F_p (INCH/HR)	A_p (DECIMAL)	SCS CN
COMMERCIAL	B	3.87	0.30	0.100	76

SUBAREA AVERAGE PERVIOUS LOSS RATE, F_p (INCH/HR) = 0.30
SUBAREA AVERAGE PERVIOUS AREA FRACTION, A_p = 0.100
TRAVEL TIME COMPUTED USING ESTIMATED FLOW (CFS) = 11.15
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY (FEET/SEC.) = 3.38
AVERAGE FLOW DEPTH (FEET) = 0.16 TRAVEL TIME (MIN.) = 1.63
 T_c (MIN.) = 12.75
SUBAREA AREA (ACRES) = 3.87 SUBAREA RUNOFF (CFS) = 12.50
EFFECTIVE AREA (ACRES) = 5.27 AREA-AVERAGED F_m (INCH/HR) = 0.03
AREA-AVERAGED F_p (INCH/HR) = 0.30 AREA-AVERAGED A_p = 0.10
TOTAL AREA (ACRES) = 5.3 PEAK FLOW RATE (CFS) = 17.02

END OF SUBAREA CHANNEL FLOW HYDRAULICS:

DEPTH (FEET) = 0.21 FLOW VELOCITY (FEET/SEC.) = 4.04
LONGEST FLOWPATH FROM NODE 24.00 TO NODE 26.00 = 650.00 FEET.

FLOW PROCESS FROM NODE 26.00 TO NODE 23.00 IS CODE = 41

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM (FEET) = 158.00 DOWNSTREAM (FEET) = 156.40
FLOW LENGTH (FEET) = 315.00 MANNING'S N = 0.013
ASSUME FULL-FLOWING PIPELINE
PIPE-FLOW VELOCITY (FEET/SEC.) = 5.42
PIPE FLOW VELOCITY = (TOTAL FLOW)/(PIPE CROSS SECTION AREA)
GIVEN PIPE DIAMETER (INCH) = 24.00 NUMBER OF PIPES = 1
PIPE-FLOW (CFS) = 17.02
PIPE TRAVEL TIME (MIN.) = 0.97 T_c (MIN.) = 13.72
LONGEST FLOWPATH FROM NODE 24.00 TO NODE 23.00 = 965.00 FEET.

FLOW PROCESS FROM NODE 23.00 TO NODE 23.00 IS CODE = 1

>>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
 >>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<

=====

TOTAL NUMBER OF STREAMS = 2
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
 TIME OF CONCENTRATION(MIN.) = 13.72
 RAINFALL INTENSITY(INCH/HR) = 3.47
 AREA-AVERAGED Fm(INCH/HR) = 0.03
 AREA-AVERAGED Fp(INCH/HR) = 0.30
 AREA-AVERAGED Ap = 0.10
 EFFECTIVE STREAM AREA(ACRES) = 5.27
 TOTAL STREAM AREA(ACRES) = 5.27
 PEAK FLOW RATE(CFS) AT CONFLUENCE = 17.02

** CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	15.78	10.11	4.134	0.30(0.03)	0.10	3.6	14.00
1	16.47	11.03	3.933	0.30(0.03)	0.10	4.0	21.00
1	16.48	11.04	3.930	0.30(0.03)	0.10	4.0	17.00
1	16.23	14.47	3.366	0.30(0.03)	0.10	4.7	18.00
1	15.61	16.03	3.174	0.30(0.03)	0.10	4.9	12.00
1	14.02	19.45	2.841	0.30(0.03)	0.10	5.0	10.00
2	17.02	13.72	3.470	0.30(0.03)	0.10	5.3	24.00

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
 CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	30.74	10.11	4.134	0.30(0.03)	0.10	7.5	14.00
2	31.99	11.03	3.933	0.30(0.03)	0.10	8.3	21.00
3	32.01	11.04	3.930	0.30(0.03)	0.10	8.3	17.00
4	33.30	13.72	3.470	0.30(0.03)	0.10	9.9	24.00
5	32.73	14.47	3.366	0.30(0.03)	0.10	10.0	18.00
6	31.17	16.03	3.174	0.30(0.03)	0.10	10.2	12.00
7	27.93	19.45	2.841	0.30(0.03)	0.10	10.2	10.00

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE(CFS) = 33.30 Tc(MIN.) = 13.72
 EFFECTIVE AREA(ACRES) = 9.85 AREA-AVERAGED Fm(INCH/HR) = 0.03
 AREA-AVERAGED Fp(INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.10
 TOTAL AREA(ACRES) = 10.2
 LONGEST FLOWPATH FROM NODE 10.00 TO NODE 23.00 = 1730.00 FEET.

 FLOW PROCESS FROM NODE 23.00 TO NODE 27.00 IS CODE = 41

>>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
 >>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 156.40 DOWNSTREAM(FEET) = 155.40
 FLOW LENGTH(FEET) = 235.00 MANNING'S N = 0.013

ASSUME FULL-FLOWING PIPELINE
 PIPE-FLOW VELOCITY (FEET/SEC.) = 6.78
 PIPE FLOW VELOCITY = (TOTAL FLOW)/(PIPE CROSS SECTION AREA)
 GIVEN PIPE DIAMETER (INCH) = 30.00 NUMBER OF PIPES = 1
 PIPE-FLOW (CFS) = 33.30
 PIPE TRAVEL TIME (MIN.) = 0.58 Tc (MIN.) = 14.30
 LONGEST FLOWPATH FROM NODE 10.00 TO NODE 27.00 = 1965.00 FEET.

 FLOW PROCESS FROM NODE 27.00 TO NODE 27.00 IS CODE = 1

 >>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

=====

TOTAL NUMBER OF STREAMS = 2
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
 TIME OF CONCENTRATION (MIN.) = 14.30
 RAINFALL INTENSITY (INCH/HR) = 3.39
 AREA-AVERAGED Fm (INCH/HR) = 0.03
 AREA-AVERAGED Fp (INCH/HR) = 0.30
 AREA-AVERAGED Ap = 0.10
 EFFECTIVE STREAM AREA (ACRES) = 9.85
 TOTAL STREAM AREA (ACRES) = 10.23
 PEAK FLOW RATE (CFS) AT CONFLUENCE = 33.30

 FLOW PROCESS FROM NODE 28.00 TO NODE 29.00 IS CODE = 21

 >>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
 >>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

=====

INITIAL SUBAREA FLOW-LENGTH (FEET) = 225.00
 ELEVATION DATA: UPSTREAM (FEET) = 170.00 DOWNSTREAM (FEET) = 166.50

Tc = K*[(LENGTH** 3.00)/(ELEVATION CHANGE)]**0.20
 SUBAREA ANALYSIS USED MINIMUM Tc (MIN.) = 6.101
 * 100 YEAR RAINFALL INTENSITY (INCH/HR) = 5.521
 SUBAREA Tc AND LOSS RATE DATA (AMC III):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN	Tc (MIN.)
COMMERCIAL	B	1.32	0.30	0.100	76	6.10

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp (INCH/HR) = 0.30
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.100
 SUBAREA RUNOFF (CFS) = 6.52
 TOTAL AREA (ACRES) = 1.32 PEAK FLOW RATE (CFS) = 6.52

 FLOW PROCESS FROM NODE 29.00 TO NODE 30.00 IS CODE = 51

 >>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<<
 >>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM (FEET) = 166.50 DOWNSTREAM (FEET) = 164.20
 CHANNEL LENGTH THRU SUBAREA (FEET) = 300.00 CHANNEL SLOPE = 0.0077
 CHANNEL BASE (FEET) = 5.00 "Z" FACTOR = 1.500

MANNING'S FACTOR = 0.015 MAXIMUM DEPTH (FEET) = 2.00

* 100 YEAR RAINFALL INTENSITY (INCH/HR) = 5.037

SUBAREA LOSS RATE DATA (AMC III):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
COMMERCIAL	B	2.72	0.30	0.100	76

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp (INCH/HR) = 0.30

SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.100

TRAVEL TIME COMPUTED USING ESTIMATED FLOW (CFS) = 12.66

TRAVEL TIME THRU SUBAREA BASED ON VELOCITY (FEET/SEC.) = 4.72

AVERAGE FLOW DEPTH (FEET) = 0.47 TRAVEL TIME (MIN.) = 1.06

Tc (MIN.) = 7.16

SUBAREA AREA (ACRES) = 2.72 SUBAREA RUNOFF (CFS) = 12.26

EFFECTIVE AREA (ACRES) = 4.04 AREA-AVERAGED Fm (INCH/HR) = 0.03

AREA-AVERAGED Fp (INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.10

TOTAL AREA (ACRES) = 4.0 PEAK FLOW RATE (CFS) = 18.20

END OF SUBAREA CHANNEL FLOW HYDRAULICS:

DEPTH (FEET) = 0.58 FLOW VELOCITY (FEET/SEC.) = 5.32

LONGEST FLOWPATH FROM NODE 28.00 TO NODE 30.00 = 525.00 FEET.

FLOW PROCESS FROM NODE 30.00 TO NODE 30.00 IS CODE = 82

>>>>ADD SUBAREA RUNOFF TO MAINLINE, AT MAINLINE Tc,<<<<<

>>>>(AND COMPUTE INITIAL SUBAREA RUNOFF)<<<<<

INITIAL SUBAREA FLOW-LENGTH (FEET) = 300.00

ELEVATION DATA: UPSTREAM (FEET) = 167.40 DOWNSTREAM (FEET) = 164.20

Tc = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20

SUBAREA ANALYSIS USED MINIMUM Tc (MIN.) = 7.381

* 100 YEAR RAINFALL INTENSITY (INCH/HR) = 4.950

SUBAREA Tc AND LOSS RATE DATA (AMC III):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN	Tc (MIN.)
COMMERCIAL	B	2.23	0.30	0.100	76	7.38

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp (INCH/HR) = 0.30

SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.100

SUBAREA AREA (ACRES) = 2.23 INITIAL SUBAREA RUNOFF (CFS) = 9.87

** ADD SUBAREA RUNOFF TO MAINLINE AT MAINLINE Tc:

MAINLINE Tc (MIN.) = 7.16

* 100 YEAR RAINFALL INTENSITY (INCH/HR) = 5.037

SUBAREA AREA (ACRES) = 2.23 SUBAREA RUNOFF (CFS) = 10.05

EFFECTIVE AREA (ACRES) = 6.27 AREA-AVERAGED Fm (INCH/HR) = 0.03

AREA-AVERAGED Fp (INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.10

TOTAL AREA (ACRES) = 6.3 PEAK FLOW RATE (CFS) = 28.25

FLOW PROCESS FROM NODE 30.00 TO NODE 27.00 IS CODE = 41

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<

>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<


```

=====
ELEVATION DATA: UPSTREAM(FEET) = 158.00 DOWNSTREAM(FEET) = 155.40
FLOW LENGTH(FEET) = 160.00 MANNING'S N = 0.013
ASSUME FULL-FLOWING PIPELINE
PIPE-FLOW VELOCITY(FEET/SEC.) = 8.99
PIPE FLOW VELOCITY = (TOTAL FLOW)/(PIPE CROSS SECTION AREA)
GIVEN PIPE DIAMETER(INCH) = 24.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 28.25
PIPE TRAVEL TIME(MIN.) = 0.30 Tc(MIN.) = 7.46
LONGEST FLOWPATH FROM NODE 28.00 TO NODE 27.00 = 685.00 FEET.

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*****
FLOW PROCESS FROM NODE 27.00 TO NODE 27.00 IS CODE = 1

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>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<

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=====
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
TIME OF CONCENTRATION(MIN.) = 7.46
RAINFALL INTENSITY(INCH/HR) = 4.92
AREA-AVERAGED Fm(INCH/HR) = 0.03
AREA-AVERAGED Fp(INCH/HR) = 0.30
AREA-AVERAGED Ap = 0.10
EFFECTIVE STREAM AREA(ACRES) = 6.27
TOTAL STREAM AREA(ACRES) = 6.27
PEAK FLOW RATE(CFS) AT CONFLUENCE = 28.25

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** CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	30.74	10.73	3.994	0.30 (0.03)	0.10	7.5	14.00
1	31.99	11.63	3.815	0.30 (0.03)	0.10	8.3	21.00
1	32.01	11.64	3.813	0.30 (0.03)	0.10	8.3	17.00
1	33.30	14.30	3.389	0.30 (0.03)	0.10	9.9	24.00
1	32.73	15.06	3.290	0.30 (0.03)	0.10	10.0	18.00
1	31.17	16.64	3.106	0.30 (0.03)	0.10	10.2	12.00
1	27.93	20.13	2.785	0.30 (0.03)	0.10	10.2	10.00
2	28.25	7.46	4.921	0.30 (0.03)	0.10	6.3	28.00

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	54.60	7.46	4.921	0.30 (0.03)	0.10	11.5	28.00
2	53.64	10.73	3.994	0.30 (0.03)	0.10	13.8	14.00
3	53.86	11.63	3.815	0.30 (0.03)	0.10	14.5	21.00
4	53.86	11.64	3.813	0.30 (0.03)	0.10	14.6	17.00
5	52.71	14.30	3.389	0.30 (0.03)	0.10	16.1	24.00
6	51.56	15.06	3.290	0.30 (0.03)	0.10	16.3	18.00
7	48.94	16.64	3.106	0.30 (0.03)	0.10	16.4	12.00
8	43.85	20.13	2.785	0.30 (0.03)	0.10	16.5	10.00

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE (CFS) = 54.60 Tc (MIN.) = 7.46
EFFECTIVE AREA (ACRES) = 11.48 AREA-AVERAGED Fm (INCH/HR) = 0.03
AREA-AVERAGED Fp (INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.10
TOTAL AREA (ACRES) = 16.5
LONGEST FLOWPATH FROM NODE 10.00 TO NODE 27.00 = 1965.00 FEET.

FLOW PROCESS FROM NODE 27.00 TO NODE 31.00 IS CODE = 41

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM (FEET) = 155.40 DOWNSTREAM (FEET) = 155.00
FLOW LENGTH (FEET) = 105.00 MANNING'S N = 0.013
ASSUME FULL-FLOWING PIPELINE
PIPE-FLOW VELOCITY (FEET/SEC.) = 7.72
PIPE FLOW VELOCITY = (TOTAL FLOW)/(PIPE CROSS SECTION AREA)
GIVEN PIPE DIAMETER (INCH) = 36.00 NUMBER OF PIPES = 1
PIPE-FLOW (CFS) = 54.60
PIPE TRAVEL TIME (MIN.) = 0.23 Tc (MIN.) = 7.68
LONGEST FLOWPATH FROM NODE 10.00 TO NODE 31.00 = 2070.00 FEET.

FLOW PROCESS FROM NODE 31.00 TO NODE 31.00 IS CODE = 10

>>>>MAIN-STREAM MEMORY COPIED ONTO MEMORY BANK # 2 <<<<<

FLOW PROCESS FROM NODE 32.00 TO NODE 33.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

=====

INITIAL SUBAREA FLOW-LENGTH (FEET) = 300.00
ELEVATION DATA: UPSTREAM (FEET) = 166.30 DOWNSTREAM (FEET) = 166.00

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$

SUBAREA ANALYSIS USED MINIMUM Tc (MIN.) = 11.850

* 100 YEAR RAINFALL INTENSITY (INCH/HR) = 3.774

SUBAREA Tc AND LOSS RATE DATA (AMC III):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN	Tc (MIN.)
COMMERCIAL	B	2.62	0.30	0.100	76	11.85

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp (INCH/HR) = 0.30

SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.100

SUBAREA RUNOFF (CFS) = 8.83

TOTAL AREA (ACRES) = 2.62 PEAK FLOW RATE (CFS) = 8.83

FLOW PROCESS FROM NODE 33.00 TO NODE 34.00 IS CODE = 61

>>>>COMPUTE STREET FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>(STANDARD CURB SECTION USED)<<<<<

=====

UPSTREAM ELEVATION(FEET) = 166.00 DOWNSTREAM ELEVATION(FEET) = 165.60
STREET LENGTH(FEET) = 485.00 CURB HEIGHT(INCHES) = 6.0
STREET HALFWIDTH(FEET) = 25.00

DISTANCE FROM CROWN TO CROSSFALL GRADEBREAK(FEET) = 10.00
INSIDE STREET CROSSFALL(DECIMAL) = 0.020
OUTSIDE STREET CROSSFALL(DECIMAL) = 0.020

SPECIFIED NUMBER OF HALFSTREETS CARRYING RUNOFF = 2
STREET PARKWAY CROSSFALL(DECIMAL) = 0.020
Manning's FRICTION FACTOR for Streetflow Section(curb-to-curb) = 0.0150
Manning's FRICTION FACTOR for Back-of-Walk Flow Section = 0.0200

**TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 15.10
STREET FLOWING FULL
STREETFLOW MODEL RESULTS USING ESTIMATED FLOW:
STREET FLOW DEPTH(FEET) = 0.63
HALFSTREET FLOOD WIDTH(FEET) = 31.53
AVERAGE FLOW VELOCITY(FEET/SEC.) = 1.09
PRODUCT OF DEPTH&VELOCITY(FT*FT/SEC.) = 0.69
STREET FLOW TRAVEL TIME(MIN.) = 7.39 Tc(MIN.) = 19.24
* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 2.859

SUBAREA LOSS RATE DATA(AMC III):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
COMMERCIAL	B	4.89	0.30	0.100	76

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.30
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.100
SUBAREA AREA(ACRES) = 4.89 SUBAREA RUNOFF(CFS) = 12.45
EFFECTIVE AREA(ACRES) = 7.51 AREA-AVERAGED Fm(INCH/HR) = 0.03
AREA-AVERAGED Fp(INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.10
TOTAL AREA(ACRES) = 7.5 PEAK FLOW RATE(CFS) = 19.12

END OF SUBAREA STREET FLOW HYDRAULICS:

DEPTH(FEET) = 0.67 HALFSTREET FLOOD WIDTH(FEET) = 33.42
FLOW VELOCITY(FEET/SEC.) = 1.18 DEPTH*VELOCITY(FT*FT/SEC.) = 0.79
*NOTE: INITIAL SUBAREA NOMOGRAPH WITH SUBAREA PARAMETERS,
AND L = 485.0 FT WITH ELEVATION-DROP = 0.4 FT, IS 14.4 CFS,
WHICH EXCEEDS THE TOP-OF-CURB STREET CAPACITY AT NODE 34.00
LONGEST FLOWPATH FROM NODE 32.00 TO NODE 34.00 = 785.00 FEET.

FLOW PROCESS FROM NODE 34.00 TO NODE 35.00 IS CODE = 51

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<<
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 165.60 DOWNSTREAM(FEET) = 165.40
CHANNEL LENGTH THRU SUBAREA(FEET) = 180.00 CHANNEL SLOPE = 0.0011
CHANNEL BASE(FEET) = 5.00 "Z" FACTOR = 0.020
MANNING'S FACTOR = 0.015 MAXIMUM DEPTH(FEET) = 2.00

CHANNEL FLOW THRU SUBAREA(CFS) = 19.12
FLOW VELOCITY(FEET/SEC.) = 2.96 FLOW DEPTH(FEET) = 1.28
TRAVEL TIME(MIN.) = 1.01 Tc(MIN.) = 20.25
LONGEST FLOWPATH FROM NODE 32.00 TO NODE 35.00 = 965.00 FEET.

FLOW PROCESS FROM NODE 35.00 TO NODE 35.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

=====

TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION(MIN.) = 20.25
RAINFALL INTENSITY(INCH/HR) = 2.78
AREA-AVERAGED Fm(INCH/HR) = 0.03
AREA-AVERAGED Fp(INCH/HR) = 0.30
AREA-AVERAGED Ap = 0.10
EFFECTIVE STREAM AREA(ACRES) = 7.51
TOTAL STREAM AREA(ACRES) = 7.51
PEAK FLOW RATE(CFS) AT CONFLUENCE = 19.12

FLOW PROCESS FROM NODE 35.00 TO NODE 35.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

=====

INITIAL SUBAREA FLOW-LENGTH(FEET) = 665.00
ELEVATION DATA: UPSTREAM(FEET) = 167.20 DOWNSTREAM(FEET) = 165.40

Tc = K*[(LENGTH** 3.00)/(ELEVATION CHANGE)]**0.20
SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) = 13.351
* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 3.524
SUBAREA Tc AND LOSS RATE DATA(AMC III):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN	Tc (MIN.)
COMMERCIAL	B	6.31	0.30	0.100	76	13.35

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.30
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.100
SUBAREA RUNOFF(CFS) = 19.84
TOTAL AREA(ACRES) = 6.31 PEAK FLOW RATE(CFS) = 19.84

FLOW PROCESS FROM NODE 35.00 TO NODE 35.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<

=====

TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
TIME OF CONCENTRATION(MIN.) = 13.35
RAINFALL INTENSITY(INCH/HR) = 3.52
AREA-AVERAGED Fm(INCH/HR) = 0.03
AREA-AVERAGED Fp(INCH/HR) = 0.30

AREA-AVERAGED $A_p = 0.10$
 EFFECTIVE STREAM AREA (ACRES) = 6.31
 TOTAL STREAM AREA (ACRES) = 6.31
 PEAK FLOW RATE (CFS) AT CONFLUENCE = 19.84

** CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp (Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	19.12	20.25	2.776	0.30 (0.03)	0.10	7.5	32.00
2	19.84	13.35	3.524	0.30 (0.03)	0.10	6.3	35.00

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
 CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp (Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	35.89	13.35	3.524	0.30 (0.03)	0.10	11.3	35.00
2	34.71	20.25	2.776	0.30 (0.03)	0.10	13.8	32.00

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE (CFS) = 35.89 Tc (MIN.) = 13.35
 EFFECTIVE AREA (ACRES) = 11.26 AREA-AVERAGED Fm (INCH/HR) = 0.03
 AREA-AVERAGED Fp (INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.10
 TOTAL AREA (ACRES) = 13.8
 LONGEST FLOWPATH FROM NODE 32.00 TO NODE 35.00 = 965.00 FEET.

FLOW PROCESS FROM NODE 35.00 TO NODE 31.00 IS CODE = 41

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
 >>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM (FEET) = 159.00 DOWNSTREAM (FEET) = 155.00
 FLOW LENGTH (FEET) = 50.00 MANNING'S N = 0.013
 ASSUME FULL-FLOWING PIPELINE
 PIPE-FLOW VELOCITY (FEET/SEC.) = 20.31
 PIPE FLOW VELOCITY = (TOTAL FLOW)/(PIPE CROSS SECTION AREA)
 GIVEN PIPE DIAMETER (INCH) = 18.00 NUMBER OF PIPES = 1
 PIPE-FLOW (CFS) = 35.89
 PIPE TRAVEL TIME (MIN.) = 0.04 Tc (MIN.) = 13.39
 LONGEST FLOWPATH FROM NODE 32.00 TO NODE 31.00 = 1015.00 FEET.

FLOW PROCESS FROM NODE 31.00 TO NODE 31.00 IS CODE = 11

>>>>CONFLUENCE MEMORY BANK # 2 WITH THE MAIN-STREAM MEMORY<<<<<

** MAIN STREAM CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp (Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	35.89	13.39	3.518	0.30 (0.03)	0.10	11.3	35.00
2	34.71	20.29	2.773	0.30 (0.03)	0.10	13.8	32.00

LONGEST FLOWPATH FROM NODE 32.00 TO NODE 31.00 = 1015.00 FEET.

** MEMORY BANK # 2 CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	54.60	7.68	4.837	0.30 (0.03)	0.10	11.5	28.00
2	53.64	10.96	3.946	0.30 (0.03)	0.10	13.8	14.00
3	53.86	11.86	3.773	0.30 (0.03)	0.10	14.5	21.00
4	53.86	11.87	3.770	0.30 (0.03)	0.10	14.6	17.00
5	52.71	14.53	3.357	0.30 (0.03)	0.10	16.1	24.00
6	51.56	15.29	3.260	0.30 (0.03)	0.10	16.3	18.00
7	48.94	16.90	3.080	0.30 (0.03)	0.10	16.4	12.00
8	43.85	20.42	2.763	0.30 (0.03)	0.10	16.5	10.00

LONGEST FLOWPATH FROM NODE 10.00 TO NODE 31.00 = 2070.00 FEET.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	82.98	7.68	4.837	0.30 (0.03)	0.10	17.9	28.00
2	86.62	10.96	3.946	0.30 (0.03)	0.10	23.0	14.00
3	87.95	11.86	3.773	0.30 (0.03)	0.10	24.5	21.00
4	87.96	11.87	3.770	0.30 (0.03)	0.10	24.5	17.00
5	89.09	13.39	3.518	0.30 (0.03)	0.10	26.7	35.00
6	88.40	14.53	3.357	0.30 (0.03)	0.10	27.8	24.00
7	87.13	15.29	3.260	0.30 (0.03)	0.10	28.2	18.00
8	84.23	16.90	3.080	0.30 (0.03)	0.10	29.0	12.00
9	78.74	20.29	2.773	0.30 (0.03)	0.10	30.3	32.00
10	78.44	20.42	2.763	0.30 (0.03)	0.10	30.3	10.00
TOTAL AREA (ACRES) =			30.3				

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE (CFS) = 89.09 Tc (MIN.) = 13.392
EFFECTIVE AREA (ACRES) = 26.71 AREA-AVERAGED Fm (INCH/HR) = 0.03
AREA-AVERAGED Fp (INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.10
TOTAL AREA (ACRES) = 30.3

LONGEST FLOWPATH FROM NODE 10.00 TO NODE 31.00 = 2070.00 FEET.

FLOW PROCESS FROM NODE 31.00 TO NODE 36.00 IS CODE = 41

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM (FEET) = 155.00 DOWNSTREAM (FEET) = 153.80
FLOW LENGTH (FEET) = 265.00 MANNING'S N = 0.013
ASSUME FULL-FLOWING PIPELINE
PIPE-FLOW VELOCITY (FEET/SEC.) = 9.26
PIPE FLOW VELOCITY = (TOTAL FLOW)/(PIPE CROSS SECTION AREA)
GIVEN PIPE DIAMETER (INCH) = 42.00 NUMBER OF PIPES = 1
PIPE-FLOW (CFS) = 89.09
PIPE TRAVEL TIME (MIN.) = 0.48 Tc (MIN.) = 13.87
LONGEST FLOWPATH FROM NODE 10.00 TO NODE 36.00 = 2335.00 FEET.

FLOW PROCESS FROM NODE 36.00 TO NODE 36.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION(MIN.) = 13.87
RAINFALL INTENSITY(INCH/HR) = 3.45
AREA-AVERAGED Fm(INCH/HR) = 0.03
AREA-AVERAGED Fp(INCH/HR) = 0.30
AREA-AVERAGED Ap = 0.10
EFFECTIVE STREAM AREA(ACRES) = 26.71
TOTAL STREAM AREA(ACRES) = 30.32
PEAK FLOW RATE(CFS) AT CONFLUENCE = 89.09

FLOW PROCESS FROM NODE 37.00 TO NODE 38.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

INITIAL SUBAREA FLOW-LENGTH(FEET) = 325.00
ELEVATION DATA: UPSTREAM(FEET) = 169.50 DOWNSTREAM(FEET) = 168.00

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$
SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) = 9.011
* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 4.415
SUBAREA Tc AND LOSS RATE DATA(AMC III):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN	Tc (MIN.)
COMMERCIAL	B	1.65	0.30	0.100	76	9.01

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.30
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.100
SUBAREA RUNOFF(CFS) = 6.51
TOTAL AREA(ACRES) = 1.65 PEAK FLOW RATE(CFS) = 6.51

FLOW PROCESS FROM NODE 38.00 TO NODE 36.00 IS CODE = 41

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

ELEVATION DATA: UPSTREAM(FEET) = 159.00 DOWNSTREAM(FEET) = 153.80
FLOW LENGTH(FEET) = 70.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 18.0 INCH PIPE IS 5.9 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 12.87
GIVEN PIPE DIAMETER(INCH) = 18.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 6.51
PIPE TRAVEL TIME(MIN.) = 0.09 Tc(MIN.) = 9.10
LONGEST FLOWPATH FROM NODE 37.00 TO NODE 36.00 = 395.00 FEET.

FLOW PROCESS FROM NODE 36.00 TO NODE 36.00 IS CODE = 1

>>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<<
 >>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<<

=====

TOTAL NUMBER OF STREAMS = 2
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
 TIME OF CONCENTRATION(MIN.) = 9.10
 RAINFALL INTENSITY(INCH/HR) = 4.39
 AREA-AVERAGED Fm(INCH/HR) = 0.03
 AREA-AVERAGED Fp(INCH/HR) = 0.30
 AREA-AVERAGED Ap = 0.10
 EFFECTIVE STREAM AREA(ACRES) = 1.65
 TOTAL STREAM AREA(ACRES) = 1.65
 PEAK FLOW RATE(CFS) AT CONFLUENCE = 6.51

** CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	82.98	8.20	4.662	0.30(0.03)	0.10	17.9	28.00
1	86.62	11.45	3.848	0.30(0.03)	0.10	23.0	14.00
1	87.95	12.34	3.687	0.30(0.03)	0.10	24.5	21.00
1	87.96	12.35	3.685	0.30(0.03)	0.10	24.5	17.00
1	89.09	13.87	3.448	0.30(0.03)	0.10	26.7	35.00
1	88.40	15.01	3.295	0.30(0.03)	0.10	27.8	24.00
1	87.13	15.78	3.202	0.30(0.03)	0.10	28.2	18.00
1	84.23	17.40	3.028	0.30(0.03)	0.10	29.0	12.00
1	78.74	20.83	2.731	0.30(0.03)	0.10	30.3	32.00
1	78.44	20.96	2.722	0.30(0.03)	0.10	30.3	10.00
2	6.51	9.10	4.390	0.30(0.03)	0.10	1.6	37.00

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
 CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	89.20	8.20	4.662	0.30(0.03)	0.10	19.4	28.00
2	90.50	9.10	4.390	0.30(0.03)	0.10	21.0	37.00
3	92.32	11.45	3.848	0.30(0.03)	0.10	24.6	14.00
4	93.41	12.34	3.687	0.30(0.03)	0.10	26.2	21.00
5	93.42	12.35	3.685	0.30(0.03)	0.10	26.2	17.00
6	94.19	13.87	3.448	0.30(0.03)	0.10	28.4	35.00
7	93.28	15.01	3.295	0.30(0.03)	0.10	29.5	24.00
8	91.87	15.78	3.202	0.30(0.03)	0.10	29.9	18.00
9	88.71	17.40	3.028	0.30(0.03)	0.10	30.6	12.00
10	82.77	20.83	2.731	0.30(0.03)	0.10	32.0	32.00
11	82.46	20.96	2.722	0.30(0.03)	0.10	32.0	10.00

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE(CFS) = 94.19 Tc(MIN.) = 13.87
 EFFECTIVE AREA(ACRES) = 28.36 AREA-AVERAGED Fm(INCH/HR) = 0.03
 AREA-AVERAGED Fp(INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.10
 TOTAL AREA(ACRES) = 32.0
 LONGEST FLOWPATH FROM NODE 10.00 TO NODE 36.00 = 2335.00 FEET.

FLOW PROCESS FROM NODE 36.00 TO NODE 39.00 IS CODE = 41

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

ELEVATION DATA: UPSTREAM(FEET) = 153.80 DOWNSTREAM(FEET) = 152.80
FLOW LENGTH(FEET) = 230.00 MANNING'S N = 0.013
ASSUME FULL-FLOWING PIPELINE
PIPE-FLOW VELOCITY(FEET/SEC.) = 9.79
PIPE FLOW VELOCITY = (TOTAL FLOW)/(PIPE CROSS SECTION AREA)
GIVEN PIPE DIAMETER(INCH) = 42.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 94.19
PIPE TRAVEL TIME(MIN.) = 0.39 Tc(MIN.) = 14.26
LONGEST FLOWPATH FROM NODE 10.00 TO NODE 39.00 = 2565.00 FEET.

FLOW PROCESS FROM NODE 39.00 TO NODE 39.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION(MIN.) = 14.26
RAINFALL INTENSITY(INCH/HR) = 3.39
AREA-AVERAGED Fm(INCH/HR) = 0.03
AREA-AVERAGED Fp(INCH/HR) = 0.30
AREA-AVERAGED Ap = 0.10
EFFECTIVE STREAM AREA(ACRES) = 28.36
TOTAL STREAM AREA(ACRES) = 31.97
PEAK FLOW RATE(CFS) AT CONFLUENCE = 94.19

FLOW PROCESS FROM NODE 40.00 TO NODE 39.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

INITIAL SUBAREA FLOW-LENGTH(FEET) = 320.00
ELEVATION DATA: UPSTREAM(FEET) = 167.40 DOWNSTREAM(FEET) = 167.20

Tc = K*[(LENGTH** 3.00)/(ELEVATION CHANGE)]**0.20

SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) = 13.359

* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 3.523

SUBAREA Tc AND LOSS RATE DATA(AMC III):

DEVELOPMENT TYPE/ SCS SOIL AREA Fp Ap SCS Tc
LAND USE GROUP (ACRES) (INCH/HR) (DECIMAL) CN (MIN.)
COMMERCIAL B 0.48 0.30 0.100 76 13.36

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.30

SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.100

SUBAREA RUNOFF(CFS) = 1.51

TOTAL AREA(ACRES) = 0.48 PEAK FLOW RATE(CFS) = 1.51

FLOW PROCESS FROM NODE 39.00 TO NODE 39.00 IS CODE = 1

>>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<<
>>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<<

TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
TIME OF CONCENTRATION(MIN.) = 13.36
RAINFALL INTENSITY(INCH/HR) = 3.52
AREA-AVERAGED Fm(INCH/HR) = 0.03
AREA-AVERAGED Fp(INCH/HR) = 0.30
AREA-AVERAGED Ap = 0.10
EFFECTIVE STREAM AREA(ACRES) = 0.48
TOTAL STREAM AREA(ACRES) = 0.48
PEAK FLOW RATE(CFS) AT CONFLUENCE = 1.51

** CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	89.20	8.61	4.532	0.30(0.03)	0.10	19.4	28.00
1	90.50	9.51	4.281	0.30(0.03)	0.10	21.0	37.00
1	92.32	11.85	3.773	0.30(0.03)	0.10	24.6	14.00
1	93.41	12.73	3.621	0.30(0.03)	0.10	26.2	21.00
1	93.42	12.75	3.619	0.30(0.03)	0.10	26.2	17.00
1	94.19	14.26	3.394	0.30(0.03)	0.10	28.4	35.00
1	93.28	15.41	3.247	0.30(0.03)	0.10	29.5	24.00
1	91.87	16.18	3.156	0.30(0.03)	0.10	29.9	18.00
1	88.71	17.82	2.987	0.30(0.03)	0.10	30.6	12.00
1	82.77	21.28	2.698	0.30(0.03)	0.10	32.0	32.00
1	82.46	21.41	2.689	0.30(0.03)	0.10	32.0	10.00
2	1.51	13.36	3.523	0.30(0.03)	0.10	0.5	40.00

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	90.46	8.61	4.532	0.30(0.03)	0.10	19.7	28.00
2	91.81	9.51	4.281	0.30(0.03)	0.10	21.3	37.00
3	93.75	11.85	3.773	0.30(0.03)	0.10	25.1	14.00
4	94.89	12.73	3.621	0.30(0.03)	0.10	26.6	21.00
5	94.90	12.75	3.619	0.30(0.03)	0.10	26.6	17.00
6	95.24	13.36	3.523	0.30(0.03)	0.10	27.5	40.00
7	95.65	14.26	3.394	0.30(0.03)	0.10	28.8	35.00
8	94.67	15.41	3.247	0.30(0.03)	0.10	29.9	24.00
9	93.22	16.18	3.156	0.30(0.03)	0.10	30.4	18.00
10	89.99	17.82	2.987	0.30(0.03)	0.10	31.1	12.00
11	83.93	21.28	2.698	0.30(0.03)	0.10	32.4	32.00
12	83.61	21.41	2.689	0.30(0.03)	0.10	32.5	10.00

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE(CFS) = 95.65 Tc(MIN.) = 14.26
EFFECTIVE AREA(ACRES) = 28.84 AREA-AVERAGED Fm(INCH/HR) = 0.03

AREA-AVERAGED F_p (INCH/HR) = 0.30 AREA-AVERAGED A_p = 0.10
 TOTAL AREA (ACRES) = 32.5
 LONGEST FLOWPATH FROM NODE 10.00 TO NODE 39.00 = 2565.00 FEET.

FLOW PROCESS FROM NODE 39.00 TO NODE 41.00 IS CODE = 41

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
 >>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 152.80 DOWNSTREAM(FEET) = 152.00
 FLOW LENGTH(FEET) = 210.00 MANNING'S N = 0.013
 ASSUME FULL-FLOWING PIPELINE
 PIPE-FLOW VELOCITY(FEET/SEC.) = 7.61
 PIPE FLOW VELOCITY = (TOTAL FLOW)/(PIPE CROSS SECTION AREA)
 GIVEN PIPE DIAMETER(INCH) = 48.00 NUMBER OF PIPES = 1
 PIPE-FLOW(CFS) = 95.65
 PIPE TRAVEL TIME(MIN.) = 0.46 T_c (MIN.) = 14.72
 LONGEST FLOWPATH FROM NODE 10.00 TO NODE 41.00 = 2775.00 FEET.

FLOW PROCESS FROM NODE 41.00 TO NODE 41.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

=====

TOTAL NUMBER OF STREAMS = 2
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
 TIME OF CONCENTRATION(MIN.) = 14.72
 RAINFALL INTENSITY(INCH/HR) = 3.33
 AREA-AVERAGED F_m (INCH/HR) = 0.03
 AREA-AVERAGED F_p (INCH/HR) = 0.30
 AREA-AVERAGED A_p = 0.10
 EFFECTIVE STREAM AREA(ACRES) = 28.84
 TOTAL STREAM AREA(ACRES) = 32.45
 PEAK FLOW RATE(CFS) AT CONFLUENCE = 95.65

FLOW PROCESS FROM NODE 42.00 TO NODE 41.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
 >>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

=====

INITIAL SUBAREA FLOW-LENGTH(FEET) = 230.00
 ELEVATION DATA: UPSTREAM(FEET) = 166.00 DOWNSTREAM(FEET) = 163.20

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$
 SUBAREA ANALYSIS USED MINIMUM T_c (MIN.) = 6.464
 * 100 YEAR RAINFALL INTENSITY(INCH/HR) = 5.341

SUBAREA T_c AND LOSS RATE DATA(AMC III):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	F_p (INCH/HR)	A_p (DECIMAL)	SCS CN	T_c (MIN.)
COMMERCIAL	B	0.66	0.30	0.100	76	6.46

SUBAREA AVERAGE PERVIOUS LOSS RATE, F_p (INCH/HR) = 0.30

SUBAREA AVERAGE PERVIOUS AREA FRACTION, A_p = 0.100

SUBAREA RUNOFF(CFS) = 3.15
 TOTAL AREA(ACRES) = 0.66 PEAK FLOW RATE(CFS) = 3.15

FLOW PROCESS FROM NODE 41.00 TO NODE 41.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
 >>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<

=====

TOTAL NUMBER OF STREAMS = 2
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
 TIME OF CONCENTRATION(MIN.) = 6.46
 RAINFALL INTENSITY(INCH/HR) = 5.34
 AREA-AVERAGED Fm(INCH/HR) = 0.03
 AREA-AVERAGED Fp(INCH/HR) = 0.30
 AREA-AVERAGED Ap = 0.10
 EFFECTIVE STREAM AREA(ACRES) = 0.66
 TOTAL STREAM AREA(ACRES) = 0.66
 PEAK FLOW RATE(CFS) AT CONFLUENCE = 3.15

** CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	90.46	9.09	4.392	0.30(0.03)	0.10	19.7	28.00
1	91.81	9.99	4.162	0.30(0.03)	0.10	21.3	37.00
1	93.75	12.32	3.690	0.30(0.03)	0.10	25.1	14.00
1	94.89	13.20	3.548	0.30(0.03)	0.10	26.6	21.00
1	94.90	13.21	3.546	0.30(0.03)	0.10	26.6	17.00
1	95.24	13.82	3.455	0.30(0.03)	0.10	27.5	40.00
1	95.65	14.72	3.333	0.30(0.03)	0.10	28.8	35.00
1	94.67	15.87	3.192	0.30(0.03)	0.10	29.9	24.00
1	93.22	16.66	3.105	0.30(0.03)	0.10	30.4	18.00
1	89.99	18.30	2.941	0.30(0.03)	0.10	31.1	12.00
1	83.93	21.73	2.666	0.30(0.03)	0.10	32.4	32.00
1	83.61	21.85	2.658	0.30(0.03)	0.10	32.5	10.00
2	3.15	6.46	5.341	0.30(0.03)	0.10	0.7	42.00

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
 CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	81.43	6.46	5.341	0.30(0.03)	0.10	14.7	42.00
2	93.05	9.09	4.392	0.30(0.03)	0.10	20.4	28.00
3	94.26	9.99	4.162	0.30(0.03)	0.10	22.0	37.00
4	95.93	12.32	3.690	0.30(0.03)	0.10	25.7	14.00
5	96.98	13.20	3.548	0.30(0.03)	0.10	27.3	21.00
6	96.99	13.21	3.546	0.30(0.03)	0.10	27.3	17.00
7	97.28	13.82	3.455	0.30(0.03)	0.10	28.2	40.00
8	97.61	14.72	3.333	0.30(0.03)	0.10	29.5	35.00
9	96.54	15.87	3.192	0.30(0.03)	0.10	30.6	24.00
10	95.04	16.66	3.105	0.30(0.03)	0.10	31.0	18.00
11	91.72	18.30	2.941	0.30(0.03)	0.10	31.8	12.00

12	85.49	21.73	2.666	0.30 (0.03)	0.10	33.1	32.00
13	85.17	21.85	2.658	0.30 (0.03)	0.10	33.1	10.00

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE (CFS) = 97.61 Tc (MIN.) = 14.72
 EFFECTIVE AREA (ACRES) = 29.50 AREA-AVERAGED Fm (INCH/HR) = 0.03
 AREA-AVERAGED Fp (INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.10
 TOTAL AREA (ACRES) = 33.1
 LONGEST FLOWPATH FROM NODE 10.00 TO NODE 41.00 = 2775.00 FEET.

FLOW PROCESS FROM NODE 41.00 TO NODE 43.00 IS CODE = 41

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
 >>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM (FEET) = 152.00 DOWNSTREAM (FEET) = 151.80
 FLOW LENGTH (FEET) = 30.00 MANNING'S N = 0.013
 DEPTH OF FLOW IN 48.0 INCH PIPE IS 34.1 INCHES
 PIPE-FLOW VELOCITY (FEET/SEC.) = 10.22
 GIVEN PIPE DIAMETER (INCH) = 48.00 NUMBER OF PIPES = 1
 PIPE-FLOW (CFS) = 97.61
 PIPE TRAVEL TIME (MIN.) = 0.05 Tc (MIN.) = 14.77
 LONGEST FLOWPATH FROM NODE 10.00 TO NODE 43.00 = 2805.00 FEET.

FLOW PROCESS FROM NODE 43.00 TO NODE 43.00 IS CODE = 10

>>>>MAIN-STREAM MEMORY COPIED ONTO MEMORY BANK # 1 <<<<<

FLOW PROCESS FROM NODE 44.00 TO NODE 45.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
 >>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

=====

INITIAL SUBAREA FLOW-LENGTH (FEET) = 350.00
 ELEVATION DATA: UPSTREAM (FEET) = 164.00 DOWNSTREAM (FEET) = 162.00

Tc = K * [(LENGTH** 3.00) / (ELEVATION CHANGE)] ** 0.20
 SUBAREA ANALYSIS USED MINIMUM Tc (MIN.) = 8.894
 * 100 YEAR RAINFALL INTENSITY (INCH/HR) = 4.448
 SUBAREA Tc AND LOSS RATE DATA (AMC III):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN	Tc (MIN.)
COMMERCIAL	B	1.55	0.30	0.100	76	8.89

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp (INCH/HR) = 0.30
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.100
 SUBAREA RUNOFF (CFS) = 6.16
 TOTAL AREA (ACRES) = 1.55 PEAK FLOW RATE (CFS) = 6.16

FLOW PROCESS FROM NODE 45.00 TO NODE 46.00 IS CODE = 51

```

-----
>>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<<
>>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 162.00 DOWNSTREAM(FEET) = 161.80
CHANNEL LENGTH THRU SUBAREA(FEET) = 165.00 CHANNEL SLOPE = 0.0012
CHANNEL BASE(FEET) = 3.00 "Z" FACTOR = 0.040
MANNING'S FACTOR = 0.015 MAXIMUM DEPTH(FEET) = 1.00
CHANNEL FLOW THRU SUBAREA(CFS) = 6.16
FLOW VELOCITY(FEET/SEC.) = 2.33 FLOW DEPTH(FEET) = 0.87
TRAVEL TIME(MIN.) = 1.18 Tc(MIN.) = 10.08
LONGEST FLOWPATH FROM NODE 44.00 TO NODE 46.00 = 515.00 FEET.

*****
FLOW PROCESS FROM NODE 46.00 TO NODE 46.00 IS CODE = 1
-----
>>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
=====
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION(MIN.) = 10.08
RAINFALL INTENSITY(INCH/HR) = 4.14
AREA-AVERAGED Fm(INCH/HR) = 0.03
AREA-AVERAGED Fp(INCH/HR) = 0.30
AREA-AVERAGED Ap = 0.10
EFFECTIVE STREAM AREA(ACRES) = 1.55
TOTAL STREAM AREA(ACRES) = 1.55
PEAK FLOW RATE(CFS) AT CONFLUENCE = 6.16

*****
FLOW PROCESS FROM NODE 47.00 TO NODE 46.00 IS CODE = 21
-----
>>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<
=====
INITIAL SUBAREA FLOW-LENGTH(FEET) = 285.00
ELEVATION DATA: UPSTREAM(FEET) = 165.00 DOWNSTREAM(FEET) = 161.80

Tc = K*[(LENGTH** 3.00)/(ELEVATION CHANGE)]**0.20
SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) = 7.157
* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 5.038
SUBAREA Tc AND LOSS RATE DATA(AMC III):
DEVELOPMENT TYPE/ SCS SOIL AREA Fp Ap SCS Tc
LAND USE GROUP (ACRES) (INCH/HR) (DECIMAL) CN (MIN.)
COMMERCIAL B 1.23 0.30 0.100 76 7.16
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.30
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.100
SUBAREA RUNOFF(CFS) = 5.54
TOTAL AREA(ACRES) = 1.23 PEAK FLOW RATE(CFS) = 5.54

*****
FLOW PROCESS FROM NODE 46.00 TO NODE 46.00 IS CODE = 1
-----
>>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

```

>>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<

=====

TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
TIME OF CONCENTRATION(MIN.) = 7.16
RAINFALL INTENSITY(INCH/HR) = 5.04
AREA-AVERAGED Fm(INCH/HR) = 0.03
AREA-AVERAGED Fp(INCH/HR) = 0.30
AREA-AVERAGED Ap = 0.10
EFFECTIVE STREAM AREA(ACRES) = 1.23
TOTAL STREAM AREA(ACRES) = 1.23
PEAK FLOW RATE(CFS) AT CONFLUENCE = 5.54

** CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	6.16	10.08	4.141	0.30(0.03)	0.10	1.5	44.00
2	5.54	7.16	5.038	0.30(0.03)	0.10	1.2	47.00

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	10.88	7.16	5.038	0.30(0.03)	0.10	2.3	47.00
2	10.71	10.08	4.141	0.30(0.03)	0.10	2.8	44.00

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE(CFS) = 10.88 Tc(MIN.) = 7.16
EFFECTIVE AREA(ACRES) = 2.33 AREA-AVERAGED Fm(INCH/HR) = 0.03
AREA-AVERAGED Fp(INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.10
TOTAL AREA(ACRES) = 2.8
LONGEST FLOWPATH FROM NODE 44.00 TO NODE 46.00 = 515.00 FEET.

FLOW PROCESS FROM NODE 46.00 TO NODE 43.00 IS CODE = 41

>>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 157.00 DOWNSTREAM(FEET) = 151.80
FLOW LENGTH(FEET) = 115.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 18.0 INCH PIPE IS 9.0 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 12.32
GIVEN PIPE DIAMETER(INCH) = 18.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 10.88
PIPE TRAVEL TIME(MIN.) = 0.16 Tc(MIN.) = 7.31
LONGEST FLOWPATH FROM NODE 44.00 TO NODE 43.00 = 630.00 FEET.

FLOW PROCESS FROM NODE 43.00 TO NODE 43.00 IS CODE = 11

>>>>>CONFLUENCE MEMORY BANK # 1 WITH THE MAIN-STREAM MEMORY<<<<<

=====

** MAIN STREAM CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	10.88	7.31	4.976	0.30 (0.03)	0.10	2.3	47.00
2	10.71	10.23	4.105	0.30 (0.03)	0.10	2.8	44.00
LONGEST FLOWPATH FROM NODE			44.00	TO NODE	43.00 =	630.00 FEET.	

** MEMORY BANK # 1 CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	81.43	6.51	5.317	0.30 (0.03)	0.10	14.7	42.00
2	93.05	9.14	4.378	0.30 (0.03)	0.10	20.4	28.00
3	94.26	10.04	4.150	0.30 (0.03)	0.10	22.0	37.00
4	95.93	12.37	3.682	0.30 (0.03)	0.10	25.7	14.00
5	96.98	13.25	3.540	0.30 (0.03)	0.10	27.3	21.00
6	96.99	13.26	3.539	0.30 (0.03)	0.10	27.3	17.00
7	97.28	13.87	3.448	0.30 (0.03)	0.10	28.2	40.00
8	97.61	14.77	3.326	0.30 (0.03)	0.10	29.5	35.00
9	96.54	15.92	3.186	0.30 (0.03)	0.10	30.6	24.00
10	95.04	16.71	3.100	0.30 (0.03)	0.10	31.0	18.00
11	91.72	18.35	2.937	0.30 (0.03)	0.10	31.8	12.00
12	85.49	21.78	2.663	0.30 (0.03)	0.10	33.1	32.00
13	85.17	21.90	2.654	0.30 (0.03)	0.10	33.1	10.00
LONGEST FLOWPATH FROM NODE			10.00	TO NODE	43.00 =	2805.00 FEET.	

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	91.79	6.51	5.317	0.30 (0.03)	0.10	16.8	42.00
2	95.84	7.31	4.976	0.30 (0.03)	0.10	18.8	47.00
3	103.82	9.14	4.378	0.30 (0.03)	0.10	23.0	28.00
4	104.99	10.04	4.150	0.30 (0.03)	0.10	24.7	37.00
5	105.12	10.23	4.105	0.30 (0.03)	0.10	25.1	44.00
6	105.53	12.37	3.682	0.30 (0.03)	0.10	28.5	14.00
7	106.21	13.25	3.540	0.30 (0.03)	0.10	30.1	21.00
8	106.21	13.26	3.539	0.30 (0.03)	0.10	30.1	17.00
9	106.26	13.87	3.448	0.30 (0.03)	0.10	31.0	40.00
10	106.27	14.77	3.326	0.30 (0.03)	0.10	32.3	35.00
11	104.84	15.92	3.186	0.30 (0.03)	0.10	33.4	24.00
12	103.11	16.71	3.100	0.30 (0.03)	0.10	33.8	18.00
13	99.36	18.35	2.937	0.30 (0.03)	0.10	34.6	12.00
14	92.42	21.78	2.663	0.30 (0.03)	0.10	35.9	32.00
15	92.07	21.90	2.654	0.30 (0.03)	0.10	35.9	10.00
TOTAL AREA (ACRES) =			35.9				

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE (CFS) = 106.27 Tc (MIN.) = 14.769
EFFECTIVE AREA (ACRES) = 32.28 AREA-AVERAGED Fm (INCH/HR) = 0.03
AREA-AVERAGED Fp (INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.10
TOTAL AREA (ACRES) = 35.9
LONGEST FLOWPATH FROM NODE 10.00 TO NODE 43.00 = 2805.00 FEET.

=====

END OF STUDY SUMMARY:

TOTAL AREA (ACRES) = 35.9 TC (MIN.) = 14.77
 EFFECTIVE AREA (ACRES) = 32.28 AREA-AVERAGED Fm (INCH/HR) = 0.03
 AREA-AVERAGED Fp (INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.100
 PEAK FLOW RATE (CFS) = 106.27

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp (Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	91.79	6.51	5.317	0.30 (0.03)	0.10	16.8	42.00
2	95.84	7.31	4.976	0.30 (0.03)	0.10	18.8	47.00
3	103.82	9.14	4.378	0.30 (0.03)	0.10	23.0	28.00
4	104.99	10.04	4.150	0.30 (0.03)	0.10	24.7	37.00
5	105.12	10.23	4.105	0.30 (0.03)	0.10	25.1	44.00
6	105.53	12.37	3.682	0.30 (0.03)	0.10	28.5	14.00
7	106.21	13.25	3.540	0.30 (0.03)	0.10	30.1	21.00
8	106.21	13.26	3.539	0.30 (0.03)	0.10	30.1	17.00
9	106.26	13.87	3.448	0.30 (0.03)	0.10	31.0	40.00
10	106.27	14.77	3.326	0.30 (0.03)	0.10	32.3	35.00
11	104.84	15.92	3.186	0.30 (0.03)	0.10	33.4	24.00
12	103.11	16.71	3.100	0.30 (0.03)	0.10	33.8	18.00
13	99.36	18.35	2.937	0.30 (0.03)	0.10	34.6	12.00
14	92.42	21.78	2.663	0.30 (0.03)	0.10	35.9	32.00
15	92.07	21.90	2.654	0.30 (0.03)	0.10	35.9	10.00

=====
 END OF RATIONAL METHOD ANALYSIS
 =====

APPENDIX 3

PROPOSED HYDROLOGY STUDIES

RATIONAL METHOD HYDROLOGY COMPUTER PROGRAM PACKAGE
(Reference: 1986 ORANGE COUNTY HYDROLOGY CRITERION)
(c) Copyright 1983-2013 Advanced Engineering Software (aes)
Ver. 20.0 Release Date: 06/01/2013 License ID 1654

Analysis prepared by:

FUSCOE ENGINEERING, Inc
16795 Von Karman Ave., #100
Irvine, CA 92606
949-474-1960

***** DESCRIPTION OF STUDY *****

* FREEMAN *
* PROPOSED CONDITION *
* 2 YEAR ANALYSIS *

FILE NAME: FRPR2.DAT
TIME/DATE OF STUDY: 09:16 05/29/2017

=====

USER SPECIFIED HYDROLOGY AND HYDRAULIC MODEL INFORMATION:

=====

--*TIME-OF-CONCENTRATION MODEL*--

USER SPECIFIED STORM EVENT (YEAR) = 2.00
SPECIFIED MINIMUM PIPE SIZE (INCH) = 12.00
SPECIFIED PERCENT OF GRADIENTS (DECIMAL) TO USE FOR FRICTION SLOPE = 0.95
DATA BANK RAINFALL USED
ANTECEDENT MOISTURE CONDITION (AMC) I ASSUMED FOR RATIONAL METHOD

USER-DEFINED STREET-SECTIONS FOR COUPLED PIPEFLOW AND STREETFLOW MODEL

NO.	HALF- WIDTH (FT)	CROWN TO CROSSFALL (FT)	STREET-CROSSFALL: IN- / OUT- / PARK- SIDE / SIDE / WAY	CURB HEIGHT (FT)	GUTTER-GEOMETRIES: WIDTH LIP HIKE (FT) (FT) (FT)	MANNING FACTOR (n)
1	24.0	19.0	0.020/0.020/0.020	0.67	2.00 0.0313 0.167	0.0150
2	18.0	13.0	0.020/0.020/0.020	0.50	1.50 0.0313 0.125	0.0150

GLOBAL STREET FLOW-DEPTH CONSTRAINTS:
1. Relative Flow-Depth = 0.50 FEET
as (Maximum Allowable Street Flow Depth) - (Top-of-Curb)
2. (Depth)*(Velocity) Constraint = 6.0 (FT*FT/S)
*SIZE PIPE WITH A FLOW CAPACITY GREATER THAN
OR EQUAL TO THE UPSTREAM TRIBUTARY PIPE.*
*USER-SPECIFIED MINIMUM TOPOGRAPHIC SLOPE ADJUSTMENT NOT SELECTED

FLOW PROCESS FROM NODE 10.00 TO NODE 32.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

INITIAL SUBAREA FLOW-LENGTH (FEET) = 265.00
ELEVATION DATA: UPSTREAM (FEET) = 167.50 DOWNSTREAM (FEET) = 166.30

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$
SUBAREA ANALYSIS USED MINIMUM T_c (MIN.) = 8.337
* 2 YEAR RAINFALL INTENSITY (INCH/HR) = 1.688

SUBAREA T_c AND LOSS RATE DATA (AMC I):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	F_p (INCH/HR)	A_p (DECIMAL)	SCS CN	T_c (MIN.)
COMMERCIAL	B	0.15	0.30	0.100	36	8.34

SUBAREA AVERAGE PERVIOUS LOSS RATE, F_p (INCH/HR) = 0.30

SUBAREA AVERAGE PERVIOUS AREA FRACTION, A_p = 0.100

SUBAREA RUNOFF (CFS) = 0.22

TOTAL AREA (ACRES) = 0.15 PEAK FLOW RATE (CFS) = 0.22

FLOW PROCESS FROM NODE 32.00 TO NODE 11.00 IS CODE = 61

>>>>COMPUTE STREET FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>(STANDARD CURB SECTION USED)<<<<<

UPSTREAM ELEVATION (FEET) = 166.30 DOWNSTREAM ELEVATION (FEET) = 165.10
STREET LENGTH (FEET) = 210.00 CURB HEIGHT (INCHES) = 6.0
STREET HALFWIDTH (FEET) = 25.00

DISTANCE FROM CROWN TO CROSSFALL GRADEBREAK (FEET) = 10.00

INSIDE STREET CROSSFALL (DECIMAL) = 0.020

OUTSIDE STREET CROSSFALL (DECIMAL) = 0.020

SPECIFIED NUMBER OF HALFSTREETS CARRYING RUNOFF = 1

STREET PARKWAY CROSSFALL (DECIMAL) = 0.020

Manning's FRICTION FACTOR for Streetflow Section (curb-to-curb) = 0.0150

Manning's FRICTION FACTOR for Back-of-Walk Flow Section = 0.0200

**TRAVEL TIME COMPUTED USING ESTIMATED FLOW (CFS) = 0.30

STREETFLOW MODEL RESULTS USING ESTIMATED FLOW:

STREET FLOW DEPTH (FEET) = 0.20

HALFSTREET FLOOD WIDTH (FEET) = 3.58

AVERAGE FLOW VELOCITY (FEET/SEC.) = 1.21

PRODUCT OF DEPTH&VELOCITY (FT*FT/SEC.) = 0.24

STREET FLOW TRAVEL TIME (MIN.) = 2.88 T_c (MIN.) = 11.22

* 2 YEAR RAINFALL INTENSITY (INCH/HR) = 1.423

SUBAREA LOSS RATE DATA (AMC I):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	F_p (INCH/HR)	A_p (DECIMAL)	SCS CN
COMMERCIAL	B	0.12	0.30	0.100	36

SUBAREA AVERAGE PERVIOUS LOSS RATE, F_p (INCH/HR) = 0.30

SUBAREA AVERAGE PERVIOUS AREA FRACTION, A_p = 0.100

SUBAREA AREA (ACRES) = 0.12 SUBAREA RUNOFF (CFS) = 0.15

EFFECTIVE AREA (ACRES) = 0.27 AREA-AVERAGED Fm (INCH/HR) = 0.03
AREA-AVERAGED Fp (INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.10
TOTAL AREA (ACRES) = 0.3 PEAK FLOW RATE (CFS) = 0.34

END OF SUBAREA STREET FLOW HYDRAULICS:

DEPTH (FEET) = 0.21 HALFSTREET FLOOD WIDTH (FEET) = 4.00
FLOW VELOCITY (FEET/SEC.) = 1.22 DEPTH*VELOCITY (FT*FT/SEC.) = 0.25
LONGEST FLOWPATH FROM NODE 10.00 TO NODE 11.00 = 475.00 FEET.

FLOW PROCESS FROM NODE 11.00 TO NODE 11.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

=====

TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION (MIN.) = 11.22
RAINFALL INTENSITY (INCH/HR) = 1.42
AREA-AVERAGED Fm (INCH/HR) = 0.03
AREA-AVERAGED Fp (INCH/HR) = 0.30
AREA-AVERAGED Ap = 0.10
EFFECTIVE STREAM AREA (ACRES) = 0.27
TOTAL STREAM AREA (ACRES) = 0.27
PEAK FLOW RATE (CFS) AT CONFLUENCE = 0.34

FLOW PROCESS FROM NODE 12.00 TO NODE 11.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<

>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

=====

INITIAL SUBAREA FLOW-LENGTH (FEET) = 210.00
ELEVATION DATA: UPSTREAM (FEET) = 166.00 DOWNSTREAM (FEET) = 165.10

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$

SUBAREA ANALYSIS USED MINIMUM Tc (MIN.) = 7.680

* 2 YEAR RAINFALL INTENSITY (INCH/HR) = 1.769

SUBAREA Tc AND LOSS RATE DATA (AMC I):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN	Tc (MIN.)
COMMERCIAL	B	0.76	0.30	0.100	36	7.68

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp (INCH/HR) = 0.30

SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.100

SUBAREA RUNOFF (CFS) = 1.19

TOTAL AREA (ACRES) = 0.76 PEAK FLOW RATE (CFS) = 1.19

FLOW PROCESS FROM NODE 11.00 TO NODE 11.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<

=====

TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:

TIME OF CONCENTRATION (MIN.) = 7.68
 RAINFALL INTENSITY (INCH/HR) = 1.77
 AREA-AVERAGED Fm (INCH/HR) = 0.03
 AREA-AVERAGED Fp (INCH/HR) = 0.30
 AREA-AVERAGED Ap = 0.10
 EFFECTIVE STREAM AREA (ACRES) = 0.76
 TOTAL STREAM AREA (ACRES) = 0.76
 PEAK FLOW RATE (CFS) AT CONFLUENCE = 1.19

** CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp (Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	0.34	11.22	1.423	0.30 (0.03)	0.10	0.3	10.00
2	1.19	7.68	1.769	0.30 (0.03)	0.10	0.8	12.00

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
 CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp (Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	1.48	7.68	1.769	0.30 (0.03)	0.10	0.9	12.00
2	1.29	11.22	1.423	0.30 (0.03)	0.10	1.0	10.00

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE (CFS) = 1.48 Tc (MIN.) = 7.68
 EFFECTIVE AREA (ACRES) = 0.94 AREA-AVERAGED Fm (INCH/HR) = 0.03
 AREA-AVERAGED Fp (INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.10
 TOTAL AREA (ACRES) = 1.0
 LONGEST FLOWPATH FROM NODE 10.00 TO NODE 11.00 = 475.00 FEET.

FLOW PROCESS FROM NODE 11.00 TO NODE 13.00 IS CODE = 62

>>>>COMPUTE STREET FLOW TRAVEL TIME THRU SUBAREA<<<<<
 >>>>(STREET TABLE SECTION # 1 USED)<<<<<

UPSTREAM ELEVATION (FEET) = 165.10 DOWNSTREAM ELEVATION (FEET) = 164.20
 STREET LENGTH (FEET) = 455.00 CURB HEIGHT (INCHES) = 8.0
 STREET HALFWIDTH (FEET) = 24.00

DISTANCE FROM CROWN TO CROSSFALL GRADEBREAK (FEET) = 19.00
 INSIDE STREET CROSSFALL (DECIMAL) = 0.020
 OUTSIDE STREET CROSSFALL (DECIMAL) = 0.020

SPECIFIED NUMBER OF HALFSTREETS CARRYING RUNOFF = 1
 STREET PARKWAY CROSSFALL (DECIMAL) = 0.020
 Manning's FRICTION FACTOR for Streetflow Section (curb-to-curb) = 0.0150
 Manning's FRICTION FACTOR for Back-of-Walk Flow Section = 0.0200

**TRAVEL TIME COMPUTED USING ESTIMATED FLOW (CFS) = 1.67
 STREETFLOW MODEL RESULTS USING ESTIMATED FLOW:
 STREET FLOW DEPTH (FEET) = 0.39
 HALFSTREET FLOOD WIDTH (FEET) = 11.49

AVERAGE FLOW VELOCITY (FEET/SEC.) = 1.11
 PRODUCT OF DEPTH&VELOCITY (FT*FT/SEC.) = 0.43
 STREET FLOW TRAVEL TIME (MIN.) = 6.84 Tc (MIN.) = 14.52
 * 2 YEAR RAINFALL INTENSITY (INCH/HR) = 1.227
 SUBAREA LOSS RATE DATA (AMC I):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
COMMERCIAL	B	0.36	0.30	0.100	36

 SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp (INCH/HR) = 0.30
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.100
 SUBAREA AREA (ACRES) = 0.36 SUBAREA RUNOFF (CFS) = 0.39
 EFFECTIVE AREA (ACRES) = 1.30 AREA-AVERAGED Fm (INCH/HR) = 0.03
 AREA-AVERAGED Fp (INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.10
 TOTAL AREA (ACRES) = 1.4 PEAK FLOW RATE (CFS) = 1.48
 NOTE: PEAK FLOW RATE DEFAULTED TO UPSTREAM VALUE

END OF SUBAREA STREET FLOW HYDRAULICS:
 DEPTH (FEET) = 0.37 HALFSTREET FLOOD WIDTH (FEET) = 10.83
 FLOW VELOCITY (FEET/SEC.) = 1.09 DEPTH*VELOCITY (FT*FT/SEC.) = 0.41
 LONGEST FLOWPATH FROM NODE 10.00 TO NODE 13.00 = 930.00 FEET.

 FLOW PROCESS FROM NODE 13.00 TO NODE 13.00 IS CODE = 1

 >>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

=====
 TOTAL NUMBER OF STREAMS = 2
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
 TIME OF CONCENTRATION (MIN.) = 14.52
 RAINFALL INTENSITY (INCH/HR) = 1.23
 AREA-AVERAGED Fm (INCH/HR) = 0.03
 AREA-AVERAGED Fp (INCH/HR) = 0.30
 AREA-AVERAGED Ap = 0.10
 EFFECTIVE STREAM AREA (ACRES) = 1.30
 TOTAL STREAM AREA (ACRES) = 1.39
 PEAK FLOW RATE (CFS) AT CONFLUENCE = 1.48

 FLOW PROCESS FROM NODE 14.00 TO NODE 15.00 IS CODE = 21

 >>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
 >>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

=====
 INITIAL SUBAREA FLOW-LENGTH (FEET) = 125.00
 ELEVATION DATA: UPSTREAM (FEET) = 164.60 DOWNSTREAM (FEET) = 164.20

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$
 SUBAREA ANALYSIS USED MINIMUM Tc (MIN.) = 6.616
 * 2 YEAR RAINFALL INTENSITY (INCH/HR) = 1.928

SUBAREA Tc AND LOSS RATE DATA (AMC I):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN	Tc (MIN.)
COMMERCIAL	B	0.15	0.30	0.100	36	6.62

 SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp (INCH/HR) = 0.30

SUBAREA AVERAGE PERVIOUS AREA FRACTION, $A_p = 0.100$
 SUBAREA RUNOFF (CFS) = 0.26
 TOTAL AREA (ACRES) = 0.15 PEAK FLOW RATE (CFS) = 0.26

 FLOW PROCESS FROM NODE 15.00 TO NODE 13.00 IS CODE = 41

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
 >>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM (FEET) = 159.30 DOWNSTREAM (FEET) = 159.20
 FLOW LENGTH (FEET) = 50.00 MANNING'S N = 0.013
 DEPTH OF FLOW IN 18.0 INCH PIPE IS 2.9 INCHES
 PIPE-FLOW VELOCITY (FEET/SEC.) = 1.41
 GIVEN PIPE DIAMETER (INCH) = 18.00 NUMBER OF PIPES = 1
 PIPE-FLOW (CFS) = 0.26
 PIPE TRAVEL TIME (MIN.) = 0.59 T_c (MIN.) = 7.21
 LONGEST FLOWPATH FROM NODE 14.00 TO NODE 13.00 = 175.00 FEET.

 FLOW PROCESS FROM NODE 13.00 TO NODE 13.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
 >>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<

=====

TOTAL NUMBER OF STREAMS = 2
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
 TIME OF CONCENTRATION (MIN.) = 7.21
 RAINFALL INTENSITY (INCH/HR) = 1.83
 AREA-AVERAGED F_m (INCH/HR) = 0.03
 AREA-AVERAGED F_p (INCH/HR) = 0.30
 AREA-AVERAGED $A_p = 0.10$
 EFFECTIVE STREAM AREA (ACRES) = 0.15
 TOTAL STREAM AREA (ACRES) = 0.15
 PEAK FLOW RATE (CFS) AT CONFLUENCE = 0.26

** CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	T_c (MIN.)	Intensity (INCH/HR)	$F_p(F_m)$ (INCH/HR)	A_p	A_e (ACRES)	HEADWATER NODE
1	1.48	14.52	1.227	0.30 (0.03)	0.10	1.3	12.00
1	1.31	18.29	1.075	0.30 (0.03)	0.10	1.4	10.00
2	0.26	7.21	1.835	0.30 (0.03)	0.10	0.2	14.00

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
 CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	T_c (MIN.)	Intensity (INCH/HR)	$F_p(F_m)$ (INCH/HR)	A_p	A_e (ACRES)	HEADWATER NODE
1	1.36	7.21	1.835	0.30 (0.03)	0.10	0.8	14.00
2	1.65	14.52	1.227	0.30 (0.03)	0.10	1.5	12.00
3	1.46	18.29	1.075	0.30 (0.03)	0.10	1.5	10.00

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE (CFS) = 1.65 Tc (MIN.) = 14.52
 EFFECTIVE AREA (ACRES) = 1.45 AREA-AVERAGED Fm (INCH/HR) = 0.03
 AREA-AVERAGED Fp (INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.10
 TOTAL AREA (ACRES) = 1.5
 LONGEST FLOWPATH FROM NODE 10.00 TO NODE 13.00 = 930.00 FEET.

 FLOW PROCESS FROM NODE 13.00 TO NODE 16.00 IS CODE = 41

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
 >>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM (FEET) = 159.20 DOWNSTREAM (FEET) = 151.55
 FLOW LENGTH (FEET) = 235.00 MANNING'S N = 0.013
 DEPTH OF FLOW IN 18.0 INCH PIPE IS 3.6 INCHES
 PIPE-FLOW VELOCITY (FEET/SEC.) = 6.47
 GIVEN PIPE DIAMETER (INCH) = 18.00 NUMBER OF PIPES = 1
 PIPE-FLOW (CFS) = 1.65
 PIPE TRAVEL TIME (MIN.) = 0.61 Tc (MIN.) = 15.13
 LONGEST FLOWPATH FROM NODE 10.00 TO NODE 16.00 = 1165.00 FEET.

 FLOW PROCESS FROM NODE 16.00 TO NODE 16.00 IS CODE = 10

>>>>MAIN-STREAM MEMORY COPIED ONTO MEMORY BANK # 1 <<<<<

 FLOW PROCESS FROM NODE 17.00 TO NODE 18.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
 >>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

=====

INITIAL SUBAREA FLOW-LENGTH (FEET) = 150.00
 ELEVATION DATA: UPSTREAM (FEET) = 166.90 DOWNSTREAM (FEET) = 166.50

$$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$$

SUBAREA ANALYSIS USED MINIMUM Tc (MIN.) = 8.741
 * 2 YEAR RAINFALL INTENSITY (INCH/HR) = 1.643

SUBAREA Tc AND LOSS RATE DATA (AMC I):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN	Tc (MIN.)
CONDOMINIUMS	B	0.36	0.30	0.350	36	8.74

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp (INCH/HR) = 0.30

SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.350

SUBAREA RUNOFF (CFS) = 0.50

TOTAL AREA (ACRES) = 0.36 PEAK FLOW RATE (CFS) = 0.50

 FLOW PROCESS FROM NODE 18.00 TO NODE 19.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
 >>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 153.00 DOWNSTREAM(FEET) = 151.60
 FLOW LENGTH(FEET) = 65.00 MANNING'S N = 0.013
 ESTIMATED PIPE DIAMETER(INCH) INCREASED TO 12.000
 DEPTH OF FLOW IN 12.0 INCH PIPE IS 2.5 INCHES
 PIPE-FLOW VELOCITY(FEET/SEC.) = 4.12
 ESTIMATED PIPE DIAMETER(INCH) = 12.000 NUMBER OF PIPES = 1
 PIPE-FLOW(CFS) = 0.50
 PIPE TRAVEL TIME(MIN.) = 0.26 Tc(MIN.) = 9.00
 LONGEST FLOWPATH FROM NODE 17.00 TO NODE 19.00 = 215.00 FEET.

FLOW PROCESS FROM NODE 19.00 TO NODE 19.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

=====

TOTAL NUMBER OF STREAMS = 2
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
 TIME OF CONCENTRATION(MIN.) = 9.00
 RAINFALL INTENSITY(INCH/HR) = 1.62
 AREA-AVERAGED Fm(INCH/HR) = 0.11
 AREA-AVERAGED Fp(INCH/HR) = 0.30
 AREA-AVERAGED Ap = 0.35
 EFFECTIVE STREAM AREA(ACRES) = 0.36
 TOTAL STREAM AREA(ACRES) = 0.36
 PEAK FLOW RATE(CFS) AT CONFLUENCE = 0.50

FLOW PROCESS FROM NODE 20.00 TO NODE 21.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
 >>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

=====

INITIAL SUBAREA FLOW-LENGTH(FEET) = 150.00
 ELEVATION DATA: UPSTREAM(FEET) = 167.30 DOWNSTREAM(FEET) = 166.80

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$
 SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) = 8.359
 * 2 YEAR RAINFALL INTENSITY(INCH/HR) = 1.685
 SUBAREA Tc AND LOSS RATE DATA(AMC I):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN	Tc (MIN.)
CONDOMINIUMS	A	0.50	0.40	0.350	17	8.36

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.40
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.350
 SUBAREA RUNOFF(CFS) = 0.70
 TOTAL AREA(ACRES) = 0.50 PEAK FLOW RATE(CFS) = 0.70

FLOW PROCESS FROM NODE 21.00 TO NODE 19.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
 >>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 153.00 DOWNSTREAM(FEET) = 151.60

FLOW LENGTH (FEET) = 45.00 MANNING'S N = 0.013
 ESTIMATED PIPE DIAMETER (INCH) INCREASED TO 12.000
 DEPTH OF FLOW IN 12.0 INCH PIPE IS 2.7 INCHES
 PIPE-FLOW VELOCITY (FEET/SEC.) = 5.17
 ESTIMATED PIPE DIAMETER (INCH) = 12.000 NUMBER OF PIPES = 1
 PIPE-FLOW (CFS) = 0.70
 PIPE TRAVEL TIME (MIN.) = 0.15 Tc (MIN.) = 8.50
 LONGEST FLOWPATH FROM NODE 20.00 TO NODE 19.00 = 195.00 FEET.

 FLOW PROCESS FROM NODE 19.00 TO NODE 19.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
 >>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<

=====
 TOTAL NUMBER OF STREAMS = 2
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
 TIME OF CONCENTRATION (MIN.) = 8.50
 RAINFALL INTENSITY (INCH/HR) = 1.67
 AREA-AVERAGED Fm (INCH/HR) = 0.14
 AREA-AVERAGED Fp (INCH/HR) = 0.40
 AREA-AVERAGED Ap = 0.35
 EFFECTIVE STREAM AREA (ACRES) = 0.50
 TOTAL STREAM AREA (ACRES) = 0.50
 PEAK FLOW RATE (CFS) AT CONFLUENCE = 0.70

** CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	0.50	9.00	1.615	0.30 (0.11)	0.35	0.4	17.00
2	0.70	8.50	1.669	0.40 (0.14)	0.35	0.5	20.00

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
 CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	1.18	8.50	1.669	0.36 (0.13)	0.35	0.8	20.00
2	1.17	9.00	1.615	0.36 (0.13)	0.35	0.9	17.00

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
 PEAK FLOW RATE (CFS) = 1.18 Tc (MIN.) = 8.50
 EFFECTIVE AREA (ACRES) = 0.84 AREA-AVERAGED Fm (INCH/HR) = 0.13
 AREA-AVERAGED Fp (INCH/HR) = 0.36 AREA-AVERAGED Ap = 0.35
 TOTAL AREA (ACRES) = 0.9
 LONGEST FLOWPATH FROM NODE 17.00 TO NODE 19.00 = 215.00 FEET.

 FLOW PROCESS FROM NODE 19.00 TO NODE 19.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

=====

MAINLINE Tc (MIN.) = 8.50

* 2 YEAR RAINFALL INTENSITY (INCH/HR) = 1.669
 SUBAREA LOSS RATE DATA (AMC I):
 DEVELOPMENT TYPE/ SCS SOIL AREA Fp Ap SCS
 LAND USE GROUP (ACRES) (INCH/HR) (DECIMAL) CN
 CONDOMINIUMS B 0.42 0.30 0.350 36
 SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp (INCH/HR) = 0.30
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.350
 SUBAREA AREA (ACRES) = 0.42 SUBAREA RUNOFF (CFS) = 0.59
 EFFECTIVE AREA (ACRES) = 1.26 AREA-AVERAGED Fm (INCH/HR) = 0.12
 AREA-AVERAGED Fp (INCH/HR) = 0.34 AREA-AVERAGED Ap = 0.35
 TOTAL AREA (ACRES) = 1.3 PEAK FLOW RATE (CFS) = 1.76

FLOW PROCESS FROM NODE 19.00 TO NODE 16.00 IS CODE = 41

>>>> COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA <<<<<
 >>>> USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT) <<<<<

=====

ELEVATION DATA: UPSTREAM (FEET) = 151.60 DOWNSTREAM (FEET) = 151.55
 FLOW LENGTH (FEET) = 30.00 MANNING'S N = 0.013
 DEPTH OF FLOW IN 18.0 INCH PIPE IS 8.1 INCHES
 PIPE-FLOW VELOCITY (FEET/SEC.) = 2.26
 GIVEN PIPE DIAMETER (INCH) = 18.00 NUMBER OF PIPES = 1
 PIPE-FLOW (CFS) = 1.76
 PIPE TRAVEL TIME (MIN.) = 0.22 Tc (MIN.) = 8.73
 LONGEST FLOWPATH FROM NODE 17.00 TO NODE 16.00 = 245.00 FEET.

FLOW PROCESS FROM NODE 16.00 TO NODE 16.00 IS CODE = 11

>>>> CONFLUENCE MEMORY BANK # 1 WITH THE MAIN-STREAM MEMORY <<<<<

** MAIN STREAM CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp (Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	1.76	8.73	1.644	0.34 (0.12)	0.35	1.3	20.00
2	1.72	9.23	1.593	0.34 (0.12)	0.35	1.3	17.00
LONGEST FLOWPATH FROM NODE 17.00 TO NODE 16.00 = 245.00 FEET.							

** MEMORY BANK # 1 CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp (Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	1.36	7.85	1.747	0.30 (0.03)	0.10	0.8	14.00
2	1.65	15.13	1.199	0.30 (0.03)	0.10	1.5	12.00
3	1.46	18.92	1.055	0.30 (0.03)	0.10	1.5	10.00
LONGEST FLOWPATH FROM NODE 10.00 TO NODE 16.00 = 1165.00 FEET.							

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp (Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	3.05	7.85	1.747	0.33 (0.08)	0.25	1.9	14.00
2	3.15	8.73	1.644	0.33 (0.08)	0.25	2.1	20.00
3	3.14	9.23	1.593	0.33 (0.08)	0.25	2.2	17.00

4	2.91	15.13	1.199	0.33 (0.07)	0.22	2.7	12.00
5	2.55	18.92	1.055	0.33 (0.07)	0.21	2.8	10.00
TOTAL AREA (ACRES) =			2.8				

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE (CFS) = 3.15 Tc (MIN.) = 8.725
EFFECTIVE AREA (ACRES) = 2.14 AREA-AVERAGED Fm (INCH/HR) = 0.08
AREA-AVERAGED Fp (INCH/HR) = 0.33 AREA-AVERAGED Ap = 0.25
TOTAL AREA (ACRES) = 2.8
LONGEST FLOWPATH FROM NODE 10.00 TO NODE 16.00 = 1165.00 FEET.

FLOW PROCESS FROM NODE 16.00 TO NODE 22.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<

=====

ELEVATION DATA: UPSTREAM (FEET) = 151.55 DOWNSTREAM (FEET) = 151.30
FLOW LENGTH (FEET) = 115.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 18.0 INCH PIPE IS 10.7 INCHES
PIPE-FLOW VELOCITY (FEET/SEC.) = 2.88
ESTIMATED PIPE DIAMETER (INCH) = 18.00 NUMBER OF PIPES = 1
PIPE-FLOW (CFS) = 3.15
PIPE TRAVEL TIME (MIN.) = 0.66 Tc (MIN.) = 9.39
LONGEST FLOWPATH FROM NODE 10.00 TO NODE 22.00 = 1280.00 FEET.

FLOW PROCESS FROM NODE 22.00 TO NODE 22.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

=====

MAINLINE Tc (MIN.) = 9.39
* 2 YEAR RAINFALL INTENSITY (INCH/HR) = 1.577
SUBAREA LOSS RATE DATA (AMC I):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
CONDOMINIUMS	A	0.31	0.40	0.350	17

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp (INCH/HR) = 0.40
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.350
SUBAREA AREA (ACRES) = 0.31 SUBAREA RUNOFF (CFS) = 0.40
EFFECTIVE AREA (ACRES) = 2.45 AREA-AVERAGED Fm (INCH/HR) = 0.09
AREA-AVERAGED Fp (INCH/HR) = 0.34 AREA-AVERAGED Ap = 0.26
TOTAL AREA (ACRES) = 3.1 PEAK FLOW RATE (CFS) = 3.27

FLOW PROCESS FROM NODE 22.00 TO NODE 22.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

=====

TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION (MIN.) = 9.39
RAINFALL INTENSITY (INCH/HR) = 1.58
AREA-AVERAGED Fm (INCH/HR) = 0.09

AREA-AVERAGED F_p (INCH/HR) = 0.34
 AREA-AVERAGED A_p = 0.26
 EFFECTIVE STREAM AREA (ACRES) = 2.45
 TOTAL STREAM AREA (ACRES) = 3.13
 PEAK FLOW RATE (CFS) AT CONFLUENCE = 3.27

FLOW PROCESS FROM NODE 23.00 TO NODE 22.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
 >>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

INITIAL SUBAREA FLOW-LENGTH (FEET) = 225.00
 ELEVATION DATA: UPSTREAM (FEET) = 166.30 DOWNSTREAM (FEET) = 165.50

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$
 SUBAREA ANALYSIS USED MINIMUM T_c (MIN.) = 9.705
 * 2 YEAR RAINFALL INTENSITY (INCH/HR) = 1.547

SUBAREA T_c AND LOSS RATE DATA (AMC I):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	F_p (INCH/HR)	A_p (DECIMAL)	SCS CN	T_c (MIN.)
CONDOMINIUMS	B	0.71	0.30	0.350	36	9.70

SUBAREA AVERAGE PERVIOUS LOSS RATE, F_p (INCH/HR) = 0.30
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, A_p = 0.350
 SUBAREA RUNOFF (CFS) = 0.92
 TOTAL AREA (ACRES) = 0.71 PEAK FLOW RATE (CFS) = 0.92

FLOW PROCESS FROM NODE 22.00 TO NODE 22.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
 >>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<

TOTAL NUMBER OF STREAMS = 2
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
 TIME OF CONCENTRATION (MIN.) = 9.70
 RAINFALL INTENSITY (INCH/HR) = 1.55
 AREA-AVERAGED F_m (INCH/HR) = 0.11
 AREA-AVERAGED F_p (INCH/HR) = 0.30
 AREA-AVERAGED A_p = 0.35
 EFFECTIVE STREAM AREA (ACRES) = 0.71
 TOTAL STREAM AREA (ACRES) = 0.71
 PEAK FLOW RATE (CFS) AT CONFLUENCE = 0.92

** CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	T_c (MIN.)	Intensity (INCH/HR)	F_p (F_m) (INCH/HR)	A_p	A_e (ACRES)	HEADWATER NODE
1	3.18	8.52	1.667	0.35 (0.09)	0.26	2.2	14.00
1	3.27	9.39	1.577	0.34 (0.09)	0.26	2.4	20.00
1	3.26	9.89	1.530	0.34 (0.09)	0.26	2.5	17.00
1	2.99	15.83	1.168	0.34 (0.08)	0.23	3.0	12.00
1	2.69	19.63	1.033	0.34 (0.08)	0.23	3.1	10.00
2	0.92	9.70	1.547	0.30 (0.11)	0.35	0.7	23.00

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
 CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	4.06	8.52	1.667	0.33(0.09)	0.28	2.9	14.00
2	4.18	9.39	1.577	0.33(0.09)	0.28	3.1	20.00
3	4.19	9.70	1.547	0.33(0.09)	0.28	3.2	23.00
4	4.17	9.89	1.530	0.33(0.09)	0.28	3.2	17.00
5	3.67	15.83	1.168	0.33(0.08)	0.25	3.8	12.00
6	3.28	19.63	1.033	0.33(0.08)	0.25	3.8	10.00

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE (CFS) = 4.19 Tc (MIN.) = 9.70
 EFFECTIVE AREA (ACRES) = 3.20 AREA-AVERAGED Fm (INCH/HR) = 0.09
 AREA-AVERAGED Fp (INCH/HR) = 0.33 AREA-AVERAGED Ap = 0.28
 TOTAL AREA (ACRES) = 3.8
 LONGEST FLOWPATH FROM NODE 10.00 TO NODE 22.00 = 1280.00 FEET.

FLOW PROCESS FROM NODE 22.00 TO NODE 22.00 IS CODE = 10

>>>>MAIN-STREAM MEMORY COPIED ONTO MEMORY BANK # 2 <<<<<

FLOW PROCESS FROM NODE 24.00 TO NODE 25.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
 >>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

INITIAL SUBAREA FLOW-LENGTH (FEET) = 270.00
 ELEVATION DATA: UPSTREAM (FEET) = 166.50 DOWNSTREAM (FEET) = 165.50

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$
 SUBAREA ANALYSIS USED MINIMUM Tc (MIN.) = 10.354
 * 2 YEAR RAINFALL INTENSITY (INCH/HR) = 1.491
 SUBAREA Tc AND LOSS RATE DATA (AMC I):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN	Tc (MIN.)
CONDOMINIUMS	B	0.51	0.30	0.350	36	10.35

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp (INCH/HR) = 0.30
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.350
 SUBAREA RUNOFF (CFS) = 0.64
 TOTAL AREA (ACRES) = 0.51 PEAK FLOW RATE (CFS) = 0.64

FLOW PROCESS FROM NODE 25.00 TO NODE 25.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

TOTAL NUMBER OF STREAMS = 2
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:

TIME OF CONCENTRATION(MIN.) = 10.35
 RAINFALL INTENSITY(INCH/HR) = 1.49
 AREA-AVERAGED Fm(INCH/HR) = 0.11
 AREA-AVERAGED Fp(INCH/HR) = 0.30
 AREA-AVERAGED Ap = 0.35
 EFFECTIVE STREAM AREA(ACRES) = 0.51
 TOTAL STREAM AREA(ACRES) = 0.51
 PEAK FLOW RATE(CFS) AT CONFLUENCE = 0.64

 FLOW PROCESS FROM NODE 26.00 TO NODE 25.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
 >>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

INITIAL SUBAREA FLOW-LENGTH(FEET) = 155.00
 ELEVATION DATA: UPSTREAM(FEET) = 166.50 DOWNSTREAM(FEET) = 165.50

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$
 SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) = 7.422
 * 2 YEAR RAINFALL INTENSITY(INCH/HR) = 1.805

SUBAREA Tc AND LOSS RATE DATA(AMC I):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN	Tc (MIN.)
CONDOMINIUMS	B	0.30	0.30	0.350	36	7.42

 SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.30
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.350
 SUBAREA RUNOFF(CFS) = 0.46
 TOTAL AREA(ACRES) = 0.30 PEAK FLOW RATE(CFS) = 0.46

 FLOW PROCESS FROM NODE 25.00 TO NODE 25.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
 >>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<

TOTAL NUMBER OF STREAMS = 2
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
 TIME OF CONCENTRATION(MIN.) = 7.42
 RAINFALL INTENSITY(INCH/HR) = 1.80
 AREA-AVERAGED Fm(INCH/HR) = 0.10
 AREA-AVERAGED Fp(INCH/HR) = 0.30
 AREA-AVERAGED Ap = 0.35
 EFFECTIVE STREAM AREA(ACRES) = 0.30
 TOTAL STREAM AREA(ACRES) = 0.30
 PEAK FLOW RATE(CFS) AT CONFLUENCE = 0.46

** CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	0.64	10.35	1.491	0.30 (0.11)	0.35	0.5	24.00
2	0.46	7.42	1.805	0.30 (0.10)	0.35	0.3	26.00

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO

CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	1.02	7.42	1.805	0.30 (0.10)	0.35	0.7	26.00
2	1.01	10.35	1.491	0.30 (0.11)	0.35	0.8	24.00

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE (CFS) = 1.02 Tc (MIN.) = 7.42
EFFECTIVE AREA (ACRES) = 0.67 AREA-AVERAGED Fm (INCH/HR) = 0.10
AREA-AVERAGED Fp (INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.35
TOTAL AREA (ACRES) = 0.8
LONGEST FLOWPATH FROM NODE 24.00 TO NODE 25.00 = 270.00 FEET.

FLOW PROCESS FROM NODE 25.00 TO NODE 28.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<

=====

ELEVATION DATA: UPSTREAM (FEET) = 161.00 DOWNSTREAM (FEET) = 158.00
FLOW LENGTH (FEET) = 175.00 MANNING'S N = 0.013
ESTIMATED PIPE DIAMETER (INCH) INCREASED TO 12.000
DEPTH OF FLOW IN 12.0 INCH PIPE IS 3.9 INCHES
PIPE-FLOW VELOCITY (FEET/SEC.) = 4.65
ESTIMATED PIPE DIAMETER (INCH) = 12.000 NUMBER OF PIPES = 1
PIPE-FLOW (CFS) = 1.02
PIPE TRAVEL TIME (MIN.) = 0.63 Tc (MIN.) = 8.05
LONGEST FLOWPATH FROM NODE 24.00 TO NODE 28.00 = 445.00 FEET.

FLOW PROCESS FROM NODE 28.00 TO NODE 28.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

=====

TOTAL NUMBER OF STREAMS = 3
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION (MIN.) = 8.05
RAINFALL INTENSITY (INCH/HR) = 1.72
AREA-AVERAGED Fm (INCH/HR) = 0.10
AREA-AVERAGED Fp (INCH/HR) = 0.30
AREA-AVERAGED Ap = 0.35
EFFECTIVE STREAM AREA (ACRES) = 0.67
TOTAL STREAM AREA (ACRES) = 0.81
PEAK FLOW RATE (CFS) AT CONFLUENCE = 1.02

FLOW PROCESS FROM NODE 27.00 TO NODE 28.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

=====

INITIAL SUBAREA FLOW-LENGTH (FEET) = 180.00

ELEVATION DATA: UPSTREAM(FEET) = 166.50 DOWNSTREAM(FEET) = 165.40

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$

SUBAREA ANALYSIS USED MINIMUM T_c (MIN.) = 7.965

* 2 YEAR RAINFALL INTENSITY(INCH/HR) = 1.733

SUBAREA T_c AND LOSS RATE DATA(AMC I):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	F_p (INCH/HR)	A_p (DECIMAL)	SCS CN	T_c (MIN.)
CONDOMINIUMS	B	0.18	0.30	0.350	36	7.97

SUBAREA AVERAGE PERVIOUS LOSS RATE, F_p (INCH/HR) = 0.30

SUBAREA AVERAGE PERVIOUS AREA FRACTION, A_p = 0.350

SUBAREA RUNOFF(CFS) = 0.26

TOTAL AREA(ACRES) = 0.18 PEAK FLOW RATE(CFS) = 0.26

FLOW PROCESS FROM NODE 28.00 TO NODE 28.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

TOTAL NUMBER OF STREAMS = 3

CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:

TIME OF CONCENTRATION(MIN.) = 7.97

RAINFALL INTENSITY(INCH/HR) = 1.73

AREA-AVERAGED F_m (INCH/HR) = 0.11

AREA-AVERAGED F_p (INCH/HR) = 0.30

AREA-AVERAGED A_p = 0.35

EFFECTIVE STREAM AREA(ACRES) = 0.18

TOTAL STREAM AREA(ACRES) = 0.18

PEAK FLOW RATE(CFS) AT CONFLUENCE = 0.26

FLOW PROCESS FROM NODE 29.00 TO NODE 28.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<

>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

INITIAL SUBAREA FLOW-LENGTH(FEET) = 230.00

ELEVATION DATA: UPSTREAM(FEET) = 166.50 DOWNSTREAM(FEET) = 165.40

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$

SUBAREA ANALYSIS USED MINIMUM T_c (MIN.) = 9.227

* 2 YEAR RAINFALL INTENSITY(INCH/HR) = 1.593

SUBAREA T_c AND LOSS RATE DATA(AMC I):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	F_p (INCH/HR)	A_p (DECIMAL)	SCS CN	T_c (MIN.)
CONDOMINIUMS	B	0.48	0.30	0.350	36	9.23

SUBAREA AVERAGE PERVIOUS LOSS RATE, F_p (INCH/HR) = 0.30

SUBAREA AVERAGE PERVIOUS AREA FRACTION, A_p = 0.350

SUBAREA RUNOFF(CFS) = 0.64

TOTAL AREA(ACRES) = 0.48 PEAK FLOW RATE(CFS) = 0.64

FLOW PROCESS FROM NODE 28.00 TO NODE 28.00 IS CODE = 1

>>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
>>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<

=====

TOTAL NUMBER OF STREAMS = 3
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 3 ARE:
TIME OF CONCENTRATION(MIN.) = 9.23
RAINFALL INTENSITY(INCH/HR) = 1.59
AREA-AVERAGED Fm(INCH/HR) = 0.11
AREA-AVERAGED Fp(INCH/HR) = 0.30
AREA-AVERAGED Ap = 0.35
EFFECTIVE STREAM AREA(ACRES) = 0.48
TOTAL STREAM AREA(ACRES) = 0.48
PEAK FLOW RATE(CFS) AT CONFLUENCE = 0.64

** CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	1.02	8.05	1.722	0.30(0.10)	0.35	0.7	26.00
1	1.01	10.98	1.441	0.30(0.11)	0.35	0.8	24.00
2	0.26	7.97	1.733	0.30(0.11)	0.35	0.2	27.00
3	0.64	9.23	1.593	0.30(0.11)	0.35	0.5	29.00

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
CONFLUENCE FORMULA USED FOR 3 STREAMS.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	1.88	7.97	1.733	0.30(0.10)	0.35	1.3	27.00
2	1.89	8.05	1.722	0.30(0.10)	0.35	1.3	26.00
3	1.90	9.23	1.593	0.30(0.10)	0.35	1.4	29.00
4	1.80	10.98	1.441	0.30(0.11)	0.35	1.5	24.00

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE(CFS) = 1.90 Tc(MIN.) = 9.23
EFFECTIVE AREA(ACRES) = 1.38 AREA-AVERAGED Fm(INCH/HR) = 0.10
AREA-AVERAGED Fp(INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.35
TOTAL AREA(ACRES) = 1.5
LONGEST FLOWPATH FROM NODE 24.00 TO NODE 28.00 = 445.00 FEET.

FLOW PROCESS FROM NODE 28.00 TO NODE 22.00 IS CODE = 31

>>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 158.00 DOWNSTREAM(FEET) = 151.30
FLOW LENGTH(FEET) = 250.00 MANNING'S N = 0.013
ESTIMATED PIPE DIAMETER(INCH) INCREASED TO 12.000
DEPTH OF FLOW IN 12.0 INCH PIPE IS 4.8 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 6.51
ESTIMATED PIPE DIAMETER(INCH) = 12.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 1.90
PIPE TRAVEL TIME(MIN.) = 0.64 Tc(MIN.) = 9.87

LONGEST FLOWPATH FROM NODE 24.00 TO NODE 22.00 = 695.00 FEET.

FLOW PROCESS FROM NODE 22.00 TO NODE 22.00 IS CODE = 11

>>>>CONFLUENCE MEMORY BANK # 2 WITH THE MAIN-STREAM MEMORY<<<<<

=====
** MAIN STREAM CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	1.88	8.61	1.658	0.30(0.10)	0.35	1.3	27.00
2	1.89	8.69	1.649	0.30(0.10)	0.35	1.3	26.00
3	1.90	9.87	1.532	0.30(0.10)	0.35	1.4	29.00
4	1.80	11.63	1.395	0.30(0.11)	0.35	1.5	24.00

LONGEST FLOWPATH FROM NODE 24.00 TO NODE 22.00 = 695.00 FEET.

** MEMORY BANK # 2 CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	4.06	8.52	1.667	0.33(0.09)	0.28	2.9	14.00
2	4.18	9.39	1.577	0.33(0.09)	0.28	3.1	20.00
3	4.19	9.70	1.547	0.33(0.09)	0.28	3.2	23.00
4	4.17	9.89	1.530	0.33(0.09)	0.28	3.2	17.00
5	3.67	15.83	1.168	0.33(0.08)	0.25	3.8	12.00
6	3.28	19.63	1.033	0.33(0.08)	0.25	3.8	10.00

LONGEST FLOWPATH FROM NODE 10.00 TO NODE 22.00 = 1280.00 FEET.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	5.93	8.52	1.667	0.32(0.10)	0.30	4.1	14.00
2	5.95	8.61	1.658	0.32(0.10)	0.30	4.1	27.00
3	5.97	8.69	1.649	0.32(0.10)	0.30	4.2	26.00
4	6.08	9.39	1.577	0.32(0.10)	0.30	4.5	20.00
5	6.09	9.70	1.547	0.32(0.10)	0.30	4.6	23.00
6	6.07	9.87	1.532	0.32(0.10)	0.30	4.6	29.00
7	6.07	9.89	1.530	0.32(0.10)	0.30	4.6	17.00
8	5.83	11.63	1.395	0.32(0.09)	0.29	4.8	24.00
9	5.15	15.83	1.168	0.32(0.09)	0.28	5.2	12.00
10	4.58	19.63	1.033	0.32(0.09)	0.28	5.3	10.00

TOTAL AREA (ACRES) = 5.3

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE (CFS) = 6.09 Tc (MIN.) = 9.705
EFFECTIVE AREA (ACRES) = 4.56 AREA-AVERAGED Fm (INCH/HR) = 0.10
AREA-AVERAGED Fp (INCH/HR) = 0.32 AREA-AVERAGED Ap = 0.30
TOTAL AREA (ACRES) = 5.3
LONGEST FLOWPATH FROM NODE 10.00 TO NODE 22.00 = 1280.00 FEET.

FLOW PROCESS FROM NODE 22.00 TO NODE 33.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<

>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 151.30 DOWNSTREAM(FEET) = 150.50
FLOW LENGTH(FEET) = 425.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 21.0 INCH PIPE IS 15.7 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 3.16
ESTIMATED PIPE DIAMETER(INCH) = 21.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 6.09
PIPE TRAVEL TIME(MIN.) = 2.24 Tc(MIN.) = 11.95
LONGEST FLOWPATH FROM NODE 10.00 TO NODE 33.00 = 1705.00 FEET.

FLOW PROCESS FROM NODE 33.00 TO NODE 33.00 IS CODE = 10

>>>>MAIN-STREAM MEMORY COPIED ONTO MEMORY BANK # 3<<<<<

FLOW PROCESS FROM NODE 30.00 TO NODE 31.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

=====

INITIAL SUBAREA FLOW-LENGTH(FEET) = 150.00
ELEVATION DATA: UPSTREAM(FEET) = 166.50 DOWNSTREAM(FEET) = 165.50

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$
SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) = 7.277
* 2 YEAR RAINFALL INTENSITY(INCH/HR) = 1.825
SUBAREA Tc AND LOSS RATE DATA(AMC I):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN	Tc (MIN.)
CONDOMINIUMS	B	0.29	0.30	0.350	36	7.28

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.30
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.350
SUBAREA RUNOFF(CFS) = 0.45
TOTAL AREA(ACRES) = 0.29 PEAK FLOW RATE(CFS) = 0.45

FLOW PROCESS FROM NODE 31.00 TO NODE 31.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

=====

MAINLINE Tc(MIN.) = 7.28
* 2 YEAR RAINFALL INTENSITY(INCH/HR) = 1.825
SUBAREA LOSS RATE DATA(AMC I):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
CONDOMINIUMS	B	0.65	0.30	0.350	36

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.30
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.350
SUBAREA AREA(ACRES) = 0.65 SUBAREA RUNOFF(CFS) = 1.01
EFFECTIVE AREA(ACRES) = 0.94 AREA-AVERAGED Fm(INCH/HR) = 0.11
AREA-AVERAGED Fp(INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.35

TOTAL AREA (ACRES) = 0.9 PEAK FLOW RATE (CFS) = 1.46

FLOW PROCESS FROM NODE 31.00 TO NODE 33.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<

=====

ELEVATION DATA: UPSTREAM (FEET) =	157.00	DOWNSTREAM (FEET) =	150.50
FLOW LENGTH (FEET) =	380.00	MANNING'S N =	0.013
ESTIMATED PIPE DIAMETER (INCH) INCREASED TO	12.000		
DEPTH OF FLOW IN 12.0 INCH PIPE IS	4.7 INCHES		
PIPE-FLOW VELOCITY (FEET/SEC.) =	5.15		
ESTIMATED PIPE DIAMETER (INCH) =	12.00	NUMBER OF PIPES =	1
PIPE-FLOW (CFS) =	1.46		
PIPE TRAVEL TIME (MIN.) =	1.23	Tc (MIN.) =	8.51
LONGEST FLOWPATH FROM NODE	30.00 TO NODE	33.00 =	530.00 FEET.

FLOW PROCESS FROM NODE 33.00 TO NODE 33.00 IS CODE = 11

>>>>CONFLUENCE MEMORY BANK # 3 WITH THE MAIN-STREAM MEMORY<<<<<

=====

** MAIN STREAM CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp (Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	1.46	8.51	1.669	0.30 (0.11)	0.35	0.9	30.00
LONGEST FLOWPATH FROM NODE			30.00 TO NODE		33.00 =		530.00 FEET.

** MEMORY BANK # 3 CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp (Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	5.93	10.77	1.457	0.32 (0.10)	0.30	4.1	14.00
2	5.95	10.85	1.451	0.32 (0.10)	0.30	4.1	27.00
3	5.97	10.94	1.444	0.32 (0.10)	0.30	4.2	26.00
4	6.08	11.63	1.394	0.32 (0.10)	0.30	4.5	20.00
5	6.09	11.95	1.373	0.32 (0.10)	0.30	4.6	23.00
6	6.07	12.11	1.362	0.32 (0.10)	0.30	4.6	29.00
7	6.07	12.14	1.361	0.32 (0.10)	0.30	4.6	17.00
8	5.83	13.88	1.260	0.32 (0.09)	0.29	4.8	24.00
9	5.15	18.14	1.080	0.32 (0.09)	0.28	5.2	12.00
10	4.58	21.99	0.967	0.32 (0.09)	0.28	5.3	10.00
LONGEST FLOWPATH FROM NODE			10.00 TO NODE		33.00 =		1705.00 FEET.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp (Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	6.87	8.51	1.669	0.32 (0.10)	0.31	4.2	30.00
2	7.19	10.77	1.457	0.32 (0.10)	0.31	5.0	14.00
3	7.21	10.85	1.451	0.32 (0.10)	0.31	5.1	27.00
4	7.22	10.94	1.444	0.32 (0.10)	0.31	5.1	26.00
5	7.28	11.63	1.394	0.32 (0.10)	0.31	5.4	20.00
6	7.27	11.95	1.373	0.32 (0.10)	0.31	5.5	23.00

7	7.24	12.11	1.362	0.32 (0.10)	0.31	5.5	29.00
8	7.24	12.14	1.361	0.32 (0.10)	0.31	5.5	17.00
9	6.90	13.88	1.260	0.32 (0.10)	0.30	5.8	24.00
10	6.06	18.14	1.080	0.32 (0.09)	0.29	6.2	12.00
11	5.38	21.99	0.967	0.32 (0.09)	0.29	6.2	10.00
TOTAL AREA (ACRES) =			6.2				

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE (CFS) = 7.28 Tc (MIN.) = 11.635
EFFECTIVE AREA (ACRES) = 5.41 AREA-AVERAGED Fm (INCH/HR) = 0.10
AREA-AVERAGED Fp (INCH/HR) = 0.32 AREA-AVERAGED Ap = 0.31
TOTAL AREA (ACRES) = 6.2
LONGEST FLOWPATH FROM NODE 10.00 TO NODE 33.00 = 1705.00 FEET.

FLOW PROCESS FROM NODE 16.00 TO NODE 16.00 IS CODE = 12

>>>>CLEAR MEMORY BANK # 1 <<<<<

FLOW PROCESS FROM NODE 22.00 TO NODE 22.00 IS CODE = 12

>>>>CLEAR MEMORY BANK # 2 <<<<<

FLOW PROCESS FROM NODE 33.00 TO NODE 33.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

=====

TOTAL NUMBER OF STREAMS = 5
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION (MIN.) = 11.63
RAINFALL INTENSITY (INCH/HR) = 1.39
AREA-AVERAGED Fm (INCH/HR) = 0.10
AREA-AVERAGED Fp (INCH/HR) = 0.32
AREA-AVERAGED Ap = 0.31
EFFECTIVE STREAM AREA (ACRES) = 5.41
TOTAL STREAM AREA (ACRES) = 6.25
PEAK FLOW RATE (CFS) AT CONFLUENCE = 7.28

FLOW PROCESS FROM NODE 30.00 TO NODE 33.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

=====

INITIAL SUBAREA FLOW-LENGTH (FEET) = 185.00
ELEVATION DATA: UPSTREAM (FEET) = 166.60 DOWNSTREAM (FEET) = 166.00

Tc = K * [(LENGTH** 3.00) / (ELEVATION CHANGE)]**0.20
SUBAREA ANALYSIS USED MINIMUM Tc (MIN.) = 9.141
* 2 YEAR RAINFALL INTENSITY (INCH/HR) = 1.601

SUBAREA Tc AND LOSS RATE DATA(AMC I):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN	Tc (MIN.)
CONDOMINIUMS	B	0.67	0.30	0.350	36	9.14

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.30
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.350
SUBAREA RUNOFF(CFS) = 0.90
TOTAL AREA(ACRES) = 0.67 PEAK FLOW RATE(CFS) = 0.90

FLOW PROCESS FROM NODE 33.00 TO NODE 33.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

=====

TOTAL NUMBER OF STREAMS = 5
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
TIME OF CONCENTRATION(MIN.) = 9.14
RAINFALL INTENSITY(INCH/HR) = 1.60
AREA-AVERAGED Fm(INCH/HR) = 0.11
AREA-AVERAGED Fp(INCH/HR) = 0.30
AREA-AVERAGED Ap = 0.35
EFFECTIVE STREAM AREA(ACRES) = 0.67
TOTAL STREAM AREA(ACRES) = 0.67
PEAK FLOW RATE(CFS) AT CONFLUENCE = 0.90

FLOW PROCESS FROM NODE 34.00 TO NODE 35.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<

>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

=====

INITIAL SUBAREA FLOW-LENGTH(FEET) = 180.00
ELEVATION DATA: UPSTREAM(FEET) = 166.20 DOWNSTREAM(FEET) = 166.10

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$
SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) = 12.867
* 2 YEAR RAINFALL INTENSITY(INCH/HR) = 1.316

SUBAREA Tc AND LOSS RATE DATA(AMC I):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN	Tc (MIN.)
CONDOMINIUMS	B	0.50	0.30	0.350	36	12.87

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.30
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.350
SUBAREA RUNOFF(CFS) = 0.54
TOTAL AREA(ACRES) = 0.50 PEAK FLOW RATE(CFS) = 0.54

FLOW PROCESS FROM NODE 33.00 TO NODE 33.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

=====

TOTAL NUMBER OF STREAMS = 5
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 3 ARE:
TIME OF CONCENTRATION(MIN.) = 12.87

RAINFALL INTENSITY (INCH/HR) = 1.32
 AREA-AVERAGED F_m (INCH/HR) = 0.11
 AREA-AVERAGED F_p (INCH/HR) = 0.30
 AREA-AVERAGED A_p = 0.35
 EFFECTIVE STREAM AREA (ACRES) = 0.50
 TOTAL STREAM AREA (ACRES) = 0.50
 PEAK FLOW RATE (CFS) AT CONFLUENCE = 0.54

 FLOW PROCESS FROM NODE 36.00 TO NODE 37.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
 >>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

INITIAL SUBAREA FLOW-LENGTH (FEET) = 150.00
 ELEVATION DATA: UPSTREAM (FEET) = 166.70 DOWNSTREAM (FEET) = 166.40

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$
 SUBAREA ANALYSIS USED MINIMUM T_c (MIN.) = 9.258
 * 2 YEAR RAINFALL INTENSITY (INCH/HR) = 1.589

SUBAREA T_c AND LOSS RATE DATA (AMC I):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	F_p (INCH/HR)	A_p (DECIMAL)	SCS CN	T_c (MIN.)
CONDOMINIUMS	B	0.29	0.30	0.350	36	9.26

SUBAREA AVERAGE PERVIOUS LOSS RATE, F_p (INCH/HR) = 0.30
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, A_p = 0.350
 SUBAREA RUNOFF (CFS) = 0.39
 TOTAL AREA (ACRES) = 0.29 PEAK FLOW RATE (CFS) = 0.39

 FLOW PROCESS FROM NODE 33.00 TO NODE 33.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

TOTAL NUMBER OF STREAMS = 5
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 4 ARE:
 TIME OF CONCENTRATION (MIN.) = 9.26
 RAINFALL INTENSITY (INCH/HR) = 1.59
 AREA-AVERAGED F_m (INCH/HR) = 0.11
 AREA-AVERAGED F_p (INCH/HR) = 0.30
 AREA-AVERAGED A_p = 0.35
 EFFECTIVE STREAM AREA (ACRES) = 0.29
 TOTAL STREAM AREA (ACRES) = 0.29
 PEAK FLOW RATE (CFS) AT CONFLUENCE = 0.39

 FLOW PROCESS FROM NODE 38.00 TO NODE 33.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
 >>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

INITIAL SUBAREA FLOW-LENGTH (FEET) = 150.00
 ELEVATION DATA: UPSTREAM (FEET) = 166.40 DOWNSTREAM (FEET) = 166.00

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$
 SUBAREA ANALYSIS USED MINIMUM T_c (MIN.) = 8.741
 * 2 YEAR RAINFALL INTENSITY (INCH/HR) = 1.643
 SUBAREA T_c AND LOSS RATE DATA (AMC I):
 DEVELOPMENT TYPE/ LAND USE SCS SOIL GROUP AREA (ACRES) F_p (INCH/HR) A_p (DECIMAL) SCS CN T_c (MIN.)
 CONDOMINIUMS B 0.18 0.30 0.350 36 8.74
 SUBAREA AVERAGE PERVIOUS LOSS RATE, F_p (INCH/HR) = 0.30
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, A_p = 0.350
 SUBAREA RUNOFF (CFS) = 0.25
 TOTAL AREA (ACRES) = 0.18 PEAK FLOW RATE (CFS) = 0.25

FLOW PROCESS FROM NODE 33.00 TO NODE 33.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<<
 >>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<<

=====
 TOTAL NUMBER OF STREAMS = 5
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 5 ARE:
 TIME OF CONCENTRATION (MIN.) = 8.74
 RAINFALL INTENSITY (INCH/HR) = 1.64
 AREA-AVERAGED F_m (INCH/HR) = 0.11
 AREA-AVERAGED F_p (INCH/HR) = 0.30
 AREA-AVERAGED A_p = 0.35
 EFFECTIVE STREAM AREA (ACRES) = 0.18
 TOTAL STREAM AREA (ACRES) = 0.18
 PEAK FLOW RATE (CFS) AT CONFLUENCE = 0.25

** CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	T_c (MIN.)	Intensity (INCH/HR)	F_p (F_m) (INCH/HR)	A_p	A_e (ACRES)	HEADWATER NODE
1	6.87	8.51	1.669	0.32 (0.10)	0.31	4.2	30.00
1	7.19	10.77	1.457	0.32 (0.10)	0.31	5.0	14.00
1	7.21	10.85	1.451	0.32 (0.10)	0.31	5.1	27.00
1	7.22	10.94	1.444	0.32 (0.10)	0.31	5.1	26.00
1	7.28	11.63	1.394	0.32 (0.10)	0.31	5.4	20.00
1	7.27	11.95	1.373	0.32 (0.10)	0.31	5.5	23.00
1	7.24	12.11	1.362	0.32 (0.10)	0.31	5.5	29.00
1	7.24	12.14	1.361	0.32 (0.10)	0.31	5.5	17.00
1	6.90	13.88	1.260	0.32 (0.10)	0.30	5.8	24.00
1	6.06	18.14	1.080	0.32 (0.09)	0.29	6.2	12.00
1	5.38	21.99	0.967	0.32 (0.09)	0.29	6.2	10.00
2	0.90	9.14	1.601	0.30 (0.11)	0.35	0.7	30.00
3	0.54	12.87	1.316	0.30 (0.11)	0.35	0.5	34.00
4	0.39	9.26	1.589	0.30 (0.11)	0.35	0.3	36.00
5	0.25	8.74	1.643	0.30 (0.11)	0.35	0.2	38.00

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
 CONFLUENCE FORMULA USED FOR 5 STREAMS.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	T_c (MIN.)	Intensity (INCH/HR)	F_p (F_m) (INCH/HR)	A_p	A_e (ACRES)	HEADWATER NODE
1	6.87	8.51	1.669	0.32 (0.10)	0.31	4.2	30.00
1	7.19	10.77	1.457	0.32 (0.10)	0.31	5.0	14.00
1	7.21	10.85	1.451	0.32 (0.10)	0.31	5.1	27.00
1	7.22	10.94	1.444	0.32 (0.10)	0.31	5.1	26.00
1	7.28	11.63	1.394	0.32 (0.10)	0.31	5.4	20.00
1	7.27	11.95	1.373	0.32 (0.10)	0.31	5.5	23.00
1	7.24	12.11	1.362	0.32 (0.10)	0.31	5.5	29.00
1	7.24	12.14	1.361	0.32 (0.10)	0.31	5.5	17.00
1	6.90	13.88	1.260	0.32 (0.10)	0.30	5.8	24.00
1	6.06	18.14	1.080	0.32 (0.09)	0.29	6.2	12.00
1	5.38	21.99	0.967	0.32 (0.09)	0.29	6.2	10.00
2	0.90	9.14	1.601	0.30 (0.11)	0.35	0.7	30.00
3	0.54	12.87	1.316	0.30 (0.11)	0.35	0.5	34.00
4	0.39	9.26	1.589	0.30 (0.11)	0.35	0.3	36.00
5	0.25	8.74	1.643	0.30 (0.11)	0.35	0.2	38.00

1	8.83	8.51	1.669	0.31	(0.10)	0.32	5.6	30.00
2	8.89	8.74	1.643	0.31	(0.10)	0.32	5.7	38.00
3	8.97	9.14	1.601	0.31	(0.10)	0.32	5.9	30.00
4	8.98	9.26	1.589	0.31	(0.10)	0.32	6.0	36.00
5	9.09	10.77	1.457	0.31	(0.10)	0.32	6.6	14.00
6	9.10	10.85	1.451	0.31	(0.10)	0.32	6.6	27.00
7	9.10	10.94	1.444	0.31	(0.10)	0.32	6.7	26.00
8	9.13	11.63	1.394	0.31	(0.10)	0.32	7.0	20.00
9	9.10	11.95	1.373	0.31	(0.10)	0.32	7.1	23.00
10	9.07	12.11	1.362	0.31	(0.10)	0.32	7.2	29.00
11	9.06	12.14	1.361	0.31	(0.10)	0.32	7.2	17.00
12	8.89	12.87	1.316	0.31	(0.10)	0.32	7.3	34.00
13	8.60	13.88	1.260	0.31	(0.10)	0.31	7.4	24.00
14	7.50	18.14	1.080	0.31	(0.09)	0.30	7.8	12.00
15	6.66	21.99	0.967	0.31	(0.09)	0.30	7.9	10.00

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE (CFS) = 9.13 Tc (MIN.) = 11.63
EFFECTIVE AREA (ACRES) = 7.00 AREA-AVERAGED Fm (INCH/HR) = 0.10
AREA-AVERAGED Fp (INCH/HR) = 0.31 AREA-AVERAGED Ap = 0.32
TOTAL AREA (ACRES) = 7.9
LONGEST FLOWPATH FROM NODE 10.00 TO NODE 33.00 = 1705.00 FEET.

FLOW PROCESS FROM NODE 33.00 TO NODE 38.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<

>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW) <<<<<

=====

ELEVATION DATA: UPSTREAM (FEET) = 150.50 DOWNSTREAM (FEET) = 150.20
FLOW LENGTH (FEET) = 150.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 24.0 INCH PIPE IS 18.2 INCHES
PIPE-FLOW VELOCITY (FEET/SEC.) = 3.56
ESTIMATED PIPE DIAMETER (INCH) = 24.00 NUMBER OF PIPES = 1
PIPE-FLOW (CFS) = 9.13
PIPE TRAVEL TIME (MIN.) = 0.70 Tc (MIN.) = 12.34
LONGEST FLOWPATH FROM NODE 10.00 TO NODE 38.00 = 1855.00 FEET.

FLOW PROCESS FROM NODE 38.00 TO NODE 38.00 IS CODE = 10

>>>>MAIN-STREAM MEMORY COPIED ONTO MEMORY BANK # 1 <<<<<

FLOW PROCESS FROM NODE 39.00 TO NODE 40.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<

>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

=====

INITIAL SUBAREA FLOW-LENGTH (FEET) = 300.00
ELEVATION DATA: UPSTREAM (FEET) = 168.00 DOWNSTREAM (FEET) = 166.90

$$Tc = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$$

SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) = 10.822
 * 2 YEAR RAINFALL INTENSITY(INCH/HR) = 1.453
 SUBAREA Tc AND LOSS RATE DATA(AMC I):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN	Tc (MIN.)
CONDOMINIUMS	B	0.80	0.30	0.350	36	10.82

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.30
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.350
 SUBAREA RUNOFF(CFS) = 0.97
 TOTAL AREA(ACRES) = 0.80 PEAK FLOW RATE(CFS) = 0.97

 FLOW PROCESS FROM NODE 40.00 TO NODE 41.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
 >>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 154.00 DOWNSTREAM(FEET) = 151.10
 FLOW LENGTH(FEET) = 40.00 MANNING'S N = 0.013
 ESTIMATED PIPE DIAMETER(INCH) INCREASED TO 12.000
 DEPTH OF FLOW IN 12.0 INCH PIPE IS 2.6 INCHES
 PIPE-FLOW VELOCITY(FEET/SEC.) = 7.69
 ESTIMATED PIPE DIAMETER(INCH) = 12.000 NUMBER OF PIPES = 1
 PIPE-FLOW(CFS) = 0.97
 PIPE TRAVEL TIME(MIN.) = 0.09 Tc(MIN.) = 10.91
 LONGEST FLOWPATH FROM NODE 39.00 TO NODE 41.00 = 340.00 FEET.

 FLOW PROCESS FROM NODE 41.00 TO NODE 41.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

=====

MAINLINE Tc(MIN.) = 10.91
 * 2 YEAR RAINFALL INTENSITY(INCH/HR) = 1.447
 SUBAREA LOSS RATE DATA(AMC I):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
CONDOMINIUMS	B	0.45	0.30	0.350	36

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.30
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.350
 SUBAREA AREA(ACRES) = 0.45 SUBAREA RUNOFF(CFS) = 0.54
 EFFECTIVE AREA(ACRES) = 1.25 AREA-AVERAGED Fm(INCH/HR) = 0.10
 AREA-AVERAGED Fp(INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.35
 TOTAL AREA(ACRES) = 1.2 PEAK FLOW RATE(CFS) = 1.51

 FLOW PROCESS FROM NODE 41.00 TO NODE 42.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
 >>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 151.10 DOWNSTREAM(FEET) = 150.60
 FLOW LENGTH(FEET) = 270.00 MANNING'S N = 0.013
 DEPTH OF FLOW IN 15.0 INCH PIPE IS 8.0 INCHES

PIPE-FLOW VELOCITY (FEET/SEC.) = 2.27
 ESTIMATED PIPE DIAMETER (INCH) = 15.00 NUMBER OF PIPES = 1
 PIPE-FLOW (CFS) = 1.51
 PIPE TRAVEL TIME (MIN.) = 1.98 Tc (MIN.) = 12.89
 LONGEST FLOWPATH FROM NODE 39.00 TO NODE 42.00 = 610.00 FEET.

FLOW PROCESS FROM NODE 42.00 TO NODE 42.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<

MAINLINE Tc (MIN.) = 12.89
 * 2 YEAR RAINFALL INTENSITY (INCH/HR) = 1.314
 SUBAREA LOSS RATE DATA (AMC I):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
CONDOMINIUMS	B	1.15	0.30	0.350	36

 SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp (INCH/HR) = 0.30
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.350
 SUBAREA AREA (ACRES) = 1.15 SUBAREA RUNOFF (CFS) = 1.25
 EFFECTIVE AREA (ACRES) = 2.40 AREA-AVERAGED Fm (INCH/HR) = 0.10
 AREA-AVERAGED Fp (INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.35
 TOTAL AREA (ACRES) = 2.4 PEAK FLOW RATE (CFS) = 2.61

FLOW PROCESS FROM NODE 42.00 TO NODE 42.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<

TOTAL NUMBER OF STREAMS = 3
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
 TIME OF CONCENTRATION (MIN.) = 12.89
 RAINFALL INTENSITY (INCH/HR) = 1.31
 AREA-AVERAGED Fm (INCH/HR) = 0.10
 AREA-AVERAGED Fp (INCH/HR) = 0.30
 AREA-AVERAGED Ap = 0.35
 EFFECTIVE STREAM AREA (ACRES) = 2.40
 TOTAL STREAM AREA (ACRES) = 2.40
 PEAK FLOW RATE (CFS) AT CONFLUENCE = 2.61

FLOW PROCESS FROM NODE 43.00 TO NODE 44.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<
 >>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

INITIAL SUBAREA FLOW-LENGTH (FEET) = 180.00
 ELEVATION DATA: UPSTREAM (FEET) = 168.00 DOWNSTREAM (FEET) = 166.20

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$
 SUBAREA ANALYSIS USED MINIMUM Tc (MIN.) = 7.218
 * 2 YEAR RAINFALL INTENSITY (INCH/HR) = 1.834
 SUBAREA Tc AND LOSS RATE DATA (AMC I):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN	Tc (MIN.)

LAND USE	GROUP	(ACRES)	(INCH/HR)	(DECIMAL)	CN	(MIN.)
CONDOMINIUMS	B	0.44	0.30	0.350	36	7.22

SUBAREA AVERAGE PERVIOUS LOSS RATE, F_p (INCH/HR) = 0.30
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, A_p = 0.350
 SUBAREA RUNOFF (CFS) = 0.68
 TOTAL AREA (ACRES) = 0.44 PEAK FLOW RATE (CFS) = 0.68

FLOW PROCESS FROM NODE 44.00 TO NODE 42.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
 >>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<

ELEVATION DATA: UPSTREAM (FEET) = 153.00 DOWNSTREAM (FEET) = 150.60
 FLOW LENGTH (FEET) = 75.00 MANNING'S N = 0.013
 ESTIMATED PIPE DIAMETER (INCH) INCREASED TO 12.000
 DEPTH OF FLOW IN 12.0 INCH PIPE IS 2.7 INCHES
 PIPE-FLOW VELOCITY (FEET/SEC.) = 5.21
 ESTIMATED PIPE DIAMETER (INCH) = 12.000 NUMBER OF PIPES = 1
 PIPE-FLOW (CFS) = 0.68
 PIPE TRAVEL TIME (MIN.) = 0.24 T_c (MIN.) = 7.46
 LONGEST FLOWPATH FROM NODE 43.00 TO NODE 42.00 = 255.00 FEET.

FLOW PROCESS FROM NODE 42.00 TO NODE 42.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

TOTAL NUMBER OF STREAMS = 3
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
 TIME OF CONCENTRATION (MIN.) = 7.46
 RAINFALL INTENSITY (INCH/HR) = 1.80
 AREA-AVERAGED F_m (INCH/HR) = 0.10
 AREA-AVERAGED F_p (INCH/HR) = 0.30
 AREA-AVERAGED A_p = 0.35
 EFFECTIVE STREAM AREA (ACRES) = 0.44
 TOTAL STREAM AREA (ACRES) = 0.44
 PEAK FLOW RATE (CFS) AT CONFLUENCE = 0.68

FLOW PROCESS FROM NODE 45.00 TO NODE 46.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
 >>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

INITIAL SUBAREA FLOW-LENGTH (FEET) = 180.00
 ELEVATION DATA: UPSTREAM (FEET) = 168.00 DOWNSTREAM (FEET) = 166.40

$$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$$

SUBAREA ANALYSIS USED MINIMUM T_c (MIN.) = 7.390
 * 2 YEAR RAINFALL INTENSITY (INCH/HR) = 1.809

SUBAREA T_c AND LOSS RATE DATA (AMC I):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	F_p (INCH/HR)	A_p (DECIMAL)	SCS CN	T_c (MIN.)
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CONDOMINIUMS B 0.45 0.30 0.350 36 7.39
 SUBAREA AVERAGE PERVIOUS LOSS RATE, F_p (INCH/HR) = 0.30
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, A_p = 0.350
 SUBAREA RUNOFF (CFS) = 0.69
 TOTAL AREA (ACRES) = 0.45 PEAK FLOW RATE (CFS) = 0.69

 FLOW PROCESS FROM NODE 46.00 TO NODE 42.00 IS CODE = 31

>>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
 >>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<

=====
 ELEVATION DATA: UPSTREAM(FEET) = 153.00 DOWNSTREAM(FEET) = 150.60
 FLOW LENGTH(FEET) = 75.00 MANNING'S N = 0.013
 ESTIMATED PIPE DIAMETER(INCH) INCREASED TO 12.000
 DEPTH OF FLOW IN 12.0 INCH PIPE IS 2.7 INCHES
 PIPE-FLOW VELOCITY(FEET/SEC.) = 5.22
 ESTIMATED PIPE DIAMETER(INCH) = 12.00 NUMBER OF PIPES = 1
 PIPE-FLOW(CFS) = 0.69
 PIPE TRAVEL TIME(MIN.) = 0.24 T_c (MIN.) = 7.63
 LONGEST FLOWPATH FROM NODE 45.00 TO NODE 42.00 = 255.00 FEET.

 FLOW PROCESS FROM NODE 42.00 TO NODE 42.00 IS CODE = 1

>>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
 >>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<

=====
 TOTAL NUMBER OF STREAMS = 3
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 3 ARE:
 TIME OF CONCENTRATION(MIN.) = 7.63
 RAINFALL INTENSITY(INCH/HR) = 1.78
 AREA-AVERAGED F_m (INCH/HR) = 0.10
 AREA-AVERAGED F_p (INCH/HR) = 0.30
 AREA-AVERAGED A_p = 0.35
 EFFECTIVE STREAM AREA(ACRES) = 0.45
 TOTAL STREAM AREA(ACRES) = 0.45
 PEAK FLOW RATE(CFS) AT CONFLUENCE = 0.69

** CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	T_c (MIN.)	Intensity (INCH/HR)	$F_p(F_m)$ (INCH/HR)	A_p	A_e (ACRES)	HEADWATER NODE
1	2.61	12.89	1.314	0.30 (0.10)	0.35	2.4	39.00
2	0.68	7.46	1.799	0.30 (0.11)	0.35	0.4	43.00
3	0.69	7.63	1.776	0.30 (0.10)	0.35	0.4	45.00

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
 CONFLUENCE FORMULA USED FOR 3 STREAMS.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	T_c (MIN.)	Intensity (INCH/HR)	$F_p(F_m)$ (INCH/HR)	A_p	A_e (ACRES)	HEADWATER NODE
1	3.49	7.46	1.799	0.30 (0.10)	0.35	2.3	43.00
2	3.50	7.63	1.776	0.30 (0.10)	0.35	2.3	45.00

3 3.60 12.89 1.314 0.30 (0.10) 0.35 3.3 39.00

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE (CFS) = 3.60 Tc (MIN.) = 12.89
EFFECTIVE AREA (ACRES) = 3.29 AREA-AVERAGED Fm (INCH/HR) = 0.10
AREA-AVERAGED Fp (INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.35
TOTAL AREA (ACRES) = 3.3
LONGEST FLOWPATH FROM NODE 39.00 TO NODE 42.00 = 610.00 FEET.

FLOW PROCESS FROM NODE 42.00 TO NODE 38.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<

=====

ELEVATION DATA: UPSTREAM (FEET) = 150.60 DOWNSTREAM (FEET) = 150.20
FLOW LENGTH (FEET) = 190.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 18.0 INCH PIPE IS 11.8 INCHES
PIPE-FLOW VELOCITY (FEET/SEC.) = 2.93
ESTIMATED PIPE DIAMETER (INCH) = 18.00 NUMBER OF PIPES = 1
PIPE-FLOW (CFS) = 3.60
PIPE TRAVEL TIME (MIN.) = 1.08 Tc (MIN.) = 13.97
LONGEST FLOWPATH FROM NODE 39.00 TO NODE 38.00 = 800.00 FEET.

FLOW PROCESS FROM NODE 38.00 TO NODE 38.00 IS CODE = 11

>>>>CONFLUENCE MEMORY BANK # 1 WITH THE MAIN-STREAM MEMORY<<<<<

** MAIN STREAM CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp (Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	3.49	8.55	1.664	0.30 (0.10)	0.35	2.3	43.00
2	3.50	8.72	1.645	0.30 (0.10)	0.35	2.3	45.00
3	3.60	13.97	1.255	0.30 (0.10)	0.35	3.3	39.00

LONGEST FLOWPATH FROM NODE 39.00 TO NODE 38.00 = 800.00 FEET.

** MEMORY BANK # 1 CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp (Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	8.83	9.21	1.594	0.31 (0.10)	0.32	5.6	30.00
2	8.89	9.44	1.571	0.31 (0.10)	0.32	5.7	38.00
3	8.97	9.84	1.534	0.31 (0.10)	0.32	5.9	30.00
4	8.98	9.96	1.524	0.31 (0.10)	0.32	6.0	36.00
5	9.09	11.47	1.405	0.31 (0.10)	0.32	6.6	14.00
6	9.10	11.56	1.399	0.31 (0.10)	0.32	6.6	27.00
7	9.10	11.64	1.394	0.31 (0.10)	0.32	6.7	26.00
8	9.13	12.34	1.348	0.31 (0.10)	0.32	7.0	20.00
9	9.10	12.65	1.329	0.31 (0.10)	0.32	7.1	23.00
10	9.07	12.81	1.319	0.31 (0.10)	0.32	7.2	29.00
11	9.06	12.84	1.317	0.31 (0.10)	0.32	7.2	17.00
12	8.89	13.57	1.276	0.31 (0.10)	0.32	7.3	34.00
13	8.60	14.59	1.224	0.31 (0.10)	0.31	7.4	24.00

14 7.50 18.86 1.056 0.31(0.09) 0.30 7.8 12.00
 15 6.66 22.75 0.949 0.31(0.09) 0.30 7.9 10.00
 LONGEST FLOWPATH FROM NODE 10.00 TO NODE 38.00 = 1855.00 FEET.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE	
1	12.07	8.55	1.664	0.31(0.10)	0.33	7.4	43.00	
2	12.15	8.72	1.645	0.31(0.10)	0.33	7.6	45.00	
3	12.35	9.21	1.594	0.31(0.10)	0.33	8.0	30.00	
4	12.40	9.44	1.571	0.31(0.10)	0.33	8.2	38.00	
5	12.49	9.84	1.534	0.31(0.10)	0.33	8.4	30.00	
6	12.51	9.96	1.524	0.31(0.10)	0.33	8.5	36.00	
7	12.64	11.47	1.405	0.31(0.10)	0.33	9.4	14.00	
8	12.65	11.56	1.399	0.31(0.10)	0.33	9.5	27.00	
9	12.66	11.64	1.394	0.31(0.10)	0.33	9.5	26.00	
10	12.70	12.34	1.348	0.31(0.10)	0.33	10.0	20.00	
11	12.67	12.65	1.329	0.31(0.10)	0.33	10.2	23.00	
12	12.64	12.81	1.319	0.31(0.10)	0.33	10.2	29.00	
13	12.64	12.84	1.317	0.31(0.10)	0.33	10.2	17.00	
14	12.48	13.57	1.276	0.31(0.10)	0.33	10.5	34.00	
15	12.38	13.97	1.255	0.31(0.10)	0.33	10.6	39.00	
16	12.11	14.59	1.224	0.31(0.10)	0.33	10.7	24.00	
17	10.48	18.86	1.056	0.31(0.10)	0.32	11.1	12.00	
18	9.30	22.75	0.949	0.31(0.10)	0.32	11.2	10.00	
TOTAL AREA (ACRES) =			11.2					

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE (CFS) = 12.70 Tc (MIN.) = 12.336
 EFFECTIVE AREA (ACRES) = 9.99 AREA-AVERAGED Fm (INCH/HR) = 0.10
 AREA-AVERAGED Fp (INCH/HR) = 0.31 AREA-AVERAGED Ap = 0.33
 TOTAL AREA (ACRES) = 11.2
 LONGEST FLOWPATH FROM NODE 10.00 TO NODE 38.00 = 1855.00 FEET.

 FLOW PROCESS FROM NODE 38.00 TO NODE 38.00 IS CODE = 12

>>>>CLEAR MEMORY BANK # 1 <<<<<

 FLOW PROCESS FROM NODE 38.00 TO NODE 47.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
 >>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<

ELEVATION DATA: UPSTREAM (FEET) = 150.20 DOWNSTREAM (FEET) = 149.90
 FLOW LENGTH (FEET) = 150.00 MANNING'S N = 0.013
 DEPTH OF FLOW IN 27.0 INCH PIPE IS 20.8 INCHES
 PIPE-FLOW VELOCITY (FEET/SEC.) = 3.86
 ESTIMATED PIPE DIAMETER (INCH) = 27.00 NUMBER OF PIPES = 1
 PIPE-FLOW (CFS) = 12.70
 PIPE TRAVEL TIME (MIN.) = 0.65 Tc (MIN.) = 12.98
 LONGEST FLOWPATH FROM NODE 10.00 TO NODE 47.00 = 2005.00 FEET.

FLOW PROCESS FROM NODE 47.00 TO NODE 47.00 IS CODE = 10

>>>>MAIN-STREAM MEMORY COPIED ONTO MEMORY BANK # 1 <<<<<
=====

FLOW PROCESS FROM NODE 32.00 TO NODE 48.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<
=====

INITIAL SUBAREA FLOW-LENGTH (FEET) = 300.00
ELEVATION DATA: UPSTREAM (FEET) = 166.30 DOWNSTREAM (FEET) = 166.00

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$
SUBAREA ANALYSIS USED MINIMUM T_c (MIN.) = 11.850
* 2 YEAR RAINFALL INTENSITY (INCH/HR) = 1.379

SUBAREA T_c AND LOSS RATE DATA (AMC I):
DEVELOPMENT TYPE/ SCS SOIL AREA F_p A_p SCS T_c
LAND USE GROUP (ACRES) (INCH/HR) (DECIMAL) CN (MIN.)
COMMERCIAL B 2.62 0.30 0.100 36 11.85
SUBAREA AVERAGE PERVIOUS LOSS RATE, F_p (INCH/HR) = 0.30
SUBAREA AVERAGE PERVIOUS AREA FRACTION, A_p = 0.100
SUBAREA RUNOFF (CFS) = 3.18
TOTAL AREA (ACRES) = 2.62 PEAK FLOW RATE (CFS) = 3.18

FLOW PROCESS FROM NODE 48.00 TO NODE 49.00 IS CODE = 61

>>>>COMPUTE STREET FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>(STANDARD CURB SECTION USED)<<<<<
=====

UPSTREAM ELEVATION (FEET) = 166.00 DOWNSTREAM ELEVATION (FEET) = 165.60
STREET LENGTH (FEET) = 485.00 CURB HEIGHT (INCHES) = 6.0
STREET HALFWIDTH (FEET) = 25.00

DISTANCE FROM CROWN TO CROSSFALL GRADEBREAK (FEET) = 10.00
INSIDE STREET CROSSFALL (DECIMAL) = 0.020
OUTSIDE STREET CROSSFALL (DECIMAL) = 0.020

SPECIFIED NUMBER OF HALFSTREETS CARRYING RUNOFF = 2
STREET PARKWAY CROSSFALL (DECIMAL) = 0.020
Manning's FRICTION FACTOR for Streetflow Section (curb-to-curb) = 0.0150
Manning's FRICTION FACTOR for Back-of-Walk Flow Section = 0.0200

**TRAVEL TIME COMPUTED USING ESTIMATED FLOW (CFS) = 5.32
STREETFLOW MODEL RESULTS USING ESTIMATED FLOW:
STREET FLOW DEPTH (FEET) = 0.47
HALFSTREET FLOOD WIDTH (FEET) = 17.07
AVERAGE FLOW VELOCITY (FEET/SEC.) = 0.88
PRODUCT OF DEPTH&VELOCITY (FT*FT/SEC.) = 0.41
STREET FLOW TRAVEL TIME (MIN.) = 9.21 T_c (MIN.) = 21.06

* 2 YEAR RAINFALL INTENSITY (INCH/HR) = 0.992
SUBAREA LOSS RATE DATA (AMC I):
DEVELOPMENT TYPE/ SCS SOIL AREA Fp Ap SCS
LAND USE GROUP (ACRES) (INCH/HR) (DECIMAL) CN
COMMERCIAL B 4.89 0.30 0.100 36
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp (INCH/HR) = 0.30
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.100
SUBAREA AREA (ACRES) = 4.89 SUBAREA RUNOFF (CFS) = 4.23
EFFECTIVE AREA (ACRES) = 7.51 AREA-AVERAGED Fm (INCH/HR) = 0.03
AREA-AVERAGED Fp (INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.10
TOTAL AREA (ACRES) = 7.5 PEAK FLOW RATE (CFS) = 6.50

END OF SUBAREA STREET FLOW HYDRAULICS:
DEPTH (FEET) = 0.49 HALFSTREET FLOOD WIDTH (FEET) = 18.40
FLOW VELOCITY (FEET/SEC.) = 0.93 DEPTH*VELOCITY (FT*FT/SEC.) = 0.46
LONGEST FLOWPATH FROM NODE 32.00 TO NODE 49.00 = 785.00 FEET.

FLOW PROCESS FROM NODE 49.00 TO NODE 50.00 IS CODE = 51

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<<
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM (FEET) = 165.60 DOWNSTREAM (FEET) = 165.40
CHANNEL LENGTH THRU SUBAREA (FEET) = 180.00 CHANNEL SLOPE = 0.0011
CHANNEL BASE (FEET) = 5.00 "Z" FACTOR = 0.020
MANNING'S FACTOR = 0.015 MAXIMUM DEPTH (FEET) = 2.00
CHANNEL FLOW THRU SUBAREA (CFS) = 6.50
FLOW VELOCITY (FEET/SEC.) = 2.08 FLOW DEPTH (FEET) = 0.62
TRAVEL TIME (MIN.) = 1.44 Tc (MIN.) = 22.50
LONGEST FLOWPATH FROM NODE 32.00 TO NODE 50.00 = 965.00 FEET.

FLOW PROCESS FROM NODE 50.00 TO NODE 50.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

=====

TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION (MIN.) = 22.50
RAINFALL INTENSITY (INCH/HR) = 0.95
AREA-AVERAGED Fm (INCH/HR) = 0.03
AREA-AVERAGED Fp (INCH/HR) = 0.30
AREA-AVERAGED Ap = 0.10
EFFECTIVE STREAM AREA (ACRES) = 7.51
TOTAL STREAM AREA (ACRES) = 7.51
PEAK FLOW RATE (CFS) AT CONFLUENCE = 6.50

FLOW PROCESS FROM NODE 50.00 TO NODE 50.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

=====

INITIAL SUBAREA FLOW-LENGTH (FEET) = 665.00
 ELEVATION DATA: UPSTREAM (FEET) = 167.20 DOWNSTREAM (FEET) = 165.40

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$
 SUBAREA ANALYSIS USED MINIMUM T_c (MIN.) = 13.351
 * 2 YEAR RAINFALL INTENSITY (INCH/HR) = 1.288

SUBAREA T_c AND LOSS RATE DATA (AMC I):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	F_p (INCH/HR)	A_p (DECIMAL)	SCS CN	T_c (MIN.)
COMMERCIAL	B	6.31	0.30	0.100	36	13.35

SUBAREA AVERAGE PERVIOUS LOSS RATE, F_p (INCH/HR) = 0.30

SUBAREA AVERAGE PERVIOUS AREA FRACTION, A_p = 0.100

SUBAREA RUNOFF (CFS) = 7.15

TOTAL AREA (ACRES) = 6.31 PEAK FLOW RATE (CFS) = 7.15

FLOW PROCESS FROM NODE 50.00 TO NODE 50.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<<
 >>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<<

=====

TOTAL NUMBER OF STREAMS = 2
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
 TIME OF CONCENTRATION (MIN.) = 13.35
 RAINFALL INTENSITY (INCH/HR) = 1.29
 AREA-AVERAGED F_m (INCH/HR) = 0.03
 AREA-AVERAGED F_p (INCH/HR) = 0.30
 AREA-AVERAGED A_p = 0.10
 EFFECTIVE STREAM AREA (ACRES) = 6.31
 TOTAL STREAM AREA (ACRES) = 6.31
 PEAK FLOW RATE (CFS) AT CONFLUENCE = 7.15

** CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	T_c (MIN.)	Intensity (INCH/HR)	F_p (F_m) (INCH/HR)	A_p	A_e (ACRES)	HEADWATER NODE
1	6.50	22.50	0.955	0.30 (0.03)	0.10	7.5	32.00
2	7.15	13.35	1.288	0.30 (0.03)	0.10	6.3	50.00

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
 CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	T_c (MIN.)	Intensity (INCH/HR)	F_p (F_m) (INCH/HR)	A_p	A_e (ACRES)	HEADWATER NODE
1	12.39	13.35	1.288	0.30 (0.03)	0.10	10.8	50.00
2	11.75	22.50	0.955	0.30 (0.03)	0.10	13.8	32.00

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE (CFS) = 12.39 T_c (MIN.) = 13.35
 EFFECTIVE AREA (ACRES) = 10.77 AREA-AVERAGED F_m (INCH/HR) = 0.03
 AREA-AVERAGED F_p (INCH/HR) = 0.30 AREA-AVERAGED A_p = 0.10
 TOTAL AREA (ACRES) = 13.8
 LONGEST FLOWPATH FROM NODE 32.00 TO NODE 50.00 = 965.00 FEET.

FLOW PROCESS FROM NODE 50.00 TO NODE 47.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<

ELEVATION DATA: UPSTREAM(FEET) = 162.40 DOWNSTREAM(FEET) = 149.90
FLOW LENGTH(FEET) = 185.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 15.0 INCH PIPE IS 9.8 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 14.67
ESTIMATED PIPE DIAMETER(INCH) = 15.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 12.39
PIPE TRAVEL TIME(MIN.) = 0.21 Tc(MIN.) = 13.56
LONGEST FLOWPATH FROM NODE 32.00 TO NODE 47.00 = 1150.00 FEET.

FLOW PROCESS FROM NODE 47.00 TO NODE 47.00 IS CODE = 11

>>>>CONFLUENCE MEMORY BANK # 1 WITH THE MAIN-STREAM MEMORY<<<<<

** MAIN STREAM CONFLUENCE DATA **

Table with 8 columns: STREAM NUMBER, Q (CFS), Tc (MIN.), Intensity (INCH/HR), Fp(Fm) (INCH/HR), Ap, Ae (ACRES), HEADWATER NODE. Contains 2 rows of data and a summary line for longest flowpath.

** MEMORY BANK # 1 CONFLUENCE DATA **

Table with 8 columns: STREAM NUMBER, Q (CFS), Tc (MIN.), Intensity (INCH/HR), Fp(Fm) (INCH/HR), Ap, Ae (ACRES), HEADWATER NODE. Contains 18 rows of data and a summary line for longest flowpath.

** PEAK FLOW RATE TABLE **

Table with 8 columns: STREAM NUMBER, Q (CFS), Tc (MIN.), Intensity (INCH/HR), Fp(Fm) (INCH/HR), Ap, Ae (ACRES), HEADWATER NODE.

1	22.62	9.20	1.596	0.31 (0.07)	0.22	14.7	43.00
2	22.78	9.37	1.579	0.31 (0.07)	0.22	15.0	45.00
3	23.21	9.86	1.533	0.31 (0.07)	0.22	15.8	30.00
4	23.37	10.09	1.513	0.31 (0.07)	0.22	16.2	38.00
5	23.64	10.49	1.479	0.31 (0.07)	0.22	16.8	30.00
6	23.70	10.61	1.470	0.31 (0.07)	0.22	16.9	36.00
7	24.47	12.12	1.362	0.31 (0.07)	0.21	19.0	14.00
8	24.52	12.20	1.356	0.31 (0.07)	0.21	19.2	27.00
9	24.56	12.29	1.351	0.31 (0.07)	0.21	19.3	26.00
10	24.87	12.98	1.309	0.31 (0.07)	0.21	20.3	20.00
11	24.96	13.30	1.291	0.31 (0.06)	0.21	20.7	23.00
12	25.00	13.46	1.282	0.31 (0.06)	0.21	20.9	29.00
13	25.00	13.49	1.281	0.31 (0.06)	0.21	20.9	17.00
14	25.01	13.56	1.277	0.31 (0.06)	0.21	21.0	50.00
15	24.82	14.22	1.242	0.31 (0.06)	0.21	21.5	34.00
16	24.69	14.62	1.223	0.31 (0.06)	0.21	21.8	39.00
17	24.38	15.24	1.194	0.31 (0.06)	0.21	22.0	24.00
18	22.45	19.53	1.036	0.31 (0.06)	0.20	23.9	12.00
19	21.27	22.72	0.949	0.31 (0.06)	0.20	25.0	32.00
20	20.83	23.45	0.932	0.31 (0.06)	0.20	25.0	10.00
TOTAL AREA (ACRES) =			25.0				

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE (CFS) = 25.01 Tc (MIN.) = 13.561
EFFECTIVE AREA (ACRES) = 21.03 AREA-AVERAGED Fm (INCH/HR) = 0.06
AREA-AVERAGED Fp (INCH/HR) = 0.31 AREA-AVERAGED Ap = 0.22
TOTAL AREA (ACRES) = 25.0
LONGEST FLOWPATH FROM NODE 10.00 TO NODE 47.00 = 2005.00 FEET.

FLOW PROCESS FROM NODE 47.00 TO NODE 47.00 IS CODE = 12

>>>>CLEAR MEMORY BANK # 1 <<<<<

FLOW PROCESS FROM NODE 47.00 TO NODE 47.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

TOTAL NUMBER OF STREAMS = 2

CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:

TIME OF CONCENTRATION (MIN.) = 13.56
RAINFALL INTENSITY (INCH/HR) = 1.28
AREA-AVERAGED Fm (INCH/HR) = 0.06
AREA-AVERAGED Fp (INCH/HR) = 0.31
AREA-AVERAGED Ap = 0.21
EFFECTIVE STREAM AREA (ACRES) = 21.03
TOTAL STREAM AREA (ACRES) = 25.00
PEAK FLOW RATE (CFS) AT CONFLUENCE = 25.01

FLOW PROCESS FROM NODE 51.00 TO NODE 52.00 IS CODE = 21

>>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

=====

INITIAL SUBAREA FLOW-LENGTH (FEET) = 140.00
ELEVATION DATA: UPSTREAM (FEET) = 166.70 DOWNSTREAM (FEET) = 166.30

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$
SUBAREA ANALYSIS USED MINIMUM T_c (MIN.) = 8.386
* 2 YEAR RAINFALL INTENSITY (INCH/HR) = 1.682

SUBAREA T_c AND LOSS RATE DATA (AMC I):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	F_p (INCH/HR)	A_p (DECIMAL)	SCS CN	T_c (MIN.)
CONDOMINIUMS	B	0.64	0.30	0.350	36	8.39

SUBAREA AVERAGE PERVIOUS LOSS RATE, F_p (INCH/HR) = 0.30

SUBAREA AVERAGE PERVIOUS AREA FRACTION, A_p = 0.350

SUBAREA RUNOFF (CFS) = 0.91

TOTAL AREA (ACRES) = 0.64 PEAK FLOW RATE (CFS) = 0.91

FLOW PROCESS FROM NODE 52.00 TO NODE 47.00 IS CODE = 31

>>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<

>>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<

=====

ELEVATION DATA: UPSTREAM (FEET) = 156.00 DOWNSTREAM (FEET) = 149.90

FLOW LENGTH (FEET) = 40.00 MANNING'S N = 0.013

ESTIMATED PIPE DIAMETER (INCH) INCREASED TO 12.000

DEPTH OF FLOW IN 12.0 INCH PIPE IS 2.1 INCHES

PIPE-FLOW VELOCITY (FEET/SEC.) = 9.80

ESTIMATED PIPE DIAMETER (INCH) = 12.000 NUMBER OF PIPES = 1

PIPE-FLOW (CFS) = 0.91

PIPE TRAVEL TIME (MIN.) = 0.07 T_c (MIN.) = 8.45

LONGEST FLOWPATH FROM NODE 51.00 TO NODE 47.00 = 180.00 FEET.

FLOW PROCESS FROM NODE 47.00 TO NODE 47.00 IS CODE = 1

>>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

>>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<

=====

TOTAL NUMBER OF STREAMS = 2

CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:

TIME OF CONCENTRATION (MIN.) = 8.45

RAINFALL INTENSITY (INCH/HR) = 1.67

AREA-AVERAGED F_m (INCH/HR) = 0.10

AREA-AVERAGED F_p (INCH/HR) = 0.30

AREA-AVERAGED A_p = 0.35

EFFECTIVE STREAM AREA (ACRES) = 0.64

TOTAL STREAM AREA (ACRES) = 0.64

PEAK FLOW RATE (CFS) AT CONFLUENCE = 0.91

** CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	T_c (MIN.)	Intensity (INCH/HR)	F_p (F_m) (INCH/HR)	A_p	A_e (ACRES)	HEADWATER NODE
------------------	------------	-----------------	------------------------	------------------------------	-------	------------------	-------------------

1	22.62	9.20	1.596	0.31 (0.07)	0.22	14.7	43.00
1	22.78	9.37	1.579	0.31 (0.07)	0.22	15.0	45.00
1	23.21	9.86	1.533	0.31 (0.07)	0.22	15.8	30.00
1	23.37	10.09	1.513	0.31 (0.07)	0.22	16.2	38.00
1	23.64	10.49	1.479	0.31 (0.07)	0.22	16.8	30.00
1	23.70	10.61	1.470	0.31 (0.07)	0.22	16.9	36.00
1	24.47	12.12	1.362	0.31 (0.07)	0.21	19.0	14.00
1	24.52	12.20	1.356	0.31 (0.07)	0.21	19.2	27.00
1	24.56	12.29	1.351	0.31 (0.07)	0.21	19.3	26.00
1	24.87	12.98	1.309	0.31 (0.07)	0.21	20.3	20.00
1	24.96	13.30	1.291	0.31 (0.06)	0.21	20.7	23.00
1	25.00	13.46	1.282	0.31 (0.06)	0.21	20.9	29.00
1	25.00	13.49	1.281	0.31 (0.06)	0.21	20.9	17.00
1	25.01	13.56	1.277	0.31 (0.06)	0.21	21.0	50.00
1	24.82	14.22	1.242	0.31 (0.06)	0.21	21.5	34.00
1	24.69	14.62	1.223	0.31 (0.06)	0.21	21.8	39.00
1	24.38	15.24	1.194	0.31 (0.06)	0.21	22.0	24.00
1	22.45	19.53	1.036	0.31 (0.06)	0.20	23.9	12.00
1	21.27	22.72	0.949	0.31 (0.06)	0.20	25.0	32.00
1	20.83	23.45	0.932	0.31 (0.06)	0.20	25.0	10.00
2	0.91	8.45	1.674	0.30 (0.10)	0.35	0.6	51.00

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	22.78	8.45	1.674	0.31 (0.07)	0.22	14.2	51.00
2	23.48	9.20	1.596	0.31 (0.07)	0.22	15.4	43.00
3	23.64	9.37	1.579	0.31 (0.07)	0.22	15.7	45.00
4	24.03	9.86	1.533	0.31 (0.07)	0.22	16.4	30.00
5	24.19	10.09	1.513	0.31 (0.07)	0.22	16.8	38.00
6	24.43	10.49	1.479	0.31 (0.07)	0.22	17.4	30.00
7	24.49	10.61	1.470	0.31 (0.07)	0.22	17.6	36.00
8	25.20	12.12	1.362	0.31 (0.07)	0.22	19.7	14.00
9	25.24	12.20	1.356	0.31 (0.07)	0.22	19.8	27.00
10	25.28	12.29	1.351	0.31 (0.07)	0.22	19.9	26.00
11	25.57	12.98	1.309	0.31 (0.07)	0.22	20.9	20.00
12	25.65	13.30	1.291	0.31 (0.07)	0.22	21.3	23.00
13	25.68	13.46	1.282	0.31 (0.07)	0.22	21.6	29.00
14	25.68	13.49	1.281	0.31 (0.07)	0.22	21.6	17.00
15	25.69	13.56	1.277	0.31 (0.07)	0.22	21.7	50.00
16	25.48	14.22	1.242	0.31 (0.07)	0.21	22.1	34.00
17	25.34	14.62	1.223	0.31 (0.07)	0.21	22.4	39.00
18	25.01	15.24	1.194	0.31 (0.07)	0.21	22.7	24.00
19	22.99	19.53	1.036	0.31 (0.06)	0.20	24.5	12.00
20	21.76	22.72	0.949	0.31 (0.06)	0.20	25.6	32.00
21	21.31	23.45	0.932	0.31 (0.06)	0.20	25.6	10.00

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE (CFS) = 25.69 Tc (MIN.) = 13.56
EFFECTIVE AREA (ACRES) = 21.67 AREA-AVERAGED Fm (INCH/HR) = 0.07
AREA-AVERAGED Fp (INCH/HR) = 0.31 AREA-AVERAGED Ap = 0.22

TOTAL AREA (ACRES) = 25.6
LONGEST FLOWPATH FROM NODE 10.00 TO NODE 47.00 = 2005.00 FEET.

FLOW PROCESS FROM NODE 47.00 TO NODE 53.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<

=====

ELEVATION DATA: UPSTREAM (FEET) = 149.80 DOWNSTREAM (FEET) = 149.70
FLOW LENGTH (FEET) = 100.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 42.0 INCH PIPE IS 29.2 INCHES
PIPE-FLOW VELOCITY (FEET/SEC.) = 3.60
ESTIMATED PIPE DIAMETER (INCH) = 42.00 NUMBER OF PIPES = 1
PIPE-FLOW (CFS) = 25.69
PIPE TRAVEL TIME (MIN.) = 0.46 Tc (MIN.) = 14.02
LONGEST FLOWPATH FROM NODE 10.00 TO NODE 53.00 = 2105.00 FEET.

FLOW PROCESS FROM NODE 53.00 TO NODE 53.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

=====

TOTAL NUMBER OF STREAMS = 3
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION (MIN.) = 14.02
RAINFALL INTENSITY (INCH/HR) = 1.25
AREA-AVERAGED Fm (INCH/HR) = 0.07
AREA-AVERAGED Fp (INCH/HR) = 0.31
AREA-AVERAGED Ap = 0.22
EFFECTIVE STREAM AREA (ACRES) = 21.67
TOTAL STREAM AREA (ACRES) = 25.64
PEAK FLOW RATE (CFS) AT CONFLUENCE = 25.69

FLOW PROCESS FROM NODE 38.00 TO NODE 54.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

=====

INITIAL SUBAREA FLOW-LENGTH (FEET) = 265.00
ELEVATION DATA: UPSTREAM (FEET) = 166.40 DOWNSTREAM (FEET) = 165.80

Tc = K * [(LENGTH** 3.00) / (ELEVATION CHANGE)] ** 0.20
SUBAREA ANALYSIS USED MINIMUM Tc (MIN.) = 11.340
* 2 YEAR RAINFALL INTENSITY (INCH/HR) = 1.415
SUBAREA Tc AND LOSS RATE DATA (AMC I):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN	Tc (MIN.)
CONDOMINIUMS	B	0.80	0.30	0.350	36	11.34

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp (INCH/HR) = 0.30
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.350
SUBAREA RUNOFF (CFS) = 0.94
TOTAL AREA (ACRES) = 0.80 PEAK FLOW RATE (CFS) = 0.94

FLOW PROCESS FROM NODE 54.00 TO NODE 53.00 IS CODE = 41

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) =	149.80	DOWNSTREAM(FEET) =	149.70
FLOW LENGTH(FEET) =	70.00	MANNING'S N =	0.013
DEPTH OF FLOW IN 18.0 INCH PIPE IS	6.1 INCHES		
PIPE-FLOW VELOCITY(FEET/SEC.) =	1.80		
GIVEN PIPE DIAMETER(INCH) =	18.00	NUMBER OF PIPES =	1
PIPE-FLOW(CFS) =	0.94		
PIPE TRAVEL TIME(MIN.) =	0.65	Tc(MIN.) =	11.99
LONGEST FLOWPATH FROM NODE	38.00 TO NODE	53.00 =	335.00 FEET.

FLOW PROCESS FROM NODE 53.00 TO NODE 53.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

=====

TOTAL NUMBER OF STREAMS =	3
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:	
TIME OF CONCENTRATION(MIN.) =	11.99
RAINFALL INTENSITY(INCH/HR) =	1.37
AREA-AVERAGED Fm(INCH/HR) =	0.11
AREA-AVERAGED Fp(INCH/HR) =	0.30
AREA-AVERAGED Ap =	0.35
EFFECTIVE STREAM AREA(ACRES) =	0.80
TOTAL STREAM AREA(ACRES) =	0.80
PEAK FLOW RATE(CFS) AT CONFLUENCE =	0.94

FLOW PROCESS FROM NODE 55.00 TO NODE 56.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

=====

INITIAL SUBAREA FLOW-LENGTH(FEET) =	130.00		
ELEVATION DATA: UPSTREAM(FEET) =	166.50	DOWNSTREAM(FEET) =	166.30

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$							
SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) =	9.214						
* 2 YEAR RAINFALL INTENSITY(INCH/HR) =	1.594						
SUBAREA Tc AND LOSS RATE DATA(AMC I):							
DEVELOPMENT TYPE/	SCS SOIL	AREA	Fp	Ap	SCS	Tc	
LAND USE	GROUP	(ACRES)	(INCH/HR)	(DECIMAL)	CN	(MIN.)	
CONDOMINIUMS	B	0.19	0.30	0.350	36	9.21	
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.30							
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.350							
SUBAREA RUNOFF(CFS) = 0.25							
TOTAL AREA(ACRES) =	0.19	PEAK FLOW RATE(CFS) =	0.25				

FLOW PROCESS FROM NODE 56.00 TO NODE 53.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<

ELEVATION DATA: UPSTREAM(FEET) = 156.00 DOWNSTREAM(FEET) = 149.70
FLOW LENGTH(FEET) = 40.00 MANNING'S N = 0.013
ESTIMATED PIPE DIAMETER(INCH) INCREASED TO 12.000
DEPTH OF FLOW IN 12.0 INCH PIPE IS 1.1 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 6.81
ESTIMATED PIPE DIAMETER(INCH) = 12.000 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 0.25
PIPE TRAVEL TIME(MIN.) = 0.10 Tc(MIN.) = 9.31
LONGEST FLOWPATH FROM NODE 55.00 TO NODE 53.00 = 170.00 FEET.

FLOW PROCESS FROM NODE 53.00 TO NODE 53.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<

TOTAL NUMBER OF STREAMS = 3
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 3 ARE:
TIME OF CONCENTRATION(MIN.) = 9.31
RAINFALL INTENSITY(INCH/HR) = 1.58
AREA-AVERAGED Fm(INCH/HR) = 0.11
AREA-AVERAGED Fp(INCH/HR) = 0.30
AREA-AVERAGED Ap = 0.35
EFFECTIVE STREAM AREA(ACRES) = 0.19
TOTAL STREAM AREA(ACRES) = 0.19
PEAK FLOW RATE(CFS) AT CONFLUENCE = 0.25

** CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	22.78	8.93	1.622	0.31(0.07)	0.22	14.2	51.00
1	23.48	9.68	1.550	0.31(0.07)	0.22	15.4	43.00
1	23.64	9.85	1.534	0.31(0.07)	0.22	15.7	45.00
1	24.03	10.34	1.492	0.31(0.07)	0.22	16.4	30.00
1	24.19	10.57	1.473	0.31(0.07)	0.22	16.8	38.00
1	24.43	10.97	1.442	0.31(0.07)	0.22	17.4	30.00
1	24.49	11.09	1.433	0.31(0.07)	0.22	17.6	36.00
1	25.20	12.60	1.332	0.31(0.07)	0.22	19.7	14.00
1	25.24	12.68	1.327	0.31(0.07)	0.22	19.8	27.00
1	25.28	12.76	1.322	0.31(0.07)	0.22	19.9	26.00
1	25.57	13.45	1.283	0.31(0.07)	0.22	20.9	20.00
1	25.65	13.76	1.266	0.31(0.07)	0.22	21.3	23.00
1	25.68	13.92	1.257	0.31(0.07)	0.22	21.6	29.00
1	25.68	13.95	1.256	0.31(0.07)	0.22	21.6	17.00
1	25.69	14.02	1.252	0.31(0.07)	0.22	21.7	50.00
1	25.48	14.68	1.220	0.31(0.07)	0.21	22.1	34.00
1	25.34	15.10	1.200	0.31(0.07)	0.21	22.4	39.00
1	25.01	15.72	1.173	0.31(0.07)	0.21	22.7	24.00
1	22.99	20.01	1.021	0.31(0.06)	0.20	24.5	12.00

1	21.76	23.20	0.938	0.31 (0.06)	0.20	25.6	32.00
1	21.31	23.94	0.921	0.31 (0.06)	0.20	25.6	10.00
2	0.94	11.99	1.370	0.30 (0.11)	0.35	0.8	38.00
3	0.25	9.31	1.584	0.30 (0.11)	0.35	0.2	55.00

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
CONFLUENCE FORMULA USED FOR 3 STREAMS.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp (Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	23.87	8.93	1.622	0.31 (0.07)	0.23	15.0	51.00
2	24.25	9.31	1.584	0.31 (0.07)	0.23	15.6	55.00
3	24.60	9.68	1.550	0.31 (0.07)	0.23	16.2	43.00
4	24.76	9.85	1.534	0.31 (0.07)	0.23	16.5	45.00
5	25.16	10.34	1.492	0.31 (0.07)	0.23	17.3	30.00
6	25.32	10.57	1.473	0.31 (0.07)	0.23	17.7	38.00
7	25.58	10.97	1.442	0.31 (0.07)	0.23	18.3	30.00
8	25.64	11.09	1.433	0.31 (0.07)	0.23	18.5	36.00
9	26.08	11.99	1.370	0.31 (0.07)	0.23	19.8	38.00
10	26.33	12.60	1.332	0.31 (0.07)	0.22	20.7	14.00
11	26.36	12.68	1.327	0.31 (0.07)	0.22	20.8	27.00
12	26.40	12.76	1.322	0.31 (0.07)	0.22	20.9	26.00
13	26.65	13.45	1.283	0.31 (0.07)	0.22	21.9	20.00
14	26.72	13.76	1.266	0.31 (0.07)	0.22	22.3	23.00
15	26.74	13.92	1.257	0.31 (0.07)	0.22	22.5	29.00
16	26.74	13.95	1.256	0.31 (0.07)	0.22	22.6	17.00
17	26.75	14.02	1.252	0.31 (0.07)	0.22	22.7	50.00
18	26.51	14.68	1.220	0.31 (0.07)	0.22	23.1	34.00
19	26.35	15.10	1.200	0.31 (0.07)	0.22	23.4	39.00
20	25.99	15.72	1.173	0.31 (0.07)	0.22	23.7	24.00
21	23.83	20.01	1.021	0.31 (0.06)	0.21	25.5	12.00
22	22.52	23.20	0.938	0.31 (0.06)	0.21	26.6	32.00
23	22.06	23.94	0.921	0.31 (0.06)	0.21	26.6	10.00

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE (CFS) = 26.75 Tc (MIN.) = 14.02
EFFECTIVE AREA (ACRES) = 22.66 AREA-AVERAGED Fm (INCH/HR) = 0.07
AREA-AVERAGED Fp (INCH/HR) = 0.31 AREA-AVERAGED Ap = 0.22
TOTAL AREA (ACRES) = 26.6
LONGEST FLOWPATH FROM NODE 10.00 TO NODE 53.00 = 2105.00 FEET.

FLOW PROCESS FROM NODE 53.00 TO NODE 57.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<

=====

ELEVATION DATA: UPSTREAM (FEET) = 149.70 DOWNSTREAM (FEET) = 149.10
FLOW LENGTH (FEET) = 280.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 36.0 INCH PIPE IS 26.4 INCHES
PIPE-FLOW VELOCITY (FEET/SEC.) = 4.81
ESTIMATED PIPE DIAMETER (INCH) = 36.00 NUMBER OF PIPES = 1
PIPE-FLOW (CFS) = 26.75

PIPE TRAVEL TIME(MIN.) = 0.97 Tc(MIN.) = 14.99
LONGEST FLOWPATH FROM NODE 10.00 TO NODE 57.00 = 2385.00 FEET.

FLOW PROCESS FROM NODE 57.00 TO NODE 57.00 IS CODE = 10

>>>>MAIN-STREAM MEMORY COPIED ONTO MEMORY BANK # 1 <<<<<
=====

FLOW PROCESS FROM NODE 58.00 TO NODE 57.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<
=====

INITIAL SUBAREA FLOW-LENGTH(FEET) = 240.00
ELEVATION DATA: UPSTREAM(FEET) = 166.50 DOWNSTREAM(FEET) = 165.60

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$

SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) = 9.853

* 2 YEAR RAINFALL INTENSITY(INCH/HR) = 1.534

SUBAREA Tc AND LOSS RATE DATA(AMC I):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN	Tc (MIN.)
CONDOMINIUMS	B	0.46	0.30	0.350	36	9.85

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.30

SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.350

SUBAREA RUNOFF(CFS) = 0.59

TOTAL AREA(ACRES) = 0.46 PEAK FLOW RATE(CFS) = 0.59

FLOW PROCESS FROM NODE 57.00 TO NODE 57.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
=====

TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:

TIME OF CONCENTRATION(MIN.) = 9.85

RAINFALL INTENSITY(INCH/HR) = 1.53

AREA-AVERAGED Fm(INCH/HR) = 0.11

AREA-AVERAGED Fp(INCH/HR) = 0.30

AREA-AVERAGED Ap = 0.35

EFFECTIVE STREAM AREA(ACRES) = 0.46

TOTAL STREAM AREA(ACRES) = 0.46

PEAK FLOW RATE(CFS) AT CONFLUENCE = 0.59

FLOW PROCESS FROM NODE 59.00 TO NODE 57.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<
=====

INITIAL SUBAREA FLOW-LENGTH(FEET) = 265.00
ELEVATION DATA: UPSTREAM(FEET) = 166.80 DOWNSTREAM(FEET) = 165.80

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$
 SUBAREA ANALYSIS USED MINIMUM T_c (MIN.) = 10.239
 * 2 YEAR RAINFALL INTENSITY (INCH/HR) = 1.500
 SUBAREA T_c AND LOSS RATE DATA (AMC I):
 DEVELOPMENT TYPE/ LAND USE SCS SOIL GROUP AREA (ACRES) F_p (INCH/HR) A_p (DECIMAL) SCS CN T_c (MIN.)
 CONDOMINIUMS B 0.57 0.30 0.350 36 10.24
 SUBAREA AVERAGE PERVIOUS LOSS RATE, F_p (INCH/HR) = 0.30
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, A_p = 0.350
 SUBAREA RUNOFF (CFS) = 0.72
 TOTAL AREA (ACRES) = 0.57 PEAK FLOW RATE (CFS) = 0.72

 FLOW PROCESS FROM NODE 57.00 TO NODE 57.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<<
 >>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<<

=====
 TOTAL NUMBER OF STREAMS = 2
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
 TIME OF CONCENTRATION (MIN.) = 10.24
 RAINFALL INTENSITY (INCH/HR) = 1.50
 AREA-AVERAGED F_m (INCH/HR) = 0.11
 AREA-AVERAGED F_p (INCH/HR) = 0.30
 AREA-AVERAGED A_p = 0.35
 EFFECTIVE STREAM AREA (ACRES) = 0.57
 TOTAL STREAM AREA (ACRES) = 0.57
 PEAK FLOW RATE (CFS) AT CONFLUENCE = 0.72

** CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	T_c (MIN.)	Intensity (INCH/HR)	F_p (F_m) (INCH/HR)	A_p	A_e (ACRES)	HEADWATER NODE
1	0.59	9.85	1.534	0.30 (0.11)	0.35	0.5	58.00
2	0.72	10.24	1.500	0.30 (0.11)	0.35	0.6	59.00

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
 CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	T_c (MIN.)	Intensity (INCH/HR)	F_p (F_m) (INCH/HR)	A_p	A_e (ACRES)	HEADWATER NODE
1	1.30	9.85	1.534	0.30 (0.11)	0.35	1.0	58.00
2	1.29	10.24	1.500	0.30 (0.11)	0.35	1.0	59.00

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE (CFS) = 1.30 T_c (MIN.) = 9.85
 EFFECTIVE AREA (ACRES) = 1.01 AREA-AVERAGED F_m (INCH/HR) = 0.11
 AREA-AVERAGED F_p (INCH/HR) = 0.30 AREA-AVERAGED A_p = 0.35
 TOTAL AREA (ACRES) = 1.0
 LONGEST FLOWPATH FROM NODE 59.00 TO NODE 57.00 = 265.00 FEET.

 FLOW PROCESS FROM NODE 57.00 TO NODE 57.00 IS CODE = 11

=====
 >>>>CONFLUENCE MEMORY BANK # 1 WITH THE MAIN-STREAM MEMORY<<<<<
 =====

** MAIN STREAM CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp (Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	1.30	9.85	1.534	0.30 (0.11)	0.35	1.0	58.00
2	1.29	10.24	1.500	0.30 (0.11)	0.35	1.0	59.00
LONGEST FLOWPATH FROM NODE			59.00	TO NODE	57.00 =	265.00 FEET.	

** MEMORY BANK # 1 CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp (Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	23.87	9.92	1.527	0.31 (0.07)	0.23	15.0	51.00
2	24.25	10.30	1.495	0.31 (0.07)	0.23	15.6	55.00
3	24.60	10.66	1.466	0.31 (0.07)	0.23	16.2	43.00
4	24.76	10.83	1.453	0.31 (0.07)	0.23	16.5	45.00
5	25.16	11.32	1.416	0.31 (0.07)	0.23	17.3	30.00
6	25.32	11.55	1.400	0.31 (0.07)	0.23	17.7	38.00
7	25.58	11.95	1.373	0.31 (0.07)	0.23	18.3	30.00
8	25.64	12.06	1.365	0.31 (0.07)	0.23	18.5	36.00
9	26.08	12.96	1.310	0.31 (0.07)	0.23	19.8	38.00
10	26.33	13.57	1.276	0.31 (0.07)	0.22	20.7	14.00
11	26.36	13.65	1.272	0.31 (0.07)	0.22	20.8	27.00
12	26.40	13.73	1.267	0.31 (0.07)	0.22	20.9	26.00
13	26.65	14.42	1.233	0.31 (0.07)	0.22	21.9	20.00
14	26.72	14.73	1.217	0.31 (0.07)	0.22	22.3	23.00
15	26.74	14.89	1.210	0.31 (0.07)	0.22	22.5	29.00
16	26.74	14.92	1.209	0.31 (0.07)	0.22	22.6	17.00
17	26.75	14.99	1.205	0.31 (0.07)	0.22	22.7	50.00
18	26.51	15.65	1.176	0.31 (0.07)	0.22	23.1	34.00
19	26.35	16.07	1.158	0.31 (0.07)	0.22	23.4	39.00
20	25.99	16.69	1.133	0.31 (0.07)	0.22	23.7	24.00
21	23.83	21.03	0.993	0.31 (0.06)	0.21	25.5	12.00
22	22.52	24.22	0.915	0.31 (0.06)	0.21	26.6	32.00
23	22.06	24.96	0.899	0.31 (0.06)	0.21	26.6	10.00
LONGEST FLOWPATH FROM NODE			10.00	TO NODE	57.00 =	2385.00 FEET.	

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp (Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	25.10	9.85	1.534	0.30 (0.07)	0.24	15.9	58.00
2	25.17	9.92	1.527	0.30 (0.07)	0.24	16.0	51.00
3	25.48	10.24	1.500	0.30 (0.07)	0.24	16.5	59.00
4	25.54	10.30	1.495	0.30 (0.07)	0.24	16.6	55.00
5	25.86	10.66	1.466	0.30 (0.07)	0.24	17.3	43.00
6	26.01	10.83	1.453	0.30 (0.07)	0.24	17.5	45.00
7	26.38	11.32	1.416	0.30 (0.07)	0.23	18.4	30.00
8	26.52	11.55	1.400	0.30 (0.07)	0.23	18.7	38.00
9	26.75	11.95	1.373	0.30 (0.07)	0.23	19.4	30.00
10	26.81	12.06	1.365	0.31 (0.07)	0.23	19.5	36.00
11	27.19	12.96	1.310	0.31 (0.07)	0.23	20.9	38.00
12	27.41	13.57	1.276	0.31 (0.07)	0.23	21.7	14.00

13	27.45	13.65	1.272	0.31 (0.07)	0.23	21.8	27.00
14	27.47	13.73	1.267	0.31 (0.07)	0.23	22.0	26.00
15	27.69	14.42	1.233	0.31 (0.07)	0.23	23.0	20.00
16	27.75	14.73	1.217	0.31 (0.07)	0.23	23.4	23.00
17	27.76	14.89	1.210	0.31 (0.07)	0.23	23.6	29.00
18	27.76	14.92	1.209	0.31 (0.07)	0.23	23.6	17.00
19	27.76	14.99	1.205	0.31 (0.07)	0.23	23.7	50.00
20	27.50	15.65	1.176	0.31 (0.07)	0.23	24.1	34.00
21	27.32	16.07	1.158	0.31 (0.07)	0.23	24.4	39.00
22	26.95	16.69	1.133	0.31 (0.07)	0.22	24.7	24.00
23	24.65	21.03	0.993	0.30 (0.07)	0.22	26.5	12.00
24	23.27	24.22	0.915	0.30 (0.06)	0.21	27.6	32.00
25	22.79	24.96	0.899	0.30 (0.06)	0.21	27.7	10.00
TOTAL AREA (ACRES) =			27.7				

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE (CFS) = 27.76 Tc (MIN.) = 14.994
EFFECTIVE AREA (ACRES) = 23.69 AREA-AVERAGED Fm (INCH/HR) = 0.07
AREA-AVERAGED Fp (INCH/HR) = 0.31 AREA-AVERAGED Ap = 0.23
TOTAL AREA (ACRES) = 27.7
LONGEST FLOWPATH FROM NODE 10.00 TO NODE 57.00 = 2385.00 FEET.

FLOW PROCESS FROM NODE 57.00 TO NODE 57.00 IS CODE = 12

>>>>CLEAR MEMORY BANK # 1 <<<<<

FLOW PROCESS FROM NODE 57.00 TO NODE 57.00 IS CODE = 10

>>>>MAIN-STREAM MEMORY COPIED ONTO MEMORY BANK # 1 <<<<<

FLOW PROCESS FROM NODE 60.00 TO NODE 61.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

INITIAL SUBAREA FLOW-LENGTH (FEET) = 200.00
ELEVATION DATA: UPSTREAM (FEET) = 167.00 DOWNSTREAM (FEET) = 166.20

Tc = K * [(LENGTH** 3.00) / (ELEVATION CHANGE)] ** 0.20
SUBAREA ANALYSIS USED MINIMUM Tc (MIN.) = 9.043
* 2 YEAR RAINFALL INTENSITY (INCH/HR) = 1.611
SUBAREA Tc AND LOSS RATE DATA (AMC I):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN	Tc (MIN.)
CONDOMINIUMS	B	0.27	0.30	0.350	36	9.04

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp (INCH/HR) = 0.30
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.350
SUBAREA RUNOFF (CFS) = 0.37
TOTAL AREA (ACRES) = 0.27 PEAK FLOW RATE (CFS) = 0.37

FLOW PROCESS FROM NODE 61.00 TO NODE 62.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) =	166.20	DOWNSTREAM(FEET) =	165.30
FLOW LENGTH(FEET) =	210.00	MANNING'S N =	0.013
ESTIMATED PIPE DIAMETER(INCH) INCREASED TO	12.000		
DEPTH OF FLOW IN 12.0 INCH PIPE IS	3.3	INCHES	
PIPE-FLOW VELOCITY(FEET/SEC.) =	2.12		
ESTIMATED PIPE DIAMETER(INCH) =	12.00	NUMBER OF PIPES =	1
PIPE-FLOW(CFS) =	0.37		
PIPE TRAVEL TIME(MIN.) =	1.65	Tc(MIN.) =	10.69
LONGEST FLOWPATH FROM NODE 60.00 TO NODE 62.00 =	410.00	FEET.	

FLOW PROCESS FROM NODE 62.00 TO NODE 62.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

=====

TOTAL NUMBER OF STREAMS =	3
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:	
TIME OF CONCENTRATION(MIN.) =	10.69
RAINFALL INTENSITY(INCH/HR) =	1.46
AREA-AVERAGED Fm(INCH/HR) =	0.10
AREA-AVERAGED Fp(INCH/HR) =	0.30
AREA-AVERAGED Ap =	0.35
EFFECTIVE STREAM AREA(ACRES) =	0.27
TOTAL STREAM AREA(ACRES) =	0.27
PEAK FLOW RATE(CFS) AT CONFLUENCE =	0.37

FLOW PROCESS FROM NODE 63.00 TO NODE 62.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

=====

INITIAL SUBAREA FLOW-LENGTH(FEET) =	270.00		
ELEVATION DATA: UPSTREAM(FEET) =	166.50	DOWNSTREAM(FEET) =	165.30

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$
SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) = 9.984
* 2 YEAR RAINFALL INTENSITY(INCH/HR) = 1.522

SUBAREA Tc AND LOSS RATE DATA(AMC I):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN	Tc (MIN.)
CONDOMINIUMS	B	0.70	0.30	0.350	36	9.98
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.30						
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.350						
SUBAREA RUNOFF(CFS) = 0.89						
TOTAL AREA(ACRES) = 0.70 PEAK FLOW RATE(CFS) = 0.89						

FLOW PROCESS FROM NODE 62.00 TO NODE 62.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

=====

TOTAL NUMBER OF STREAMS = 3
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
 TIME OF CONCENTRATION(MIN.) = 9.98
 RAINFALL INTENSITY(INCH/HR) = 1.52
 AREA-AVERAGED Fm(INCH/HR) = 0.11
 AREA-AVERAGED Fp(INCH/HR) = 0.30
 AREA-AVERAGED Ap = 0.35
 EFFECTIVE STREAM AREA(ACRES) = 0.70
 TOTAL STREAM AREA(ACRES) = 0.70
 PEAK FLOW RATE(CFS) AT CONFLUENCE = 0.89

FLOW PROCESS FROM NODE 64.00 TO NODE 62.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

=====

INITIAL SUBAREA FLOW-LENGTH(FEET) = 295.00
 ELEVATION DATA: UPSTREAM(FEET) = 166.50 DOWNSTREAM(FEET) = 165.30

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$
 SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) = 20.647
 * 2 YEAR RAINFALL INTENSITY(INCH/HR) = 1.003
 SUBAREA Tc AND LOSS RATE DATA(AMC I):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN	Tc (MIN.)
AGRICULTURAL FAIR COVER "PASTURE, IRRIGATED"	B	0.27	0.30	1.000	45	20.65

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.30
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 1.000
 SUBAREA RUNOFF(CFS) = 0.17
 TOTAL AREA(ACRES) = 0.27 PEAK FLOW RATE(CFS) = 0.17

FLOW PROCESS FROM NODE 62.00 TO NODE 62.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<

=====

TOTAL NUMBER OF STREAMS = 3
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 3 ARE:
 TIME OF CONCENTRATION(MIN.) = 20.65
 RAINFALL INTENSITY(INCH/HR) = 1.00
 AREA-AVERAGED Fm(INCH/HR) = 0.30
 AREA-AVERAGED Fp(INCH/HR) = 0.30
 AREA-AVERAGED Ap = 1.00
 EFFECTIVE STREAM AREA(ACRES) = 0.27
 TOTAL STREAM AREA(ACRES) = 0.27
 PEAK FLOW RATE(CFS) AT CONFLUENCE = 0.17

** CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	0.37	10.69	1.463	0.30 (0.10)	0.35	0.3	60.00
2	0.89	9.98	1.522	0.30 (0.11)	0.35	0.7	63.00
3	0.17	20.65	1.003	0.30 (0.30)	1.00	0.3	64.00

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
CONFLUENCE FORMULA USED FOR 3 STREAMS.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	1.39	9.98	1.522	0.30 (0.13)	0.43	1.1	63.00
2	1.37	10.69	1.463	0.30 (0.13)	0.43	1.1	60.00
3	0.98	20.65	1.003	0.30 (0.15)	0.49	1.2	64.00

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE (CFS) = 1.39 Tc (MIN.) = 9.98
 EFFECTIVE AREA (ACRES) = 1.08 AREA-AVERAGED Fm (INCH/HR) = 0.13
 AREA-AVERAGED Fp (INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.43
 TOTAL AREA (ACRES) = 1.2
 LONGEST FLOWPATH FROM NODE 60.00 TO NODE 62.00 = 410.00 FEET.

FLOW PROCESS FROM NODE 62.00 TO NODE 57.00 IS CODE = 31

 >>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
 >>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW) <<<<<

=====

ELEVATION DATA: UPSTREAM (FEET) = 156.00 DOWNSTREAM (FEET) = 149.10
 FLOW LENGTH (FEET) = 245.00 MANNING'S N = 0.013
 ESTIMATED PIPE DIAMETER (INCH) INCREASED TO 12.000
 DEPTH OF FLOW IN 12.0 INCH PIPE IS 4.0 INCHES
 PIPE-FLOW VELOCITY (FEET/SEC.) = 6.10
 ESTIMATED PIPE DIAMETER (INCH) = 12.000 NUMBER OF PIPES = 1
 PIPE-FLOW (CFS) = 1.39
 PIPE TRAVEL TIME (MIN.) = 0.67 Tc (MIN.) = 10.65
 LONGEST FLOWPATH FROM NODE 60.00 TO NODE 57.00 = 655.00 FEET.

FLOW PROCESS FROM NODE 57.00 TO NODE 57.00 IS CODE = 11

 >>>>CONFLUENCE MEMORY BANK # 1 WITH THE MAIN-STREAM MEMORY<<<<<

** MAIN STREAM CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	1.39	10.65	1.466	0.30 (0.13)	0.43	1.1	63.00
2	1.37	11.37	1.413	0.30 (0.13)	0.43	1.1	60.00
3	0.98	21.39	0.983	0.30 (0.15)	0.49	1.2	64.00

LONGEST FLOWPATH FROM NODE 60.00 TO NODE 57.00 = 655.00 FEET.

** MEMORY BANK # 1 CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp (Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	25.10	9.85	1.534	0.30 (0.07)	0.24	15.9	58.00
2	25.17	9.92	1.527	0.30 (0.07)	0.24	16.0	51.00
3	25.48	10.24	1.500	0.30 (0.07)	0.24	16.5	59.00
4	25.54	10.30	1.495	0.30 (0.07)	0.24	16.6	55.00
5	25.86	10.66	1.466	0.30 (0.07)	0.24	17.3	43.00
6	26.01	10.83	1.453	0.30 (0.07)	0.24	17.5	45.00
7	26.38	11.32	1.416	0.30 (0.07)	0.23	18.4	30.00
8	26.52	11.55	1.400	0.30 (0.07)	0.23	18.7	38.00
9	26.75	11.95	1.373	0.30 (0.07)	0.23	19.4	30.00
10	26.81	12.06	1.365	0.31 (0.07)	0.23	19.5	36.00
11	27.19	12.96	1.310	0.31 (0.07)	0.23	20.9	38.00
12	27.41	13.57	1.276	0.31 (0.07)	0.23	21.7	14.00
13	27.45	13.65	1.272	0.31 (0.07)	0.23	21.8	27.00
14	27.47	13.73	1.267	0.31 (0.07)	0.23	22.0	26.00
15	27.69	14.42	1.233	0.31 (0.07)	0.23	23.0	20.00
16	27.75	14.73	1.217	0.31 (0.07)	0.23	23.4	23.00
17	27.76	14.89	1.210	0.31 (0.07)	0.23	23.6	29.00
18	27.76	14.92	1.209	0.31 (0.07)	0.23	23.6	17.00
19	27.76	14.99	1.205	0.31 (0.07)	0.23	23.7	50.00
20	27.50	15.65	1.176	0.31 (0.07)	0.23	24.1	34.00
21	27.32	16.07	1.158	0.31 (0.07)	0.23	24.4	39.00
22	26.95	16.69	1.133	0.31 (0.07)	0.22	24.7	24.00
23	24.65	21.03	0.993	0.30 (0.07)	0.22	26.5	12.00
24	23.27	24.22	0.915	0.30 (0.06)	0.21	27.6	32.00
25	22.79	24.96	0.899	0.30 (0.06)	0.21	27.7	10.00
LONGEST FLOWPATH FROM NODE			10.00	TO NODE	57.00 =	2385.00	FEET.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp (Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	26.45	9.85	1.534	0.30 (0.08)	0.25	16.9	58.00
2	26.52	9.92	1.527	0.30 (0.08)	0.25	17.0	51.00
3	26.85	10.24	1.500	0.30 (0.08)	0.25	17.6	59.00
4	26.91	10.30	1.495	0.30 (0.08)	0.25	17.7	55.00
5	27.25	10.65	1.466	0.30 (0.08)	0.25	18.3	63.00
6	27.26	10.66	1.466	0.30 (0.08)	0.25	18.3	43.00
7	27.39	10.83	1.453	0.30 (0.08)	0.25	18.6	45.00
8	27.75	11.32	1.416	0.30 (0.07)	0.25	19.5	30.00
9	27.78	11.37	1.413	0.30 (0.07)	0.25	19.5	60.00
10	27.88	11.55	1.400	0.30 (0.07)	0.25	19.8	38.00
11	28.10	11.95	1.373	0.30 (0.07)	0.24	20.5	30.00
12	28.15	12.06	1.365	0.30 (0.07)	0.24	20.7	36.00
13	28.50	12.96	1.310	0.30 (0.07)	0.24	22.0	38.00
14	28.70	13.57	1.276	0.30 (0.07)	0.24	22.8	14.00
15	28.72	13.65	1.272	0.30 (0.07)	0.24	23.0	27.00
16	28.75	13.73	1.267	0.30 (0.07)	0.24	23.1	26.00
17	28.94	14.42	1.233	0.30 (0.07)	0.24	24.1	20.00
18	28.98	14.73	1.217	0.30 (0.07)	0.24	24.5	23.00
19	28.99	14.89	1.210	0.30 (0.07)	0.24	24.7	29.00
20	28.99	14.92	1.209	0.30 (0.07)	0.24	24.8	17.00

21	28.99	14.99	1.205	0.30	(0.07)	0.24	24.8	50.00
22	28.70	15.65	1.176	0.30	(0.07)	0.24	25.3	34.00
23	28.51	16.07	1.158	0.30	(0.07)	0.24	25.6	39.00
24	28.11	16.69	1.133	0.30	(0.07)	0.24	25.9	24.00
25	25.65	21.03	0.993	0.30	(0.07)	0.23	27.7	12.00
26	25.48	21.39	0.983	0.30	(0.07)	0.23	27.9	64.00
27	24.17	24.22	0.915	0.30	(0.07)	0.22	28.9	32.00
28	23.67	24.96	0.899	0.30	(0.07)	0.22	28.9	10.00
TOTAL AREA (ACRES) =			28.9					

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE (CFS) = 28.99 Tc (MIN.) = 14.895
EFFECTIVE AREA (ACRES) = 24.73 AREA-AVERAGED Fm (INCH/HR) = 0.07
AREA-AVERAGED Fp (INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.24
TOTAL AREA (ACRES) = 28.9
LONGEST FLOWPATH FROM NODE 10.00 TO NODE 57.00 = 2385.00 FEET.

FLOW PROCESS FROM NODE 57.00 TO NODE 57.00 IS CODE = 12

>>>>CLEAR MEMORY BANK # 1 <<<<<
=====

FLOW PROCESS FROM NODE 57.00 TO NODE 57.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
=====

TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION (MIN.) = 14.89
RAINFALL INTENSITY (INCH/HR) = 1.21
AREA-AVERAGED Fm (INCH/HR) = 0.07
AREA-AVERAGED Fp (INCH/HR) = 0.30
AREA-AVERAGED Ap = 0.24
EFFECTIVE STREAM AREA (ACRES) = 24.73
TOTAL STREAM AREA (ACRES) = 28.90
PEAK FLOW RATE (CFS) AT CONFLUENCE = 28.99

FLOW PROCESS FROM NODE 65.00 TO NODE 66.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<
=====

INITIAL SUBAREA FLOW-LENGTH (FEET) = 135.00
ELEVATION DATA: UPSTREAM (FEET) = 166.70 DOWNSTREAM (FEET) = 166.30

Tc = K * [(LENGTH** 3.00) / (ELEVATION CHANGE)]**0.20
SUBAREA ANALYSIS USED MINIMUM Tc (MIN.) = 8.205
* 2 YEAR RAINFALL INTENSITY (INCH/HR) = 1.703
SUBAREA Tc AND LOSS RATE DATA (AMC I):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN	Tc (MIN.)
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CONDOMINIUMS B 0.19 0.30 0.350 36 8.21
 SUBAREA AVERAGE PERVIOUS LOSS RATE, F_p (INCH/HR) = 0.30
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, A_p = 0.350
 SUBAREA RUNOFF (CFS) = 0.27
 TOTAL AREA (ACRES) = 0.19 PEAK FLOW RATE (CFS) = 0.27

 FLOW PROCESS FROM NODE 66.00 TO NODE 57.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
 >>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<

=====
 ELEVATION DATA: UPSTREAM (FEET) = 156.00 DOWNSTREAM (FEET) = 149.10
 FLOW LENGTH (FEET) = 45.00 MANNING'S N = 0.013
 ESTIMATED PIPE DIAMETER (INCH) INCREASED TO 12.000
 DEPTH OF FLOW IN 12.0 INCH PIPE IS 1.2 INCHES
 PIPE-FLOW VELOCITY (FEET/SEC.) = 6.89
 ESTIMATED PIPE DIAMETER (INCH) = 12.000 NUMBER OF PIPES = 1
 PIPE-FLOW (CFS) = 0.27
 PIPE TRAVEL TIME (MIN.) = 0.11 T_c (MIN.) = 8.31
 LONGEST FLOWPATH FROM NODE 65.00 TO NODE 57.00 = 180.00 FEET.

 FLOW PROCESS FROM NODE 57.00 TO NODE 57.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
 >>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<

=====
 TOTAL NUMBER OF STREAMS = 2
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
 TIME OF CONCENTRATION (MIN.) = 8.31
 RAINFALL INTENSITY (INCH/HR) = 1.69
 AREA-AVERAGED F_m (INCH/HR) = 0.11
 AREA-AVERAGED F_p (INCH/HR) = 0.30
 AREA-AVERAGED A_p = 0.35
 EFFECTIVE STREAM AREA (ACRES) = 0.19
 TOTAL STREAM AREA (ACRES) = 0.19
 PEAK FLOW RATE (CFS) AT CONFLUENCE = 0.27

** CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	T_c (MIN.)	Intensity (INCH/HR)	F_p (F_m) (INCH/HR)	A_p	A_e (ACRES)	HEADWATER NODE
1	26.45	9.85	1.534	0.30 (0.08)	0.25	16.9	58.00
1	26.52	9.92	1.527	0.30 (0.08)	0.25	17.0	51.00
1	26.85	10.24	1.500	0.30 (0.08)	0.25	17.6	59.00
1	26.91	10.30	1.495	0.30 (0.08)	0.25	17.7	55.00
1	27.25	10.65	1.466	0.30 (0.08)	0.25	18.3	63.00
1	27.26	10.66	1.466	0.30 (0.08)	0.25	18.3	43.00
1	27.39	10.83	1.453	0.30 (0.08)	0.25	18.6	45.00
1	27.75	11.32	1.416	0.30 (0.07)	0.25	19.5	30.00
1	27.78	11.37	1.413	0.30 (0.07)	0.25	19.5	60.00
1	27.88	11.55	1.400	0.30 (0.07)	0.25	19.8	38.00
1	28.10	11.95	1.373	0.30 (0.07)	0.24	20.5	30.00
1	28.15	12.06	1.365	0.30 (0.07)	0.24	20.7	36.00

1	28.50	12.96	1.310	0.30 (0.07)	0.24	22.0	38.00
1	28.70	13.57	1.276	0.30 (0.07)	0.24	22.8	14.00
1	28.72	13.65	1.272	0.30 (0.07)	0.24	23.0	27.00
1	28.75	13.73	1.267	0.30 (0.07)	0.24	23.1	26.00
1	28.94	14.42	1.233	0.30 (0.07)	0.24	24.1	20.00
1	28.98	14.73	1.217	0.30 (0.07)	0.24	24.5	23.00
1	28.99	14.89	1.210	0.30 (0.07)	0.24	24.7	29.00
1	28.99	14.92	1.209	0.30 (0.07)	0.24	24.8	17.00
1	28.99	14.99	1.205	0.30 (0.07)	0.24	24.8	50.00
1	28.70	15.65	1.176	0.30 (0.07)	0.24	25.3	34.00
1	28.51	16.07	1.158	0.30 (0.07)	0.24	25.6	39.00
1	28.11	16.69	1.133	0.30 (0.07)	0.24	25.9	24.00
1	25.65	21.03	0.993	0.30 (0.07)	0.23	27.7	12.00
1	25.48	21.39	0.983	0.30 (0.07)	0.23	27.9	64.00
1	24.17	24.22	0.915	0.30 (0.07)	0.22	28.9	32.00
1	23.67	24.96	0.899	0.30 (0.07)	0.22	28.9	10.00
2	0.27	8.31	1.691	0.30 (0.11)	0.35	0.2	65.00

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	25.00	8.31	1.691	0.30 (0.08)	0.25	14.4	65.00
2	26.70	9.85	1.534	0.30 (0.08)	0.25	17.1	58.00
3	26.77	9.92	1.527	0.30 (0.08)	0.25	17.2	51.00
4	27.09	10.24	1.500	0.30 (0.08)	0.25	17.8	59.00
5	27.15	10.30	1.495	0.30 (0.08)	0.25	17.9	55.00
6	27.49	10.65	1.466	0.30 (0.08)	0.25	18.5	63.00
7	27.49	10.66	1.466	0.30 (0.08)	0.25	18.5	43.00
8	27.63	10.83	1.453	0.30 (0.08)	0.25	18.8	45.00
9	27.98	11.32	1.416	0.30 (0.08)	0.25	19.7	30.00
10	28.00	11.37	1.413	0.30 (0.08)	0.25	19.7	60.00
11	28.11	11.55	1.400	0.30 (0.08)	0.25	20.0	38.00
12	28.32	11.95	1.373	0.30 (0.07)	0.25	20.7	30.00
13	28.37	12.06	1.365	0.30 (0.07)	0.25	20.8	36.00
14	28.71	12.96	1.310	0.30 (0.07)	0.24	22.2	38.00
15	28.90	13.57	1.276	0.30 (0.07)	0.24	23.0	14.00
16	28.93	13.65	1.272	0.30 (0.07)	0.24	23.2	27.00
17	28.95	13.73	1.267	0.30 (0.07)	0.24	23.3	26.00
18	29.14	14.42	1.233	0.30 (0.07)	0.24	24.3	20.00
19	29.18	14.73	1.217	0.30 (0.07)	0.24	24.7	23.00
20	29.18	14.89	1.210	0.30 (0.07)	0.24	24.9	29.00
21	29.18	14.92	1.209	0.30 (0.07)	0.24	24.9	17.00
22	29.18	14.99	1.205	0.30 (0.07)	0.24	25.0	50.00
23	28.88	15.65	1.176	0.30 (0.07)	0.24	25.5	34.00
24	28.69	16.07	1.158	0.30 (0.07)	0.24	25.8	39.00
25	28.28	16.69	1.133	0.30 (0.07)	0.24	26.1	24.00
26	25.80	21.03	0.993	0.30 (0.07)	0.23	27.9	12.00
27	25.63	21.39	0.983	0.30 (0.07)	0.23	28.1	64.00
28	24.31	24.22	0.915	0.30 (0.07)	0.22	29.1	32.00
29	23.81	24.96	0.899	0.30 (0.07)	0.22	29.1	10.00

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE (CFS) = 29.18 Tc (MIN.) = 14.89
EFFECTIVE AREA (ACRES) = 24.92 AREA-AVERAGED Fm (INCH/HR) = 0.07
AREA-AVERAGED Fp (INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.24
TOTAL AREA (ACRES) = 29.1
LONGEST FLOWPATH FROM NODE 10.00 TO NODE 57.00 = 2385.00 FEET.

FLOW PROCESS FROM NODE 57.00 TO NODE 67.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<

=====

ELEVATION DATA: UPSTREAM (FEET) = 149.10 DOWNSTREAM (FEET) = 148.50
FLOW LENGTH (FEET) = 275.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 36.0 INCH PIPE IS 28.3 INCHES
PIPE-FLOW VELOCITY (FEET/SEC.) = 4.89
ESTIMATED PIPE DIAMETER (INCH) = 36.00 NUMBER OF PIPES = 1
PIPE-FLOW (CFS) = 29.18
PIPE TRAVEL TIME (MIN.) = 0.94 Tc (MIN.) = 15.83
LONGEST FLOWPATH FROM NODE 10.00 TO NODE 67.00 = 2660.00 FEET.

FLOW PROCESS FROM NODE 67.00 TO NODE 67.00 IS CODE = 10

>>>>MAIN-STREAM MEMORY COPIED ONTO MEMORY BANK # 1 <<<<<

FLOW PROCESS FROM NODE 68.00 TO NODE 69.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

=====

INITIAL SUBAREA FLOW-LENGTH (FEET) = 365.00
ELEVATION DATA: UPSTREAM (FEET) = 168.20 DOWNSTREAM (FEET) = 167.30

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$
SUBAREA ANALYSIS USED MINIMUM Tc (MIN.) = 11.404
* 2 YEAR RAINFALL INTENSITY (INCH/HR) = 1.410
SUBAREA Tc AND LOSS RATE DATA (AMC I):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN	Tc (MIN.)
APARTMENTS	B	1.00	0.30	0.200	36	11.40

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp (INCH/HR) = 0.30
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.200
SUBAREA RUNOFF (CFS) = 1.22
TOTAL AREA (ACRES) = 1.00 PEAK FLOW RATE (CFS) = 1.22

FLOW PROCESS FROM NODE 69.00 TO NODE 70.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<


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ELEVATION DATA: UPSTREAM(FEET) = 162.00 DOWNSTREAM(FEET) = 159.00
FLOW LENGTH(FEET) = 185.00 MANNING'S N = 0.013
ESTIMATED PIPE DIAMETER(INCH) INCREASED TO 12.000
DEPTH OF FLOW IN 12.0 INCH PIPE IS 4.3 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 4.81
ESTIMATED PIPE DIAMETER(INCH) = 12.000 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 1.22
PIPE TRAVEL TIME(MIN.) = 0.64 Tc(MIN.) = 12.05
LONGEST FLOWPATH FROM NODE 68.00 TO NODE 70.00 = 550.00 FEET.

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FLOW PROCESS FROM NODE 70.00 TO NODE 71.00 IS CODE = 1

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>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
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TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION(MIN.) = 12.05
RAINFALL INTENSITY(INCH/HR) = 1.37
AREA-AVERAGED Fm(INCH/HR) = 0.06
AREA-AVERAGED Fp(INCH/HR) = 0.30
AREA-AVERAGED Ap = 0.20
EFFECTIVE STREAM AREA(ACRES) = 1.00
TOTAL STREAM AREA(ACRES) = 1.00
PEAK FLOW RATE(CFS) AT CONFLUENCE = 1.22

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FLOW PROCESS FROM NODE 71.00 TO NODE 70.00 IS CODE = 21

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>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<
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INITIAL SUBAREA FLOW-LENGTH(FEET) = 250.00
ELEVATION DATA: UPSTREAM(FEET) = 167.20 DOWNSTREAM(FEET) = 166.80

Tc = K*[(LENGTH** 3.00)/(ELEVATION CHANGE)]**0.20
SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) = 10.688
* 2 YEAR RAINFALL INTENSITY(INCH/HR) = 1.464
SUBAREA Tc AND LOSS RATE DATA(AMC I ):
DEVELOPMENT TYPE/          SCS SOIL  AREA      Fp          Ap          SCS  Tc
LAND USE                   GROUP   (ACRES) (INCH/HR) (DECIMAL) CN (MIN.)
APARTMENTS                  B       0.66    0.30    0.200    36   10.69
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.30
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.200
SUBAREA RUNOFF(CFS) = 0.83
TOTAL AREA(ACRES) = 0.66 PEAK FLOW RATE(CFS) = 0.83

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FLOW PROCESS FROM NODE 70.00 TO NODE 70.00 IS CODE = 1

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>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<
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TOTAL NUMBER OF STREAMS = 2
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
 TIME OF CONCENTRATION(MIN.) = 10.69
 RAINFALL INTENSITY(INCH/HR) = 1.46
 AREA-AVERAGED Fm(INCH/HR) = 0.06
 AREA-AVERAGED Fp(INCH/HR) = 0.30
 AREA-AVERAGED Ap = 0.20
 EFFECTIVE STREAM AREA(ACRES) = 0.66
 TOTAL STREAM AREA(ACRES) = 0.66
 PEAK FLOW RATE(CFS) AT CONFLUENCE = 0.83

** CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	1.22	12.05	1.367	0.30(0.06)	0.20	1.0	68.00
2	0.83	10.69	1.464	0.30(0.06)	0.20	0.7	71.00

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
 CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	1.99	10.69	1.464	0.30(0.06)	0.20	1.5	71.00
2	1.99	12.05	1.367	0.30(0.06)	0.20	1.7	68.00

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE(CFS) = 1.99 Tc(MIN.) = 10.69
 EFFECTIVE AREA(ACRES) = 1.55 AREA-AVERAGED Fm(INCH/HR) = 0.06
 AREA-AVERAGED Fp(INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.20
 TOTAL AREA(ACRES) = 1.7
 LONGEST FLOWPATH FROM NODE 68.00 TO NODE 70.00 = 550.00 FEET.

FLOW PROCESS FROM NODE 33.00 TO NODE 33.00 IS CODE = 12

>>>>CLEAR MEMORY BANK # 3 <<<<<

FLOW PROCESS FROM NODE 70.00 TO NODE 70.00 IS CODE = 10

>>>>MAIN-STREAM MEMORY COPIED ONTO MEMORY BANK # 3 <<<<<

FLOW PROCESS FROM NODE 72.00 TO NODE 73.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<

>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

INITIAL SUBAREA FLOW-LENGTH(FEET) = 230.00

ELEVATION DATA: UPSTREAM(FEET) = 168.00 DOWNSTREAM(FEET) = 166.80

$$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$$

SUBAREA ANALYSIS USED MINIMUM T_c (MIN.) = 8.161

* 2 YEAR RAINFALL INTENSITY (INCH/HR) = 1.709

SUBAREA T_c AND LOSS RATE DATA (AMC I):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	F_p (INCH/HR)	A_p (DECIMAL)	SCS CN	T_c (MIN.)
APARTMENTS	B	0.28	0.30	0.200	36	8.16

SUBAREA AVERAGE PERVIOUS LOSS RATE, F_p (INCH/HR) = 0.30
SUBAREA AVERAGE PERVIOUS AREA FRACTION, A_p = 0.200
SUBAREA RUNOFF (CFS) = 0.42
TOTAL AREA (ACRES) = 0.28 PEAK FLOW RATE (CFS) = 0.42

FLOW PROCESS FROM NODE 73.00 TO NODE 70.00 IS CODE = 31

>>>> COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA <<<<<

>>>> USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW) <<<<<

ELEVATION DATA: UPSTREAM (FEET) = 160.00 DOWNSTREAM (FEET) = 159.00

FLOW LENGTH (FEET) = 270.00 MANNING'S N = 0.013

ESTIMATED PIPE DIAMETER (INCH) INCREASED TO 12.000

DEPTH OF FLOW IN 12.0 INCH PIPE IS 3.6 INCHES

PIPE-FLOW VELOCITY (FEET/SEC.) = 2.10

ESTIMATED PIPE DIAMETER (INCH) = 12.000 NUMBER OF PIPES = 1

PIPE-FLOW (CFS) = 0.42

PIPE TRAVEL TIME (MIN.) = 2.14 T_c (MIN.) = 10.30

LONGEST FLOWPATH FROM NODE 72.00 TO NODE 70.00 = 500.00 FEET.

FLOW PROCESS FROM NODE 70.00 TO NODE 70.00 IS CODE = 1

>>>> DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE <<<<<

TOTAL NUMBER OF STREAMS = 2

CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:

TIME OF CONCENTRATION (MIN.) = 10.30

RAINFALL INTENSITY (INCH/HR) = 1.49

AREA-AVERAGED F_m (INCH/HR) = 0.06

AREA-AVERAGED F_p (INCH/HR) = 0.30

AREA-AVERAGED A_p = 0.20

EFFECTIVE STREAM AREA (ACRES) = 0.28

TOTAL STREAM AREA (ACRES) = 0.28

PEAK FLOW RATE (CFS) AT CONFLUENCE = 0.42

FLOW PROCESS FROM NODE 72.00 TO NODE 70.00 IS CODE = 21

>>>> RATIONAL METHOD INITIAL SUBAREA ANALYSIS <<<<<

>> USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA <<

INITIAL SUBAREA FLOW-LENGTH (FEET) = 390.00

ELEVATION DATA: UPSTREAM (FEET) = 167.20 DOWNSTREAM (FEET) = 166.80

$$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$$

SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) = 13.956
 * 2 YEAR RAINFALL INTENSITY(INCH/HR) = 1.256
 SUBAREA Tc AND LOSS RATE DATA(AMC I):
 DEVELOPMENT TYPE/ SCS SOIL AREA Fp Ap SCS Tc
 LAND USE GROUP (ACRES) (INCH/HR) (DECIMAL) CN (MIN.)
 APARTMENTS B 1.49 0.30 0.200 36 13.96
 SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.30
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.200
 SUBAREA RUNOFF(CFS) = 1.60
 TOTAL AREA(ACRES) = 1.49 PEAK FLOW RATE(CFS) = 1.60

 FLOW PROCESS FROM NODE 70.00 TO NODE 70.00 IS CODE = 1

 >>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
 >>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<

=====

TOTAL NUMBER OF STREAMS = 2
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
 TIME OF CONCENTRATION(MIN.) = 13.96
 RAINFALL INTENSITY(INCH/HR) = 1.26
 AREA-AVERAGED Fm(INCH/HR) = 0.06
 AREA-AVERAGED Fp(INCH/HR) = 0.30
 AREA-AVERAGED Ap = 0.20
 EFFECTIVE STREAM AREA(ACRES) = 1.49
 TOTAL STREAM AREA(ACRES) = 1.49
 PEAK FLOW RATE(CFS) AT CONFLUENCE = 1.60

** CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	0.42	10.30	1.495	0.30 (0.06)	0.20	0.3	72.00
2	1.60	13.96	1.256	0.30 (0.06)	0.20	1.5	72.00

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
 CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	1.84	10.30	1.495	0.30 (0.06)	0.20	1.4	72.00
2	1.95	13.96	1.256	0.30 (0.06)	0.20	1.8	72.00

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE(CFS) = 1.95 Tc(MIN.) = 13.96
 EFFECTIVE AREA(ACRES) = 1.77 AREA-AVERAGED Fm(INCH/HR) = 0.06
 AREA-AVERAGED Fp(INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.20
 TOTAL AREA(ACRES) = 1.8
 LONGEST FLOWPATH FROM NODE 72.00 TO NODE 70.00 = 500.00 FEET.

 FLOW PROCESS FROM NODE 70.00 TO NODE 70.00 IS CODE = 11

 >>>>CONFLUENCE MEMORY BANK # 3 WITH THE MAIN-STREAM MEMORY<<<<<

=====
** MAIN STREAM CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	1.84	10.30	1.495	0.30 (0.06)	0.20	1.4	72.00
2	1.95	13.96	1.256	0.30 (0.06)	0.20	1.8	72.00

LONGEST FLOWPATH FROM NODE 72.00 TO NODE 70.00 = 500.00 FEET.

** MEMORY BANK # 3 CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	1.99	10.69	1.464	0.30 (0.06)	0.20	1.5	71.00
2	1.99	12.05	1.367	0.30 (0.06)	0.20	1.7	68.00

LONGEST FLOWPATH FROM NODE 68.00 TO NODE 70.00 = 550.00 FEET.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	3.80	10.30	1.495	0.30 (0.06)	0.20	2.9	72.00
2	3.84	10.69	1.464	0.30 (0.06)	0.20	3.0	71.00
3	3.88	12.05	1.367	0.30 (0.06)	0.20	3.2	68.00
4	3.77	13.96	1.256	0.30 (0.06)	0.20	3.4	72.00

TOTAL AREA (ACRES) = 3.4

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE (CFS) = 3.88 Tc (MIN.) = 12.045
EFFECTIVE AREA (ACRES) = 3.23 AREA-AVERAGED Fm (INCH/HR) = 0.06
AREA-AVERAGED Fp (INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.20
TOTAL AREA (ACRES) = 3.4
LONGEST FLOWPATH FROM NODE 68.00 TO NODE 70.00 = 550.00 FEET.

FLOW PROCESS FROM NODE 70.00 TO NODE 70.00 IS CODE = 12

=====
>>>>CLEAR MEMORY BANK # 3 <<<<<

FLOW PROCESS FROM NODE 70.00 TO NODE 67.00 IS CODE = 31

=====
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<

>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW) <<<<<

=====
ELEVATION DATA: UPSTREAM (FEET) = 155.00 DOWNSTREAM (FEET) = 148.50
FLOW LENGTH (FEET) = 30.00 MANNING'S N = 0.013
ESTIMATED PIPE DIAMETER (INCH) INCREASED TO 12.000
DEPTH OF FLOW IN 12.0 INCH PIPE IS 4.0 INCHES
PIPE-FLOW VELOCITY (FEET/SEC.) = 16.91
ESTIMATED PIPE DIAMETER (INCH) = 12.00 NUMBER OF PIPES = 1
PIPE-FLOW (CFS) = 3.88
PIPE TRAVEL TIME (MIN.) = 0.03 Tc (MIN.) = 12.07
LONGEST FLOWPATH FROM NODE 68.00 TO NODE 67.00 = 580.00 FEET.

FLOW PROCESS FROM NODE 67.00 TO NODE 67.00 IS CODE = 11

>>>>CONFLUENCE MEMORY BANK # 1 WITH THE MAIN-STREAM MEMORY<<<<<

** MAIN STREAM CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	3.80	10.33	1.492	0.30 (0.06)	0.20	2.9	72.00
2	3.84	10.72	1.461	0.30 (0.06)	0.20	3.0	71.00
3	3.88	12.07	1.365	0.30 (0.06)	0.20	3.2	68.00
4	3.77	13.99	1.254	0.30 (0.06)	0.20	3.4	72.00
LONGEST FLOWPATH FROM NODE				68.00 TO NODE	67.00 =	580.00 FEET.	

** MEMORY BANK # 1 CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	25.00	9.27	1.588	0.30 (0.08)	0.25	14.4	65.00
2	26.70	10.80	1.455	0.30 (0.08)	0.25	17.1	58.00
3	26.77	10.87	1.450	0.30 (0.08)	0.25	17.2	51.00
4	27.09	11.18	1.426	0.30 (0.08)	0.25	17.8	59.00
5	27.15	11.24	1.422	0.30 (0.08)	0.25	17.9	55.00
6	27.49	11.60	1.397	0.30 (0.08)	0.25	18.5	63.00
7	27.49	11.60	1.396	0.30 (0.08)	0.25	18.5	43.00
8	27.63	11.77	1.385	0.30 (0.08)	0.25	18.8	45.00
9	27.98	12.26	1.353	0.30 (0.08)	0.25	19.7	30.00
10	28.00	12.31	1.350	0.30 (0.08)	0.25	19.7	60.00
11	28.11	12.49	1.339	0.30 (0.08)	0.25	20.0	38.00
12	28.32	12.88	1.315	0.30 (0.07)	0.25	20.7	30.00
13	28.37	13.00	1.308	0.30 (0.07)	0.25	20.8	36.00
14	28.71	13.90	1.259	0.30 (0.07)	0.24	22.2	38.00
15	28.90	14.51	1.228	0.30 (0.07)	0.24	23.0	14.00
16	28.93	14.59	1.224	0.30 (0.07)	0.24	23.2	27.00
17	28.95	14.67	1.220	0.30 (0.07)	0.24	23.3	26.00
18	29.14	15.35	1.189	0.30 (0.07)	0.24	24.3	20.00
19	29.18	15.67	1.175	0.30 (0.07)	0.24	24.7	23.00
20	29.18	15.83	1.168	0.30 (0.07)	0.24	24.9	29.00
21	29.18	15.86	1.167	0.30 (0.07)	0.24	24.9	17.00
22	29.18	15.93	1.164	0.30 (0.07)	0.24	25.0	50.00
23	28.88	16.59	1.137	0.30 (0.07)	0.24	25.5	34.00
24	28.69	17.01	1.121	0.30 (0.07)	0.24	25.8	39.00
25	28.28	17.63	1.098	0.30 (0.07)	0.24	26.1	24.00
26	25.80	21.98	0.968	0.30 (0.07)	0.23	27.9	12.00
27	25.63	22.34	0.959	0.30 (0.07)	0.23	28.1	64.00
28	24.31	25.18	0.895	0.30 (0.07)	0.22	29.1	32.00
29	23.81	25.95	0.880	0.30 (0.07)	0.22	29.1	10.00
LONGEST FLOWPATH FROM NODE				10.00 TO NODE	67.00 =	2660.00 FEET.	

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	28.63	9.27	1.588	0.30 (0.07)	0.24	17.0	65.00
2	29.98	10.33	1.492	0.30 (0.07)	0.24	19.1	72.00

3	30.45	10.72	1.461	0.30	(0.07)	0.24	19.9	71.00
4	30.54	10.80	1.455	0.30	(0.07)	0.24	20.1	58.00
5	30.61	10.87	1.450	0.30	(0.07)	0.24	20.2	51.00
6	30.95	11.18	1.426	0.30	(0.07)	0.24	20.8	59.00
7	31.01	11.24	1.422	0.30	(0.07)	0.24	20.9	55.00
8	31.35	11.60	1.397	0.30	(0.07)	0.24	21.6	63.00
9	31.36	11.60	1.396	0.30	(0.07)	0.24	21.7	43.00
10	31.50	11.77	1.385	0.30	(0.07)	0.24	22.0	45.00
11	31.73	12.07	1.365	0.30	(0.07)	0.24	22.6	68.00
12	31.85	12.26	1.353	0.30	(0.07)	0.24	22.9	30.00
13	31.87	12.31	1.350	0.30	(0.07)	0.24	23.0	60.00
14	31.96	12.49	1.339	0.30	(0.07)	0.24	23.3	38.00
15	32.15	12.88	1.315	0.30	(0.07)	0.24	24.0	30.00
16	32.19	13.00	1.308	0.30	(0.07)	0.24	24.2	36.00
17	32.49	13.90	1.259	0.30	(0.07)	0.24	25.6	38.00
18	32.51	13.99	1.254	0.30	(0.07)	0.24	25.7	72.00
19	32.59	14.51	1.228	0.30	(0.07)	0.24	26.5	14.00
20	32.60	14.59	1.224	0.30	(0.07)	0.24	26.6	27.00
21	32.62	14.67	1.220	0.30	(0.07)	0.24	26.7	26.00
22	32.70	15.35	1.189	0.30	(0.07)	0.23	27.7	20.00
23	32.70	15.67	1.175	0.30	(0.07)	0.23	28.1	23.00
24	32.68	15.83	1.168	0.30	(0.07)	0.23	28.3	29.00
25	32.68	15.86	1.167	0.30	(0.07)	0.23	28.4	17.00
26	32.67	15.93	1.164	0.30	(0.07)	0.23	28.5	50.00
27	32.29	16.59	1.137	0.30	(0.07)	0.23	28.9	34.00
28	32.04	17.01	1.121	0.30	(0.07)	0.23	29.2	39.00
29	31.56	17.63	1.098	0.30	(0.07)	0.23	29.5	24.00
30	28.67	21.98	0.968	0.30	(0.07)	0.23	31.4	12.00
31	28.47	22.34	0.959	0.30	(0.07)	0.23	31.5	64.00
32	26.95	25.18	0.895	0.30	(0.07)	0.22	32.5	32.00
33	26.40	25.95	0.880	0.30	(0.07)	0.22	32.5	10.00
TOTAL AREA (ACRES) =			32.5					

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE (CFS) = 32.70 Tc (MIN.) = 15.354
EFFECTIVE AREA (ACRES) = 27.72 AREA-AVERAGED Fm (INCH/HR) = 0.07
AREA-AVERAGED Fp (INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.23
TOTAL AREA (ACRES) = 32.5
LONGEST FLOWPATH FROM NODE 10.00 TO NODE 67.00 = 2660.00 FEET.

FLOW PROCESS FROM NODE 67.00 TO NODE 74.00 IS CODE = 31

>>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<

=====

ELEVATION DATA: UPSTREAM (FEET) = 148.50 DOWNSTREAM (FEET) = 148.00
FLOW LENGTH (FEET) = 280.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 39.0 INCH PIPE IS 30.7 INCHES
PIPE-FLOW VELOCITY (FEET/SEC.) = 4.67
ESTIMATED PIPE DIAMETER (INCH) = 39.00 NUMBER OF PIPES = 1
PIPE-FLOW (CFS) = 32.70
PIPE TRAVEL TIME (MIN.) = 1.00 Tc (MIN.) = 16.35
LONGEST FLOWPATH FROM NODE 10.00 TO NODE 74.00 = 2940.00 FEET.

FLOW PROCESS FROM NODE 74.00 TO NODE 74.00 IS CODE = 10

>>>>MAIN-STREAM MEMORY COPIED ONTO MEMORY BANK # 3 <<<<<
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FLOW PROCESS FROM NODE 75.00 TO NODE 76.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<
=====

INITIAL SUBAREA FLOW-LENGTH (FEET) = 275.00
ELEVATION DATA: UPSTREAM (FEET) = 167.00 DOWNSTREAM (FEET) = 165.30

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$
SUBAREA ANALYSIS USED MINIMUM T_c (MIN.) = 9.415
* 2 YEAR RAINFALL INTENSITY (INCH/HR) = 1.574

SUBAREA T_c AND LOSS RATE DATA (AMC I):
DEVELOPMENT TYPE/ SCS SOIL AREA F_p A_p SCS T_c
LAND USE GROUP (ACRES) (INCH/HR) (DECIMAL) CN (MIN.)
CONDOMINIUMS B 0.70 0.30 0.350 36 9.41
SUBAREA AVERAGE PERVIOUS LOSS RATE, F_p (INCH/HR) = 0.30
SUBAREA AVERAGE PERVIOUS AREA FRACTION, A_p = 0.350
SUBAREA RUNOFF (CFS) = 0.93
TOTAL AREA (ACRES) = 0.70 PEAK FLOW RATE (CFS) = 0.93

FLOW PROCESS FROM NODE 76.00 TO NODE 76.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
=====

TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION (MIN.) = 9.41
RAINFALL INTENSITY (INCH/HR) = 1.57
AREA-AVERAGED F_m (INCH/HR) = 0.11
AREA-AVERAGED F_p (INCH/HR) = 0.30
AREA-AVERAGED A_p = 0.35
EFFECTIVE STREAM AREA (ACRES) = 0.70
TOTAL STREAM AREA (ACRES) = 0.70
PEAK FLOW RATE (CFS) AT CONFLUENCE = 0.93

FLOW PROCESS FROM NODE 77.00 TO NODE 76.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<
=====

INITIAL SUBAREA FLOW-LENGTH (FEET) = 255.00
ELEVATION DATA: UPSTREAM (FEET) = 167.00 DOWNSTREAM (FEET) = 166.30

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$

SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) = 10.745
 * 2 YEAR RAINFALL INTENSITY(INCH/HR) = 1.459
 SUBAREA Tc AND LOSS RATE DATA(AMC I):
 DEVELOPMENT TYPE/ SCS SOIL AREA Fp Ap SCS Tc
 LAND USE GROUP (ACRES) (INCH/HR) (DECIMAL) CN (MIN.)
 CONDOMINIUMS B 0.51 0.30 0.350 36 10.75
 SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.30
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.350
 SUBAREA RUNOFF(CFS) = 0.62
 TOTAL AREA(ACRES) = 0.51 PEAK FLOW RATE(CFS) = 0.62

 FLOW PROCESS FROM NODE 76.00 TO NODE 76.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
 >>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<

=====

TOTAL NUMBER OF STREAMS = 2
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
 TIME OF CONCENTRATION(MIN.) = 10.75
 RAINFALL INTENSITY(INCH/HR) = 1.46
 AREA-AVERAGED Fm(INCH/HR) = 0.11
 AREA-AVERAGED Fp(INCH/HR) = 0.30
 AREA-AVERAGED Ap = 0.35
 EFFECTIVE STREAM AREA(ACRES) = 0.51
 TOTAL STREAM AREA(ACRES) = 0.51
 PEAK FLOW RATE(CFS) AT CONFLUENCE = 0.62

** CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	0.93	9.41	1.574	0.30 (0.11)	0.35	0.7	75.00
2	0.62	10.75	1.459	0.30 (0.11)	0.35	0.5	77.00

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
 CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	1.52	9.41	1.574	0.30 (0.11)	0.35	1.1	75.00
2	1.47	10.75	1.459	0.30 (0.10)	0.35	1.2	77.00

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE(CFS) = 1.52 Tc(MIN.) = 9.41
 EFFECTIVE AREA(ACRES) = 1.15 AREA-AVERAGED Fm(INCH/HR) = 0.10
 AREA-AVERAGED Fp(INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.35
 TOTAL AREA(ACRES) = 1.2
 LONGEST FLOWPATH FROM NODE 75.00 TO NODE 76.00 = 275.00 FEET.

 FLOW PROCESS FROM NODE 76.00 TO NODE 79.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<

>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 154.00 DOWNSTREAM(FEET) = 152.00
FLOW LENGTH(FEET) = 300.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 12.0 INCH PIPE IS 6.2 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 3.68
ESTIMATED PIPE DIAMETER(INCH) = 12.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 1.52
PIPE TRAVEL TIME(MIN.) = 1.36 Tc(MIN.) = 10.77
LONGEST FLOWPATH FROM NODE 75.00 TO NODE 79.00 = 575.00 FEET.

FLOW PROCESS FROM NODE 79.00 TO NODE 79.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

=====

TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION(MIN.) = 10.77
RAINFALL INTENSITY(INCH/HR) = 1.46
AREA-AVERAGED Fm(INCH/HR) = 0.10
AREA-AVERAGED Fp(INCH/HR) = 0.30
AREA-AVERAGED Ap = 0.35
EFFECTIVE STREAM AREA(ACRES) = 1.15
TOTAL STREAM AREA(ACRES) = 1.21
PEAK FLOW RATE(CFS) AT CONFLUENCE = 1.52

FLOW PROCESS FROM NODE 78.00 TO NODE 79.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

=====

INITIAL SUBAREA FLOW-LENGTH(FEET) = 200.00
ELEVATION DATA: UPSTREAM(FEET) = 165.90 DOWNSTREAM(FEET) = 165.30

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$
SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) = 9.578
* 2 YEAR RAINFALL INTENSITY(INCH/HR) = 1.559

SUBAREA Tc AND LOSS RATE DATA(AMC I):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN	Tc (MIN.)
CONDOMINIUMS	B	0.40	0.30	0.350	36	9.58

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.30

SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.350

SUBAREA RUNOFF(CFS) = 0.52

TOTAL AREA(ACRES) = 0.40 PEAK FLOW RATE(CFS) = 0.52

FLOW PROCESS FROM NODE 79.00 TO NODE 79.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<

TOTAL NUMBER OF STREAMS = 2
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
 TIME OF CONCENTRATION(MIN.) = 9.58
 RAINFALL INTENSITY(INCH/HR) = 1.56
 AREA-AVERAGED Fm(INCH/HR) = 0.11
 AREA-AVERAGED Fp(INCH/HR) = 0.30
 AREA-AVERAGED Ap = 0.35
 EFFECTIVE STREAM AREA(ACRES) = 0.40
 TOTAL STREAM AREA(ACRES) = 0.40
 PEAK FLOW RATE(CFS) AT CONFLUENCE = 0.52

** CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	1.52	10.77	1.457	0.30(0.11)	0.35	1.1	75.00
1	1.47	12.12	1.362	0.30(0.10)	0.35	1.2	77.00
2	0.52	9.58	1.559	0.30(0.11)	0.35	0.4	78.00

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
 CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	1.97	9.58	1.559	0.30(0.10)	0.35	1.4	78.00
2	2.00	10.77	1.457	0.30(0.11)	0.35	1.5	75.00
3	1.93	12.12	1.362	0.30(0.10)	0.35	1.6	77.00

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE(CFS) = 2.00 Tc(MIN.) = 10.77
 EFFECTIVE AREA(ACRES) = 1.55 AREA-AVERAGED Fm(INCH/HR) = 0.11
 AREA-AVERAGED Fp(INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.35
 TOTAL AREA(ACRES) = 1.6
 LONGEST FLOWPATH FROM NODE 75.00 TO NODE 79.00 = 575.00 FEET.

 FLOW PROCESS FROM NODE 79.00 TO NODE 74.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
 >>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 152.00 DOWNSTREAM(FEET) = 148.00
 FLOW LENGTH(FEET) = 50.00 MANNING'S N = 0.013
 ESTIMATED PIPE DIAMETER(INCH) INCREASED TO 12.000
 DEPTH OF FLOW IN 12.0 INCH PIPE IS 3.7 INCHES
 PIPE-FLOW VELOCITY(FEET/SEC.) = 9.82
 ESTIMATED PIPE DIAMETER(INCH) = 12.00 NUMBER OF PIPES = 1
 PIPE-FLOW(CFS) = 2.00
 PIPE TRAVEL TIME(MIN.) = 0.08 Tc(MIN.) = 10.86
 LONGEST FLOWPATH FROM NODE 75.00 TO NODE 74.00 = 625.00 FEET.

 FLOW PROCESS FROM NODE 74.00 TO NODE 74.00 IS CODE = 11

>>>>>CONFLUENCE MEMORY BANK # 3 WITH THE MAIN-STREAM MEMORY<<<<<

=====
 ** MAIN STREAM CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	1.97	9.66	1.551	0.30 (0.10)	0.35	1.4	78.00
2	2.00	10.86	1.450	0.30 (0.11)	0.35	1.5	75.00
3	1.93	12.20	1.356	0.30 (0.10)	0.35	1.6	77.00
LONGEST FLOWPATH FROM NODE			75.00	TO NODE	74.00 =	625.00 FEET.	

** MEMORY BANK # 3 CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	28.63	10.29	1.496	0.30 (0.07)	0.24	17.0	65.00
2	29.98	11.34	1.415	0.30 (0.07)	0.24	19.1	72.00
3	30.45	11.72	1.388	0.30 (0.07)	0.24	19.9	71.00
4	30.54	11.81	1.382	0.30 (0.07)	0.24	20.1	58.00
5	30.61	11.87	1.378	0.30 (0.07)	0.24	20.2	51.00
6	30.95	12.19	1.357	0.30 (0.07)	0.24	20.8	59.00
7	31.01	12.25	1.354	0.30 (0.07)	0.24	20.9	55.00
8	31.35	12.60	1.332	0.30 (0.07)	0.24	21.6	63.00
9	31.36	12.60	1.332	0.30 (0.07)	0.24	21.7	43.00
10	31.50	12.77	1.321	0.30 (0.07)	0.24	22.0	45.00
11	31.73	13.08	1.304	0.30 (0.07)	0.24	22.6	68.00
12	31.85	13.26	1.293	0.30 (0.07)	0.24	22.9	30.00
13	31.87	13.31	1.291	0.30 (0.07)	0.24	23.0	60.00
14	31.96	13.49	1.281	0.30 (0.07)	0.24	23.3	38.00
15	32.15	13.89	1.259	0.30 (0.07)	0.24	24.0	30.00
16	32.19	14.00	1.253	0.30 (0.07)	0.24	24.2	36.00
17	32.49	14.90	1.210	0.30 (0.07)	0.24	25.6	38.00
18	32.51	14.99	1.206	0.30 (0.07)	0.24	25.7	72.00
19	32.59	15.51	1.182	0.30 (0.07)	0.24	26.5	14.00
20	32.60	15.59	1.179	0.30 (0.07)	0.24	26.6	27.00
21	32.62	15.67	1.175	0.30 (0.07)	0.24	26.7	26.00
22	32.70	16.35	1.147	0.30 (0.07)	0.23	27.7	20.00
23	32.70	16.67	1.134	0.30 (0.07)	0.23	28.1	23.00
24	32.68	16.83	1.128	0.30 (0.07)	0.23	28.3	29.00
25	32.68	16.86	1.127	0.30 (0.07)	0.23	28.4	17.00
26	32.67	16.93	1.124	0.30 (0.07)	0.23	28.5	50.00
27	32.29	17.59	1.100	0.30 (0.07)	0.23	28.9	34.00
28	32.04	18.01	1.085	0.30 (0.07)	0.23	29.2	39.00
29	31.56	18.63	1.064	0.30 (0.07)	0.23	29.5	24.00
30	28.67	22.99	0.943	0.30 (0.07)	0.23	31.4	12.00
31	28.47	23.36	0.934	0.30 (0.07)	0.23	31.5	64.00
32	26.95	26.23	0.874	0.30 (0.07)	0.22	32.5	32.00
33	26.40	27.01	0.860	0.30 (0.07)	0.22	32.5	10.00
LONGEST FLOWPATH FROM NODE			10.00	TO NODE	74.00 =	2940.00 FEET.	

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	29.91	9.66	1.551	0.30 (0.08)	0.25	17.4	78.00
2	30.62	10.29	1.496	0.30 (0.08)	0.25	18.5	65.00

3	31.37	10.86	1.450	0.30	(0.08)	0.25	19.7	75.00
4	31.96	11.34	1.415	0.30	(0.08)	0.25	20.7	72.00
5	32.40	11.72	1.388	0.30	(0.08)	0.25	21.5	71.00
6	32.49	11.81	1.382	0.30	(0.08)	0.25	21.6	58.00
7	32.56	11.87	1.378	0.30	(0.08)	0.25	21.8	51.00
8	32.88	12.19	1.357	0.30	(0.08)	0.25	22.4	59.00
9	32.89	12.20	1.356	0.30	(0.08)	0.25	22.5	77.00
10	32.93	12.25	1.354	0.30	(0.08)	0.25	22.6	55.00
11	33.24	12.60	1.332	0.30	(0.08)	0.25	23.3	63.00
12	33.25	12.60	1.332	0.30	(0.08)	0.25	23.3	43.00
13	33.37	12.77	1.321	0.30	(0.08)	0.25	23.6	45.00
14	33.57	13.08	1.304	0.30	(0.08)	0.25	24.2	68.00
15	33.68	13.26	1.293	0.30	(0.08)	0.25	24.5	30.00
16	33.70	13.31	1.291	0.30	(0.08)	0.25	24.6	60.00
17	33.77	13.49	1.281	0.30	(0.08)	0.25	24.9	38.00
18	33.93	13.89	1.259	0.30	(0.07)	0.25	25.6	30.00
19	33.96	14.00	1.253	0.30	(0.07)	0.25	25.8	36.00
20	34.19	14.90	1.210	0.30	(0.07)	0.24	27.2	38.00
21	34.20	14.99	1.206	0.30	(0.07)	0.24	27.3	72.00
22	34.25	15.51	1.182	0.30	(0.07)	0.24	28.1	14.00
23	34.26	15.59	1.179	0.30	(0.07)	0.24	28.2	27.00
24	34.26	15.67	1.175	0.30	(0.07)	0.24	28.3	26.00
25	34.31	16.35	1.147	0.30	(0.07)	0.24	29.3	20.00
26	34.28	16.67	1.134	0.30	(0.07)	0.24	29.8	23.00
27	34.26	16.83	1.128	0.30	(0.07)	0.24	30.0	29.00
28	34.25	16.86	1.127	0.30	(0.07)	0.24	30.0	17.00
29	34.24	16.93	1.124	0.30	(0.07)	0.24	30.1	50.00
30	33.82	17.59	1.100	0.30	(0.07)	0.24	30.5	34.00
31	33.55	18.01	1.085	0.30	(0.07)	0.24	30.8	39.00
32	33.04	18.63	1.064	0.30	(0.07)	0.24	31.1	24.00
33	29.96	22.99	0.943	0.30	(0.07)	0.23	33.0	12.00
34	29.74	23.36	0.934	0.30	(0.07)	0.23	33.1	64.00
35	28.14	26.23	0.874	0.30	(0.07)	0.23	34.1	32.00
36	27.56	27.01	0.860	0.30	(0.07)	0.23	34.1	10.00
TOTAL AREA (ACRES) =			34.1					

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE (CFS) = 34.31 Tc (MIN.) = 16.354
EFFECTIVE AREA (ACRES) = 29.33 AREA-AVERAGED Fm (INCH/HR) = 0.07
AREA-AVERAGED Fp (INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.24
TOTAL AREA (ACRES) = 34.1
LONGEST FLOWPATH FROM NODE 10.00 TO NODE 74.00 = 2940.00 FEET.

FLOW PROCESS FROM NODE 74.00 TO NODE 74.00 IS CODE = 12

>>>>CLEAR MEMORY BANK # 3 <<<<<
=====

FLOW PROCESS FROM NODE 74.00 TO NODE 74.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
=====

TOTAL NUMBER OF STREAMS = 2
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
 TIME OF CONCENTRATION(MIN.) = 16.35
 RAINFALL INTENSITY(INCH/HR) = 1.15
 AREA-AVERAGED Fm(INCH/HR) = 0.07
 AREA-AVERAGED Fp(INCH/HR) = 0.30
 AREA-AVERAGED Ap = 0.24
 EFFECTIVE STREAM AREA(ACRES) = 29.33
 TOTAL STREAM AREA(ACRES) = 34.13
 PEAK FLOW RATE(CFS) AT CONFLUENCE = 34.31

 FLOW PROCESS FROM NODE 59.00 TO NODE 74.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
 >>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

=====

INITIAL SUBAREA FLOW-LENGTH(FEET) = 300.00
 ELEVATION DATA: UPSTREAM(FEET) = 166.80 DOWNSTREAM(FEET) = 165.00

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$
 SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) = 9.807
 * 2 YEAR RAINFALL INTENSITY(INCH/HR) = 1.538

SUBAREA Tc AND LOSS RATE DATA(AMC I):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN	Tc (MIN.)
CONDOMINIUMS	B	0.41	0.30	0.350	36	9.81

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.30

SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.350

SUBAREA RUNOFF(CFS) = 0.53

TOTAL AREA(ACRES) = 0.41 PEAK FLOW RATE(CFS) = 0.53

 FLOW PROCESS FROM NODE 74.00 TO NODE 74.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
 >>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<

=====

TOTAL NUMBER OF STREAMS = 2
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
 TIME OF CONCENTRATION(MIN.) = 9.81
 RAINFALL INTENSITY(INCH/HR) = 1.54
 AREA-AVERAGED Fm(INCH/HR) = 0.11
 AREA-AVERAGED Fp(INCH/HR) = 0.30
 AREA-AVERAGED Ap = 0.35
 EFFECTIVE STREAM AREA(ACRES) = 0.41
 TOTAL STREAM AREA(ACRES) = 0.41
 PEAK FLOW RATE(CFS) AT CONFLUENCE = 0.53

** CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	29.91	9.66	1.551	0.30(0.08)	0.25	17.4	78.00
1	30.62	10.29	1.496	0.30(0.08)	0.25	18.5	65.00

1	31.37	10.86	1.450	0.30 (0.08)	0.25	19.7	75.00
1	31.96	11.34	1.415	0.30 (0.08)	0.25	20.7	72.00
1	32.40	11.72	1.388	0.30 (0.08)	0.25	21.5	71.00
1	32.49	11.81	1.382	0.30 (0.08)	0.25	21.6	58.00
1	32.56	11.87	1.378	0.30 (0.08)	0.25	21.8	51.00
1	32.88	12.19	1.357	0.30 (0.08)	0.25	22.4	59.00
1	32.89	12.20	1.356	0.30 (0.08)	0.25	22.5	77.00
1	32.93	12.25	1.354	0.30 (0.08)	0.25	22.6	55.00
1	33.24	12.60	1.332	0.30 (0.08)	0.25	23.3	63.00
1	33.25	12.60	1.332	0.30 (0.08)	0.25	23.3	43.00
1	33.37	12.77	1.321	0.30 (0.08)	0.25	23.6	45.00
1	33.57	13.08	1.304	0.30 (0.08)	0.25	24.2	68.00
1	33.68	13.26	1.293	0.30 (0.08)	0.25	24.5	30.00
1	33.70	13.31	1.291	0.30 (0.08)	0.25	24.6	60.00
1	33.77	13.49	1.281	0.30 (0.08)	0.25	24.9	38.00
1	33.93	13.89	1.259	0.30 (0.07)	0.25	25.6	30.00
1	33.96	14.00	1.253	0.30 (0.07)	0.25	25.8	36.00
1	34.19	14.90	1.210	0.30 (0.07)	0.24	27.2	38.00
1	34.20	14.99	1.206	0.30 (0.07)	0.24	27.3	72.00
1	34.25	15.51	1.182	0.30 (0.07)	0.24	28.1	14.00
1	34.26	15.59	1.179	0.30 (0.07)	0.24	28.2	27.00
1	34.26	15.67	1.175	0.30 (0.07)	0.24	28.3	26.00
1	34.31	16.35	1.147	0.30 (0.07)	0.24	29.3	20.00
1	34.28	16.67	1.134	0.30 (0.07)	0.24	29.8	23.00
1	34.26	16.83	1.128	0.30 (0.07)	0.24	30.0	29.00
1	34.25	16.86	1.127	0.30 (0.07)	0.24	30.0	17.00
1	34.24	16.93	1.124	0.30 (0.07)	0.24	30.1	50.00
1	33.82	17.59	1.100	0.30 (0.07)	0.24	30.5	34.00
1	33.55	18.01	1.085	0.30 (0.07)	0.24	30.8	39.00
1	33.04	18.63	1.064	0.30 (0.07)	0.24	31.1	24.00
1	29.96	22.99	0.943	0.30 (0.07)	0.23	33.0	12.00
1	29.74	23.36	0.934	0.30 (0.07)	0.23	33.1	64.00
1	28.14	26.23	0.874	0.30 (0.07)	0.23	34.1	32.00
1	27.56	27.01	0.860	0.30 (0.07)	0.23	34.1	10.00
2	0.53	9.81	1.538	0.30 (0.11)	0.35	0.4	59.00

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	30.43	9.66	1.551	0.30 (0.08)	0.25	17.8	78.00
2	30.60	9.81	1.538	0.30 (0.08)	0.25	18.1	59.00
3	31.14	10.29	1.496	0.30 (0.08)	0.25	18.9	65.00
4	31.86	10.86	1.450	0.30 (0.08)	0.25	20.1	75.00
5	32.44	11.34	1.415	0.30 (0.08)	0.25	21.1	72.00
6	32.88	11.72	1.388	0.30 (0.08)	0.25	21.9	71.00
7	32.96	11.81	1.382	0.30 (0.08)	0.25	22.1	58.00
8	33.03	11.87	1.378	0.30 (0.08)	0.25	22.2	51.00
9	33.34	12.19	1.357	0.30 (0.08)	0.25	22.8	59.00
10	33.35	12.20	1.356	0.30 (0.08)	0.25	22.9	77.00
11	33.39	12.25	1.354	0.30 (0.08)	0.25	23.0	55.00
12	33.69	12.60	1.332	0.30 (0.08)	0.25	23.7	63.00

13	33.70	12.60	1.332	0.30	(0.08)	0.25	23.7	43.00
14	33.82	12.77	1.321	0.30	(0.08)	0.25	24.0	45.00
15	34.02	13.08	1.304	0.30	(0.08)	0.25	24.6	68.00
16	34.12	13.26	1.293	0.30	(0.08)	0.25	24.9	30.00
17	34.14	13.31	1.291	0.30	(0.08)	0.25	25.0	60.00
18	34.21	13.49	1.281	0.30	(0.08)	0.25	25.3	38.00
19	34.35	13.89	1.259	0.30	(0.08)	0.25	26.0	30.00
20	34.39	14.00	1.253	0.30	(0.08)	0.25	26.2	36.00
21	34.59	14.90	1.210	0.30	(0.07)	0.25	27.6	38.00
22	34.61	14.99	1.206	0.30	(0.07)	0.25	27.7	72.00
23	34.64	15.51	1.182	0.30	(0.07)	0.24	28.5	14.00
24	34.65	15.59	1.179	0.30	(0.07)	0.24	28.6	27.00
25	34.66	15.67	1.175	0.30	(0.07)	0.24	28.7	26.00
26	34.69	16.35	1.147	0.30	(0.07)	0.24	29.7	20.00
27	34.66	16.67	1.134	0.30	(0.07)	0.24	30.2	23.00
28	34.64	16.83	1.128	0.30	(0.07)	0.24	30.4	29.00
29	34.63	16.86	1.127	0.30	(0.07)	0.24	30.4	17.00
30	34.61	16.93	1.124	0.30	(0.07)	0.24	30.5	50.00
31	34.19	17.59	1.100	0.30	(0.07)	0.24	31.0	34.00
32	33.91	18.01	1.085	0.30	(0.07)	0.24	31.2	39.00
33	33.39	18.63	1.064	0.30	(0.07)	0.24	31.5	24.00
34	30.27	22.99	0.943	0.30	(0.07)	0.23	33.4	12.00
35	30.05	23.36	0.934	0.30	(0.07)	0.23	33.5	64.00
36	28.42	26.23	0.874	0.30	(0.07)	0.23	34.5	32.00
37	27.84	27.01	0.860	0.30	(0.07)	0.23	34.5	10.00

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE (CFS) = 34.69 Tc (MIN.) = 16.35
EFFECTIVE AREA (ACRES) = 29.74 AREA-AVERAGED Fm (INCH/HR) = 0.07
AREA-AVERAGED Fp (INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.24
TOTAL AREA (ACRES) = 34.5
LONGEST FLOWPATH FROM NODE 10.00 TO NODE 74.00 = 2940.00 FEET.

FLOW PROCESS FROM NODE 80.00 TO NODE 81.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

INITIAL SUBAREA FLOW-LENGTH (FEET) = 290.00
ELEVATION DATA: UPSTREAM (FEET) = 166.40 DOWNSTREAM (FEET) = 165.90

Tc = K * [(LENGTH** 3.00) / (ELEVATION CHANGE)] ** 0.20
SUBAREA ANALYSIS USED MINIMUM Tc (MIN.) = 10.484
* 2 YEAR RAINFALL INTENSITY (INCH/HR) = 1.480

SUBAREA Tc AND LOSS RATE DATA (AMC I):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN	Tc (MIN.)
COMMERCIAL	B	0.76	0.30	0.100	36	10.48

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp (INCH/HR) = 0.30

SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.100

SUBAREA RUNOFF (CFS) = 0.99

TOTAL AREA (ACRES) = 0.76 PEAK FLOW RATE (CFS) = 0.99

FLOW PROCESS FROM NODE 81.00 TO NODE 82.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<

ELEVATION DATA: UPSTREAM(FEET) = 153.00 DOWNSTREAM(FEET) = 147.60
FLOW LENGTH(FEET) = 320.00 MANNING'S N = 0.013
ESTIMATED PIPE DIAMETER(INCH) INCREASED TO 12.000
DEPTH OF FLOW IN 12.0 INCH PIPE IS 3.8 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 4.61
ESTIMATED PIPE DIAMETER(INCH) = 12.000 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 0.99
PIPE TRAVEL TIME(MIN.) = 1.16 Tc(MIN.) = 11.64
LONGEST FLOWPATH FROM NODE 80.00 TO NODE 82.00 = 610.00 FEET.

FLOW PROCESS FROM NODE 82.00 TO NODE 82.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION(MIN.) = 11.64
RAINFALL INTENSITY(INCH/HR) = 1.39
AREA-AVERAGED Fm(INCH/HR) = 0.03
AREA-AVERAGED Fp(INCH/HR) = 0.30
AREA-AVERAGED Ap = 0.10
EFFECTIVE STREAM AREA(ACRES) = 0.76
TOTAL STREAM AREA(ACRES) = 0.76
PEAK FLOW RATE(CFS) AT CONFLUENCE = 0.99

FLOW PROCESS FROM NODE 81.00 TO NODE 82.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

INITIAL SUBAREA FLOW-LENGTH(FEET) = 320.00
ELEVATION DATA: UPSTREAM(FEET) = 165.90 DOWNSTREAM(FEET) = 165.40

Tc = K*[(LENGTH** 3.00)/(ELEVATION CHANGE)]**0.20

SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) = 11.122

* 2 YEAR RAINFALL INTENSITY(INCH/HR) = 1.431

SUBAREA Tc AND LOSS RATE DATA(AMC I):

DEVELOPMENT TYPE/ SCS SOIL AREA Fp Ap SCS Tc
LAND USE GROUP (ACRES) (INCH/HR) (DECIMAL) CN (MIN.)
COMMERCIAL B 0.59 0.30 0.100 36 11.12

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.30

SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.100

SUBAREA RUNOFF(CFS) = 0.74

TOTAL AREA(ACRES) = 0.59 PEAK FLOW RATE(CFS) = 0.74

FLOW PROCESS FROM NODE 82.00 TO NODE 82.00 IS CODE = 1

>>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<<
>>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<<

TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
TIME OF CONCENTRATION(MIN.) = 11.12
RAINFALL INTENSITY(INCH/HR) = 1.43
AREA-AVERAGED Fm(INCH/HR) = 0.03
AREA-AVERAGED Fp(INCH/HR) = 0.30
AREA-AVERAGED Ap = 0.10
EFFECTIVE STREAM AREA(ACRES) = 0.59
TOTAL STREAM AREA(ACRES) = 0.59
PEAK FLOW RATE(CFS) AT CONFLUENCE = 0.74

** CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	0.99	11.64	1.394	0.30(0.03)	0.10	0.8	80.00
2	0.74	11.12	1.431	0.30(0.03)	0.10	0.6	81.00

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	1.72	11.12	1.431	0.30(0.03)	0.10	1.3	81.00
2	1.72	11.64	1.394	0.30(0.03)	0.10	1.3	80.00

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE(CFS) = 1.72 Tc(MIN.) = 11.12
EFFECTIVE AREA(ACRES) = 1.32 AREA-AVERAGED Fm(INCH/HR) = 0.03
AREA-AVERAGED Fp(INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.10
TOTAL AREA(ACRES) = 1.3
LONGEST FLOWPATH FROM NODE 80.00 TO NODE 82.00 = 610.00 FEET.

END OF STUDY SUMMARY:

TOTAL AREA(ACRES) = 1.3 TC(MIN.) = 11.12
EFFECTIVE AREA(ACRES) = 1.32 AREA-AVERAGED Fm(INCH/HR) = 0.03
AREA-AVERAGED Fp(INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.100
PEAK FLOW RATE(CFS) = 1.72

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	1.72	11.12	1.431	0.30(0.03)	0.10	1.3	81.00
2	1.72	11.64	1.394	0.30(0.03)	0.10	1.3	80.00

END OF RATIONAL METHOD ANALYSIS

RATIONAL METHOD HYDROLOGY COMPUTER PROGRAM PACKAGE
(Reference: 1986 ORANGE COUNTY HYDROLOGY CRITERION)
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Analysis prepared by:

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***** DESCRIPTION OF STUDY *****
* FREEMAN *
* PROPOSED CONDITION *
* 10 YEAR ANALYSIS *

FILE NAME: FRPR10.DAT
TIME/DATE OF STUDY: 09:20 05/29/2017

=====

USER SPECIFIED HYDROLOGY AND HYDRAULIC MODEL INFORMATION:

=====

--*TIME-OF-CONCENTRATION MODEL*--

USER SPECIFIED STORM EVENT (YEAR) = 10.00
SPECIFIED MINIMUM PIPE SIZE (INCH) = 12.00
SPECIFIED PERCENT OF GRADIENTS (DECIMAL) TO USE FOR FRICTION SLOPE = 0.95
DATA BANK RAINFALL USED
ANTECEDENT MOISTURE CONDITION (AMC) II ASSUMED FOR RATIONAL METHOD

USER-DEFINED STREET-SECTIONS FOR COUPLED PIPEFLOW AND STREETFLOW MODEL

NO.	HALF-WIDTH (FT)	CROWN TO CROSSFALL (FT)	STREET-CROSSFALL: IN- / OUT- / SIDE / SIDE / WAY	CURB HEIGHT (FT)	GUTTER-GEOMETRIES: WIDTH (FT)	LIP (FT)	HIKE (FT)	MANNING FACTOR (n)
1	24.0	19.0	0.020/0.020/0.020	0.67	2.00	0.0313	0.167	0.0150
2	18.0	13.0	0.020/0.020/0.020	0.50	1.50	0.0313	0.125	0.0150

GLOBAL STREET FLOW-DEPTH CONSTRAINTS:
1. Relative Flow-Depth = 0.50 FEET
as (Maximum Allowable Street Flow Depth) - (Top-of-Curb)
2. (Depth)*(Velocity) Constraint = 6.0 (FT*FT/S)
*SIZE PIPE WITH A FLOW CAPACITY GREATER THAN
OR EQUAL TO THE UPSTREAM TRIBUTARY PIPE.*
*USER-SPECIFIED MINIMUM TOPOGRAPHIC SLOPE ADJUSTMENT NOT SELECTED

FLOW PROCESS FROM NODE 10.00 TO NODE 32.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

INITIAL SUBAREA FLOW-LENGTH (FEET) = 265.00
ELEVATION DATA: UPSTREAM (FEET) = 167.50 DOWNSTREAM (FEET) = 166.30

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$
SUBAREA ANALYSIS USED MINIMUM T_c (MIN.) = 8.337
* 10 YEAR RAINFALL INTENSITY (INCH/HR) = 3.029

SUBAREA T_c AND LOSS RATE DATA (AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	F_p (INCH/HR)	A_p (DECIMAL)	SCS CN	T_c (MIN.)
COMMERCIAL	B	0.15	0.30	0.100	56	8.34

SUBAREA AVERAGE PERVIOUS LOSS RATE, F_p (INCH/HR) = 0.30

SUBAREA AVERAGE PERVIOUS AREA FRACTION, A_p = 0.100

SUBAREA RUNOFF (CFS) = 0.40

TOTAL AREA (ACRES) = 0.15 PEAK FLOW RATE (CFS) = 0.40

FLOW PROCESS FROM NODE 32.00 TO NODE 11.00 IS CODE = 61

>>>>COMPUTE STREET FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>(STANDARD CURB SECTION USED)<<<<<

UPSTREAM ELEVATION (FEET) = 166.30 DOWNSTREAM ELEVATION (FEET) = 165.10
STREET LENGTH (FEET) = 210.00 CURB HEIGHT (INCHES) = 6.0
STREET HALFWIDTH (FEET) = 25.00

DISTANCE FROM CROWN TO CROSSFALL GRADEBREAK (FEET) = 10.00

INSIDE STREET CROSSFALL (DECIMAL) = 0.020

OUTSIDE STREET CROSSFALL (DECIMAL) = 0.020

SPECIFIED NUMBER OF HALFSTREETS CARRYING RUNOFF = 1

STREET PARKWAY CROSSFALL (DECIMAL) = 0.020

Manning's FRICTION FACTOR for Streetflow Section (curb-to-curb) = 0.0150

Manning's FRICTION FACTOR for Back-of-Walk Flow Section = 0.0200

**TRAVEL TIME COMPUTED USING ESTIMATED FLOW (CFS) = 0.54

STREETFLOW MODEL RESULTS USING ESTIMATED FLOW:

STREET FLOW DEPTH (FEET) = 0.24

HALFSTREET FLOOD WIDTH (FEET) = 5.48

AVERAGE FLOW VELOCITY (FEET/SEC.) = 1.30

PRODUCT OF DEPTH&VELOCITY (FT*FT/SEC.) = 0.31

STREET FLOW TRAVEL TIME (MIN.) = 2.70 T_c (MIN.) = 11.04

* 10 YEAR RAINFALL INTENSITY (INCH/HR) = 2.579

SUBAREA LOSS RATE DATA (AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	F_p (INCH/HR)	A_p (DECIMAL)	SCS CN
COMMERCIAL	B	0.12	0.30	0.100	56

SUBAREA AVERAGE PERVIOUS LOSS RATE, F_p (INCH/HR) = 0.30

SUBAREA AVERAGE PERVIOUS AREA FRACTION, A_p = 0.100

SUBAREA AREA (ACRES) = 0.12 SUBAREA RUNOFF (CFS) = 0.28

EFFECTIVE AREA (ACRES) = 0.27 AREA-AVERAGED Fm (INCH/HR) = 0.03
AREA-AVERAGED Fp (INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.10
TOTAL AREA (ACRES) = 0.3 PEAK FLOW RATE (CFS) = 0.62

END OF SUBAREA STREET FLOW HYDRAULICS:

DEPTH (FEET) = 0.24 HALFSTREET FLOOD WIDTH (FEET) = 5.90
FLOW VELOCITY (FEET/SEC.) = 1.33 DEPTH*VELOCITY (FT*FT/SEC.) = 0.32
LONGEST FLOWPATH FROM NODE 10.00 TO NODE 11.00 = 475.00 FEET.

FLOW PROCESS FROM NODE 11.00 TO NODE 11.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

=====

TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION (MIN.) = 11.04
RAINFALL INTENSITY (INCH/HR) = 2.58
AREA-AVERAGED Fm (INCH/HR) = 0.03
AREA-AVERAGED Fp (INCH/HR) = 0.30
AREA-AVERAGED Ap = 0.10
EFFECTIVE STREAM AREA (ACRES) = 0.27
TOTAL STREAM AREA (ACRES) = 0.27
PEAK FLOW RATE (CFS) AT CONFLUENCE = 0.62

FLOW PROCESS FROM NODE 12.00 TO NODE 11.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<

>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

=====

INITIAL SUBAREA FLOW-LENGTH (FEET) = 210.00
ELEVATION DATA: UPSTREAM (FEET) = 166.00 DOWNSTREAM (FEET) = 165.10

$$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$$

SUBAREA ANALYSIS USED MINIMUM Tc (MIN.) = 7.680

* 10 YEAR RAINFALL INTENSITY (INCH/HR) = 3.174

SUBAREA Tc AND LOSS RATE DATA (AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN	Tc (MIN.)
COMMERCIAL	B	0.76	0.30	0.100	56	7.68

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp (INCH/HR) = 0.30

SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.100

SUBAREA RUNOFF (CFS) = 2.15

TOTAL AREA (ACRES) = 0.76 PEAK FLOW RATE (CFS) = 2.15

FLOW PROCESS FROM NODE 11.00 TO NODE 11.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<

=====

TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:

TIME OF CONCENTRATION(MIN.) = 7.68
 RAINFALL INTENSITY(INCH/HR) = 3.17
 AREA-AVERAGED Fm(INCH/HR) = 0.03
 AREA-AVERAGED Fp(INCH/HR) = 0.30
 AREA-AVERAGED Ap = 0.10
 EFFECTIVE STREAM AREA(ACRES) = 0.76
 TOTAL STREAM AREA(ACRES) = 0.76
 PEAK FLOW RATE(CFS) AT CONFLUENCE = 2.15

** CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	0.62	11.04	2.579	0.30(0.03)	0.10	0.3	10.00
2	2.15	7.68	3.174	0.30(0.03)	0.10	0.8	12.00

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
 CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	2.68	7.68	3.174	0.30(0.03)	0.10	0.9	12.00
2	2.36	11.04	2.579	0.30(0.03)	0.10	1.0	10.00

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE(CFS) = 2.68 Tc(MIN.) = 7.68
 EFFECTIVE AREA(ACRES) = 0.95 AREA-AVERAGED Fm(INCH/HR) = 0.03
 AREA-AVERAGED Fp(INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.10
 TOTAL AREA(ACRES) = 1.0
 LONGEST FLOWPATH FROM NODE 10.00 TO NODE 11.00 = 475.00 FEET.

FLOW PROCESS FROM NODE 11.00 TO NODE 13.00 IS CODE = 62

>>>>COMPUTE STREET FLOW TRAVEL TIME THRU SUBAREA<<<<<
 >>>>(STREET TABLE SECTION # 1 USED)<<<<<

UPSTREAM ELEVATION(FEET) = 165.10 DOWNSTREAM ELEVATION(FEET) = 164.20
 STREET LENGTH(FEET) = 455.00 CURB HEIGHT(INCHES) = 8.0
 STREET HALFWIDTH(FEET) = 24.00

DISTANCE FROM CROWN TO CROSSFALL GRADEBREAK(FEET) = 19.00
 INSIDE STREET CROSSFALL(DECIMAL) = 0.020
 OUTSIDE STREET CROSSFALL(DECIMAL) = 0.020

SPECIFIED NUMBER OF HALFSTREETS CARRYING RUNOFF = 1
 STREET PARKWAY CROSSFALL(DECIMAL) = 0.020
 Manning's FRICTION FACTOR for Streetflow Section(curbs-to-curbs) = 0.0150
 Manning's FRICTION FACTOR for Back-of-Walk Flow Section = 0.0200

**TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 3.05
 STREETFLOW MODEL RESULTS USING ESTIMATED FLOW:
 STREET FLOW DEPTH(FEET) = 0.45
 HALFSTREET FLOOD WIDTH(FEET) = 14.83

AVERAGE FLOW VELOCITY (FEET/SEC.) = 1.28
 PRODUCT OF DEPTH&VELOCITY (FT*FT/SEC.) = 0.58
 STREET FLOW TRAVEL TIME (MIN.) = 5.94 Tc (MIN.) = 13.62
 * 10 YEAR RAINFALL INTENSITY (INCH/HR) = 2.286
 SUBAREA LOSS RATE DATA (AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
COMMERCIAL	B	0.36	0.30	0.100	56

 SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp (INCH/HR) = 0.30
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.100
 SUBAREA AREA (ACRES) = 0.36 SUBAREA RUNOFF (CFS) = 0.73
 EFFECTIVE AREA (ACRES) = 1.31 AREA-AVERAGED Fm (INCH/HR) = 0.03
 AREA-AVERAGED Fp (INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.10
 TOTAL AREA (ACRES) = 1.4 PEAK FLOW RATE (CFS) = 2.68
 NOTE: PEAK FLOW RATE DEFAULTED TO UPSTREAM VALUE

END OF SUBAREA STREET FLOW HYDRAULICS:
 DEPTH (FEET) = 0.44 HALFSTREET FLOOD WIDTH (FEET) = 14.09
 FLOW VELOCITY (FEET/SEC.) = 1.23 DEPTH*VELOCITY (FT*FT/SEC.) = 0.54
 LONGEST FLOWPATH FROM NODE 10.00 TO NODE 13.00 = 930.00 FEET.

 FLOW PROCESS FROM NODE 13.00 TO NODE 13.00 IS CODE = 1

 >>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

=====
 TOTAL NUMBER OF STREAMS = 2
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
 TIME OF CONCENTRATION (MIN.) = 13.62
 RAINFALL INTENSITY (INCH/HR) = 2.29
 AREA-AVERAGED Fm (INCH/HR) = 0.03
 AREA-AVERAGED Fp (INCH/HR) = 0.30
 AREA-AVERAGED Ap = 0.10
 EFFECTIVE STREAM AREA (ACRES) = 1.31
 TOTAL STREAM AREA (ACRES) = 1.39
 PEAK FLOW RATE (CFS) AT CONFLUENCE = 2.68

 FLOW PROCESS FROM NODE 14.00 TO NODE 15.00 IS CODE = 21

 >>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
 >>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

=====
 INITIAL SUBAREA FLOW-LENGTH (FEET) = 125.00
 ELEVATION DATA: UPSTREAM (FEET) = 164.60 DOWNSTREAM (FEET) = 164.20

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$
 SUBAREA ANALYSIS USED MINIMUM Tc (MIN.) = 6.616
 * 10 YEAR RAINFALL INTENSITY (INCH/HR) = 3.458

SUBAREA Tc AND LOSS RATE DATA (AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN	Tc (MIN.)
COMMERCIAL	B	0.15	0.30	0.100	56	6.62

 SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp (INCH/HR) = 0.30

SUBAREA AVERAGE PERVIOUS AREA FRACTION, $A_p = 0.100$
 SUBAREA RUNOFF (CFS) = 0.46
 TOTAL AREA (ACRES) = 0.15 PEAK FLOW RATE (CFS) = 0.46

FLOW PROCESS FROM NODE 15.00 TO NODE 13.00 IS CODE = 41

 >>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
 >>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM (FEET) = 159.30 DOWNSTREAM (FEET) = 159.20
 FLOW LENGTH (FEET) = 50.00 MANNING'S N = 0.013
 DEPTH OF FLOW IN 18.0 INCH PIPE IS 3.9 INCHES
 PIPE-FLOW VELOCITY (FEET/SEC.) = 1.66
 GIVEN PIPE DIAMETER (INCH) = 18.00 NUMBER OF PIPES = 1
 PIPE-FLOW (CFS) = 0.46
 PIPE TRAVEL TIME (MIN.) = 0.50 T_c (MIN.) = 7.12
 LONGEST FLOWPATH FROM NODE 14.00 TO NODE 13.00 = 175.00 FEET.

FLOW PROCESS FROM NODE 13.00 TO NODE 13.00 IS CODE = 1

 >>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
 >>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<

=====

TOTAL NUMBER OF STREAMS = 2
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
 TIME OF CONCENTRATION (MIN.) = 7.12
 RAINFALL INTENSITY (INCH/HR) = 3.32
 AREA-AVERAGED F_m (INCH/HR) = 0.03
 AREA-AVERAGED F_p (INCH/HR) = 0.30
 AREA-AVERAGED $A_p = 0.10$
 EFFECTIVE STREAM AREA (ACRES) = 0.15
 TOTAL STREAM AREA (ACRES) = 0.15
 PEAK FLOW RATE (CFS) AT CONFLUENCE = 0.46

** CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	T_c (MIN.)	Intensity (INCH/HR)	$F_p(F_m)$ (INCH/HR)	A_p	A_e (ACRES)	HEADWATER NODE
1	2.68	13.62	2.286	0.30 (0.03)	0.10	1.3	12.00
1	2.47	17.18	2.001	0.30 (0.03)	0.10	1.4	10.00
2	0.46	7.12	3.316	0.30 (0.03)	0.10	0.2	14.00

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
 CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	T_c (MIN.)	Intensity (INCH/HR)	$F_p(F_m)$ (INCH/HR)	A_p	A_e (ACRES)	HEADWATER NODE
1	2.50	7.12	3.316	0.30 (0.03)	0.10	0.8	14.00
2	3.00	13.62	2.286	0.30 (0.03)	0.10	1.5	12.00
3	2.74	17.18	2.001	0.30 (0.03)	0.10	1.5	10.00

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE (CFS) = 3.00 Tc (MIN.) = 13.62
 EFFECTIVE AREA (ACRES) = 1.46 AREA-AVERAGED Fm (INCH/HR) = 0.03
 AREA-AVERAGED Fp (INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.10
 TOTAL AREA (ACRES) = 1.5
 LONGEST FLOWPATH FROM NODE 10.00 TO NODE 13.00 = 930.00 FEET.

 FLOW PROCESS FROM NODE 13.00 TO NODE 16.00 IS CODE = 41

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
 >>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM (FEET) = 159.20 DOWNSTREAM (FEET) = 151.55
 FLOW LENGTH (FEET) = 235.00 MANNING'S N = 0.013
 DEPTH OF FLOW IN 18.0 INCH PIPE IS 4.9 INCHES
 PIPE-FLOW VELOCITY (FEET/SEC.) = 7.68
 GIVEN PIPE DIAMETER (INCH) = 18.00 NUMBER OF PIPES = 1
 PIPE-FLOW (CFS) = 3.00
 PIPE TRAVEL TIME (MIN.) = 0.51 Tc (MIN.) = 14.13
 LONGEST FLOWPATH FROM NODE 10.00 TO NODE 16.00 = 1165.00 FEET.

 FLOW PROCESS FROM NODE 16.00 TO NODE 16.00 IS CODE = 10

>>>>MAIN-STREAM MEMORY COPIED ONTO MEMORY BANK # 1 <<<<<

 FLOW PROCESS FROM NODE 17.00 TO NODE 18.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
 >>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

=====

INITIAL SUBAREA FLOW-LENGTH (FEET) = 150.00
 ELEVATION DATA: UPSTREAM (FEET) = 166.90 DOWNSTREAM (FEET) = 166.50

$$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$$

SUBAREA ANALYSIS USED MINIMUM Tc (MIN.) = 8.741
 * 10 YEAR RAINFALL INTENSITY (INCH/HR) = 2.948

SUBAREA Tc AND LOSS RATE DATA (AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN	Tc (MIN.)
CONDOMINIUMS	B	0.36	0.30	0.350	56	8.74

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp (INCH/HR) = 0.30

SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.350

SUBAREA RUNOFF (CFS) = 0.92

TOTAL AREA (ACRES) = 0.36 PEAK FLOW RATE (CFS) = 0.92

 FLOW PROCESS FROM NODE 18.00 TO NODE 19.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
 >>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 153.00 DOWNSTREAM(FEET) = 151.60
 FLOW LENGTH(FEET) = 65.00 MANNING'S N = 0.013
 ESTIMATED PIPE DIAMETER(INCH) INCREASED TO 12.000
 DEPTH OF FLOW IN 12.0 INCH PIPE IS 3.5 INCHES
 PIPE-FLOW VELOCITY(FEET/SEC.) = 4.93
 ESTIMATED PIPE DIAMETER(INCH) = 12.000 NUMBER OF PIPES = 1
 PIPE-FLOW(CFS) = 0.92
 PIPE TRAVEL TIME(MIN.) = 0.22 Tc(MIN.) = 8.96
 LONGEST FLOWPATH FROM NODE 17.00 TO NODE 19.00 = 215.00 FEET.

FLOW PROCESS FROM NODE 19.00 TO NODE 19.00 IS CODE = 1

 >>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

=====

TOTAL NUMBER OF STREAMS = 2
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
 TIME OF CONCENTRATION(MIN.) = 8.96
 RAINFALL INTENSITY(INCH/HR) = 2.91
 AREA-AVERAGED Fm(INCH/HR) = 0.11
 AREA-AVERAGED Fp(INCH/HR) = 0.30
 AREA-AVERAGED Ap = 0.35
 EFFECTIVE STREAM AREA(ACRES) = 0.36
 TOTAL STREAM AREA(ACRES) = 0.36
 PEAK FLOW RATE(CFS) AT CONFLUENCE = 0.92

FLOW PROCESS FROM NODE 20.00 TO NODE 21.00 IS CODE = 21

 >>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
 >>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

=====

INITIAL SUBAREA FLOW-LENGTH(FEET) = 150.00
 ELEVATION DATA: UPSTREAM(FEET) = 167.30 DOWNSTREAM(FEET) = 166.80

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$
 SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) = 8.359
 * 10 YEAR RAINFALL INTENSITY(INCH/HR) = 3.024
 SUBAREA Tc AND LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN	Tc (MIN.)
CONDOMINIUMS	A	0.50	0.40	0.350	32	8.36

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.40
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.350
 SUBAREA RUNOFF(CFS) = 1.30
 TOTAL AREA(ACRES) = 0.50 PEAK FLOW RATE(CFS) = 1.30

FLOW PROCESS FROM NODE 21.00 TO NODE 19.00 IS CODE = 31

 >>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
 >>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 153.00 DOWNSTREAM(FEET) = 151.60

FLOW LENGTH (FEET) = 45.00 MANNING'S N = 0.013
 ESTIMATED PIPE DIAMETER (INCH) INCREASED TO 12.000
 DEPTH OF FLOW IN 12.0 INCH PIPE IS 3.7 INCHES
 PIPE-FLOW VELOCITY (FEET/SEC.) = 6.20
 ESTIMATED PIPE DIAMETER (INCH) = 12.000 NUMBER OF PIPES = 1
 PIPE-FLOW (CFS) = 1.30
 PIPE TRAVEL TIME (MIN.) = 0.12 Tc (MIN.) = 8.48
 LONGEST FLOWPATH FROM NODE 20.00 TO NODE 19.00 = 195.00 FEET.

 FLOW PROCESS FROM NODE 19.00 TO NODE 19.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
 >>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<

=====
 TOTAL NUMBER OF STREAMS = 2
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
 TIME OF CONCENTRATION (MIN.) = 8.48
 RAINFALL INTENSITY (INCH/HR) = 3.00
 AREA-AVERAGED Fm (INCH/HR) = 0.14
 AREA-AVERAGED Fp (INCH/HR) = 0.40
 AREA-AVERAGED Ap = 0.35
 EFFECTIVE STREAM AREA (ACRES) = 0.50
 TOTAL STREAM AREA (ACRES) = 0.50
 PEAK FLOW RATE (CFS) AT CONFLUENCE = 1.30

** CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	0.92	8.96	2.906	0.30 (0.11)	0.35	0.4	17.00
2	1.30	8.48	2.999	0.40 (0.14)	0.35	0.5	20.00

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
 CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	2.20	8.48	2.999	0.36 (0.13)	0.35	0.8	20.00
2	2.18	8.96	2.906	0.36 (0.13)	0.35	0.9	17.00

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE (CFS) = 2.20 Tc (MIN.) = 8.48
 EFFECTIVE AREA (ACRES) = 0.84 AREA-AVERAGED Fm (INCH/HR) = 0.13
 AREA-AVERAGED Fp (INCH/HR) = 0.36 AREA-AVERAGED Ap = 0.35
 TOTAL AREA (ACRES) = 0.9
 LONGEST FLOWPATH FROM NODE 17.00 TO NODE 19.00 = 215.00 FEET.

 FLOW PROCESS FROM NODE 19.00 TO NODE 19.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

=====

MAINLINE Tc (MIN.) = 8.48

* 10 YEAR RAINFALL INTENSITY (INCH/HR) = 2.999
 SUBAREA LOSS RATE DATA (AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
CONDOMINIUMS	B	0.42	0.30	0.350	56

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp (INCH/HR) = 0.30
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.350
 SUBAREA AREA (ACRES) = 0.42 SUBAREA RUNOFF (CFS) = 1.09
 EFFECTIVE AREA (ACRES) = 1.26 AREA-AVERAGED Fm (INCH/HR) = 0.12
 AREA-AVERAGED Fp (INCH/HR) = 0.34 AREA-AVERAGED Ap = 0.35
 TOTAL AREA (ACRES) = 1.3 PEAK FLOW RATE (CFS) = 3.27

FLOW PROCESS FROM NODE 19.00 TO NODE 16.00 IS CODE = 41

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
 >>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM (FEET) = 151.60 DOWNSTREAM (FEET) = 151.55
 FLOW LENGTH (FEET) = 30.00 MANNING'S N = 0.013
 DEPTH OF FLOW IN 18.0 INCH PIPE IS 12.0 INCHES
 PIPE-FLOW VELOCITY (FEET/SEC.) = 2.62
 GIVEN PIPE DIAMETER (INCH) = 18.00 NUMBER OF PIPES = 1
 PIPE-FLOW (CFS) = 3.27
 PIPE TRAVEL TIME (MIN.) = 0.19 Tc (MIN.) = 8.67
 LONGEST FLOWPATH FROM NODE 17.00 TO NODE 16.00 = 245.00 FEET.

FLOW PROCESS FROM NODE 16.00 TO NODE 16.00 IS CODE = 11

>>>>CONFLUENCE MEMORY BANK # 1 WITH THE MAIN-STREAM MEMORY<<<<<

** MAIN STREAM CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp (Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	3.27	8.67	2.961	0.34 (0.12)	0.35	1.3	20.00
2	3.21	9.15	2.871	0.34 (0.12)	0.35	1.3	17.00

LONGEST FLOWPATH FROM NODE 17.00 TO NODE 16.00 = 245.00 FEET.

** MEMORY BANK # 1 CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp (Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	2.50	7.65	3.181	0.30 (0.03)	0.10	0.8	14.00
2	3.00	14.13	2.238	0.30 (0.03)	0.10	1.5	12.00
3	2.74	17.71	1.967	0.30 (0.03)	0.10	1.5	10.00

LONGEST FLOWPATH FROM NODE 10.00 TO NODE 16.00 = 1165.00 FEET.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp (Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	5.61	7.65	3.181	0.33 (0.08)	0.24	1.9	14.00
2	5.85	8.67	2.961	0.33 (0.08)	0.24	2.2	20.00
3	5.83	9.15	2.871	0.33 (0.08)	0.24	2.3	17.00

4	5.47	14.13	2.238	0.33 (0.07)	0.22	2.7	12.00
5	4.90	17.71	1.967	0.33 (0.07)	0.21	2.8	10.00
TOTAL AREA (ACRES) =			2.8				

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE (CFS) = 5.85 Tc (MIN.) = 8.671
EFFECTIVE AREA (ACRES) = 2.19 AREA-AVERAGED Fm (INCH/HR) = 0.08
AREA-AVERAGED Fp (INCH/HR) = 0.33 AREA-AVERAGED Ap = 0.24
TOTAL AREA (ACRES) = 2.8
LONGEST FLOWPATH FROM NODE 10.00 TO NODE 16.00 = 1165.00 FEET.

FLOW PROCESS FROM NODE 16.00 TO NODE 22.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<

=====

ELEVATION DATA: UPSTREAM (FEET) = 151.55 DOWNSTREAM (FEET) = 151.30
FLOW LENGTH (FEET) = 115.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 21.0 INCH PIPE IS 14.4 INCHES
PIPE-FLOW VELOCITY (FEET/SEC.) = 3.34
ESTIMATED PIPE DIAMETER (INCH) = 21.00 NUMBER OF PIPES = 1
PIPE-FLOW (CFS) = 5.85
PIPE TRAVEL TIME (MIN.) = 0.57 Tc (MIN.) = 9.25
LONGEST FLOWPATH FROM NODE 10.00 TO NODE 22.00 = 1280.00 FEET.

FLOW PROCESS FROM NODE 22.00 TO NODE 22.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

=====

MAINLINE Tc (MIN.) = 9.25
* 10 YEAR RAINFALL INTENSITY (INCH/HR) = 2.854
SUBAREA LOSS RATE DATA (AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
CONDOMINIUMS	A	0.31	0.40	0.350	32

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp (INCH/HR) = 0.40
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.350
SUBAREA AREA (ACRES) = 0.31 SUBAREA RUNOFF (CFS) = 0.76
EFFECTIVE AREA (ACRES) = 2.50 AREA-AVERAGED Fm (INCH/HR) = 0.09
AREA-AVERAGED Fp (INCH/HR) = 0.34 AREA-AVERAGED Ap = 0.26
TOTAL AREA (ACRES) = 3.1 PEAK FLOW RATE (CFS) = 6.23

FLOW PROCESS FROM NODE 22.00 TO NODE 22.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

=====

TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION (MIN.) = 9.25
RAINFALL INTENSITY (INCH/HR) = 2.85
AREA-AVERAGED Fm (INCH/HR) = 0.09

AREA-AVERAGED F_p (INCH/HR) = 0.34
 AREA-AVERAGED A_p = 0.26
 EFFECTIVE STREAM AREA (ACRES) = 2.50
 TOTAL STREAM AREA (ACRES) = 3.13
 PEAK FLOW RATE (CFS) AT CONFLUENCE = 6.23

FLOW PROCESS FROM NODE 23.00 TO NODE 22.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
 >>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

INITIAL SUBAREA FLOW-LENGTH (FEET) = 225.00
 ELEVATION DATA: UPSTREAM (FEET) = 166.30 DOWNSTREAM (FEET) = 165.50

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$
 SUBAREA ANALYSIS USED MINIMUM T_c (MIN.) = 9.705
 * 10 YEAR RAINFALL INTENSITY (INCH/HR) = 2.776

SUBAREA T_c AND LOSS RATE DATA (AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	F_p (INCH/HR)	A_p (DECIMAL)	SCS CN	T_c (MIN.)
CONDOMINIUMS	B	0.71	0.30	0.350	56	9.70

SUBAREA AVERAGE PERVIOUS LOSS RATE, F_p (INCH/HR) = 0.30
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, A_p = 0.350
 SUBAREA RUNOFF (CFS) = 1.71
 TOTAL AREA (ACRES) = 0.71 PEAK FLOW RATE (CFS) = 1.71

FLOW PROCESS FROM NODE 22.00 TO NODE 22.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
 >>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<

TOTAL NUMBER OF STREAMS = 2
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
 TIME OF CONCENTRATION (MIN.) = 9.70
 RAINFALL INTENSITY (INCH/HR) = 2.78
 AREA-AVERAGED F_m (INCH/HR) = 0.11
 AREA-AVERAGED F_p (INCH/HR) = 0.30
 AREA-AVERAGED A_p = 0.35
 EFFECTIVE STREAM AREA (ACRES) = 0.71
 TOTAL STREAM AREA (ACRES) = 0.71
 PEAK FLOW RATE (CFS) AT CONFLUENCE = 1.71

** CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	T_c (MIN.)	Intensity (INCH/HR)	F_p (F_m) (INCH/HR)	A_p	A_e (ACRES)	HEADWATER NODE
1	6.01	8.23	3.051	0.35 (0.09)	0.26	2.3	14.00
1	6.23	9.25	2.854	0.34 (0.09)	0.26	2.5	20.00
1	6.20	9.73	2.773	0.34 (0.09)	0.25	2.6	17.00
1	5.78	14.72	2.187	0.34 (0.08)	0.23	3.0	12.00
1	5.22	18.30	1.930	0.34 (0.08)	0.23	3.1	10.00
2	1.71	9.70	2.776	0.30 (0.11)	0.35	0.7	23.00

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
 CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	7.61	8.23	3.051	0.33(0.09)	0.28	2.9	14.00
2	7.90	9.25	2.854	0.33(0.09)	0.28	3.2	20.00
3	7.91	9.70	2.776	0.33(0.09)	0.28	3.3	23.00
4	7.91	9.73	2.773	0.33(0.09)	0.28	3.3	17.00
5	7.11	14.72	2.187	0.33(0.08)	0.25	3.8	12.00
6	6.39	18.30	1.930	0.33(0.08)	0.25	3.8	10.00

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE (CFS) = 7.91 Tc (MIN.) = 9.70
 EFFECTIVE AREA (ACRES) = 3.27 AREA-AVERAGED Fm (INCH/HR) = 0.09
 AREA-AVERAGED Fp (INCH/HR) = 0.33 AREA-AVERAGED Ap = 0.28
 TOTAL AREA (ACRES) = 3.8
 LONGEST FLOWPATH FROM NODE 10.00 TO NODE 22.00 = 1280.00 FEET.

FLOW PROCESS FROM NODE 22.00 TO NODE 22.00 IS CODE = 10

>>>>MAIN-STREAM MEMORY COPIED ONTO MEMORY BANK # 2 <<<<<

FLOW PROCESS FROM NODE 24.00 TO NODE 25.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<

>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

INITIAL SUBAREA FLOW-LENGTH (FEET) = 270.00
 ELEVATION DATA: UPSTREAM (FEET) = 166.50 DOWNSTREAM (FEET) = 165.50

$$Tc = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$$

SUBAREA ANALYSIS USED MINIMUM Tc (MIN.) = 10.354

* 10 YEAR RAINFALL INTENSITY (INCH/HR) = 2.675

SUBAREA Tc AND LOSS RATE DATA (AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN	Tc (MIN.)
CONDOMINIUMS	B	0.51	0.30	0.350	56	10.35

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp (INCH/HR) = 0.30

SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.350

SUBAREA RUNOFF (CFS) = 1.18

TOTAL AREA (ACRES) = 0.51 PEAK FLOW RATE (CFS) = 1.18

FLOW PROCESS FROM NODE 25.00 TO NODE 25.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

TOTAL NUMBER OF STREAMS = 2

CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:

TIME OF CONCENTRATION(MIN.) = 10.35
 RAINFALL INTENSITY(INCH/HR) = 2.67
 AREA-AVERAGED Fm(INCH/HR) = 0.11
 AREA-AVERAGED Fp(INCH/HR) = 0.30
 AREA-AVERAGED Ap = 0.35
 EFFECTIVE STREAM AREA(ACRES) = 0.51
 TOTAL STREAM AREA(ACRES) = 0.51
 PEAK FLOW RATE(CFS) AT CONFLUENCE = 1.18

 FLOW PROCESS FROM NODE 26.00 TO NODE 25.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
 >>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

INITIAL SUBAREA FLOW-LENGTH(FEET) = 155.00
 ELEVATION DATA: UPSTREAM(FEET) = 166.50 DOWNSTREAM(FEET) = 165.50

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$
 SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) = 7.422
 * 10 YEAR RAINFALL INTENSITY(INCH/HR) = 3.237

SUBAREA Tc AND LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN	Tc (MIN.)
CONDOMINIUMS	B	0.30	0.30	0.350	56	7.42

 SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.30
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.350
 SUBAREA RUNOFF(CFS) = 0.85
 TOTAL AREA(ACRES) = 0.30 PEAK FLOW RATE(CFS) = 0.85

 FLOW PROCESS FROM NODE 25.00 TO NODE 25.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
 >>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<

TOTAL NUMBER OF STREAMS = 2
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
 TIME OF CONCENTRATION(MIN.) = 7.42
 RAINFALL INTENSITY(INCH/HR) = 3.24
 AREA-AVERAGED Fm(INCH/HR) = 0.10
 AREA-AVERAGED Fp(INCH/HR) = 0.30
 AREA-AVERAGED Ap = 0.35
 EFFECTIVE STREAM AREA(ACRES) = 0.30
 TOTAL STREAM AREA(ACRES) = 0.30
 PEAK FLOW RATE(CFS) AT CONFLUENCE = 0.85

** CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	1.18	10.35	2.675	0.30 (0.11)	0.35	0.5	24.00
2	0.85	7.42	3.237	0.30 (0.10)	0.35	0.3	26.00

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO

CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	1.88	7.42	3.237	0.30 (0.10)	0.35	0.7	26.00
2	1.87	10.35	2.675	0.30 (0.11)	0.35	0.8	24.00

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE (CFS) = 1.88 Tc (MIN.) = 7.42
EFFECTIVE AREA (ACRES) = 0.67 AREA-AVERAGED Fm (INCH/HR) = 0.10
AREA-AVERAGED Fp (INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.35
TOTAL AREA (ACRES) = 0.8
LONGEST FLOWPATH FROM NODE 24.00 TO NODE 25.00 = 270.00 FEET.

FLOW PROCESS FROM NODE 25.00 TO NODE 28.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<

=====

ELEVATION DATA: UPSTREAM (FEET) = 161.00 DOWNSTREAM (FEET) = 158.00
FLOW LENGTH (FEET) = 175.00 MANNING'S N = 0.013
ESTIMATED PIPE DIAMETER (INCH) INCREASED TO 12.000
DEPTH OF FLOW IN 12.0 INCH PIPE IS 5.4 INCHES
PIPE-FLOW VELOCITY (FEET/SEC.) = 5.51
ESTIMATED PIPE DIAMETER (INCH) = 12.000 NUMBER OF PIPES = 1
PIPE-FLOW (CFS) = 1.88
PIPE TRAVEL TIME (MIN.) = 0.53 Tc (MIN.) = 7.95
LONGEST FLOWPATH FROM NODE 24.00 TO NODE 28.00 = 445.00 FEET.

FLOW PROCESS FROM NODE 28.00 TO NODE 28.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

=====

TOTAL NUMBER OF STREAMS = 3
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION (MIN.) = 7.95
RAINFALL INTENSITY (INCH/HR) = 3.11
AREA-AVERAGED Fm (INCH/HR) = 0.10
AREA-AVERAGED Fp (INCH/HR) = 0.30
AREA-AVERAGED Ap = 0.35
EFFECTIVE STREAM AREA (ACRES) = 0.67
TOTAL STREAM AREA (ACRES) = 0.81
PEAK FLOW RATE (CFS) AT CONFLUENCE = 1.88

FLOW PROCESS FROM NODE 27.00 TO NODE 28.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

=====

INITIAL SUBAREA FLOW-LENGTH (FEET) = 180.00

ELEVATION DATA: UPSTREAM(FEET) = 166.50 DOWNSTREAM(FEET) = 165.40

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$

SUBAREA ANALYSIS USED MINIMUM T_c (MIN.) = 7.965

* 10 YEAR RAINFALL INTENSITY(INCH/HR) = 3.109

SUBAREA T_c AND LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	F_p (INCH/HR)	A_p (DECIMAL)	SCS CN	T_c (MIN.)
CONDOMINIUMS	B	0.18	0.30	0.350	56	7.97

SUBAREA AVERAGE PERVIOUS LOSS RATE, F_p (INCH/HR) = 0.30

SUBAREA AVERAGE PERVIOUS AREA FRACTION, A_p = 0.350

SUBAREA RUNOFF(CFS) = 0.49

TOTAL AREA(ACRES) = 0.18 PEAK FLOW RATE(CFS) = 0.49

FLOW PROCESS FROM NODE 28.00 TO NODE 28.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

TOTAL NUMBER OF STREAMS = 3

CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:

TIME OF CONCENTRATION(MIN.) = 7.97

RAINFALL INTENSITY(INCH/HR) = 3.11

AREA-AVERAGED F_m (INCH/HR) = 0.11

AREA-AVERAGED F_p (INCH/HR) = 0.30

AREA-AVERAGED A_p = 0.35

EFFECTIVE STREAM AREA(ACRES) = 0.18

TOTAL STREAM AREA(ACRES) = 0.18

PEAK FLOW RATE(CFS) AT CONFLUENCE = 0.49

FLOW PROCESS FROM NODE 29.00 TO NODE 28.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<

>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

INITIAL SUBAREA FLOW-LENGTH(FEET) = 230.00

ELEVATION DATA: UPSTREAM(FEET) = 166.50 DOWNSTREAM(FEET) = 165.40

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$

SUBAREA ANALYSIS USED MINIMUM T_c (MIN.) = 9.227

* 10 YEAR RAINFALL INTENSITY(INCH/HR) = 2.858

SUBAREA T_c AND LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	F_p (INCH/HR)	A_p (DECIMAL)	SCS CN	T_c (MIN.)
CONDOMINIUMS	B	0.48	0.30	0.350	56	9.23

SUBAREA AVERAGE PERVIOUS LOSS RATE, F_p (INCH/HR) = 0.30

SUBAREA AVERAGE PERVIOUS AREA FRACTION, A_p = 0.350

SUBAREA RUNOFF(CFS) = 1.19

TOTAL AREA(ACRES) = 0.48 PEAK FLOW RATE(CFS) = 1.19

FLOW PROCESS FROM NODE 28.00 TO NODE 28.00 IS CODE = 1

>>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
>>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<

=====

TOTAL NUMBER OF STREAMS = 3
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 3 ARE:
TIME OF CONCENTRATION(MIN.) = 9.23
RAINFALL INTENSITY(INCH/HR) = 2.86
AREA-AVERAGED Fm(INCH/HR) = 0.11
AREA-AVERAGED Fp(INCH/HR) = 0.30
AREA-AVERAGED Ap = 0.35
EFFECTIVE STREAM AREA(ACRES) = 0.48
TOTAL STREAM AREA(ACRES) = 0.48
PEAK FLOW RATE(CFS) AT CONFLUENCE = 1.19

** CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	1.88	7.95	3.112	0.30(0.10)	0.35	0.7	26.00
1	1.87	10.88	2.600	0.30(0.11)	0.35	0.8	24.00
2	0.49	7.97	3.109	0.30(0.11)	0.35	0.2	27.00
3	1.19	9.23	2.858	0.30(0.11)	0.35	0.5	29.00

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
CONFLUENCE FORMULA USED FOR 3 STREAMS.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	3.48	7.95	3.112	0.30(0.10)	0.35	1.3	26.00
2	3.48	7.97	3.109	0.30(0.10)	0.35	1.3	27.00
3	3.52	9.23	2.858	0.30(0.11)	0.35	1.4	29.00
4	3.36	10.88	2.600	0.30(0.11)	0.35	1.5	24.00

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE(CFS) = 3.52 Tc(MIN.) = 9.23
EFFECTIVE AREA(ACRES) = 1.39 AREA-AVERAGED Fm(INCH/HR) = 0.11
AREA-AVERAGED Fp(INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.35
TOTAL AREA(ACRES) = 1.5
LONGEST FLOWPATH FROM NODE 24.00 TO NODE 28.00 = 445.00 FEET.

FLOW PROCESS FROM NODE 28.00 TO NODE 22.00 IS CODE = 31

>>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<

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ELEVATION DATA: UPSTREAM(FEET) = 158.00 DOWNSTREAM(FEET) = 151.30
FLOW LENGTH(FEET) = 250.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 12.0 INCH PIPE IS 6.8 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 7.62
ESTIMATED PIPE DIAMETER(INCH) = 12.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 3.52
PIPE TRAVEL TIME(MIN.) = 0.55 Tc(MIN.) = 9.77
LONGEST FLOWPATH FROM NODE 24.00 TO NODE 22.00 = 695.00 FEET.

 FLOW PROCESS FROM NODE 22.00 TO NODE 22.00 IS CODE = 11

>>>>CONFLUENCE MEMORY BANK # 2 WITH THE MAIN-STREAM MEMORY<<<<<

** MAIN STREAM CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	3.48	8.50	2.995	0.30(0.10)	0.35	1.3	26.00
2	3.48	8.51	2.993	0.30(0.10)	0.35	1.3	27.00
3	3.52	9.77	2.765	0.30(0.11)	0.35	1.4	29.00
4	3.36	11.44	2.527	0.30(0.11)	0.35	1.5	24.00
LONGEST FLOWPATH FROM NODE 24.00 TO NODE 22.00 =					695.00 FEET.		

** MEMORY BANK # 2 CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	7.61	8.23	3.051	0.33(0.09)	0.28	2.9	14.00
2	7.90	9.25	2.854	0.33(0.09)	0.28	3.2	20.00
3	7.91	9.70	2.776	0.33(0.09)	0.28	3.3	23.00
4	7.91	9.73	2.773	0.33(0.09)	0.28	3.3	17.00
5	7.11	14.72	2.187	0.33(0.08)	0.25	3.8	12.00
6	6.39	18.30	1.930	0.33(0.08)	0.25	3.8	10.00
LONGEST FLOWPATH FROM NODE 10.00 TO NODE 22.00 =					1280.00 FEET.		

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	11.05	8.23	3.051	0.32(0.10)	0.30	4.1	14.00
2	11.17	8.50	2.995	0.32(0.10)	0.30	4.2	26.00
3	11.17	8.51	2.993	0.32(0.10)	0.30	4.2	27.00
4	11.40	9.25	2.854	0.32(0.10)	0.30	4.5	20.00
5	11.43	9.70	2.776	0.32(0.10)	0.30	4.7	23.00
6	11.42	9.73	2.773	0.32(0.10)	0.30	4.7	17.00
7	11.42	9.77	2.765	0.32(0.10)	0.30	4.7	29.00
8	10.99	11.44	2.527	0.32(0.09)	0.29	4.9	24.00
9	10.00	14.72	2.187	0.32(0.09)	0.28	5.2	12.00
10	8.91	18.30	1.930	0.32(0.09)	0.28	5.3	10.00
TOTAL AREA (ACRES) =					5.3		

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE (CFS) = 11.43 Tc (MIN.) = 9.705
 EFFECTIVE AREA (ACRES) = 4.66 AREA-AVERAGED Fm (INCH/HR) = 0.10
 AREA-AVERAGED Fp (INCH/HR) = 0.32 AREA-AVERAGED Ap = 0.30
 TOTAL AREA (ACRES) = 5.3
 LONGEST FLOWPATH FROM NODE 10.00 TO NODE 22.00 = 1280.00 FEET.

 FLOW PROCESS FROM NODE 22.00 TO NODE 33.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<

>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<

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ELEVATION DATA: UPSTREAM(FEET) = 151.30 DOWNSTREAM(FEET) = 150.50
FLOW LENGTH(FEET) = 425.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 27.0 INCH PIPE IS 19.5 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 3.71
ESTIMATED PIPE DIAMETER(INCH) = 27.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 11.43
PIPE TRAVEL TIME(MIN.) = 1.91 Tc(MIN.) = 11.61
LONGEST FLOWPATH FROM NODE 10.00 TO NODE 33.00 = 1705.00 FEET.

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FLOW PROCESS FROM NODE 33.00 TO NODE 33.00 IS CODE = 10

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>>>>MAIN-STREAM MEMORY COPIED ONTO MEMORY BANK # 3 <<<<
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FLOW PROCESS FROM NODE 30.00 TO NODE 31.00 IS CODE = 21

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>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<
>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<
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INITIAL SUBAREA FLOW-LENGTH(FEET) = 150.00
ELEVATION DATA: UPSTREAM(FEET) = 166.50 DOWNSTREAM(FEET) = 165.50

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Tc = K*[(LENGTH** 3.00)/(ELEVATION CHANGE)]**0.20
SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) = 7.277
* 10 YEAR RAINFALL INTENSITY(INCH/HR) = 3.274
SUBAREA Tc AND LOSS RATE DATA(AMC II):
DEVELOPMENT TYPE/      SCS SOIL  AREA      Fp          Ap      SCS  Tc
LAND USE                GROUP   (ACRES) (INCH/HR) (DECIMAL) CN (MIN.)
CONDOMINIUMS           B       0.29     0.30     0.350     56   7.28
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.30
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.350
SUBAREA RUNOFF(CFS) = 0.83
TOTAL AREA(ACRES) = 0.29 PEAK FLOW RATE(CFS) = 0.83

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FLOW PROCESS FROM NODE 31.00 TO NODE 31.00 IS CODE = 81

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>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<
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MAINLINE Tc(MIN.) = 7.28
* 10 YEAR RAINFALL INTENSITY(INCH/HR) = 3.274
SUBAREA LOSS RATE DATA(AMC II):
DEVELOPMENT TYPE/      SCS SOIL  AREA      Fp          Ap      SCS
LAND USE                GROUP   (ACRES) (INCH/HR) (DECIMAL) CN
CONDOMINIUMS           B       0.65     0.30     0.350     56
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.30
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.350
SUBAREA AREA(ACRES) = 0.65 SUBAREA RUNOFF(CFS) = 1.85
EFFECTIVE AREA(ACRES) = 0.94 AREA-AVERAGED Fm(INCH/HR) = 0.11
AREA-AVERAGED Fp(INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.35
TOTAL AREA(ACRES) = 0.9 PEAK FLOW RATE(CFS) = 2.68

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 FLOW PROCESS FROM NODE 31.00 TO NODE 33.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
 >>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 157.00 DOWNSTREAM(FEET) = 150.50
 FLOW LENGTH(FEET) = 380.00 MANNING'S N = 0.013
 DEPTH OF FLOW IN 12.0 INCH PIPE IS 6.6 INCHES
 PIPE-FLOW VELOCITY(FEET/SEC.) = 6.02
 ESTIMATED PIPE DIAMETER(INCH) = 12.00 NUMBER OF PIPES = 1
 PIPE-FLOW(CFS) = 2.68
 PIPE TRAVEL TIME(MIN.) = 1.05 Tc(MIN.) = 8.33
 LONGEST FLOWPATH FROM NODE 30.00 TO NODE 33.00 = 530.00 FEET.

 FLOW PROCESS FROM NODE 33.00 TO NODE 33.00 IS CODE = 11

>>>>CONFLUENCE MEMORY BANK # 3 WITH THE MAIN-STREAM MEMORY<<<<<

** MAIN STREAM CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	2.68	8.33	3.030	0.30 (0.11)	0.35	0.9	30.00
LONGEST FLOWPATH FROM NODE 30.00 TO NODE 33.00 =					530.00 FEET.		

** MEMORY BANK # 3 CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	11.05	10.15	2.706	0.32 (0.10)	0.30	4.1	14.00
2	11.17	10.41	2.666	0.32 (0.10)	0.30	4.2	26.00
3	11.17	10.43	2.664	0.32 (0.10)	0.30	4.2	27.00
4	11.40	11.15	2.563	0.32 (0.10)	0.30	4.5	20.00
5	11.43	11.61	2.505	0.32 (0.10)	0.30	4.7	23.00
6	11.42	11.64	2.502	0.32 (0.10)	0.30	4.7	17.00
7	11.42	11.68	2.496	0.32 (0.10)	0.30	4.7	29.00
8	10.99	13.36	2.312	0.32 (0.09)	0.29	4.9	24.00
9	10.00	16.67	2.036	0.32 (0.09)	0.28	5.2	12.00
10	8.91	20.35	1.816	0.32 (0.09)	0.28	5.3	10.00
LONGEST FLOWPATH FROM NODE 10.00 TO NODE 33.00 =					1705.00 FEET.		

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	12.87	8.33	3.030	0.32 (0.10)	0.31	4.3	30.00
2	13.43	10.15	2.706	0.32 (0.10)	0.31	5.0	14.00
3	13.52	10.41	2.666	0.32 (0.10)	0.31	5.1	26.00
4	13.52	10.43	2.664	0.32 (0.10)	0.31	5.1	27.00
5	13.66	11.15	2.563	0.32 (0.10)	0.31	5.5	20.00
6	13.63	11.61	2.505	0.32 (0.10)	0.31	5.6	23.00
7	13.62	11.64	2.502	0.32 (0.10)	0.31	5.6	17.00
8	13.61	11.68	2.496	0.32 (0.10)	0.31	5.6	29.00

9	13.02	13.36	2.312	0.32 (0.10)	0.30	5.9	24.00
10	11.77	16.67	2.036	0.32 (0.09)	0.29	6.2	12.00
11	10.48	20.35	1.816	0.32 (0.09)	0.29	6.2	10.00
TOTAL AREA (ACRES) =			6.2				

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE (CFS) = 13.66 Tc (MIN.) = 11.154
EFFECTIVE AREA (ACRES) = 5.45 AREA-AVERAGED Fm (INCH/HR) = 0.10
AREA-AVERAGED Fp (INCH/HR) = 0.32 AREA-AVERAGED Ap = 0.31
TOTAL AREA (ACRES) = 6.2
LONGEST FLOWPATH FROM NODE 10.00 TO NODE 33.00 = 1705.00 FEET.

FLOW PROCESS FROM NODE 16.00 TO NODE 16.00 IS CODE = 12

>>>>CLEAR MEMORY BANK # 1 <<<<<

FLOW PROCESS FROM NODE 22.00 TO NODE 22.00 IS CODE = 12

>>>>CLEAR MEMORY BANK # 2 <<<<<

FLOW PROCESS FROM NODE 33.00 TO NODE 33.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

TOTAL NUMBER OF STREAMS = 5
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION (MIN.) = 11.15
RAINFALL INTENSITY (INCH/HR) = 2.56
AREA-AVERAGED Fm (INCH/HR) = 0.10
AREA-AVERAGED Fp (INCH/HR) = 0.32
AREA-AVERAGED Ap = 0.31
EFFECTIVE STREAM AREA (ACRES) = 5.45
TOTAL STREAM AREA (ACRES) = 6.25
PEAK FLOW RATE (CFS) AT CONFLUENCE = 13.66

FLOW PROCESS FROM NODE 30.00 TO NODE 33.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

INITIAL SUBAREA FLOW-LENGTH (FEET) = 185.00
ELEVATION DATA: UPSTREAM (FEET) = 166.60 DOWNSTREAM (FEET) = 166.00

Tc = K * [(LENGTH** 3.00) / (ELEVATION CHANGE)]**0.20
SUBAREA ANALYSIS USED MINIMUM Tc (MIN.) = 9.141
* 10 YEAR RAINFALL INTENSITY (INCH/HR) = 2.873

SUBAREA Tc AND LOSS RATE DATA (AMC II):
DEVELOPMENT TYPE/ SCS SOIL AREA Fp Ap SCS Tc

LAND USE GROUP (ACRES) (INCH/HR) (DECIMAL) CN (MIN.)
 CONDOMINIUMS B 0.67 0.30 0.350 56 9.14
 SUBAREA AVERAGE PERVIOUS LOSS RATE, F_p (INCH/HR) = 0.30
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, A_p = 0.350
 SUBAREA RUNOFF (CFS) = 1.67
 TOTAL AREA (ACRES) = 0.67 PEAK FLOW RATE (CFS) = 1.67

FLOW PROCESS FROM NODE 33.00 TO NODE 33.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

TOTAL NUMBER OF STREAMS = 5
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
 TIME OF CONCENTRATION (MIN.) = 9.14
 RAINFALL INTENSITY (INCH/HR) = 2.87
 AREA-AVERAGED F_m (INCH/HR) = 0.11
 AREA-AVERAGED F_p (INCH/HR) = 0.30
 AREA-AVERAGED A_p = 0.35
 EFFECTIVE STREAM AREA (ACRES) = 0.67
 TOTAL STREAM AREA (ACRES) = 0.67
 PEAK FLOW RATE (CFS) AT CONFLUENCE = 1.67

FLOW PROCESS FROM NODE 34.00 TO NODE 35.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
 >>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

INITIAL SUBAREA FLOW-LENGTH (FEET) = 180.00
 ELEVATION DATA: UPSTREAM (FEET) = 166.20 DOWNSTREAM (FEET) = 166.10

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$
 SUBAREA ANALYSIS USED MINIMUM T_c (MIN.) = 12.867
 * 10 YEAR RAINFALL INTENSITY (INCH/HR) = 2.362
 SUBAREA T_c AND LOSS RATE DATA (AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	F_p (INCH/HR)	A_p (DECIMAL)	SCS CN	T_c (MIN.)
CONDOMINIUMS	B	0.50	0.30	0.350	56	12.87

 SUBAREA AVERAGE PERVIOUS LOSS RATE, F_p (INCH/HR) = 0.30
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, A_p = 0.350
 SUBAREA RUNOFF (CFS) = 1.02
 TOTAL AREA (ACRES) = 0.50 PEAK FLOW RATE (CFS) = 1.02

FLOW PROCESS FROM NODE 33.00 TO NODE 33.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

TOTAL NUMBER OF STREAMS = 5
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 3 ARE:
 TIME OF CONCENTRATION (MIN.) = 12.87
 RAINFALL INTENSITY (INCH/HR) = 2.36
 AREA-AVERAGED F_m (INCH/HR) = 0.11

AREA-AVERAGED F_p (INCH/HR) = 0.30
AREA-AVERAGED A_p = 0.35
EFFECTIVE STREAM AREA (ACRES) = 0.50
TOTAL STREAM AREA (ACRES) = 0.50
PEAK FLOW RATE (CFS) AT CONFLUENCE = 1.02

FLOW PROCESS FROM NODE 36.00 TO NODE 37.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

=====

INITIAL SUBAREA FLOW-LENGTH (FEET) = 150.00
ELEVATION DATA: UPSTREAM (FEET) = 166.70 DOWNSTREAM (FEET) = 166.40

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$
SUBAREA ANALYSIS USED MINIMUM T_c (MIN.) = 9.258
* 10 YEAR RAINFALL INTENSITY (INCH/HR) = 2.852

SUBAREA T_c AND LOSS RATE DATA (AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	F_p (INCH/HR)	A_p (DECIMAL)	SCS CN	T_c (MIN.)
CONDOMINIUMS	B	0.29	0.30	0.350	56	9.26

SUBAREA AVERAGE PERVIOUS LOSS RATE, F_p (INCH/HR) = 0.30

SUBAREA AVERAGE PERVIOUS AREA FRACTION, A_p = 0.350

SUBAREA RUNOFF (CFS) = 0.72

TOTAL AREA (ACRES) = 0.29 PEAK FLOW RATE (CFS) = 0.72

FLOW PROCESS FROM NODE 33.00 TO NODE 33.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

=====

TOTAL NUMBER OF STREAMS = 5
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 4 ARE:
TIME OF CONCENTRATION (MIN.) = 9.26
RAINFALL INTENSITY (INCH/HR) = 2.85
AREA-AVERAGED F_m (INCH/HR) = 0.11
AREA-AVERAGED F_p (INCH/HR) = 0.30
AREA-AVERAGED A_p = 0.35
EFFECTIVE STREAM AREA (ACRES) = 0.29
TOTAL STREAM AREA (ACRES) = 0.29
PEAK FLOW RATE (CFS) AT CONFLUENCE = 0.72

FLOW PROCESS FROM NODE 38.00 TO NODE 33.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

=====

INITIAL SUBAREA FLOW-LENGTH (FEET) = 150.00
ELEVATION DATA: UPSTREAM (FEET) = 166.40 DOWNSTREAM (FEET) = 166.00

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$
SUBAREA ANALYSIS USED MINIMUM T_c (MIN.) = 8.741

* 10 YEAR RAINFALL INTENSITY (INCH/HR) = 2.948
 SUBAREA Tc AND LOSS RATE DATA (AMC II):
 DEVELOPMENT TYPE/ SCS SOIL AREA Fp Ap SCS Tc
 LAND USE GROUP (ACRES) (INCH/HR) (DECIMAL) CN (MIN.)
 CONDOMINIUMS B 0.18 0.30 0.350 56 8.74
 SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp (INCH/HR) = 0.30
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.350
 SUBAREA RUNOFF (CFS) = 0.46
 TOTAL AREA (ACRES) = 0.18 PEAK FLOW RATE (CFS) = 0.46

FLOW PROCESS FROM NODE 33.00 TO NODE 33.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
 >>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<

=====

TOTAL NUMBER OF STREAMS = 5
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 5 ARE:
 TIME OF CONCENTRATION (MIN.) = 8.74
 RAINFALL INTENSITY (INCH/HR) = 2.95
 AREA-AVERAGED Fm (INCH/HR) = 0.11
 AREA-AVERAGED Fp (INCH/HR) = 0.30
 AREA-AVERAGED Ap = 0.35
 EFFECTIVE STREAM AREA (ACRES) = 0.18
 TOTAL STREAM AREA (ACRES) = 0.18
 PEAK FLOW RATE (CFS) AT CONFLUENCE = 0.46

** CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	12.87	8.33	3.030	0.32 (0.10)	0.31	4.3	30.00
1	13.43	10.15	2.706	0.32 (0.10)	0.31	5.0	14.00
1	13.52	10.41	2.666	0.32 (0.10)	0.31	5.1	26.00
1	13.52	10.43	2.664	0.32 (0.10)	0.31	5.1	27.00
1	13.66	11.15	2.563	0.32 (0.10)	0.31	5.5	20.00
1	13.63	11.61	2.505	0.32 (0.10)	0.31	5.6	23.00
1	13.62	11.64	2.502	0.32 (0.10)	0.31	5.6	17.00
1	13.61	11.68	2.496	0.32 (0.10)	0.31	5.6	29.00
1	13.02	13.36	2.312	0.32 (0.10)	0.30	5.9	24.00
1	11.77	16.67	2.036	0.32 (0.09)	0.29	6.2	12.00
1	10.48	20.35	1.816	0.32 (0.09)	0.29	6.2	10.00
2	1.67	9.14	2.873	0.30 (0.11)	0.35	0.7	30.00
3	1.02	12.87	2.362	0.30 (0.11)	0.35	0.5	34.00
4	0.72	9.26	2.852	0.30 (0.11)	0.35	0.3	36.00
5	0.46	8.74	2.948	0.30 (0.11)	0.35	0.2	38.00

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
 CONFLUENCE FORMULA USED FOR 5 STREAMS.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	16.47	8.33	3.030	0.31 (0.10)	0.32	5.7	30.00
2	16.67	8.74	2.948	0.31 (0.10)	0.32	5.9	38.00

3	16.84	9.14	2.873	0.31	(0.10)	0.32	6.1	30.00
4	16.87	9.26	2.852	0.31	(0.10)	0.32	6.2	36.00
5	17.02	10.15	2.706	0.31	(0.10)	0.32	6.6	14.00
6	17.08	10.41	2.666	0.31	(0.10)	0.32	6.7	26.00
7	17.08	10.43	2.664	0.31	(0.10)	0.32	6.7	27.00
8	17.14	11.15	2.563	0.31	(0.10)	0.32	7.0	20.00
9	17.06	11.61	2.505	0.31	(0.10)	0.32	7.2	23.00
10	17.05	11.64	2.502	0.31	(0.10)	0.32	7.2	17.00
11	17.04	11.68	2.496	0.31	(0.10)	0.32	7.2	29.00
12	16.52	12.87	2.362	0.31	(0.10)	0.31	7.4	34.00
13	16.27	13.36	2.312	0.31	(0.10)	0.31	7.5	24.00
14	14.62	16.67	2.036	0.31	(0.09)	0.30	7.8	12.00
15	13.01	20.35	1.816	0.31	(0.09)	0.30	7.9	10.00

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE (CFS) = 17.14 Tc (MIN.) = 11.15
EFFECTIVE AREA (ACRES) = 7.03 AREA-AVERAGED Fm (INCH/HR) = 0.10
AREA-AVERAGED Fp (INCH/HR) = 0.31 AREA-AVERAGED Ap = 0.32
TOTAL AREA (ACRES) = 7.9
LONGEST FLOWPATH FROM NODE 10.00 TO NODE 33.00 = 1705.00 FEET.

FLOW PROCESS FROM NODE 33.00 TO NODE 38.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<

=====

ELEVATION DATA: UPSTREAM (FEET) = 150.50 DOWNSTREAM (FEET) = 150.20
FLOW LENGTH (FEET) = 150.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 30.0 INCH PIPE IS 23.5 INCHES
PIPE-FLOW VELOCITY (FEET/SEC.) = 4.15
ESTIMATED PIPE DIAMETER (INCH) = 30.00 NUMBER OF PIPES = 1
PIPE-FLOW (CFS) = 17.14
PIPE TRAVEL TIME (MIN.) = 0.60 Tc (MIN.) = 11.76
LONGEST FLOWPATH FROM NODE 10.00 TO NODE 38.00 = 1855.00 FEET.

FLOW PROCESS FROM NODE 38.00 TO NODE 38.00 IS CODE = 10

>>>>MAIN-STREAM MEMORY COPIED ONTO MEMORY BANK # 1 <<<<<

FLOW PROCESS FROM NODE 39.00 TO NODE 40.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

=====

INITIAL SUBAREA FLOW-LENGTH (FEET) = 300.00
ELEVATION DATA: UPSTREAM (FEET) = 168.00 DOWNSTREAM (FEET) = 166.90

Tc = K * [(LENGTH** 3.00) / (ELEVATION CHANGE)] ** 0.20
SUBAREA ANALYSIS USED MINIMUM Tc (MIN.) = 10.822
* 10 YEAR RAINFALL INTENSITY (INCH/HR) = 2.608

SUBAREA Tc AND LOSS RATE DATA (AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN	Tc (MIN.)
CONDOMINIUMS	B	0.80	0.30	0.350	56	10.82

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp (INCH/HR) = 0.30
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.350
SUBAREA RUNOFF (CFS) = 1.80
TOTAL AREA (ACRES) = 0.80 PEAK FLOW RATE (CFS) = 1.80

FLOW PROCESS FROM NODE 40.00 TO NODE 41.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<

=====

ELEVATION DATA: UPSTREAM (FEET) = 154.00 DOWNSTREAM (FEET) = 151.10
FLOW LENGTH (FEET) = 40.00 MANNING'S N = 0.013
ESTIMATED PIPE DIAMETER (INCH) INCREASED TO 12.000
DEPTH OF FLOW IN 12.0 INCH PIPE IS 3.6 INCHES
PIPE-FLOW VELOCITY (FEET/SEC.) = 9.19
ESTIMATED PIPE DIAMETER (INCH) = 12.000 NUMBER OF PIPES = 1
PIPE-FLOW (CFS) = 1.80
PIPE TRAVEL TIME (MIN.) = 0.07 Tc (MIN.) = 10.89
LONGEST FLOWPATH FROM NODE 39.00 TO NODE 41.00 = 340.00 FEET.

FLOW PROCESS FROM NODE 41.00 TO NODE 41.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

=====

MAINLINE Tc (MIN.) = 10.89
* 10 YEAR RAINFALL INTENSITY (INCH/HR) = 2.598
SUBAREA LOSS RATE DATA (AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
CONDOMINIUMS	B	0.45	0.30	0.350	56

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp (INCH/HR) = 0.30
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.350
SUBAREA AREA (ACRES) = 0.45 SUBAREA RUNOFF (CFS) = 1.01
EFFECTIVE AREA (ACRES) = 1.25 AREA-AVERAGED Fm (INCH/HR) = 0.10
AREA-AVERAGED Fp (INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.35
TOTAL AREA (ACRES) = 1.2 PEAK FLOW RATE (CFS) = 2.80

FLOW PROCESS FROM NODE 41.00 TO NODE 42.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<

=====

ELEVATION DATA: UPSTREAM (FEET) = 151.10 DOWNSTREAM (FEET) = 150.60
FLOW LENGTH (FEET) = 270.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 18.0 INCH PIPE IS 10.4 INCHES
PIPE-FLOW VELOCITY (FEET/SEC.) = 2.65
ESTIMATED PIPE DIAMETER (INCH) = 18.00 NUMBER OF PIPES = 1

PIPE-FLOW (CFS) = 2.80
 PIPE TRAVEL TIME (MIN.) = 1.70 Tc (MIN.) = 12.60
 LONGEST FLOWPATH FROM NODE 39.00 TO NODE 42.00 = 610.00 FEET.

FLOW PROCESS FROM NODE 42.00 TO NODE 42.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<

=====

MAINLINE Tc (MIN.) = 12.60
 * 10 YEAR RAINFALL INTENSITY (INCH/HR) = 2.391
 SUBAREA LOSS RATE DATA (AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
CONDOMINIUMS	B	1.15	0.30	0.350	56

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp (INCH/HR) = 0.30
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.350
 SUBAREA AREA (ACRES) = 1.15 SUBAREA RUNOFF (CFS) = 2.37
 EFFECTIVE AREA (ACRES) = 2.40 AREA-AVERAGED Fm (INCH/HR) = 0.10
 AREA-AVERAGED Fp (INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.35
 TOTAL AREA (ACRES) = 2.4 PEAK FLOW RATE (CFS) = 4.94

FLOW PROCESS FROM NODE 42.00 TO NODE 42.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<

=====

TOTAL NUMBER OF STREAMS = 3
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
 TIME OF CONCENTRATION (MIN.) = 12.60
 RAINFALL INTENSITY (INCH/HR) = 2.39
 AREA-AVERAGED Fm (INCH/HR) = 0.10
 AREA-AVERAGED Fp (INCH/HR) = 0.30
 AREA-AVERAGED Ap = 0.35
 EFFECTIVE STREAM AREA (ACRES) = 2.40
 TOTAL STREAM AREA (ACRES) = 2.40
 PEAK FLOW RATE (CFS) AT CONFLUENCE = 4.94

FLOW PROCESS FROM NODE 43.00 TO NODE 44.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<
 >>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

=====

INITIAL SUBAREA FLOW-LENGTH (FEET) = 180.00
 ELEVATION DATA: UPSTREAM (FEET) = 168.00 DOWNSTREAM (FEET) = 166.20

Tc = K * [(LENGTH** 3.00) / (ELEVATION CHANGE)]**0.20
 SUBAREA ANALYSIS USED MINIMUM Tc (MIN.) = 7.218
 * 10 YEAR RAINFALL INTENSITY (INCH/HR) = 3.289
 SUBAREA Tc AND LOSS RATE DATA (AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN	Tc (MIN.)
CONDOMINIUMS	B	0.44	0.30	0.350	56	7.22

SUBAREA AVERAGE PERVIOUS LOSS RATE, F_p (INCH/HR) = 0.30
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, A_p = 0.350
 SUBAREA RUNOFF (CFS) = 1.26
 TOTAL AREA (ACRES) = 0.44 PEAK FLOW RATE (CFS) = 1.26

 FLOW PROCESS FROM NODE 44.00 TO NODE 42.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
 >>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<

=====

ELEVATION DATA: UPSTREAM (FEET) = 153.00 DOWNSTREAM (FEET) = 150.60
 FLOW LENGTH (FEET) = 75.00 MANNING'S N = 0.013
 ESTIMATED PIPE DIAMETER (INCH) INCREASED TO 12.000
 DEPTH OF FLOW IN 12.0 INCH PIPE IS 3.7 INCHES
 PIPE-FLOW VELOCITY (FEET/SEC.) = 6.20
 ESTIMATED PIPE DIAMETER (INCH) = 12.000 NUMBER OF PIPES = 1
 PIPE-FLOW (CFS) = 1.26
 PIPE TRAVEL TIME (MIN.) = 0.20 T_c (MIN.) = 7.42
 LONGEST FLOWPATH FROM NODE 43.00 TO NODE 42.00 = 255.00 FEET.

 FLOW PROCESS FROM NODE 42.00 TO NODE 42.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

=====

TOTAL NUMBER OF STREAMS = 3
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
 TIME OF CONCENTRATION (MIN.) = 7.42
 RAINFALL INTENSITY (INCH/HR) = 3.24
 AREA-AVERAGED F_m (INCH/HR) = 0.10
 AREA-AVERAGED F_p (INCH/HR) = 0.30
 AREA-AVERAGED A_p = 0.35
 EFFECTIVE STREAM AREA (ACRES) = 0.44
 TOTAL STREAM AREA (ACRES) = 0.44
 PEAK FLOW RATE (CFS) AT CONFLUENCE = 1.26

 FLOW PROCESS FROM NODE 45.00 TO NODE 46.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
 >>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

=====

INITIAL SUBAREA FLOW-LENGTH (FEET) = 180.00
 ELEVATION DATA: UPSTREAM (FEET) = 168.00 DOWNSTREAM (FEET) = 166.40

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$
 SUBAREA ANALYSIS USED MINIMUM T_c (MIN.) = 7.390
 * 10 YEAR RAINFALL INTENSITY (INCH/HR) = 3.245

SUBAREA T_c AND LOSS RATE DATA (AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	F_p (INCH/HR)	A_p (DECIMAL)	SCS CN	T_c (MIN.)
CONDOMINIUMS	B	0.45	0.30	0.350	56	7.39
SUBAREA AVERAGE PERVIOUS LOSS RATE, F_p (INCH/HR) = 0.30						

SUBAREA AVERAGE PERVIOUS AREA FRACTION, $A_p = 0.350$
 SUBAREA RUNOFF (CFS) = 1.27
 TOTAL AREA (ACRES) = 0.45 PEAK FLOW RATE (CFS) = 1.27

 FLOW PROCESS FROM NODE 46.00 TO NODE 42.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
 >>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<

=====

ELEVATION DATA: UPSTREAM (FEET) = 153.00 DOWNSTREAM (FEET) = 150.60
 FLOW LENGTH (FEET) = 75.00 MANNING'S N = 0.013
 ESTIMATED PIPE DIAMETER (INCH) INCREASED TO 12.000
 DEPTH OF FLOW IN 12.0 INCH PIPE IS 3.7 INCHES
 PIPE-FLOW VELOCITY (FEET/SEC.) = 6.22
 ESTIMATED PIPE DIAMETER (INCH) = 12.00 NUMBER OF PIPES = 1
 PIPE-FLOW (CFS) = 1.27
 PIPE TRAVEL TIME (MIN.) = 0.20 T_c (MIN.) = 7.59
 LONGEST FLOWPATH FROM NODE 45.00 TO NODE 42.00 = 255.00 FEET.

 FLOW PROCESS FROM NODE 42.00 TO NODE 42.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
 >>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<

=====

TOTAL NUMBER OF STREAMS = 3
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 3 ARE:
 TIME OF CONCENTRATION (MIN.) = 7.59
 RAINFALL INTENSITY (INCH/HR) = 3.20
 AREA-AVERAGED F_m (INCH/HR) = 0.10
 AREA-AVERAGED F_p (INCH/HR) = 0.30
 AREA-AVERAGED $A_p = 0.35$
 EFFECTIVE STREAM AREA (ACRES) = 0.45
 TOTAL STREAM AREA (ACRES) = 0.45
 PEAK FLOW RATE (CFS) AT CONFLUENCE = 1.27

** CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	T_c (MIN.)	Intensity (INCH/HR)	$F_p(F_m)$ (INCH/HR)	A_p	A_e (ACRES)	HEADWATER NODE
1	4.94	12.60	2.391	0.30 (0.10)	0.35	2.4	39.00
2	1.26	7.42	3.238	0.30 (0.11)	0.35	0.4	43.00
3	1.27	7.59	3.196	0.30 (0.10)	0.35	0.4	45.00

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
 CONFLUENCE FORMULA USED FOR 3 STREAMS.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	T_c (MIN.)	Intensity (INCH/HR)	$F_p(F_m)$ (INCH/HR)	A_p	A_e (ACRES)	HEADWATER NODE
1	6.51	7.42	3.238	0.30 (0.10)	0.35	2.3	43.00
2	6.54	7.59	3.196	0.30 (0.10)	0.35	2.3	45.00
3	6.80	12.60	2.391	0.30 (0.10)	0.35	3.3	39.00

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE (CFS) = 6.80 Tc (MIN.) = 12.60
 EFFECTIVE AREA (ACRES) = 3.29 AREA-AVERAGED Fm (INCH/HR) = 0.10
 AREA-AVERAGED Fp (INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.35
 TOTAL AREA (ACRES) = 3.3
 LONGEST FLOWPATH FROM NODE 39.00 TO NODE 42.00 = 610.00 FEET.

FLOW PROCESS FROM NODE 42.00 TO NODE 38.00 IS CODE = 31

 >>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
 >>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<

=====

ELEVATION DATA: UPSTREAM (FEET) = 150.60 DOWNSTREAM (FEET) = 150.20
 FLOW LENGTH (FEET) = 190.00 MANNING'S N = 0.013
 DEPTH OF FLOW IN 21.0 INCH PIPE IS 16.5 INCHES
 PIPE-FLOW VELOCITY (FEET/SEC.) = 3.35
 ESTIMATED PIPE DIAMETER (INCH) = 21.00 NUMBER OF PIPES = 1
 PIPE-FLOW (CFS) = 6.80
 PIPE TRAVEL TIME (MIN.) = 0.94 Tc (MIN.) = 13.54
 LONGEST FLOWPATH FROM NODE 39.00 TO NODE 38.00 = 800.00 FEET.

FLOW PROCESS FROM NODE 38.00 TO NODE 38.00 IS CODE = 11

 >>>>CONFLUENCE MEMORY BANK # 1 WITH THE MAIN-STREAM MEMORY<<<<<

** MAIN STREAM CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp (Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	6.51	8.37	3.022	0.30 (0.10)	0.35	2.3	43.00
2	6.54	8.54	2.988	0.30 (0.10)	0.35	2.3	45.00
3	6.80	13.54	2.294	0.30 (0.10)	0.35	3.3	39.00

LONGEST FLOWPATH FROM NODE 39.00 TO NODE 38.00 = 800.00 FEET.

** MEMORY BANK # 1 CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp (Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	16.47	8.93	2.911	0.31 (0.10)	0.32	5.7	30.00
2	16.67	9.34	2.837	0.31 (0.10)	0.32	5.9	38.00
3	16.84	9.74	2.770	0.31 (0.10)	0.32	6.1	30.00
4	16.87	9.86	2.751	0.31 (0.10)	0.32	6.2	36.00
5	17.02	10.75	2.617	0.31 (0.10)	0.32	6.6	14.00
6	17.08	11.02	2.581	0.31 (0.10)	0.32	6.7	26.00
7	17.08	11.03	2.580	0.31 (0.10)	0.32	6.7	27.00
8	17.14	11.76	2.487	0.31 (0.10)	0.32	7.0	20.00
9	17.06	12.22	2.433	0.31 (0.10)	0.32	7.2	23.00
10	17.05	12.24	2.431	0.31 (0.10)	0.32	7.2	17.00
11	17.04	12.29	2.425	0.31 (0.10)	0.32	7.2	29.00
12	16.52	13.47	2.301	0.31 (0.10)	0.31	7.4	34.00
13	16.27	13.96	2.254	0.31 (0.10)	0.31	7.5	24.00
14	14.62	17.28	1.994	0.31 (0.09)	0.30	7.8	12.00
15	13.01	20.99	1.784	0.31 (0.09)	0.30	7.9	10.00

LONGEST FLOWPATH FROM NODE 10.00 TO NODE 38.00 = 1855.00 FEET.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp (Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	22.55	8.37	3.022	0.31 (0.10)	0.33	7.6	43.00
2	22.71	8.54	2.988	0.31 (0.10)	0.33	7.7	45.00
3	23.03	8.93	2.911	0.31 (0.10)	0.33	8.1	30.00
4	23.25	9.34	2.837	0.31 (0.10)	0.33	8.4	38.00
5	23.44	9.74	2.770	0.31 (0.10)	0.33	8.7	30.00
6	23.47	9.86	2.751	0.31 (0.10)	0.33	8.7	36.00
7	23.68	10.75	2.617	0.31 (0.10)	0.33	9.3	14.00
8	23.74	11.02	2.581	0.31 (0.10)	0.33	9.5	26.00
9	23.75	11.03	2.580	0.31 (0.10)	0.33	9.5	27.00
10	23.84	11.76	2.487	0.31 (0.10)	0.33	10.0	20.00
11	23.79	12.22	2.433	0.31 (0.10)	0.33	10.2	23.00
12	23.79	12.24	2.431	0.31 (0.10)	0.33	10.2	17.00
13	23.77	12.29	2.425	0.31 (0.10)	0.33	10.3	29.00
14	23.32	13.47	2.301	0.31 (0.10)	0.32	10.7	34.00
15	23.29	13.54	2.294	0.31 (0.10)	0.32	10.7	39.00
16	22.95	13.96	2.254	0.31 (0.10)	0.32	10.8	24.00
17	20.49	17.28	1.994	0.31 (0.10)	0.32	11.1	12.00
18	18.22	20.99	1.784	0.31 (0.10)	0.32	11.2	10.00
TOTAL AREA (ACRES) =		11.2					

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE (CFS) = 23.84 Tc (MIN.) = 11.757
EFFECTIVE AREA (ACRES) = 9.98 AREA-AVERAGED Fm (INCH/HR) = 0.10
AREA-AVERAGED Fp (INCH/HR) = 0.31 AREA-AVERAGED Ap = 0.33
TOTAL AREA (ACRES) = 11.2
LONGEST FLOWPATH FROM NODE 10.00 TO NODE 38.00 = 1855.00 FEET.

FLOW PROCESS FROM NODE 38.00 TO NODE 38.00 IS CODE = 12

>>>>CLEAR MEMORY BANK # 1 <<<<<

FLOW PROCESS FROM NODE 38.00 TO NODE 47.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<

>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<

ELEVATION DATA: UPSTREAM (FEET) = 150.20 DOWNSTREAM (FEET) = 149.90
FLOW LENGTH (FEET) = 150.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 36.0 INCH PIPE IS 24.8 INCHES
PIPE-FLOW VELOCITY (FEET/SEC.) = 4.59
ESTIMATED PIPE DIAMETER (INCH) = 36.00 NUMBER OF PIPES = 1
PIPE-FLOW (CFS) = 23.84
PIPE TRAVEL TIME (MIN.) = 0.54 Tc (MIN.) = 12.30
LONGEST FLOWPATH FROM NODE 10.00 TO NODE 47.00 = 2005.00 FEET.

FLOW PROCESS FROM NODE 47.00 TO NODE 47.00 IS CODE = 10

>>>>MAIN-STREAM MEMORY COPIED ONTO MEMORY BANK # 1 <<<<<

FLOW PROCESS FROM NODE 32.00 TO NODE 48.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<

>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

INITIAL SUBAREA FLOW-LENGTH (FEET) = 300.00

ELEVATION DATA: UPSTREAM (FEET) = 166.30 DOWNSTREAM (FEET) = 166.00

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$

SUBAREA ANALYSIS USED MINIMUM T_c (MIN.) = 11.850

* 10 YEAR RAINFALL INTENSITY (INCH/HR) = 2.476

SUBAREA T_c AND LOSS RATE DATA (AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	F_p (INCH/HR)	A_p (DECIMAL)	SCS CN	T_c (MIN.)
COMMERCIAL	B	2.62	0.30	0.100	56	11.85

SUBAREA AVERAGE PERVIOUS LOSS RATE, F_p (INCH/HR) = 0.30

SUBAREA AVERAGE PERVIOUS AREA FRACTION, A_p = 0.100

SUBAREA RUNOFF (CFS) = 5.77

TOTAL AREA (ACRES) = 2.62 PEAK FLOW RATE (CFS) = 5.77

FLOW PROCESS FROM NODE 48.00 TO NODE 49.00 IS CODE = 61

>>>>COMPUTE STREET FLOW TRAVEL TIME THRU SUBAREA<<<<<

>>>>(STANDARD CURB SECTION USED)<<<<<

UPSTREAM ELEVATION (FEET) = 166.00 DOWNSTREAM ELEVATION (FEET) = 165.60

STREET LENGTH (FEET) = 485.00 CURB HEIGHT (INCHES) = 6.0

STREET HALFWIDTH (FEET) = 25.00

DISTANCE FROM CROWN TO CROSSFALL GRADEBREAK (FEET) = 10.00

INSIDE STREET CROSSFALL (DECIMAL) = 0.020

OUTSIDE STREET CROSSFALL (DECIMAL) = 0.020

SPECIFIED NUMBER OF HALFSTREETS CARRYING RUNOFF = 2

STREET PARKWAY CROSSFALL (DECIMAL) = 0.020

Manning's FRICTION FACTOR for Streetflow Section (curb-to-curb) = 0.0150

Manning's FRICTION FACTOR for Back-of-Walk Flow Section = 0.0200

**TRAVEL TIME COMPUTED USING ESTIMATED FLOW (CFS) = 9.78

STREETFLOW MODEL RESULTS USING ESTIMATED FLOW:

STREET FLOW DEPTH (FEET) = 0.56

HALFSTREET FLOOD WIDTH (FEET) = 24.36

AVERAGE FLOW VELOCITY (FEET/SEC.) = 1.01

PRODUCT OF DEPTH&VELOCITY (FT*FT/SEC.) = 0.56

STREET FLOW TRAVEL TIME (MIN.) = 7.99 T_c (MIN.) = 19.84

* 10 YEAR RAINFALL INTENSITY (INCH/HR) = 1.843

SUBAREA LOSS RATE DATA (AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
COMMERCIAL	B	4.89	0.30	0.100	56
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp (INCH/HR) = 0.30					
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.100					
SUBAREA AREA (ACRES) =		4.89	SUBAREA RUNOFF (CFS) =		7.98
EFFECTIVE AREA (ACRES) =		7.51	AREA-AVERAGED Fm (INCH/HR) =		0.03
AREA-AVERAGED Fp (INCH/HR) =		0.30	AREA-AVERAGED Ap =		0.10
TOTAL AREA (ACRES) =		7.5	PEAK FLOW RATE (CFS) =		12.25

END OF SUBAREA STREET FLOW HYDRAULICS:

DEPTH (FEET) = 0.60 HALFSTREET FLOOD WIDTH (FEET) = 28.27
FLOW VELOCITY (FEET/SEC.) = 1.05 DEPTH*VELOCITY (FT*FT/SEC.) = 0.62
*NOTE: INITIAL SUBAREA NOMOGRAPH WITH SUBAREA PARAMETERS,
AND L = 485.0 FT WITH ELEVATION-DROP = 0.4 FT, IS 9.4 CFS,
WHICH EXCEEDS THE TOP-OF-CURB STREET CAPACITY AT NODE 49.00
LONGEST FLOWPATH FROM NODE 32.00 TO NODE 49.00 = 785.00 FEET.

FLOW PROCESS FROM NODE 49.00 TO NODE 50.00 IS CODE = 51

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<<
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM (FEET) = 165.60 DOWNSTREAM (FEET) = 165.40
CHANNEL LENGTH THRU SUBAREA (FEET) = 180.00 CHANNEL SLOPE = 0.0011
CHANNEL BASE (FEET) = 5.00 "Z" FACTOR = 0.020
MANNING'S FACTOR = 0.015 MAXIMUM DEPTH (FEET) = 2.00
CHANNEL FLOW THRU SUBAREA (CFS) = 12.25
FLOW VELOCITY (FEET/SEC.) = 2.57 FLOW DEPTH (FEET) = 0.95
TRAVEL TIME (MIN.) = 1.17 Tc (MIN.) = 21.00
LONGEST FLOWPATH FROM NODE 32.00 TO NODE 50.00 = 965.00 FEET.

FLOW PROCESS FROM NODE 50.00 TO NODE 50.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

=====

TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION (MIN.) = 21.00
RAINFALL INTENSITY (INCH/HR) = 1.78
AREA-AVERAGED Fm (INCH/HR) = 0.03
AREA-AVERAGED Fp (INCH/HR) = 0.30
AREA-AVERAGED Ap = 0.10
EFFECTIVE STREAM AREA (ACRES) = 7.51
TOTAL STREAM AREA (ACRES) = 7.51
PEAK FLOW RATE (CFS) AT CONFLUENCE = 12.25

FLOW PROCESS FROM NODE 50.00 TO NODE 50.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

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=====
INITIAL SUBAREA FLOW-LENGTH (FEET) = 665.00
ELEVATION DATA: UPSTREAM (FEET) = 167.20 DOWNSTREAM (FEET) = 165.40

Tc = K * [(LENGTH** 3.00) / (ELEVATION CHANGE)]**0.20
SUBAREA ANALYSIS USED MINIMUM Tc (MIN.) = 13.351
* 10 YEAR RAINFALL INTENSITY (INCH/HR) = 2.312
SUBAREA Tc AND LOSS RATE DATA (AMC II):
DEVELOPMENT TYPE/      SCS SOIL  AREA      Fp        Ap        SCS  Tc
LAND USE                GROUP   (ACRES)  (INCH/HR) (DECIMAL) CN (MIN.)
COMMERCIAL              B       6.31     0.30     0.100    56   13.35
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp (INCH/HR) = 0.30
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.100
SUBAREA RUNOFF (CFS) = 12.96
TOTAL AREA (ACRES) = 6.31 PEAK FLOW RATE (CFS) = 12.96

*****
FLOW PROCESS FROM NODE 50.00 TO NODE 50.00 IS CODE = 1
=====
>>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
>>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<
=====
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
TIME OF CONCENTRATION (MIN.) = 13.35
RAINFALL INTENSITY (INCH/HR) = 2.31
AREA-AVERAGED Fm (INCH/HR) = 0.03
AREA-AVERAGED Fp (INCH/HR) = 0.30
AREA-AVERAGED Ap = 0.10
EFFECTIVE STREAM AREA (ACRES) = 6.31
TOTAL STREAM AREA (ACRES) = 6.31
PEAK FLOW RATE (CFS) AT CONFLUENCE = 12.96

** CONFLUENCE DATA **
STREAM      Q      Tc  Intensity  Fp(Fm)  Ap      Ae      HEADWATER
NUMBER      (CFS) (MIN.) (INCH/HR) (INCH/HR) (ACRES)  NODE
1           12.25  21.00  1.784  0.30 ( 0.03)  0.10    7.5    32.00
2           12.96  13.35  2.312  0.30 ( 0.03)  0.10    6.3    50.00

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **
STREAM      Q      Tc  Intensity  Fp(Fm)  Ap      Ae      HEADWATER
NUMBER      (CFS) (MIN.) (INCH/HR) (INCH/HR) (ACRES)  NODE
1           23.10  13.35  2.312  0.30 ( 0.03)  0.10    11.1   50.00
2           22.21  21.00  1.784  0.30 ( 0.03)  0.10    13.8   32.00

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
PEAK FLOW RATE (CFS) = 23.10 Tc (MIN.) = 13.35
EFFECTIVE AREA (ACRES) = 11.08 AREA-AVERAGED Fm (INCH/HR) = 0.03
AREA-AVERAGED Fp (INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.10
TOTAL AREA (ACRES) = 13.8
LONGEST FLOWPATH FROM NODE 32.00 TO NODE 50.00 = 965.00 FEET.

```

 FLOW PROCESS FROM NODE 50.00 TO NODE 47.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
 >>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW) <<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 162.40 DOWNSTREAM(FEET) = 149.90
 FLOW LENGTH(FEET) = 185.00 MANNING'S N = 0.013
 DEPTH OF FLOW IN 18.0 INCH PIPE IS 13.0 INCHES
 PIPE-FLOW VELOCITY(FEET/SEC.) = 16.96
 ESTIMATED PIPE DIAMETER(INCH) = 18.00 NUMBER OF PIPES = 1
 PIPE-FLOW(CFS) = 23.10
 PIPE TRAVEL TIME(MIN.) = 0.18 Tc(MIN.) = 13.53
 LONGEST FLOWPATH FROM NODE 32.00 TO NODE 47.00 = 1150.00 FEET.

 FLOW PROCESS FROM NODE 47.00 TO NODE 47.00 IS CODE = 11

>>>>CONFLUENCE MEMORY BANK # 1 WITH THE MAIN-STREAM MEMORY<<<<<

** MAIN STREAM CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	23.10	13.53	2.295	0.30 (0.03)	0.10	11.1	50.00
2	22.21	21.19	1.775	0.30 (0.03)	0.10	13.8	32.00

LONGEST FLOWPATH FROM NODE 32.00 TO NODE 47.00 = 1150.00 FEET.

** MEMORY BANK # 1 CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	22.55	8.93	2.911	0.31 (0.10)	0.33	7.6	43.00
2	22.71	9.10	2.880	0.31 (0.10)	0.33	7.7	45.00
3	23.03	9.50	2.811	0.31 (0.10)	0.33	8.1	30.00
4	23.25	9.89	2.746	0.31 (0.10)	0.33	8.4	38.00
5	23.44	10.29	2.684	0.31 (0.10)	0.33	8.7	30.00
6	23.47	10.41	2.667	0.31 (0.10)	0.33	8.7	36.00
7	23.68	11.30	2.544	0.31 (0.10)	0.33	9.3	14.00
8	23.74	11.56	2.511	0.31 (0.10)	0.33	9.5	26.00
9	23.75	11.58	2.509	0.31 (0.10)	0.33	9.5	27.00
10	23.84	12.30	2.423	0.31 (0.10)	0.33	10.0	20.00
11	23.79	12.76	2.373	0.31 (0.10)	0.33	10.2	23.00
12	23.79	12.78	2.371	0.31 (0.10)	0.33	10.2	17.00
13	23.77	12.83	2.366	0.31 (0.10)	0.33	10.3	29.00
14	23.32	14.02	2.249	0.31 (0.10)	0.32	10.7	34.00
15	23.29	14.09	2.242	0.31 (0.10)	0.32	10.7	39.00
16	22.95	14.53	2.203	0.31 (0.10)	0.32	10.8	24.00
17	20.49	17.85	1.958	0.31 (0.10)	0.32	11.1	12.00
18	18.22	21.58	1.756	0.31 (0.10)	0.32	11.2	10.00

LONGEST FLOWPATH FROM NODE 10.00 TO NODE 47.00 = 2005.00 FEET.

** PEAK FLOW RATE TABLE **

STREAM	Q	Tc	Intensity	Fp(Fm)	Ap	Ae	HEADWATER
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NUMBER	(CFS)	(MIN.)	(INCH/HR)	(INCH/HR)		(ACRES)	NODE
1	41.94	8.93	2.911	0.31 (0.07)	0.22	14.9	43.00
2	42.26	9.10	2.880	0.31 (0.07)	0.22	15.2	45.00
3	42.94	9.50	2.811	0.31 (0.07)	0.22	15.8	30.00
4	43.50	9.89	2.746	0.31 (0.07)	0.22	16.5	38.00
5	44.03	10.29	2.684	0.31 (0.07)	0.22	17.1	30.00
6	44.16	10.41	2.667	0.31 (0.07)	0.22	17.3	36.00
7	45.09	11.30	2.544	0.31 (0.07)	0.21	18.6	14.00
8	45.37	11.56	2.511	0.31 (0.07)	0.21	19.0	26.00
9	45.38	11.58	2.509	0.31 (0.07)	0.21	19.0	27.00
10	46.04	12.30	2.423	0.31 (0.07)	0.21	20.1	20.00
11	46.33	12.76	2.373	0.31 (0.06)	0.21	20.7	23.00
12	46.34	12.78	2.371	0.31 (0.06)	0.21	20.7	17.00
13	46.36	12.83	2.366	0.31 (0.06)	0.21	20.8	29.00
14	46.60	13.53	2.295	0.31 (0.06)	0.21	21.6	50.00
15	46.36	14.02	2.249	0.31 (0.06)	0.21	22.0	34.00
16	46.32	14.09	2.242	0.31 (0.06)	0.21	22.0	39.00
17	45.93	14.53	2.203	0.31 (0.06)	0.21	22.2	24.00
18	43.09	17.85	1.958	0.31 (0.06)	0.20	23.7	12.00
19	40.67	21.19	1.775	0.31 (0.06)	0.20	25.0	32.00
20	40.20	21.58	1.756	0.31 (0.06)	0.20	25.0	10.00
TOTAL AREA (ACRES) =			25.0				

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE (CFS) = 46.60 Tc (MIN.) = 13.533
EFFECTIVE AREA (ACRES) = 21.60 AREA-AVERAGED Fm (INCH/HR) = 0.06
AREA-AVERAGED Fp (INCH/HR) = 0.31 AREA-AVERAGED Ap = 0.22
TOTAL AREA (ACRES) = 25.0
LONGEST FLOWPATH FROM NODE 10.00 TO NODE 47.00 = 2005.00 FEET.

FLOW PROCESS FROM NODE 47.00 TO NODE 47.00 IS CODE = 12

>>>>CLEAR MEMORY BANK # 1 <<<<<

FLOW PROCESS FROM NODE 47.00 TO NODE 47.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

TOTAL NUMBER OF STREAMS = 2

CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:

TIME OF CONCENTRATION (MIN.) = 13.53
RAINFALL INTENSITY (INCH/HR) = 2.29
AREA-AVERAGED Fm (INCH/HR) = 0.06
AREA-AVERAGED Fp (INCH/HR) = 0.31
AREA-AVERAGED Ap = 0.21
EFFECTIVE STREAM AREA (ACRES) = 21.60
TOTAL STREAM AREA (ACRES) = 25.00
PEAK FLOW RATE (CFS) AT CONFLUENCE = 46.60

FLOW PROCESS FROM NODE 51.00 TO NODE 52.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<
=====

INITIAL SUBAREA FLOW-LENGTH (FEET) = 140.00
ELEVATION DATA: UPSTREAM (FEET) = 166.70 DOWNSTREAM (FEET) = 166.30

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$
SUBAREA ANALYSIS USED MINIMUM T_c (MIN.) = 8.386
* 10 YEAR RAINFALL INTENSITY (INCH/HR) = 3.018

SUBAREA T_c AND LOSS RATE DATA (AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	F_p (INCH/HR)	A_p (DECIMAL)	SCS CN	T_c (MIN.)
CONDOMINIUMS	B	0.64	0.30	0.350	56	8.39

SUBAREA AVERAGE PERVIOUS LOSS RATE, F_p (INCH/HR) = 0.30
SUBAREA AVERAGE PERVIOUS AREA FRACTION, A_p = 0.350
SUBAREA RUNOFF (CFS) = 1.68
TOTAL AREA (ACRES) = 0.64 PEAK FLOW RATE (CFS) = 1.68

FLOW PROCESS FROM NODE 52.00 TO NODE 47.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<
=====

ELEVATION DATA: UPSTREAM (FEET) = 156.00 DOWNSTREAM (FEET) = 149.90
FLOW LENGTH (FEET) = 40.00 MANNING'S N = 0.013
ESTIMATED PIPE DIAMETER (INCH) INCREASED TO 12.000
DEPTH OF FLOW IN 12.0 INCH PIPE IS 2.9 INCHES
PIPE-FLOW VELOCITY (FEET/SEC.) = 11.74
ESTIMATED PIPE DIAMETER (INCH) = 12.000 NUMBER OF PIPES = 1
PIPE-FLOW (CFS) = 1.68
PIPE TRAVEL TIME (MIN.) = 0.06 T_c (MIN.) = 8.44
LONGEST FLOWPATH FROM NODE 51.00 TO NODE 47.00 = 180.00 FEET.

FLOW PROCESS FROM NODE 47.00 TO NODE 47.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<
=====

TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
TIME OF CONCENTRATION (MIN.) = 8.44
RAINFALL INTENSITY (INCH/HR) = 3.01
AREA-AVERAGED F_m (INCH/HR) = 0.10
AREA-AVERAGED F_p (INCH/HR) = 0.30
AREA-AVERAGED A_p = 0.35
EFFECTIVE STREAM AREA (ACRES) = 0.64
TOTAL STREAM AREA (ACRES) = 0.64
PEAK FLOW RATE (CFS) AT CONFLUENCE = 1.68

** CONFLUENCE DATA **

STREAM	Q	T_c	Intensity	F_p (F_m)	A_p	A_e	HEADWATER
--------	---	-------	-----------	-----------------	-------	-------	-----------

NUMBER	(CFS)	(MIN.)	(INCH/HR)	(INCH/HR)		(ACRES)	NODE
1	41.94	8.93	2.911	0.31 (0.07)	0.22	14.9	43.00
1	42.26	9.10	2.880	0.31 (0.07)	0.22	15.2	45.00
1	42.94	9.50	2.811	0.31 (0.07)	0.22	15.8	30.00
1	43.50	9.89	2.746	0.31 (0.07)	0.22	16.5	38.00
1	44.03	10.29	2.684	0.31 (0.07)	0.22	17.1	30.00
1	44.16	10.41	2.667	0.31 (0.07)	0.22	17.3	36.00
1	45.09	11.30	2.544	0.31 (0.07)	0.21	18.6	14.00
1	45.37	11.56	2.511	0.31 (0.07)	0.21	19.0	26.00
1	45.38	11.58	2.509	0.31 (0.07)	0.21	19.0	27.00
1	46.04	12.30	2.423	0.31 (0.07)	0.21	20.1	20.00
1	46.33	12.76	2.373	0.31 (0.06)	0.21	20.7	23.00
1	46.34	12.78	2.371	0.31 (0.06)	0.21	20.7	17.00
1	46.36	12.83	2.366	0.31 (0.06)	0.21	20.8	29.00
1	46.60	13.53	2.295	0.31 (0.06)	0.21	21.6	50.00
1	46.36	14.02	2.249	0.31 (0.06)	0.21	22.0	34.00
1	46.32	14.09	2.242	0.31 (0.06)	0.21	22.0	39.00
1	45.93	14.53	2.203	0.31 (0.06)	0.21	22.2	24.00
1	43.09	17.85	1.958	0.31 (0.06)	0.20	23.7	12.00
1	40.67	21.19	1.775	0.31 (0.06)	0.20	25.0	32.00
1	40.20	21.58	1.756	0.31 (0.06)	0.20	25.0	10.00
2	1.68	8.44	3.007	0.30 (0.10)	0.35	0.6	51.00

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	42.66	8.44	3.007	0.31 (0.07)	0.22	14.7	51.00
2	43.57	8.93	2.911	0.31 (0.07)	0.22	15.5	43.00
3	43.87	9.10	2.880	0.31 (0.07)	0.22	15.8	45.00
4	44.50	9.50	2.811	0.31 (0.07)	0.22	16.5	30.00
5	45.03	9.89	2.746	0.31 (0.07)	0.22	17.1	38.00
6	45.52	10.29	2.684	0.31 (0.07)	0.22	17.7	30.00
7	45.64	10.41	2.667	0.31 (0.07)	0.22	17.9	36.00
8	46.50	11.30	2.544	0.31 (0.07)	0.22	19.2	14.00
9	46.76	11.56	2.511	0.31 (0.07)	0.22	19.6	26.00
10	46.77	11.58	2.509	0.31 (0.07)	0.22	19.6	27.00
11	47.38	12.30	2.423	0.31 (0.07)	0.22	20.7	20.00
12	47.64	12.76	2.373	0.31 (0.07)	0.22	21.3	23.00
13	47.65	12.78	2.371	0.31 (0.07)	0.22	21.3	17.00
14	47.67	12.83	2.366	0.31 (0.07)	0.22	21.4	29.00
15	47.87	13.53	2.295	0.31 (0.07)	0.21	22.2	50.00
16	47.60	14.02	2.249	0.31 (0.07)	0.21	22.6	34.00
17	47.56	14.09	2.242	0.31 (0.07)	0.21	22.6	39.00
18	47.14	14.53	2.203	0.31 (0.07)	0.21	22.9	24.00
19	44.16	17.85	1.958	0.31 (0.06)	0.21	24.4	12.00
20	41.64	21.19	1.775	0.31 (0.06)	0.20	25.6	32.00
21	41.16	21.58	1.756	0.31 (0.06)	0.20	25.6	10.00

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE (CFS) = 47.87 Tc (MIN.) = 13.53
EFFECTIVE AREA (ACRES) = 22.24 AREA-AVERAGED Fm (INCH/HR) = 0.07

AREA-AVERAGED F_p (INCH/HR) = 0.31 AREA-AVERAGED A_p = 0.21
 TOTAL AREA (ACRES) = 25.6
 LONGEST FLOWPATH FROM NODE 10.00 TO NODE 47.00 = 2005.00 FEET.

 FLOW PROCESS FROM NODE 47.00 TO NODE 53.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
 >>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 149.80 DOWNSTREAM(FEET) = 149.70
 FLOW LENGTH(FEET) = 100.00 MANNING'S N = 0.013
 DEPTH OF FLOW IN 51.0 INCH PIPE IS 38.5 INCHES
 PIPE-FLOW VELOCITY(FEET/SEC.) = 4.16
 ESTIMATED PIPE DIAMETER(INCH) = 51.00 NUMBER OF PIPES = 1
 PIPE-FLOW (CFS) = 47.87
 PIPE TRAVEL TIME(MIN.) = 0.40 T_c (MIN.) = 13.93
 LONGEST FLOWPATH FROM NODE 10.00 TO NODE 53.00 = 2105.00 FEET.

 FLOW PROCESS FROM NODE 53.00 TO NODE 53.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

=====

TOTAL NUMBER OF STREAMS = 3
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
 TIME OF CONCENTRATION(MIN.) = 13.93
 RAINFALL INTENSITY(INCH/HR) = 2.26
 AREA-AVERAGED F_m (INCH/HR) = 0.07
 AREA-AVERAGED F_p (INCH/HR) = 0.31
 AREA-AVERAGED A_p = 0.21
 EFFECTIVE STREAM AREA (ACRES) = 22.24
 TOTAL STREAM AREA (ACRES) = 25.64
 PEAK FLOW RATE (CFS) AT CONFLUENCE = 47.87

 FLOW PROCESS FROM NODE 38.00 TO NODE 54.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
 >>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

=====

INITIAL SUBAREA FLOW-LENGTH(FEET) = 265.00
 ELEVATION DATA: UPSTREAM(FEET) = 166.40 DOWNSTREAM(FEET) = 165.80

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$
 SUBAREA ANALYSIS USED MINIMUM T_c (MIN.) = 11.340
 * 10 YEAR RAINFALL INTENSITY (INCH/HR) = 2.539

SUBAREA T_c AND LOSS RATE DATA (AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	F_p (INCH/HR)	A_p (DECIMAL)	SCS CN	T_c (MIN.)
CONDOMINIUMS	B	0.80	0.30	0.350	56	11.34

SUBAREA AVERAGE PERVIOUS LOSS RATE, F_p (INCH/HR) = 0.30

SUBAREA AVERAGE PERVIOUS AREA FRACTION, A_p = 0.350

SUBAREA RUNOFF (CFS) = 1.75

TOTAL AREA (ACRES) = 0.80 PEAK FLOW RATE (CFS) = 1.75

FLOW PROCESS FROM NODE 54.00 TO NODE 53.00 IS CODE = 41

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM (FEET) = 149.80 DOWNSTREAM (FEET) = 149.70
FLOW LENGTH (FEET) = 70.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 18.0 INCH PIPE IS 8.5 INCHES
PIPE-FLOW VELOCITY (FEET/SEC.) = 2.14
GIVEN PIPE DIAMETER (INCH) = 18.00 NUMBER OF PIPES = 1
PIPE-FLOW (CFS) = 1.75
PIPE TRAVEL TIME (MIN.) = 0.55 Tc (MIN.) = 11.89
LONGEST FLOWPATH FROM NODE 38.00 TO NODE 53.00 = 335.00 FEET.

FLOW PROCESS FROM NODE 53.00 TO NODE 53.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

=====

TOTAL NUMBER OF STREAMS = 3
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
TIME OF CONCENTRATION (MIN.) = 11.89
RAINFALL INTENSITY (INCH/HR) = 2.47
AREA-AVERAGED Fm (INCH/HR) = 0.11
AREA-AVERAGED Fp (INCH/HR) = 0.30
AREA-AVERAGED Ap = 0.35
EFFECTIVE STREAM AREA (ACRES) = 0.80
TOTAL STREAM AREA (ACRES) = 0.80
PEAK FLOW RATE (CFS) AT CONFLUENCE = 1.75

FLOW PROCESS FROM NODE 55.00 TO NODE 56.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

=====

INITIAL SUBAREA FLOW-LENGTH (FEET) = 130.00
ELEVATION DATA: UPSTREAM (FEET) = 166.50 DOWNSTREAM (FEET) = 166.30

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$

SUBAREA ANALYSIS USED MINIMUM Tc (MIN.) = 9.214

* 10 YEAR RAINFALL INTENSITY (INCH/HR) = 2.860

SUBAREA Tc AND LOSS RATE DATA (AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN	Tc (MIN.)
CONDOMINIUMS	B	0.19	0.30	0.350	56	9.21

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp (INCH/HR) = 0.30

SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.350

SUBAREA RUNOFF (CFS) = 0.47

TOTAL AREA (ACRES) = 0.19 PEAK FLOW RATE (CFS) = 0.47

FLOW PROCESS FROM NODE 56.00 TO NODE 53.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<

ELEVATION DATA: UPSTREAM(FEET) = 156.00 DOWNSTREAM(FEET) = 149.70
FLOW LENGTH(FEET) = 40.00 MANNING'S N = 0.013
ESTIMATED PIPE DIAMETER(INCH) INCREASED TO 12.000
DEPTH OF FLOW IN 12.0 INCH PIPE IS 1.5 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 8.16
ESTIMATED PIPE DIAMETER(INCH) = 12.000 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 0.47
PIPE TRAVEL TIME(MIN.) = 0.08 Tc(MIN.) = 9.30
LONGEST FLOWPATH FROM NODE 55.00 TO NODE 53.00 = 170.00 FEET.

FLOW PROCESS FROM NODE 53.00 TO NODE 53.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<

TOTAL NUMBER OF STREAMS = 3
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 3 ARE:
TIME OF CONCENTRATION(MIN.) = 9.30
RAINFALL INTENSITY(INCH/HR) = 2.85
AREA-AVERAGED Fm(INCH/HR) = 0.11
AREA-AVERAGED Fp(INCH/HR) = 0.30
AREA-AVERAGED Ap = 0.35
EFFECTIVE STREAM AREA(ACRES) = 0.19
TOTAL STREAM AREA(ACRES) = 0.19
PEAK FLOW RATE(CFS) AT CONFLUENCE = 0.47

** CONFLUENCE DATA **

Table with 8 columns: STREAM NUMBER, Q (CFS), Tc (MIN.), Intensity (INCH/HR), Fp(Fm) (INCH/HR), Ap, Ae (ACRES), HEADWATER NODE. It lists 18 rows of data for different stream numbers and their respective flow characteristics.

1	44.16	18.27	1.932	0.31 (0.06)	0.21	24.4	12.00
1	41.64	21.60	1.755	0.31 (0.06)	0.20	25.6	32.00
1	41.16	21.99	1.737	0.31 (0.06)	0.20	25.6	10.00
2	1.75	11.89	2.472	0.30 (0.11)	0.35	0.8	38.00
3	0.47	9.30	2.845	0.30 (0.11)	0.35	0.2	55.00

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
CONFLUENCE FORMULA USED FOR 3 STREAMS.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp (Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	44.67	8.86	2.925	0.31 (0.07)	0.23	15.5	51.00
2	45.53	9.30	2.845	0.31 (0.07)	0.23	16.3	55.00
3	45.63	9.35	2.836	0.31 (0.07)	0.23	16.4	43.00
4	45.93	9.52	2.807	0.31 (0.07)	0.23	16.7	45.00
5	46.59	9.90	2.744	0.31 (0.07)	0.23	17.3	30.00
6	47.12	10.30	2.684	0.31 (0.07)	0.23	18.0	38.00
7	47.63	10.69	2.626	0.31 (0.07)	0.23	18.6	30.00
8	47.76	10.81	2.610	0.31 (0.07)	0.23	18.8	36.00
9	48.65	11.70	2.494	0.31 (0.07)	0.23	20.2	14.00
10	48.84	11.89	2.472	0.31 (0.07)	0.22	20.5	38.00
11	48.91	11.96	2.462	0.31 (0.07)	0.22	20.6	26.00
12	48.92	11.98	2.461	0.31 (0.07)	0.22	20.6	27.00
13	49.45	12.70	2.379	0.31 (0.07)	0.22	21.7	20.00
14	49.67	13.16	2.331	0.31 (0.07)	0.22	22.3	23.00
15	49.68	13.18	2.329	0.31 (0.07)	0.22	22.3	17.00
16	49.69	13.23	2.324	0.31 (0.07)	0.22	22.4	29.00
17	49.83	13.93	2.257	0.31 (0.07)	0.22	23.2	50.00
18	49.52	14.42	2.213	0.31 (0.07)	0.22	23.6	34.00
19	49.47	14.49	2.207	0.31 (0.07)	0.22	23.6	39.00
20	49.03	14.93	2.169	0.31 (0.07)	0.22	23.9	24.00
21	45.82	18.27	1.932	0.31 (0.06)	0.21	25.4	12.00
22	43.14	21.60	1.755	0.31 (0.06)	0.21	26.6	32.00
23	42.65	21.99	1.737	0.31 (0.06)	0.21	26.6	10.00

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE (CFS) = 49.83 Tc (MIN.) = 13.93
EFFECTIVE AREA (ACRES) = 23.23 AREA-AVERAGED Fm (INCH/HR) = 0.07
AREA-AVERAGED Fp (INCH/HR) = 0.31 AREA-AVERAGED Ap = 0.22
TOTAL AREA (ACRES) = 26.6
LONGEST FLOWPATH FROM NODE 10.00 TO NODE 53.00 = 2105.00 FEET.

FLOW PROCESS FROM NODE 53.00 TO NODE 57.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<

=====

ELEVATION DATA: UPSTREAM (FEET) = 149.70 DOWNSTREAM (FEET) = 149.10
FLOW LENGTH (FEET) = 280.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 45.0 INCH PIPE IS 33.8 INCHES
PIPE-FLOW VELOCITY (FEET/SEC.) = 5.60
ESTIMATED PIPE DIAMETER (INCH) = 45.00 NUMBER OF PIPES = 1

PIPE-FLOW (CFS) = 49.83
PIPE TRAVEL TIME (MIN.) = 0.83 Tc (MIN.) = 14.77
LONGEST FLOWPATH FROM NODE 10.00 TO NODE 57.00 = 2385.00 FEET.

FLOW PROCESS FROM NODE 57.00 TO NODE 57.00 IS CODE = 10

>>>>MAIN-STREAM MEMORY COPIED ONTO MEMORY BANK # 1 <<<<<
=====

FLOW PROCESS FROM NODE 58.00 TO NODE 57.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<
=====

INITIAL SUBAREA FLOW-LENGTH (FEET) = 240.00
ELEVATION DATA: UPSTREAM (FEET) = 166.50 DOWNSTREAM (FEET) = 165.60

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$
SUBAREA ANALYSIS USED MINIMUM Tc (MIN.) = 9.853
* 10 YEAR RAINFALL INTENSITY (INCH/HR) = 2.752

SUBAREA Tc AND LOSS RATE DATA (AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN	Tc (MIN.)
CONDOMINIUMS	B	0.46	0.30	0.350	56	9.85

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp (INCH/HR) = 0.30

SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.350

SUBAREA RUNOFF (CFS) = 1.10

TOTAL AREA (ACRES) = 0.46 PEAK FLOW RATE (CFS) = 1.10

FLOW PROCESS FROM NODE 57.00 TO NODE 57.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
=====

TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION (MIN.) = 9.85
RAINFALL INTENSITY (INCH/HR) = 2.75
AREA-AVERAGED Fm (INCH/HR) = 0.11
AREA-AVERAGED Fp (INCH/HR) = 0.30
AREA-AVERAGED Ap = 0.35
EFFECTIVE STREAM AREA (ACRES) = 0.46
TOTAL STREAM AREA (ACRES) = 0.46
PEAK FLOW RATE (CFS) AT CONFLUENCE = 1.10

FLOW PROCESS FROM NODE 59.00 TO NODE 57.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<
=====

INITIAL SUBAREA FLOW-LENGTH (FEET) = 265.00

ELEVATION DATA: UPSTREAM(FEET) = 166.80 DOWNSTREAM(FEET) = 165.80

Tc = K*[(LENGTH** 3.00)/(ELEVATION CHANGE)]**0.20

SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) = 10.239

* 10 YEAR RAINFALL INTENSITY(INCH/HR) = 2.692

SUBAREA Tc AND LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN	Tc (MIN.)
CONDOMINIUMS	B	0.57	0.30	0.350	56	10.24

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.30

SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.350

SUBAREA RUNOFF(CFS) = 1.33

TOTAL AREA(ACRES) = 0.57 PEAK FLOW RATE(CFS) = 1.33

FLOW PROCESS FROM NODE 57.00 TO NODE 57.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<

TOTAL NUMBER OF STREAMS = 2

CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:

TIME OF CONCENTRATION(MIN.) = 10.24

RAINFALL INTENSITY(INCH/HR) = 2.69

AREA-AVERAGED Fm(INCH/HR) = 0.11

AREA-AVERAGED Fp(INCH/HR) = 0.30

AREA-AVERAGED Ap = 0.35

EFFECTIVE STREAM AREA(ACRES) = 0.57

TOTAL STREAM AREA(ACRES) = 0.57

PEAK FLOW RATE(CFS) AT CONFLUENCE = 1.33

** CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	1.10	9.85	2.752	0.30(0.11)	0.35	0.5	58.00
2	1.33	10.24	2.692	0.30(0.11)	0.35	0.6	59.00

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO

CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	2.40	9.85	2.752	0.30(0.11)	0.35	1.0	58.00
2	2.40	10.24	2.692	0.30(0.11)	0.35	1.0	59.00

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE(CFS) = 2.40 Tc(MIN.) = 9.85

EFFECTIVE AREA(ACRES) = 1.01 AREA-AVERAGED Fm(INCH/HR) = 0.11

AREA-AVERAGED Fp(INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.35

TOTAL AREA(ACRES) = 1.0

LONGEST FLOWPATH FROM NODE 59.00 TO NODE 57.00 = 265.00 FEET.

FLOW PROCESS FROM NODE 57.00 TO NODE 57.00 IS CODE = 11

=====
 >>>>>CONFLUENCE MEMORY BANK # 1 WITH THE MAIN-STREAM MEMORY<<<<<<
 =====

** MAIN STREAM CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp (Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	2.40	9.85	2.752	0.30 (0.11)	0.35	1.0	58.00
2	2.40	10.24	2.692	0.30 (0.11)	0.35	1.0	59.00
LONGEST FLOWPATH FROM NODE			59.00	TO NODE	57.00 =	265.00 FEET.	

** MEMORY BANK # 1 CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp (Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	44.67	9.73	2.773	0.31 (0.07)	0.23	15.5	51.00
2	45.53	10.14	2.707	0.31 (0.07)	0.23	16.3	55.00
3	45.63	10.19	2.699	0.31 (0.07)	0.23	16.4	43.00
4	45.93	10.36	2.674	0.31 (0.07)	0.23	16.7	45.00
5	46.59	10.74	2.619	0.31 (0.07)	0.23	17.3	30.00
6	47.12	11.13	2.566	0.31 (0.07)	0.23	18.0	38.00
7	47.63	11.53	2.515	0.31 (0.07)	0.23	18.6	30.00
8	47.76	11.65	2.500	0.31 (0.07)	0.23	18.8	36.00
9	48.65	12.54	2.397	0.31 (0.07)	0.23	20.2	14.00
10	48.84	12.72	2.377	0.31 (0.07)	0.22	20.5	38.00
11	48.91	12.80	2.369	0.31 (0.07)	0.22	20.6	26.00
12	48.92	12.81	2.368	0.31 (0.07)	0.22	20.6	27.00
13	49.45	13.54	2.294	0.31 (0.07)	0.22	21.7	20.00
14	49.67	14.00	2.251	0.31 (0.07)	0.22	22.3	23.00
15	49.68	14.02	2.249	0.31 (0.07)	0.22	22.3	17.00
16	49.69	14.06	2.244	0.31 (0.07)	0.22	22.4	29.00
17	49.83	14.77	2.183	0.31 (0.07)	0.22	23.2	50.00
18	49.52	15.25	2.142	0.31 (0.07)	0.22	23.6	34.00
19	49.47	15.32	2.137	0.31 (0.07)	0.22	23.6	39.00
20	49.03	15.76	2.103	0.31 (0.07)	0.22	23.9	24.00
21	45.82	19.11	1.883	0.31 (0.06)	0.21	25.4	12.00
22	43.14	22.47	1.716	0.31 (0.06)	0.21	26.6	32.00
23	42.65	22.86	1.699	0.31 (0.06)	0.21	26.6	10.00
LONGEST FLOWPATH FROM NODE			10.00	TO NODE	57.00 =	2385.00 FEET.	

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp (Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	47.06	9.73	2.773	0.30 (0.07)	0.24	16.5	51.00
2	47.34	9.85	2.752	0.30 (0.07)	0.24	16.7	58.00
3	47.93	10.14	2.707	0.30 (0.07)	0.24	17.3	55.00
4	48.03	10.19	2.699	0.30 (0.07)	0.24	17.4	43.00
5	48.11	10.24	2.692	0.30 (0.07)	0.24	17.5	59.00
6	48.32	10.36	2.674	0.30 (0.07)	0.24	17.7	45.00
7	48.92	10.74	2.619	0.31 (0.07)	0.23	18.4	30.00
8	49.40	11.13	2.566	0.31 (0.07)	0.23	19.0	38.00
9	49.87	11.53	2.515	0.31 (0.07)	0.23	19.7	30.00
10	49.98	11.65	2.500	0.31 (0.07)	0.23	19.9	36.00
11	50.78	12.54	2.397	0.31 (0.07)	0.23	21.2	14.00

12	50.95	12.72	2.377	0.31	(0.07)	0.23	21.5	38.00
13	51.01	12.80	2.369	0.31	(0.07)	0.23	21.6	26.00
14	51.02	12.81	2.368	0.31	(0.07)	0.23	21.6	27.00
15	51.48	13.54	2.294	0.31	(0.07)	0.23	22.7	20.00
16	51.66	14.00	2.251	0.31	(0.07)	0.23	23.3	23.00
17	51.67	14.02	2.249	0.31	(0.07)	0.23	23.4	17.00
18	51.68	14.06	2.244	0.31	(0.07)	0.23	23.4	29.00
19	51.76	14.77	2.183	0.31	(0.07)	0.23	24.3	50.00
20	51.41	15.25	2.142	0.31	(0.07)	0.22	24.6	34.00
21	51.36	15.32	2.137	0.31	(0.07)	0.22	24.7	39.00
22	50.88	15.76	2.103	0.31	(0.07)	0.22	24.9	24.00
23	47.47	19.11	1.883	0.30	(0.07)	0.22	26.4	12.00
24	44.64	22.47	1.716	0.30	(0.06)	0.21	27.7	32.00
25	44.12	22.86	1.699	0.30	(0.06)	0.21	27.7	10.00
TOTAL AREA (ACRES) =			27.7					

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE (CFS) = 51.76 Tc (MIN.) = 14.767
EFFECTIVE AREA (ACRES) = 24.26 AREA-AVERAGED Fm (INCH/HR) = 0.07
AREA-AVERAGED Fp (INCH/HR) = 0.31 AREA-AVERAGED Ap = 0.23
TOTAL AREA (ACRES) = 27.7
LONGEST FLOWPATH FROM NODE 10.00 TO NODE 57.00 = 2385.00 FEET.

FLOW PROCESS FROM NODE 57.00 TO NODE 57.00 IS CODE = 12

>>>>CLEAR MEMORY BANK # 1 <<<<<

FLOW PROCESS FROM NODE 57.00 TO NODE 57.00 IS CODE = 10

>>>>MAIN-STREAM MEMORY COPIED ONTO MEMORY BANK # 1 <<<<<

FLOW PROCESS FROM NODE 60.00 TO NODE 61.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

INITIAL SUBAREA FLOW-LENGTH (FEET) = 200.00
ELEVATION DATA: UPSTREAM (FEET) = 167.00 DOWNSTREAM (FEET) = 166.20

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$

SUBAREA ANALYSIS USED MINIMUM Tc (MIN.) = 9.043

* 10 YEAR RAINFALL INTENSITY (INCH/HR) = 2.891

SUBAREA Tc AND LOSS RATE DATA (AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN	Tc (MIN.)
CONDOMINIUMS	B	0.27	0.30	0.350	56	9.04

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp (INCH/HR) = 0.30

SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.350

SUBAREA RUNOFF (CFS) = 0.68

TOTAL AREA (ACRES) = 0.27 PEAK FLOW RATE (CFS) = 0.68

FLOW PROCESS FROM NODE 61.00 TO NODE 62.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<

=====

ELEVATION DATA: UPSTREAM (FEET) = 166.20 DOWNSTREAM (FEET) = 165.30
FLOW LENGTH (FEET) = 210.00 MANNING'S N = 0.013
ESTIMATED PIPE DIAMETER (INCH) INCREASED TO 12.000
DEPTH OF FLOW IN 12.0 INCH PIPE IS 4.5 INCHES
PIPE-FLOW VELOCITY (FEET/SEC.) = 2.52
ESTIMATED PIPE DIAMETER (INCH) = 12.000 NUMBER OF PIPES = 1
PIPE-FLOW (CFS) = 0.68
PIPE TRAVEL TIME (MIN.) = 1.39 Tc (MIN.) = 10.43
LONGEST FLOWPATH FROM NODE 60.00 TO NODE 62.00 = 410.00 FEET.

FLOW PROCESS FROM NODE 62.00 TO NODE 62.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

=====

TOTAL NUMBER OF STREAMS = 3
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION (MIN.) = 10.43
RAINFALL INTENSITY (INCH/HR) = 2.66
AREA-AVERAGED Fm (INCH/HR) = 0.10
AREA-AVERAGED Fp (INCH/HR) = 0.30
AREA-AVERAGED Ap = 0.35
EFFECTIVE STREAM AREA (ACRES) = 0.27
TOTAL STREAM AREA (ACRES) = 0.27
PEAK FLOW RATE (CFS) AT CONFLUENCE = 0.68

FLOW PROCESS FROM NODE 63.00 TO NODE 62.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

=====

INITIAL SUBAREA FLOW-LENGTH (FEET) = 270.00
ELEVATION DATA: UPSTREAM (FEET) = 166.50 DOWNSTREAM (FEET) = 165.30

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$

SUBAREA ANALYSIS USED MINIMUM Tc (MIN.) = 9.984

* 10 YEAR RAINFALL INTENSITY (INCH/HR) = 2.731

SUBAREA Tc AND LOSS RATE DATA (AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN	Tc (MIN.)
CONDOMINIUMS	B	0.70	0.30	0.350	56	9.98

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp (INCH/HR) = 0.30

SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.350

SUBAREA RUNOFF (CFS) = 1.65

TOTAL AREA (ACRES) = 0.70 PEAK FLOW RATE (CFS) = 1.65

FLOW PROCESS FROM NODE 62.00 TO NODE 62.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

=====

TOTAL NUMBER OF STREAMS = 3
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
TIME OF CONCENTRATION(MIN.) = 9.98
RAINFALL INTENSITY(INCH/HR) = 2.73
AREA-AVERAGED Fm(INCH/HR) = 0.11
AREA-AVERAGED Fp(INCH/HR) = 0.30
AREA-AVERAGED Ap = 0.35
EFFECTIVE STREAM AREA(ACRES) = 0.70
TOTAL STREAM AREA(ACRES) = 0.70
PEAK FLOW RATE(CFS) AT CONFLUENCE = 1.65

FLOW PROCESS FROM NODE 64.00 TO NODE 62.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

=====

INITIAL SUBAREA FLOW-LENGTH(FEET) = 295.00
ELEVATION DATA: UPSTREAM(FEET) = 166.50 DOWNSTREAM(FEET) = 165.30

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$
SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) = 20.647
* 10 YEAR RAINFALL INTENSITY(INCH/HR) = 1.801

SUBAREA Tc AND LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN	Tc (MIN.)
AGRICULTURAL FAIR COVER "PASTURE, IRRIGATED"	B	0.27	0.30	1.000	65	20.65

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.30
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 1.000
SUBAREA RUNOFF(CFS) = 0.36
TOTAL AREA(ACRES) = 0.27 PEAK FLOW RATE(CFS) = 0.36

FLOW PROCESS FROM NODE 62.00 TO NODE 62.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<

=====

TOTAL NUMBER OF STREAMS = 3
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 3 ARE:
TIME OF CONCENTRATION(MIN.) = 20.65
RAINFALL INTENSITY(INCH/HR) = 1.80
AREA-AVERAGED Fm(INCH/HR) = 0.30
AREA-AVERAGED Fp(INCH/HR) = 0.30
AREA-AVERAGED Ap = 1.00
EFFECTIVE STREAM AREA(ACRES) = 0.27
TOTAL STREAM AREA(ACRES) = 0.27

PEAK FLOW RATE (CFS) AT CONFLUENCE = 0.36

** CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp (Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	0.68	10.43	2.663	0.30 (0.10)	0.35	0.3	60.00
2	1.65	9.98	2.731	0.30 (0.11)	0.35	0.7	63.00
3	0.36	20.65	1.801	0.30 (0.30)	1.00	0.3	64.00

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
CONFLUENCE FORMULA USED FOR 3 STREAMS.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp (Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	2.61	9.98	2.731	0.30 (0.13)	0.43	1.1	63.00
2	2.58	10.43	2.663	0.30 (0.13)	0.43	1.1	60.00
3	1.88	20.65	1.801	0.30 (0.15)	0.49	1.2	64.00

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE (CFS) = 2.61 Tc (MIN.) = 9.98
EFFECTIVE AREA (ACRES) = 1.09 AREA-AVERAGED Fm (INCH/HR) = 0.13
AREA-AVERAGED Fp (INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.43
TOTAL AREA (ACRES) = 1.2
LONGEST FLOWPATH FROM NODE 60.00 TO NODE 62.00 = 410.00 FEET.

FLOW PROCESS FROM NODE 62.00 TO NODE 57.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<

=====

ELEVATION DATA: UPSTREAM (FEET) = 156.00 DOWNSTREAM (FEET) = 149.10
FLOW LENGTH (FEET) = 245.00 MANNING'S N = 0.013
ESTIMATED PIPE DIAMETER (INCH) INCREASED TO 12.000
DEPTH OF FLOW IN 12.0 INCH PIPE IS 5.6 INCHES
PIPE-FLOW VELOCITY (FEET/SEC.) = 7.21
ESTIMATED PIPE DIAMETER (INCH) = 12.00 NUMBER OF PIPES = 1
PIPE-FLOW (CFS) = 2.61
PIPE TRAVEL TIME (MIN.) = 0.57 Tc (MIN.) = 10.55
LONGEST FLOWPATH FROM NODE 60.00 TO NODE 57.00 = 655.00 FEET.

FLOW PROCESS FROM NODE 57.00 TO NODE 57.00 IS CODE = 11

>>>>CONFLUENCE MEMORY BANK # 1 WITH THE MAIN-STREAM MEMORY<<<<<

** MAIN STREAM CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp (Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	2.61	10.55	2.646	0.30 (0.13)	0.43	1.1	63.00
2	2.58	11.00	2.584	0.30 (0.13)	0.43	1.1	60.00
3	1.88	21.26	1.771	0.30 (0.15)	0.49	1.2	64.00

LONGEST FLOWPATH FROM NODE 60.00 TO NODE 57.00 = 655.00 FEET.

** MEMORY BANK # 1 CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp (Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	47.06	9.73	2.773	0.30 (0.07)	0.24	16.5	51.00
2	47.34	9.85	2.752	0.30 (0.07)	0.24	16.7	58.00
3	47.93	10.14	2.707	0.30 (0.07)	0.24	17.3	55.00
4	48.03	10.19	2.699	0.30 (0.07)	0.24	17.4	43.00
5	48.11	10.24	2.692	0.30 (0.07)	0.24	17.5	59.00
6	48.32	10.36	2.674	0.30 (0.07)	0.24	17.7	45.00
7	48.92	10.74	2.619	0.31 (0.07)	0.23	18.4	30.00
8	49.40	11.13	2.566	0.31 (0.07)	0.23	19.0	38.00
9	49.87	11.53	2.515	0.31 (0.07)	0.23	19.7	30.00
10	49.98	11.65	2.500	0.31 (0.07)	0.23	19.9	36.00
11	50.78	12.54	2.397	0.31 (0.07)	0.23	21.2	14.00
12	50.95	12.72	2.377	0.31 (0.07)	0.23	21.5	38.00
13	51.01	12.80	2.369	0.31 (0.07)	0.23	21.6	26.00
14	51.02	12.81	2.368	0.31 (0.07)	0.23	21.6	27.00
15	51.48	13.54	2.294	0.31 (0.07)	0.23	22.7	20.00
16	51.66	14.00	2.251	0.31 (0.07)	0.23	23.3	23.00
17	51.67	14.02	2.249	0.31 (0.07)	0.23	23.4	17.00
18	51.68	14.06	2.244	0.31 (0.07)	0.23	23.4	29.00
19	51.76	14.77	2.183	0.31 (0.07)	0.23	24.3	50.00
20	51.41	15.25	2.142	0.31 (0.07)	0.22	24.6	34.00
21	51.36	15.32	2.137	0.31 (0.07)	0.22	24.7	39.00
22	50.88	15.76	2.103	0.31 (0.07)	0.22	24.9	24.00
23	47.47	19.11	1.883	0.30 (0.07)	0.22	26.4	12.00
24	44.64	22.47	1.716	0.30 (0.06)	0.21	27.7	32.00
25	44.12	22.86	1.699	0.30 (0.06)	0.21	27.7	10.00

LONGEST FLOWPATH FROM NODE 10.00 TO NODE 57.00 = 2385.00 FEET.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp (Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	49.59	9.73	2.773	0.30 (0.08)	0.25	17.5	51.00
2	49.88	9.85	2.752	0.30 (0.08)	0.25	17.8	58.00
3	50.49	10.14	2.707	0.30 (0.08)	0.25	18.3	55.00
4	50.60	10.19	2.699	0.30 (0.08)	0.25	18.4	43.00
5	50.69	10.24	2.692	0.30 (0.08)	0.25	18.5	59.00
6	50.90	10.36	2.674	0.30 (0.08)	0.25	18.8	45.00
7	51.22	10.55	2.646	0.30 (0.07)	0.25	19.1	63.00
8	51.51	10.74	2.619	0.30 (0.07)	0.25	19.5	30.00
9	51.82	11.00	2.584	0.30 (0.07)	0.25	19.9	60.00
10	51.97	11.13	2.566	0.30 (0.07)	0.24	20.1	38.00
11	52.41	11.53	2.515	0.30 (0.07)	0.24	20.8	30.00
12	52.52	11.65	2.500	0.30 (0.07)	0.24	21.0	36.00
13	53.25	12.54	2.397	0.30 (0.07)	0.24	22.3	14.00
14	53.41	12.72	2.377	0.30 (0.07)	0.24	22.6	38.00
15	53.47	12.80	2.369	0.30 (0.07)	0.24	22.8	26.00
16	53.47	12.81	2.368	0.30 (0.07)	0.24	22.8	27.00
17	53.89	13.54	2.294	0.30 (0.07)	0.24	23.9	20.00
18	54.04	14.00	2.251	0.30 (0.07)	0.24	24.5	23.00
19	54.04	14.02	2.249	0.30 (0.07)	0.24	24.5	17.00

20	54.05	14.06	2.244	0.30	(0.07)	0.24	24.6	29.00
21	54.08	14.77	2.183	0.30	(0.07)	0.24	25.4	50.00
22	53.70	15.25	2.142	0.30	(0.07)	0.24	25.8	34.00
23	53.64	15.32	2.137	0.30	(0.07)	0.23	25.8	39.00
24	53.14	15.76	2.103	0.30	(0.07)	0.23	26.0	24.00
25	49.50	19.11	1.883	0.30	(0.07)	0.23	27.6	12.00
26	47.54	21.26	1.771	0.30	(0.07)	0.23	28.4	64.00
27	46.46	22.47	1.716	0.30	(0.07)	0.22	28.9	32.00
28	45.92	22.86	1.699	0.30	(0.07)	0.22	28.9	10.00
TOTAL AREA (ACRES) =			28.9					

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE (CFS) = 54.08 Tc (MIN.) = 14.767
EFFECTIVE AREA (ACRES) = 25.42 AREA-AVERAGED Fm (INCH/HR) = 0.07
AREA-AVERAGED Fp (INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.24
TOTAL AREA (ACRES) = 28.9
LONGEST FLOWPATH FROM NODE 10.00 TO NODE 57.00 = 2385.00 FEET.

FLOW PROCESS FROM NODE 57.00 TO NODE 57.00 IS CODE = 12

>>>>CLEAR MEMORY BANK # 1 <<<<<

FLOW PROCESS FROM NODE 57.00 TO NODE 57.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

TOTAL NUMBER OF STREAMS = 2

CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:

TIME OF CONCENTRATION (MIN.) = 14.77

RAINFALL INTENSITY (INCH/HR) = 2.18

AREA-AVERAGED Fm (INCH/HR) = 0.07

AREA-AVERAGED Fp (INCH/HR) = 0.30

AREA-AVERAGED Ap = 0.24

EFFECTIVE STREAM AREA (ACRES) = 25.42

TOTAL STREAM AREA (ACRES) = 28.90

PEAK FLOW RATE (CFS) AT CONFLUENCE = 54.08

FLOW PROCESS FROM NODE 65.00 TO NODE 66.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<

>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

INITIAL SUBAREA FLOW-LENGTH (FEET) = 135.00

ELEVATION DATA: UPSTREAM (FEET) = 166.70 DOWNSTREAM (FEET) = 166.30

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$

SUBAREA ANALYSIS USED MINIMUM Tc (MIN.) = 8.205

* 10 YEAR RAINFALL INTENSITY (INCH/HR) = 3.056

SUBAREA Tc AND LOSS RATE DATA (AMC II):

DEVELOPMENT TYPE/	SCS SOIL	AREA	Fp	Ap	SCS	Tc
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LAND USE GROUP (ACRES) (INCH/HR) (DECIMAL) CN (MIN.)
 CONDOMINIUMS B 0.19 0.30 0.350 56 8.21
 SUBAREA AVERAGE PERVIOUS LOSS RATE, F_p (INCH/HR) = 0.30
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, A_p = 0.350
 SUBAREA RUNOFF (CFS) = 0.50
 TOTAL AREA (ACRES) = 0.19 PEAK FLOW RATE (CFS) = 0.50

FLOW PROCESS FROM NODE 66.00 TO NODE 57.00 IS CODE = 31

>>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
 >>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<

=====
 ELEVATION DATA: UPSTREAM (FEET) = 156.00 DOWNSTREAM (FEET) = 149.10
 FLOW LENGTH (FEET) = 45.00 MANNING'S N = 0.013
 ESTIMATED PIPE DIAMETER (INCH) INCREASED TO 12.000
 DEPTH OF FLOW IN 12.0 INCH PIPE IS 1.6 INCHES
 PIPE-FLOW VELOCITY (FEET/SEC.) = 8.25
 ESTIMATED PIPE DIAMETER (INCH) = 12.00 NUMBER OF PIPES = 1
 PIPE-FLOW (CFS) = 0.50
 PIPE TRAVEL TIME (MIN.) = 0.09 T_c (MIN.) = 8.30
 LONGEST FLOWPATH FROM NODE 65.00 TO NODE 57.00 = 180.00 FEET.

FLOW PROCESS FROM NODE 57.00 TO NODE 57.00 IS CODE = 1

>>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
 >>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<

=====
 TOTAL NUMBER OF STREAMS = 2
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
 TIME OF CONCENTRATION (MIN.) = 8.30
 RAINFALL INTENSITY (INCH/HR) = 3.04
 AREA-AVERAGED F_m (INCH/HR) = 0.11
 AREA-AVERAGED F_p (INCH/HR) = 0.30
 AREA-AVERAGED A_p = 0.35
 EFFECTIVE STREAM AREA (ACRES) = 0.19
 TOTAL STREAM AREA (ACRES) = 0.19
 PEAK FLOW RATE (CFS) AT CONFLUENCE = 0.50

** CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	T_c (MIN.)	Intensity (INCH/HR)	F_p (F_m) (INCH/HR)	A_p	A_e (ACRES)	HEADWATER NODE
1	49.59	9.73	2.773	0.30 (0.08)	0.25	17.5	51.00
1	49.88	9.85	2.752	0.30 (0.08)	0.25	17.8	58.00
1	50.49	10.14	2.707	0.30 (0.08)	0.25	18.3	55.00
1	50.60	10.19	2.699	0.30 (0.08)	0.25	18.4	43.00
1	50.69	10.24	2.692	0.30 (0.08)	0.25	18.5	59.00
1	50.90	10.36	2.674	0.30 (0.08)	0.25	18.8	45.00
1	51.22	10.55	2.646	0.30 (0.07)	0.25	19.1	63.00
1	51.51	10.74	2.619	0.30 (0.07)	0.25	19.5	30.00
1	51.82	11.00	2.584	0.30 (0.07)	0.25	19.9	60.00
1	51.97	11.13	2.566	0.30 (0.07)	0.24	20.1	38.00
1	52.41	11.53	2.515	0.30 (0.07)	0.24	20.8	30.00

1	52.52	11.65	2.500	0.30 (0.07)	0.24	21.0	36.00
1	53.25	12.54	2.397	0.30 (0.07)	0.24	22.3	14.00
1	53.41	12.72	2.377	0.30 (0.07)	0.24	22.6	38.00
1	53.47	12.80	2.369	0.30 (0.07)	0.24	22.8	26.00
1	53.47	12.81	2.368	0.30 (0.07)	0.24	22.8	27.00
1	53.89	13.54	2.294	0.30 (0.07)	0.24	23.9	20.00
1	54.04	14.00	2.251	0.30 (0.07)	0.24	24.5	23.00
1	54.04	14.02	2.249	0.30 (0.07)	0.24	24.5	17.00
1	54.05	14.06	2.244	0.30 (0.07)	0.24	24.6	29.00
1	54.08	14.77	2.183	0.30 (0.07)	0.24	25.4	50.00
1	53.70	15.25	2.142	0.30 (0.07)	0.24	25.8	34.00
1	53.64	15.32	2.137	0.30 (0.07)	0.23	25.8	39.00
1	53.14	15.76	2.103	0.30 (0.07)	0.23	26.0	24.00
1	49.50	19.11	1.883	0.30 (0.07)	0.23	27.6	12.00
1	47.54	21.26	1.771	0.30 (0.07)	0.23	28.4	64.00
1	46.46	22.47	1.716	0.30 (0.07)	0.22	28.9	32.00
1	45.92	22.86	1.699	0.30 (0.07)	0.22	28.9	10.00
2	0.50	8.30	3.037	0.30 (0.11)	0.35	0.2	65.00

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	46.95	8.30	3.037	0.30 (0.08)	0.25	15.1	65.00
2	50.05	9.73	2.773	0.30 (0.08)	0.25	17.7	51.00
3	50.33	9.85	2.752	0.30 (0.08)	0.25	18.0	58.00
4	50.94	10.14	2.707	0.30 (0.08)	0.25	18.5	55.00
5	51.04	10.19	2.699	0.30 (0.08)	0.25	18.6	43.00
6	51.13	10.24	2.692	0.30 (0.08)	0.25	18.7	59.00
7	51.35	10.36	2.674	0.30 (0.08)	0.25	19.0	45.00
8	51.66	10.55	2.646	0.30 (0.08)	0.25	19.3	63.00
9	51.94	10.74	2.619	0.30 (0.08)	0.25	19.7	30.00
10	52.24	11.00	2.584	0.30 (0.07)	0.25	20.1	60.00
11	52.40	11.13	2.566	0.30 (0.07)	0.25	20.3	38.00
12	52.82	11.53	2.515	0.30 (0.07)	0.25	21.0	30.00
13	52.93	11.65	2.500	0.30 (0.07)	0.24	21.2	36.00
14	53.65	12.54	2.397	0.30 (0.07)	0.24	22.5	14.00
15	53.80	12.72	2.377	0.30 (0.07)	0.24	22.8	38.00
16	53.86	12.80	2.369	0.30 (0.07)	0.24	22.9	26.00
17	53.86	12.81	2.368	0.30 (0.07)	0.24	23.0	27.00
18	54.27	13.54	2.294	0.30 (0.07)	0.24	24.0	20.00
19	54.41	14.00	2.251	0.30 (0.07)	0.24	24.7	23.00
20	54.41	14.02	2.249	0.30 (0.07)	0.24	24.7	17.00
21	54.42	14.06	2.244	0.30 (0.07)	0.24	24.8	29.00
22	54.44	14.77	2.183	0.30 (0.07)	0.24	25.6	50.00
23	54.05	15.25	2.142	0.30 (0.07)	0.24	26.0	34.00
24	53.99	15.32	2.137	0.30 (0.07)	0.24	26.0	39.00
25	53.48	15.76	2.103	0.30 (0.07)	0.24	26.2	24.00
26	49.81	19.11	1.883	0.30 (0.07)	0.23	27.8	12.00
27	47.83	21.26	1.771	0.30 (0.07)	0.23	28.6	64.00
28	46.73	22.47	1.716	0.30 (0.07)	0.22	29.1	32.00
29	46.20	22.86	1.699	0.30 (0.07)	0.22	29.1	10.00

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE (CFS) = 54.44 Tc (MIN.) = 14.77
EFFECTIVE AREA (ACRES) = 25.61 AREA-AVERAGED Fm (INCH/HR) = 0.07
AREA-AVERAGED Fp (INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.24
TOTAL AREA (ACRES) = 29.1
LONGEST FLOWPATH FROM NODE 10.00 TO NODE 57.00 = 2385.00 FEET.

FLOW PROCESS FROM NODE 57.00 TO NODE 67.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<

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ELEVATION DATA: UPSTREAM (FEET) = 149.10 DOWNSTREAM (FEET) = 148.50
FLOW LENGTH (FEET) = 275.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 45.0 INCH PIPE IS 36.4 INCHES
PIPE-FLOW VELOCITY (FEET/SEC.) = 5.68
ESTIMATED PIPE DIAMETER (INCH) = 45.00 NUMBER OF PIPES = 1
PIPE-FLOW (CFS) = 54.44
PIPE TRAVEL TIME (MIN.) = 0.81 Tc (MIN.) = 15.57
LONGEST FLOWPATH FROM NODE 10.00 TO NODE 67.00 = 2660.00 FEET.

FLOW PROCESS FROM NODE 67.00 TO NODE 67.00 IS CODE = 10

>>>>MAIN-STREAM MEMORY COPIED ONTO MEMORY BANK # 1 <<<<<

FLOW PROCESS FROM NODE 68.00 TO NODE 69.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

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INITIAL SUBAREA FLOW-LENGTH (FEET) = 365.00
ELEVATION DATA: UPSTREAM (FEET) = 168.20 DOWNSTREAM (FEET) = 167.30

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$

SUBAREA ANALYSIS USED MINIMUM Tc (MIN.) = 11.404

* 10 YEAR RAINFALL INTENSITY (INCH/HR) = 2.531

SUBAREA Tc AND LOSS RATE DATA (AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN	Tc (MIN.)
APARTMENTS	B	1.00	0.30	0.200	56	11.40

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp (INCH/HR) = 0.30

SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.200

SUBAREA RUNOFF (CFS) = 2.22

TOTAL AREA (ACRES) = 1.00 PEAK FLOW RATE (CFS) = 2.22

FLOW PROCESS FROM NODE 69.00 TO NODE 70.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<

>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<

ELEVATION DATA: UPSTREAM(FEET) = 162.00 DOWNSTREAM(FEET) = 159.00
FLOW LENGTH(FEET) = 185.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 12.0 INCH PIPE IS 6.0 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 5.64
ESTIMATED PIPE DIAMETER(INCH) = 12.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 2.22
PIPE TRAVEL TIME(MIN.) = 0.55 Tc(MIN.) = 11.95
LONGEST FLOWPATH FROM NODE 68.00 TO NODE 70.00 = 550.00 FEET.

FLOW PROCESS FROM NODE 70.00 TO NODE 71.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<

TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION(MIN.) = 11.95
RAINFALL INTENSITY(INCH/HR) = 2.46
AREA-AVERAGED Fm(INCH/HR) = 0.06
AREA-AVERAGED Fp(INCH/HR) = 0.30
AREA-AVERAGED Ap = 0.20
EFFECTIVE STREAM AREA(ACRES) = 1.00
TOTAL STREAM AREA(ACRES) = 1.00
PEAK FLOW RATE(CFS) AT CONFLUENCE = 2.22

FLOW PROCESS FROM NODE 71.00 TO NODE 70.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<

>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

INITIAL SUBAREA FLOW-LENGTH(FEET) = 250.00
ELEVATION DATA: UPSTREAM(FEET) = 167.20 DOWNSTREAM(FEET) = 166.80

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$

SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) = 10.688

* 10 YEAR RAINFALL INTENSITY(INCH/HR) = 2.627

SUBAREA Tc AND LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN	Tc (MIN.)
APARTMENTS	B	0.66	0.30	0.200	56	10.69

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.30

SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.200

SUBAREA RUNOFF(CFS) = 1.52

TOTAL AREA(ACRES) = 0.66 PEAK FLOW RATE(CFS) = 1.52

FLOW PROCESS FROM NODE 70.00 TO NODE 70.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<

>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<

TOTAL NUMBER OF STREAMS = 2
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
 TIME OF CONCENTRATION(MIN.) = 10.69
 RAINFALL INTENSITY(INCH/HR) = 2.63
 AREA-AVERAGED Fm(INCH/HR) = 0.06
 AREA-AVERAGED Fp(INCH/HR) = 0.30
 AREA-AVERAGED Ap = 0.20
 EFFECTIVE STREAM AREA(ACRES) = 0.66
 TOTAL STREAM AREA(ACRES) = 0.66
 PEAK FLOW RATE(CFS) AT CONFLUENCE = 1.52

** CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	2.22	11.95	2.464	0.30(0.06)	0.20	1.0	68.00
2	1.52	10.69	2.627	0.30(0.06)	0.20	0.7	71.00

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
 CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	3.65	10.69	2.627	0.30(0.06)	0.20	1.6	71.00
2	3.65	11.95	2.464	0.30(0.06)	0.20	1.7	68.00

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE(CFS) = 3.65 Tc(MIN.) = 11.95
 EFFECTIVE AREA(ACRES) = 1.66 AREA-AVERAGED Fm(INCH/HR) = 0.06
 AREA-AVERAGED Fp(INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.20
 TOTAL AREA(ACRES) = 1.7
 LONGEST FLOWPATH FROM NODE 68.00 TO NODE 70.00 = 550.00 FEET.

FLOW PROCESS FROM NODE 33.00 TO NODE 33.00 IS CODE = 12

>>>>CLEAR MEMORY BANK # 3 <<<<<

FLOW PROCESS FROM NODE 70.00 TO NODE 70.00 IS CODE = 10

>>>>MAIN-STREAM MEMORY COPIED ONTO MEMORY BANK # 3 <<<<<

FLOW PROCESS FROM NODE 72.00 TO NODE 73.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<

>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

INITIAL SUBAREA FLOW-LENGTH(FEET) = 230.00

ELEVATION DATA: UPSTREAM(FEET) = 168.00 DOWNSTREAM(FEET) = 166.80

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$
 SUBAREA ANALYSIS USED MINIMUM T_c (MIN.) = 8.161
 * 10 YEAR RAINFALL INTENSITY (INCH/HR) = 3.066
 SUBAREA T_c AND LOSS RATE DATA (AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	F_p (INCH/HR)	A_p (DECIMAL)	SCS CN	T_c (MIN.)
APARTMENTS	B	0.28	0.30	0.200	56	8.16

 SUBAREA AVERAGE PERVIOUS LOSS RATE, F_p (INCH/HR) = 0.30
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, A_p = 0.200
 SUBAREA RUNOFF (CFS) = 0.76
 TOTAL AREA (ACRES) = 0.28 PEAK FLOW RATE (CFS) = 0.76

FLOW PROCESS FROM NODE 73.00 TO NODE 70.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
 >>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<

ELEVATION DATA: UPSTREAM (FEET) = 160.00 DOWNSTREAM (FEET) = 159.00
 FLOW LENGTH (FEET) = 270.00 MANNING'S N = 0.013
 ESTIMATED PIPE DIAMETER (INCH) INCREASED TO 12.000
 DEPTH OF FLOW IN 12.0 INCH PIPE IS 5.0 INCHES
 PIPE-FLOW VELOCITY (FEET/SEC.) = 2.47
 ESTIMATED PIPE DIAMETER (INCH) = 12.000 NUMBER OF PIPES = 1
 PIPE-FLOW (CFS) = 0.76
 PIPE TRAVEL TIME (MIN.) = 1.82 T_c (MIN.) = 9.98
 LONGEST FLOWPATH FROM NODE 72.00 TO NODE 70.00 = 500.00 FEET.

FLOW PROCESS FROM NODE 70.00 TO NODE 70.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

TOTAL NUMBER OF STREAMS = 2
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
 TIME OF CONCENTRATION (MIN.) = 9.98
 RAINFALL INTENSITY (INCH/HR) = 2.73
 AREA-AVERAGED F_m (INCH/HR) = 0.06
 AREA-AVERAGED F_p (INCH/HR) = 0.30
 AREA-AVERAGED A_p = 0.20
 EFFECTIVE STREAM AREA (ACRES) = 0.28
 TOTAL STREAM AREA (ACRES) = 0.28
 PEAK FLOW RATE (CFS) AT CONFLUENCE = 0.76

FLOW PROCESS FROM NODE 72.00 TO NODE 70.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
 >>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

INITIAL SUBAREA FLOW-LENGTH (FEET) = 390.00
 ELEVATION DATA: UPSTREAM (FEET) = 167.20 DOWNSTREAM (FEET) = 166.80

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$

SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) = 13.956
 * 10 YEAR RAINFALL INTENSITY(INCH/HR) = 2.254
 SUBAREA Tc AND LOSS RATE DATA(AMC II):
 DEVELOPMENT TYPE/ SCS SOIL AREA Fp Ap SCS Tc
 LAND USE GROUP (ACRES) (INCH/HR) (DECIMAL) CN (MIN.)
 APARTMENTS B 1.49 0.30 0.200 56 13.96
 SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.30
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.200
 SUBAREA RUNOFF(CFS) = 2.94
 TOTAL AREA(ACRES) = 1.49 PEAK FLOW RATE(CFS) = 2.94

 FLOW PROCESS FROM NODE 70.00 TO NODE 70.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
 >>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<

=====

TOTAL NUMBER OF STREAMS = 2
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
 TIME OF CONCENTRATION(MIN.) = 13.96
 RAINFALL INTENSITY(INCH/HR) = 2.25
 AREA-AVERAGED Fm(INCH/HR) = 0.06
 AREA-AVERAGED Fp(INCH/HR) = 0.30
 AREA-AVERAGED Ap = 0.20
 EFFECTIVE STREAM AREA(ACRES) = 1.49
 TOTAL STREAM AREA(ACRES) = 1.49
 PEAK FLOW RATE(CFS) AT CONFLUENCE = 2.94

** CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	0.76	9.98	2.731	0.30 (0.06)	0.20	0.3	72.00
2	2.94	13.96	2.254	0.30 (0.06)	0.20	1.5	72.00

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
 CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	3.32	9.98	2.731	0.30 (0.06)	0.20	1.3	72.00
2	3.56	13.96	2.254	0.30 (0.06)	0.20	1.8	72.00

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE(CFS) = 3.56 Tc(MIN.) = 13.96
 EFFECTIVE AREA(ACRES) = 1.77 AREA-AVERAGED Fm(INCH/HR) = 0.06
 AREA-AVERAGED Fp(INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.20
 TOTAL AREA(ACRES) = 1.8
 LONGEST FLOWPATH FROM NODE 72.00 TO NODE 70.00 = 500.00 FEET.

 FLOW PROCESS FROM NODE 70.00 TO NODE 70.00 IS CODE = 11

>>>>CONFLUENCE MEMORY BANK # 3 WITH THE MAIN-STREAM MEMORY<<<<<

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** MAIN STREAM CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	3.32	9.98	2.731	0.30 (0.06)	0.20	1.3	72.00
2	3.56	13.96	2.254	0.30 (0.06)	0.20	1.8	72.00

LONGEST FLOWPATH FROM NODE 72.00 TO NODE 70.00 = 500.00 FEET.

** MEMORY BANK # 3 CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	3.65	10.69	2.627	0.30 (0.06)	0.20	1.6	71.00
2	3.65	11.95	2.464	0.30 (0.06)	0.20	1.7	68.00

LONGEST FLOWPATH FROM NODE 68.00 TO NODE 70.00 = 550.00 FEET.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	6.87	9.98	2.731	0.30 (0.06)	0.20	2.8	72.00
2	7.01	10.69	2.627	0.30 (0.06)	0.20	3.0	71.00
3	7.09	11.95	2.464	0.30 (0.06)	0.20	3.2	68.00
4	6.90	13.96	2.254	0.30 (0.06)	0.20	3.4	72.00

TOTAL AREA (ACRES) = 3.4

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE (CFS) = 7.09 Tc (MIN.) = 11.951
EFFECTIVE AREA (ACRES) = 3.22 AREA-AVERAGED Fm (INCH/HR) = 0.06
AREA-AVERAGED Fp (INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.20
TOTAL AREA (ACRES) = 3.4
LONGEST FLOWPATH FROM NODE 68.00 TO NODE 70.00 = 550.00 FEET.

FLOW PROCESS FROM NODE 70.00 TO NODE 70.00 IS CODE = 12

=====
>>>>CLEAR MEMORY BANK # 3 <<<<<

FLOW PROCESS FROM NODE 70.00 TO NODE 67.00 IS CODE = 31

=====
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<

>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW) <<<<<

=====
ELEVATION DATA: UPSTREAM (FEET) = 155.00 DOWNSTREAM (FEET) = 148.50
FLOW LENGTH (FEET) = 30.00 MANNING'S N = 0.013
ESTIMATED PIPE DIAMETER (INCH) INCREASED TO 12.000
DEPTH OF FLOW IN 12.0 INCH PIPE IS 5.6 INCHES
PIPE-FLOW VELOCITY (FEET/SEC.) = 19.90
ESTIMATED PIPE DIAMETER (INCH) = 12.00 NUMBER OF PIPES = 1
PIPE-FLOW (CFS) = 7.09
PIPE TRAVEL TIME (MIN.) = 0.03 Tc (MIN.) = 11.98
LONGEST FLOWPATH FROM NODE 68.00 TO NODE 67.00 = 580.00 FEET.

FLOW PROCESS FROM NODE 67.00 TO NODE 67.00 IS CODE = 11

>>>>CONFLUENCE MEMORY BANK # 1 WITH THE MAIN-STREAM MEMORY<<<<<

** MAIN STREAM CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	6.87	10.01	2.727	0.30 (0.06)	0.20	2.8	72.00
2	7.01	10.71	2.623	0.30 (0.06)	0.20	3.0	71.00
3	7.09	11.98	2.461	0.30 (0.06)	0.20	3.2	68.00
4	6.90	13.98	2.252	0.30 (0.06)	0.20	3.4	72.00
LONGEST FLOWPATH FROM NODE			68.00 TO NODE	67.00 =	580.00 FEET.		

** MEMORY BANK # 1 CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	46.95	9.11	2.878	0.30 (0.08)	0.25	15.1	65.00
2	50.05	10.54	2.648	0.30 (0.08)	0.25	17.7	51.00
3	50.33	10.66	2.630	0.30 (0.08)	0.25	18.0	58.00
4	50.94	10.95	2.591	0.30 (0.08)	0.25	18.5	55.00
5	51.04	11.00	2.584	0.30 (0.08)	0.25	18.6	43.00
6	51.13	11.05	2.577	0.30 (0.08)	0.25	18.7	59.00
7	51.35	11.17	2.561	0.30 (0.08)	0.25	19.0	45.00
8	51.66	11.36	2.537	0.30 (0.08)	0.25	19.3	63.00
9	51.94	11.55	2.512	0.30 (0.08)	0.25	19.7	30.00
10	52.24	11.81	2.481	0.30 (0.07)	0.25	20.1	60.00
11	52.40	11.94	2.465	0.30 (0.07)	0.25	20.3	38.00
12	52.82	12.34	2.419	0.30 (0.07)	0.25	21.0	30.00
13	52.93	12.46	2.406	0.30 (0.07)	0.24	21.2	36.00
14	53.65	13.34	2.313	0.30 (0.07)	0.24	22.5	14.00
15	53.80	13.53	2.295	0.30 (0.07)	0.24	22.8	38.00
16	53.86	13.61	2.287	0.30 (0.07)	0.24	22.9	26.00
17	53.86	13.62	2.286	0.30 (0.07)	0.24	23.0	27.00
18	54.27	14.34	2.219	0.30 (0.07)	0.24	24.0	20.00
19	54.41	14.80	2.180	0.30 (0.07)	0.24	24.7	23.00
20	54.41	14.82	2.178	0.30 (0.07)	0.24	24.7	17.00
21	54.42	14.87	2.174	0.30 (0.07)	0.24	24.8	29.00
22	54.44	15.57	2.117	0.30 (0.07)	0.24	25.6	50.00
23	54.05	16.06	2.080	0.30 (0.07)	0.24	26.0	34.00
24	53.99	16.13	2.075	0.30 (0.07)	0.24	26.0	39.00
25	53.48	16.57	2.043	0.30 (0.07)	0.24	26.2	24.00
26	49.81	19.92	1.838	0.30 (0.07)	0.23	27.8	12.00
27	47.83	22.08	1.733	0.30 (0.07)	0.23	28.6	64.00
28	46.73	23.29	1.681	0.30 (0.07)	0.22	29.1	32.00
29	46.20	23.68	1.665	0.30 (0.07)	0.22	29.1	10.00
LONGEST FLOWPATH FROM NODE			10.00 TO NODE	67.00 =	2660.00 FEET.		

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	53.55	9.11	2.878	0.30 (0.07)	0.24	17.7	65.00
2	55.76	10.01	2.727	0.30 (0.07)	0.24	19.5	72.00

3	57.02	10.54	2.648	0.30	(0.07)	0.24	20.6	51.00
4	57.33	10.66	2.630	0.30	(0.07)	0.24	20.9	58.00
5	57.45	10.71	2.623	0.30	(0.07)	0.24	21.0	71.00
6	57.97	10.95	2.591	0.30	(0.07)	0.24	21.6	55.00
7	58.07	11.00	2.584	0.30	(0.07)	0.24	21.7	43.00
8	58.17	11.05	2.577	0.30	(0.07)	0.24	21.8	59.00
9	58.39	11.17	2.561	0.30	(0.07)	0.24	22.0	45.00
10	58.71	11.36	2.537	0.30	(0.07)	0.24	22.4	63.00
11	59.01	11.55	2.512	0.30	(0.07)	0.24	22.8	30.00
12	59.32	11.81	2.481	0.30	(0.07)	0.24	23.3	60.00
13	59.49	11.94	2.465	0.30	(0.07)	0.24	23.5	38.00
14	59.53	11.98	2.461	0.30	(0.07)	0.24	23.6	68.00
15	59.88	12.34	2.419	0.30	(0.07)	0.24	24.2	30.00
16	59.98	12.46	2.406	0.30	(0.07)	0.24	24.4	36.00
17	60.61	13.34	2.313	0.30	(0.07)	0.24	25.9	14.00
18	60.74	13.53	2.295	0.30	(0.07)	0.24	26.2	38.00
19	60.79	13.61	2.287	0.30	(0.07)	0.24	26.3	26.00
20	60.80	13.62	2.286	0.30	(0.07)	0.24	26.4	27.00
21	60.96	13.98	2.252	0.30	(0.07)	0.24	26.9	72.00
22	61.06	14.34	2.219	0.30	(0.07)	0.24	27.5	20.00
23	61.08	14.80	2.180	0.30	(0.07)	0.23	28.1	23.00
24	61.07	14.82	2.178	0.30	(0.07)	0.23	28.1	17.00
25	61.07	14.87	2.174	0.30	(0.07)	0.23	28.2	29.00
26	60.91	15.57	2.117	0.30	(0.07)	0.23	29.0	50.00
27	60.41	16.06	2.080	0.30	(0.07)	0.23	29.4	34.00
28	60.34	16.13	2.075	0.30	(0.07)	0.23	29.4	39.00
29	59.72	16.57	2.043	0.30	(0.07)	0.23	29.7	24.00
30	55.40	19.92	1.838	0.30	(0.07)	0.23	31.2	12.00
31	53.09	22.08	1.733	0.30	(0.07)	0.22	32.1	64.00
32	51.83	23.29	1.681	0.30	(0.07)	0.22	32.5	32.00
33	51.25	23.68	1.665	0.30	(0.07)	0.22	32.5	10.00
TOTAL AREA (ACRES) =			32.5					

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE (CFS) = 61.08 Tc (MIN.) = 14.802
EFFECTIVE AREA (ACRES) = 28.10 AREA-AVERAGED Fm (INCH/HR) = 0.07
AREA-AVERAGED Fp (INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.23
TOTAL AREA (ACRES) = 32.5
LONGEST FLOWPATH FROM NODE 10.00 TO NODE 67.00 = 2660.00 FEET.

FLOW PROCESS FROM NODE 67.00 TO NODE 74.00 IS CODE = 31

>>>>> COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA <<<<<
>>>>> USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW) <<<<<

=====

ELEVATION DATA: UPSTREAM (FEET) = 148.50 DOWNSTREAM (FEET) = 148.00
FLOW LENGTH (FEET) = 280.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 51.0 INCH PIPE IS 37.1 INCHES
PIPE-FLOW VELOCITY (FEET/SEC.) = 5.53
ESTIMATED PIPE DIAMETER (INCH) = 51.00 NUMBER OF PIPES = 1
PIPE-FLOW (CFS) = 61.08
PIPE TRAVEL TIME (MIN.) = 0.84 Tc (MIN.) = 15.65
LONGEST FLOWPATH FROM NODE 10.00 TO NODE 74.00 = 2940.00 FEET.

FLOW PROCESS FROM NODE 74.00 TO NODE 74.00 IS CODE = 10

>>>>MAIN-STREAM MEMORY COPIED ONTO MEMORY BANK # 3 <<<<<
=====

FLOW PROCESS FROM NODE 75.00 TO NODE 76.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<
=====

INITIAL SUBAREA FLOW-LENGTH (FEET) = 275.00
ELEVATION DATA: UPSTREAM (FEET) = 167.00 DOWNSTREAM (FEET) = 165.30

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$
SUBAREA ANALYSIS USED MINIMUM T_c (MIN.) = 9.415
* 10 YEAR RAINFALL INTENSITY (INCH/HR) = 2.825

SUBAREA T_c AND LOSS RATE DATA (AMC II):
DEVELOPMENT TYPE/ SCS SOIL AREA F_p A_p SCS T_c
LAND USE GROUP (ACRES) (INCH/HR) (DECIMAL) CN (MIN.)
CONDOMINIUMS B 0.70 0.30 0.350 56 9.41
SUBAREA AVERAGE PERVIOUS LOSS RATE, F_p (INCH/HR) = 0.30
SUBAREA AVERAGE PERVIOUS AREA FRACTION, A_p = 0.350
SUBAREA RUNOFF (CFS) = 1.71
TOTAL AREA (ACRES) = 0.70 PEAK FLOW RATE (CFS) = 1.71

FLOW PROCESS FROM NODE 76.00 TO NODE 76.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
=====

TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION (MIN.) = 9.41
RAINFALL INTENSITY (INCH/HR) = 2.82
AREA-AVERAGED F_m (INCH/HR) = 0.11
AREA-AVERAGED F_p (INCH/HR) = 0.30
AREA-AVERAGED A_p = 0.35
EFFECTIVE STREAM AREA (ACRES) = 0.70
TOTAL STREAM AREA (ACRES) = 0.70
PEAK FLOW RATE (CFS) AT CONFLUENCE = 1.71

FLOW PROCESS FROM NODE 77.00 TO NODE 76.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<
=====

INITIAL SUBAREA FLOW-LENGTH (FEET) = 255.00
ELEVATION DATA: UPSTREAM (FEET) = 167.00 DOWNSTREAM (FEET) = 166.30

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$

SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) = 10.745
 * 10 YEAR RAINFALL INTENSITY(INCH/HR) = 2.619
 SUBAREA Tc AND LOSS RATE DATA(AMC II):
 DEVELOPMENT TYPE/ SCS SOIL AREA Fp Ap SCS Tc
 LAND USE GROUP (ACRES) (INCH/HR) (DECIMAL) CN (MIN.)
 CONDOMINIUMS B 0.51 0.30 0.350 56 10.75
 SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.30
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.350
 SUBAREA RUNOFF(CFS) = 1.15
 TOTAL AREA(ACRES) = 0.51 PEAK FLOW RATE(CFS) = 1.15

 FLOW PROCESS FROM NODE 76.00 TO NODE 76.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
 >>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<

=====

TOTAL NUMBER OF STREAMS = 2
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
 TIME OF CONCENTRATION(MIN.) = 10.75
 RAINFALL INTENSITY(INCH/HR) = 2.62
 AREA-AVERAGED Fm(INCH/HR) = 0.11
 AREA-AVERAGED Fp(INCH/HR) = 0.30
 AREA-AVERAGED Ap = 0.35
 EFFECTIVE STREAM AREA(ACRES) = 0.51
 TOTAL STREAM AREA(ACRES) = 0.51
 PEAK FLOW RATE(CFS) AT CONFLUENCE = 1.15

** CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	1.71	9.41	2.825	0.30 (0.11)	0.35	0.7	75.00
2	1.15	10.75	2.619	0.30 (0.11)	0.35	0.5	77.00

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
 CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	2.81	9.41	2.825	0.30 (0.11)	0.35	1.1	75.00
2	2.74	10.75	2.619	0.30 (0.10)	0.35	1.2	77.00

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE(CFS) = 2.81 Tc(MIN.) = 9.41
 EFFECTIVE AREA(ACRES) = 1.15 AREA-AVERAGED Fm(INCH/HR) = 0.10
 AREA-AVERAGED Fp(INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.35
 TOTAL AREA(ACRES) = 1.2
 LONGEST FLOWPATH FROM NODE 75.00 TO NODE 76.00 = 275.00 FEET.

 FLOW PROCESS FROM NODE 76.00 TO NODE 79.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<

>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 154.00 DOWNSTREAM(FEET) = 152.00
FLOW LENGTH(FEET) = 300.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 12.0 INCH PIPE IS 9.7 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 4.12
ESTIMATED PIPE DIAMETER(INCH) = 12.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 2.81
PIPE TRAVEL TIME(MIN.) = 1.21 Tc(MIN.) = 10.63
LONGEST FLOWPATH FROM NODE 75.00 TO NODE 79.00 = 575.00 FEET.

FLOW PROCESS FROM NODE 79.00 TO NODE 79.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

=====

TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION(MIN.) = 10.63
RAINFALL INTENSITY(INCH/HR) = 2.64
AREA-AVERAGED Fm(INCH/HR) = 0.10
AREA-AVERAGED Fp(INCH/HR) = 0.30
AREA-AVERAGED Ap = 0.35
EFFECTIVE STREAM AREA(ACRES) = 1.15
TOTAL STREAM AREA(ACRES) = 1.21
PEAK FLOW RATE(CFS) AT CONFLUENCE = 2.81

FLOW PROCESS FROM NODE 78.00 TO NODE 79.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

=====

INITIAL SUBAREA FLOW-LENGTH(FEET) = 200.00
ELEVATION DATA: UPSTREAM(FEET) = 165.90 DOWNSTREAM(FEET) = 165.30

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$
SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) = 9.578
* 10 YEAR RAINFALL INTENSITY(INCH/HR) = 2.797

SUBAREA Tc AND LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN	Tc (MIN.)
CONDOMINIUMS	B	0.40	0.30	0.350	56	9.58

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.30

SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.350

SUBAREA RUNOFF(CFS) = 0.97

TOTAL AREA(ACRES) = 0.40 PEAK FLOW RATE(CFS) = 0.97

FLOW PROCESS FROM NODE 79.00 TO NODE 79.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<

TOTAL NUMBER OF STREAMS = 2
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
 TIME OF CONCENTRATION(MIN.) = 9.58
 RAINFALL INTENSITY(INCH/HR) = 2.80
 AREA-AVERAGED Fm(INCH/HR) = 0.11
 AREA-AVERAGED Fp(INCH/HR) = 0.30
 AREA-AVERAGED Ap = 0.35
 EFFECTIVE STREAM AREA(ACRES) = 0.40
 TOTAL STREAM AREA(ACRES) = 0.40
 PEAK FLOW RATE(CFS) AT CONFLUENCE = 0.97

** CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	2.81	10.63	2.635	0.30(0.11)	0.35	1.1	75.00
1	2.74	11.96	2.463	0.30(0.10)	0.35	1.2	77.00
2	0.97	9.58	2.797	0.30(0.11)	0.35	0.4	78.00

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
 CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	3.66	9.58	2.797	0.30(0.11)	0.35	1.4	78.00
2	3.72	10.63	2.635	0.30(0.11)	0.35	1.5	75.00
3	3.59	11.96	2.463	0.30(0.10)	0.35	1.6	77.00

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE(CFS) = 3.72 Tc(MIN.) = 10.63
 EFFECTIVE AREA(ACRES) = 1.55 AREA-AVERAGED Fm(INCH/HR) = 0.11
 AREA-AVERAGED Fp(INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.35
 TOTAL AREA(ACRES) = 1.6
 LONGEST FLOWPATH FROM NODE 75.00 TO NODE 79.00 = 575.00 FEET.

 FLOW PROCESS FROM NODE 79.00 TO NODE 74.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
 >>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 152.00 DOWNSTREAM(FEET) = 148.00
 FLOW LENGTH(FEET) = 50.00 MANNING'S N = 0.013
 ESTIMATED PIPE DIAMETER(INCH) INCREASED TO 12.000
 DEPTH OF FLOW IN 12.0 INCH PIPE IS 5.1 INCHES
 PIPE-FLOW VELOCITY(FEET/SEC.) = 11.64
 ESTIMATED PIPE DIAMETER(INCH) = 12.00 NUMBER OF PIPES = 1
 PIPE-FLOW(CFS) = 3.72
 PIPE TRAVEL TIME(MIN.) = 0.07 Tc(MIN.) = 10.70
 LONGEST FLOWPATH FROM NODE 75.00 TO NODE 74.00 = 625.00 FEET.

 FLOW PROCESS FROM NODE 74.00 TO NODE 74.00 IS CODE = 11

>>>>CONFLUENCE MEMORY BANK # 3 WITH THE MAIN-STREAM MEMORY<<<<<

** MAIN STREAM CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	3.66	9.65	2.785	0.30 (0.11)	0.35	1.4	78.00
2	3.72	10.70	2.625	0.30 (0.11)	0.35	1.5	75.00
3	3.59	12.03	2.454	0.30 (0.10)	0.35	1.6	77.00
LONGEST FLOWPATH FROM NODE			75.00	TO NODE	74.00 =	625.00 FEET.	

** MEMORY BANK # 3 CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	53.55	9.99	2.730	0.30 (0.07)	0.24	17.7	65.00
2	55.76	10.88	2.600	0.30 (0.07)	0.24	19.5	72.00
3	57.02	11.41	2.531	0.30 (0.07)	0.24	20.6	51.00
4	57.33	11.53	2.515	0.30 (0.07)	0.24	20.9	58.00
5	57.45	11.58	2.508	0.30 (0.07)	0.24	21.0	71.00
6	57.97	11.82	2.480	0.30 (0.07)	0.24	21.6	55.00
7	58.07	11.87	2.473	0.30 (0.07)	0.24	21.7	43.00
8	58.17	11.92	2.468	0.30 (0.07)	0.24	21.8	59.00
9	58.39	12.04	2.454	0.30 (0.07)	0.24	22.0	45.00
10	58.71	12.23	2.432	0.30 (0.07)	0.24	22.4	63.00
11	59.01	12.42	2.410	0.30 (0.07)	0.24	22.8	30.00
12	59.32	12.66	2.384	0.30 (0.07)	0.24	23.3	60.00
13	59.49	12.79	2.370	0.30 (0.07)	0.24	23.5	38.00
14	59.53	12.82	2.366	0.30 (0.07)	0.24	23.6	68.00
15	59.88	13.19	2.329	0.30 (0.07)	0.24	24.2	30.00
16	59.98	13.30	2.317	0.30 (0.07)	0.24	24.4	36.00
17	60.61	14.19	2.233	0.30 (0.07)	0.24	25.9	14.00
18	60.74	14.37	2.217	0.30 (0.07)	0.24	26.2	38.00
19	60.79	14.45	2.210	0.30 (0.07)	0.24	26.3	26.00
20	60.80	14.46	2.209	0.30 (0.07)	0.24	26.4	27.00
21	60.96	14.83	2.178	0.30 (0.07)	0.24	26.9	72.00
22	61.06	15.19	2.148	0.30 (0.07)	0.24	27.5	20.00
23	61.08	15.65	2.111	0.30 (0.07)	0.23	28.1	23.00
24	61.07	15.67	2.110	0.30 (0.07)	0.23	28.1	17.00
25	61.07	15.72	2.106	0.30 (0.07)	0.23	28.2	29.00
26	60.91	16.42	2.054	0.30 (0.07)	0.23	29.0	50.00
27	60.41	16.91	2.020	0.30 (0.07)	0.23	29.4	34.00
28	60.34	16.97	2.015	0.30 (0.07)	0.23	29.4	39.00
29	59.72	17.42	1.986	0.30 (0.07)	0.23	29.7	24.00
30	55.40	20.80	1.794	0.30 (0.07)	0.23	31.2	12.00
31	53.09	22.96	1.695	0.30 (0.07)	0.22	32.1	64.00
32	51.83	24.17	1.646	0.30 (0.07)	0.22	32.5	32.00
33	51.25	24.56	1.631	0.30 (0.07)	0.22	32.5	10.00
LONGEST FLOWPATH FROM NODE			10.00	TO NODE	74.00 =	2940.00 FEET.	

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	56.46	9.65	2.785	0.30 (0.08)	0.25	18.5	78.00
2	57.23	9.99	2.730	0.30 (0.08)	0.25	19.1	65.00

3	59.03	10.70	2.625	0.30	(0.08)	0.25	20.7	75.00
4	59.46	10.88	2.600	0.30	(0.08)	0.25	21.1	72.00
5	60.67	11.41	2.531	0.30	(0.08)	0.25	22.2	51.00
6	60.97	11.53	2.515	0.30	(0.08)	0.25	22.5	58.00
7	61.08	11.58	2.508	0.30	(0.08)	0.25	22.6	71.00
8	61.58	11.82	2.480	0.30	(0.08)	0.25	23.2	55.00
9	61.68	11.87	2.473	0.30	(0.08)	0.25	23.3	43.00
10	61.76	11.92	2.468	0.30	(0.08)	0.25	23.4	59.00
11	61.96	12.03	2.454	0.30	(0.08)	0.25	23.6	77.00
12	61.97	12.04	2.454	0.30	(0.08)	0.25	23.6	45.00
13	62.26	12.23	2.432	0.30	(0.08)	0.25	24.0	63.00
14	62.53	12.42	2.410	0.30	(0.08)	0.25	24.4	30.00
15	62.80	12.66	2.384	0.30	(0.08)	0.25	24.9	60.00
16	62.95	12.79	2.370	0.30	(0.07)	0.25	25.1	38.00
17	62.98	12.82	2.366	0.30	(0.07)	0.25	25.2	68.00
18	63.28	13.19	2.329	0.30	(0.07)	0.25	25.8	30.00
19	63.35	13.30	2.317	0.30	(0.07)	0.25	26.0	36.00
20	63.86	14.19	2.233	0.30	(0.07)	0.24	27.5	14.00
21	63.97	14.37	2.217	0.30	(0.07)	0.24	27.8	38.00
22	64.00	14.45	2.210	0.30	(0.07)	0.24	27.9	26.00
23	64.01	14.46	2.209	0.30	(0.07)	0.24	28.0	27.00
24	64.13	14.83	2.178	0.30	(0.07)	0.24	28.5	72.00
25	64.18	15.19	2.148	0.30	(0.07)	0.24	29.1	20.00
26	64.14	15.65	2.111	0.30	(0.07)	0.24	29.7	23.00
27	64.13	15.67	2.110	0.30	(0.07)	0.24	29.7	17.00
28	64.12	15.72	2.106	0.30	(0.07)	0.24	29.8	29.00
29	63.89	16.42	2.054	0.30	(0.07)	0.24	30.6	50.00
30	63.33	16.91	2.020	0.30	(0.07)	0.24	31.0	34.00
31	63.25	16.97	2.015	0.30	(0.07)	0.24	31.1	39.00
32	62.59	17.42	1.986	0.30	(0.07)	0.24	31.3	24.00
33	57.98	20.80	1.794	0.30	(0.07)	0.23	32.8	12.00
34	55.52	22.96	1.695	0.30	(0.07)	0.23	33.7	64.00
35	54.19	24.17	1.646	0.30	(0.07)	0.23	34.1	32.00
36	53.58	24.56	1.631	0.30	(0.07)	0.23	34.1	10.00
TOTAL AREA (ACRES) =			34.1					

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE (CFS) = 64.18 Tc (MIN.) = 15.187
EFFECTIVE AREA (ACRES) = 29.08 AREA-AVERAGED Fm (INCH/HR) = 0.07
AREA-AVERAGED Fp (INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.24
TOTAL AREA (ACRES) = 34.1
LONGEST FLOWPATH FROM NODE 10.00 TO NODE 74.00 = 2940.00 FEET.

FLOW PROCESS FROM NODE 74.00 TO NODE 74.00 IS CODE = 12

>>>>CLEAR MEMORY BANK # 3 <<<<<

FLOW PROCESS FROM NODE 74.00 TO NODE 74.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

TOTAL NUMBER OF STREAMS = 2
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
 TIME OF CONCENTRATION(MIN.) = 15.19
 RAINFALL INTENSITY(INCH/HR) = 2.15
 AREA-AVERAGED Fm(INCH/HR) = 0.07
 AREA-AVERAGED Fp(INCH/HR) = 0.30
 AREA-AVERAGED Ap = 0.24
 EFFECTIVE STREAM AREA(ACRES) = 29.08
 TOTAL STREAM AREA(ACRES) = 34.13
 PEAK FLOW RATE(CFS) AT CONFLUENCE = 64.18

 FLOW PROCESS FROM NODE 59.00 TO NODE 74.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
 >>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

=====

INITIAL SUBAREA FLOW-LENGTH(FEET) = 300.00
 ELEVATION DATA: UPSTREAM(FEET) = 166.80 DOWNSTREAM(FEET) = 165.00

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$
 SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) = 9.807
 * 10 YEAR RAINFALL INTENSITY(INCH/HR) = 2.760

SUBAREA Tc AND LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN	Tc (MIN.)
CONDOMINIUMS	B	0.41	0.30	0.350	56	9.81

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.30
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.350
 SUBAREA RUNOFF(CFS) = 0.98
 TOTAL AREA(ACRES) = 0.41 PEAK FLOW RATE(CFS) = 0.98

 FLOW PROCESS FROM NODE 74.00 TO NODE 74.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
 >>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<

=====

TOTAL NUMBER OF STREAMS = 2
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
 TIME OF CONCENTRATION(MIN.) = 9.81
 RAINFALL INTENSITY(INCH/HR) = 2.76
 AREA-AVERAGED Fm(INCH/HR) = 0.11
 AREA-AVERAGED Fp(INCH/HR) = 0.30
 AREA-AVERAGED Ap = 0.35
 EFFECTIVE STREAM AREA(ACRES) = 0.41
 TOTAL STREAM AREA(ACRES) = 0.41
 PEAK FLOW RATE(CFS) AT CONFLUENCE = 0.98

** CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	56.46	9.65	2.785	0.30(0.08)	0.25	18.5	78.00
1	57.23	9.99	2.730	0.30(0.08)	0.25	19.1	65.00

1	59.03	10.70	2.625	0.30 (0.08)	0.25	20.7	75.00
1	59.46	10.88	2.600	0.30 (0.08)	0.25	21.1	72.00
1	60.67	11.41	2.531	0.30 (0.08)	0.25	22.2	51.00
1	60.97	11.53	2.515	0.30 (0.08)	0.25	22.5	58.00
1	61.08	11.58	2.508	0.30 (0.08)	0.25	22.6	71.00
1	61.58	11.82	2.480	0.30 (0.08)	0.25	23.2	55.00
1	61.68	11.87	2.473	0.30 (0.08)	0.25	23.3	43.00
1	61.76	11.92	2.468	0.30 (0.08)	0.25	23.4	59.00
1	61.96	12.03	2.454	0.30 (0.08)	0.25	23.6	77.00
1	61.97	12.04	2.454	0.30 (0.08)	0.25	23.6	45.00
1	62.26	12.23	2.432	0.30 (0.08)	0.25	24.0	63.00
1	62.53	12.42	2.410	0.30 (0.08)	0.25	24.4	30.00
1	62.80	12.66	2.384	0.30 (0.08)	0.25	24.9	60.00
1	62.95	12.79	2.370	0.30 (0.07)	0.25	25.1	38.00
1	62.98	12.82	2.366	0.30 (0.07)	0.25	25.2	68.00
1	63.28	13.19	2.329	0.30 (0.07)	0.25	25.8	30.00
1	63.35	13.30	2.317	0.30 (0.07)	0.25	26.0	36.00
1	63.86	14.19	2.233	0.30 (0.07)	0.24	27.5	14.00
1	63.97	14.37	2.217	0.30 (0.07)	0.24	27.8	38.00
1	64.00	14.45	2.210	0.30 (0.07)	0.24	27.9	26.00
1	64.01	14.46	2.209	0.30 (0.07)	0.24	28.0	27.00
1	64.13	14.83	2.178	0.30 (0.07)	0.24	28.5	72.00
1	64.18	15.19	2.148	0.30 (0.07)	0.24	29.1	20.00
1	64.14	15.65	2.111	0.30 (0.07)	0.24	29.7	23.00
1	64.13	15.67	2.110	0.30 (0.07)	0.24	29.7	17.00
1	64.12	15.72	2.106	0.30 (0.07)	0.24	29.8	29.00
1	63.89	16.42	2.054	0.30 (0.07)	0.24	30.6	50.00
1	63.33	16.91	2.020	0.30 (0.07)	0.24	31.0	34.00
1	63.25	16.97	2.015	0.30 (0.07)	0.24	31.1	39.00
1	62.59	17.42	1.986	0.30 (0.07)	0.24	31.3	24.00
1	57.98	20.80	1.794	0.30 (0.07)	0.23	32.8	12.00
1	55.52	22.96	1.695	0.30 (0.07)	0.23	33.7	64.00
1	54.19	24.17	1.646	0.30 (0.07)	0.23	34.1	32.00
1	53.58	24.56	1.631	0.30 (0.07)	0.23	34.1	10.00
2	0.98	9.81	2.760	0.30 (0.11)	0.35	0.4	59.00

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	57.43	9.65	2.785	0.30 (0.08)	0.25	18.9	78.00
2	57.79	9.81	2.760	0.30 (0.08)	0.25	19.2	59.00
3	58.20	9.99	2.730	0.30 (0.08)	0.25	19.5	65.00
4	59.96	10.70	2.625	0.30 (0.08)	0.25	21.1	75.00
5	60.38	10.88	2.600	0.30 (0.08)	0.25	21.5	72.00
6	61.56	11.41	2.531	0.30 (0.08)	0.25	22.6	51.00
7	61.86	11.53	2.515	0.30 (0.08)	0.25	22.9	58.00
8	61.96	11.58	2.508	0.30 (0.08)	0.25	23.0	71.00
9	62.45	11.82	2.480	0.30 (0.08)	0.25	23.6	55.00
10	62.55	11.87	2.473	0.30 (0.08)	0.25	23.7	43.00
11	62.63	11.92	2.468	0.30 (0.08)	0.25	23.8	59.00
12	62.83	12.03	2.454	0.30 (0.08)	0.25	24.0	77.00

13	62.84	12.04	2.454	0.30	(0.08)	0.25	24.0	45.00
14	63.12	12.23	2.432	0.30	(0.08)	0.25	24.4	63.00
15	63.38	12.42	2.410	0.30	(0.08)	0.25	24.8	30.00
16	63.64	12.66	2.384	0.30	(0.08)	0.25	25.3	60.00
17	63.78	12.79	2.370	0.30	(0.08)	0.25	25.6	38.00
18	63.81	12.82	2.366	0.30	(0.08)	0.25	25.6	68.00
19	64.10	13.19	2.329	0.30	(0.08)	0.25	26.3	30.00
20	64.17	13.30	2.317	0.30	(0.08)	0.25	26.5	36.00
21	64.64	14.19	2.233	0.30	(0.07)	0.25	27.9	14.00
22	64.75	14.37	2.217	0.30	(0.07)	0.24	28.2	38.00
23	64.78	14.45	2.210	0.30	(0.07)	0.24	28.4	26.00
24	64.78	14.46	2.209	0.30	(0.07)	0.24	28.4	27.00
25	64.89	14.83	2.178	0.30	(0.07)	0.24	29.0	72.00
26	64.93	15.19	2.148	0.30	(0.07)	0.24	29.5	20.00
27	64.88	15.65	2.111	0.30	(0.07)	0.24	30.1	23.00
28	64.87	15.67	2.110	0.30	(0.07)	0.24	30.2	17.00
29	64.86	15.72	2.106	0.30	(0.07)	0.24	30.2	29.00
30	64.61	16.42	2.054	0.30	(0.07)	0.24	31.1	50.00
31	64.04	16.91	2.020	0.30	(0.07)	0.24	31.4	34.00
32	63.96	16.97	2.015	0.30	(0.07)	0.24	31.5	39.00
33	63.28	17.42	1.986	0.30	(0.07)	0.24	31.7	24.00
34	58.61	20.80	1.794	0.30	(0.07)	0.23	33.2	12.00
35	56.10	22.96	1.695	0.30	(0.07)	0.23	34.1	64.00
36	54.75	24.17	1.646	0.30	(0.07)	0.23	34.5	32.00
37	54.14	24.56	1.631	0.30	(0.07)	0.23	34.5	10.00

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE (CFS) = 64.93 Tc (MIN.) = 15.19
EFFECTIVE AREA (ACRES) = 29.49 AREA-AVERAGED Fm (INCH/HR) = 0.07
AREA-AVERAGED Fp (INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.24
TOTAL AREA (ACRES) = 34.5
LONGEST FLOWPATH FROM NODE 10.00 TO NODE 74.00 = 2940.00 FEET.

FLOW PROCESS FROM NODE 80.00 TO NODE 81.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

INITIAL SUBAREA FLOW-LENGTH (FEET) = 290.00
ELEVATION DATA: UPSTREAM (FEET) = 166.40 DOWNSTREAM (FEET) = 165.90

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$

SUBAREA ANALYSIS USED MINIMUM Tc (MIN.) = 10.484

* 10 YEAR RAINFALL INTENSITY (INCH/HR) = 2.656

SUBAREA Tc AND LOSS RATE DATA (AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN	Tc (MIN.)
COMMERCIAL	B	0.76	0.30	0.100	56	10.48

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp (INCH/HR) = 0.30

SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.100

SUBAREA RUNOFF (CFS) = 1.80

TOTAL AREA (ACRES) = 0.76 PEAK FLOW RATE (CFS) = 1.80

FLOW PROCESS FROM NODE 81.00 TO NODE 82.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<

ELEVATION DATA: UPSTREAM(FEET) = 153.00 DOWNSTREAM(FEET) = 147.60
FLOW LENGTH(FEET) = 320.00 MANNING'S N = 0.013
ESTIMATED PIPE DIAMETER(INCH) INCREASED TO 12.000
DEPTH OF FLOW IN 12.0 INCH PIPE IS 5.3 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 5.41
ESTIMATED PIPE DIAMETER(INCH) = 12.000 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 1.80
PIPE TRAVEL TIME(MIN.) = 0.99 Tc(MIN.) = 11.47
LONGEST FLOWPATH FROM NODE 80.00 TO NODE 82.00 = 610.00 FEET.

FLOW PROCESS FROM NODE 82.00 TO NODE 82.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION(MIN.) = 11.47
RAINFALL INTENSITY(INCH/HR) = 2.52
AREA-AVERAGED Fm(INCH/HR) = 0.03
AREA-AVERAGED Fp(INCH/HR) = 0.30
AREA-AVERAGED Ap = 0.10
EFFECTIVE STREAM AREA(ACRES) = 0.76
TOTAL STREAM AREA(ACRES) = 0.76
PEAK FLOW RATE(CFS) AT CONFLUENCE = 1.80

FLOW PROCESS FROM NODE 81.00 TO NODE 82.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

INITIAL SUBAREA FLOW-LENGTH(FEET) = 320.00
ELEVATION DATA: UPSTREAM(FEET) = 165.90 DOWNSTREAM(FEET) = 165.40

Tc = K*[(LENGTH** 3.00)/(ELEVATION CHANGE)]**0.20
SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) = 11.122
* 10 YEAR RAINFALL INTENSITY(INCH/HR) = 2.568

SUBAREA Tc AND LOSS RATE DATA(AMC II):

Table with 7 columns: DEVELOPMENT TYPE/LAND USE, SCS SOIL GROUP, AREA (ACRES), Fp (INCH/HR), Ap (DECIMAL), SCS CN, Tc (MIN.). Row 1: COMMERCIAL, B, 0.59, 0.30, 0.100, 56, 11.12

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.30
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.100
SUBAREA RUNOFF(CFS) = 1.35
TOTAL AREA(ACRES) = 0.59 PEAK FLOW RATE(CFS) = 1.35

FLOW PROCESS FROM NODE 82.00 TO NODE 82.00 IS CODE = 1

>>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<<
>>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<<

TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
TIME OF CONCENTRATION(MIN.) = 11.12
RAINFALL INTENSITY(INCH/HR) = 2.57
AREA-AVERAGED Fm(INCH/HR) = 0.03
AREA-AVERAGED Fp(INCH/HR) = 0.30
AREA-AVERAGED Ap = 0.10
EFFECTIVE STREAM AREA(ACRES) = 0.59
TOTAL STREAM AREA(ACRES) = 0.59
PEAK FLOW RATE(CFS) AT CONFLUENCE = 1.35

** CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	1.80	11.47	2.523	0.30(0.03)	0.10	0.8	80.00
2	1.35	11.12	2.568	0.30(0.03)	0.10	0.6	81.00

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	3.12	11.12	2.568	0.30(0.03)	0.10	1.3	81.00
2	3.12	11.47	2.523	0.30(0.03)	0.10	1.3	80.00

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE(CFS) = 3.12 Tc(MIN.) = 11.12
EFFECTIVE AREA(ACRES) = 1.33 AREA-AVERAGED Fm(INCH/HR) = 0.03
AREA-AVERAGED Fp(INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.10
TOTAL AREA(ACRES) = 1.3
LONGEST FLOWPATH FROM NODE 80.00 TO NODE 82.00 = 610.00 FEET.

END OF STUDY SUMMARY:

TOTAL AREA(ACRES) = 1.3 TC(MIN.) = 11.12
EFFECTIVE AREA(ACRES) = 1.33 AREA-AVERAGED Fm(INCH/HR) = 0.03
AREA-AVERAGED Fp(INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.100
PEAK FLOW RATE(CFS) = 3.12

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	3.12	11.12	2.568	0.30(0.03)	0.10	1.3	81.00
2	3.12	11.47	2.523	0.30(0.03)	0.10	1.3	80.00

END OF RATIONAL METHOD ANALYSIS

RATIONAL METHOD HYDROLOGY COMPUTER PROGRAM PACKAGE
(Reference: 1986 ORANGE COUNTY HYDROLOGY CRITERION)
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***** DESCRIPTION OF STUDY *****

* FREEMAN *
* PROPOSED CONDITION *
* 25 YEAR ANALYSIS *

FILE NAME: FRPR25.DAT
TIME/DATE OF STUDY: 19:01 05/28/2017

=====

USER SPECIFIED HYDROLOGY AND HYDRAULIC MODEL INFORMATION:

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--*TIME-OF-CONCENTRATION MODEL*--

USER SPECIFIED STORM EVENT (YEAR) = 25.00
SPECIFIED MINIMUM PIPE SIZE (INCH) = 12.00
SPECIFIED PERCENT OF GRADIENTS (DECIMAL) TO USE FOR FRICTION SLOPE = 0.95
DATA BANK RAINFALL USED
ANTECEDENT MOISTURE CONDITION (AMC) II ASSUMED FOR RATIONAL METHOD

USER-DEFINED STREET-SECTIONS FOR COUPLED PIPEFLOW AND STREETFLOW MODEL

NO.	HALF- WIDTH (FT)	CROWN TO CROSSFALL (FT)	STREET-CROSSFALL: IN- / OUT- / PARK- SIDE / SIDE / WAY	CURB HEIGHT (FT)	GUTTER-GEOMETRIES: WIDTH LIP HIKE (FT) (FT) (FT)	MANNING FACTOR (n)
1	24.0	19.0	0.020/0.020/0.020	0.67	2.00 0.0312 0.167	0.0150
2	18.0	13.0	0.020/0.020/0.020	0.50	1.50 0.0312 0.125	0.0150

GLOBAL STREET FLOW-DEPTH CONSTRAINTS:

1. Relative Flow-Depth = 0.50 FEET
as (Maximum Allowable Street Flow Depth) - (Top-of-Curb)
 2. (Depth)*(Velocity) Constraint = 6.0 (FT*FT/S)
- *SIZE PIPE WITH A FLOW CAPACITY GREATER THAN
OR EQUAL TO THE UPSTREAM TRIBUTARY PIPE.*
*USER-SPECIFIED MINIMUM TOPOGRAPHIC SLOPE ADJUSTMENT NOT SELECTED

FLOW PROCESS FROM NODE 10.00 TO NODE 32.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

INITIAL SUBAREA FLOW-LENGTH (FEET) = 265.00
ELEVATION DATA: UPSTREAM (FEET) = 167.50 DOWNSTREAM (FEET) = 166.30

Tc = K * [(LENGTH** 3.00) / (ELEVATION CHANGE)]**0.20
SUBAREA ANALYSIS USED MINIMUM Tc (MIN.) = 8.337
* 25 YEAR RAINFALL INTENSITY (INCH/HR) = 3.612

SUBAREA Tc AND LOSS RATE DATA (AMC II):

DEVELOPMENT TYPE/ SCS SOIL AREA Fp Ap SCS Tc
LAND USE GROUP (ACRES) (INCH/HR) (DECIMAL) CN (MIN.)
COMMERCIAL B 0.15 0.30 0.100 56 8.34

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp (INCH/HR) = 0.30

SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.100

SUBAREA RUNOFF (CFS) = 0.48

TOTAL AREA (ACRES) = 0.15 PEAK FLOW RATE (CFS) = 0.48

FLOW PROCESS FROM NODE 32.00 TO NODE 11.00 IS CODE = 61

>>>>COMPUTE STREET FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>(STANDARD CURB SECTION USED)<<<<<

UPSTREAM ELEVATION (FEET) = 166.30 DOWNSTREAM ELEVATION (FEET) = 165.10
STREET LENGTH (FEET) = 210.00 CURB HEIGHT (INCHES) = 6.0
STREET HALFWIDTH (FEET) = 25.00

DISTANCE FROM CROWN TO CROSSFALL GRADEBREAK (FEET) = 10.00

INSIDE STREET CROSSFALL (DECIMAL) = 0.020

OUTSIDE STREET CROSSFALL (DECIMAL) = 0.020

SPECIFIED NUMBER OF HALFSTREETS CARRYING RUNOFF = 1

STREET PARKWAY CROSSFALL (DECIMAL) = 0.020

Manning's FRICTION FACTOR for Streetflow Section (curb-to-curb) = 0.0150

Manning's FRICTION FACTOR for Back-of-Walk Flow Section = 0.0200

**TRAVEL TIME COMPUTED USING ESTIMATED FLOW (CFS) = 0.65

STREETFLOW MODEL RESULTS USING ESTIMATED FLOW:

STREET FLOW DEPTH (FEET) = 0.25

HALFSTREET FLOOD WIDTH (FEET) = 6.01

AVERAGE FLOW VELOCITY (FEET/SEC.) = 1.36

PRODUCT OF DEPTH&VELOCITY (FT*FT/SEC.) = 0.33

STREET FLOW TRAVEL TIME (MIN.) = 2.58 Tc (MIN.) = 10.92

* 25 YEAR RAINFALL INTENSITY (INCH/HR) = 3.100

SUBAREA LOSS RATE DATA (AMC II):

DEVELOPMENT TYPE/ SCS SOIL AREA Fp Ap SCS
LAND USE GROUP (ACRES) (INCH/HR) (DECIMAL) CN
COMMERCIAL B 0.12 0.30 0.100 56

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp (INCH/HR) = 0.30

SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.100

SUBAREA AREA (ACRES) = 0.12 SUBAREA RUNOFF (CFS) = 0.33
 EFFECTIVE AREA (ACRES) = 0.27 AREA-AVERAGED Fm (INCH/HR) = 0.03
 AREA-AVERAGED Fp (INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.10
 TOTAL AREA (ACRES) = 0.3 PEAK FLOW RATE (CFS) = 0.75

END OF SUBAREA STREET FLOW HYDRAULICS:

DEPTH (FEET) = 0.26 HALFSTREET FLOOD WIDTH (FEET) = 6.48
 FLOW VELOCITY (FEET/SEC.) = 1.39 DEPTH*VELOCITY (FT*FT/SEC.) = 0.35
 LONGEST FLOWPATH FROM NODE 10.00 TO NODE 11.00 = 475.00 FEET.

FLOW PROCESS FROM NODE 11.00 TO NODE 11.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

=====

TOTAL NUMBER OF STREAMS = 2
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
 TIME OF CONCENTRATION (MIN.) = 10.92
 RAINFALL INTENSITY (INCH/HR) = 3.10
 AREA-AVERAGED Fm (INCH/HR) = 0.03
 AREA-AVERAGED Fp (INCH/HR) = 0.30
 AREA-AVERAGED Ap = 0.10
 EFFECTIVE STREAM AREA (ACRES) = 0.27
 TOTAL STREAM AREA (ACRES) = 0.27
 PEAK FLOW RATE (CFS) AT CONFLUENCE = 0.75

FLOW PROCESS FROM NODE 12.00 TO NODE 11.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<

>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

=====

INITIAL SUBAREA FLOW-LENGTH (FEET) = 210.00
 ELEVATION DATA: UPSTREAM (FEET) = 166.00 DOWNSTREAM (FEET) = 165.10

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$

SUBAREA ANALYSIS USED MINIMUM Tc (MIN.) = 7.680

* 25 YEAR RAINFALL INTENSITY (INCH/HR) = 3.783

SUBAREA Tc AND LOSS RATE DATA (AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN	Tc (MIN.)
COMMERCIAL	B	0.76	0.30	0.100	56	7.68

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp (INCH/HR) = 0.30

SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.100

SUBAREA RUNOFF (CFS) = 2.57

TOTAL AREA (ACRES) = 0.76 PEAK FLOW RATE (CFS) = 2.57

FLOW PROCESS FROM NODE 11.00 TO NODE 11.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<

=====

TOTAL NUMBER OF STREAMS = 2

CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:

TIME OF CONCENTRATION(MIN.) = 7.68
RAINFALL INTENSITY(INCH/HR) = 3.78
AREA-AVERAGED Fm(INCH/HR) = 0.03
AREA-AVERAGED Fp(INCH/HR) = 0.30
AREA-AVERAGED Ap = 0.10
EFFECTIVE STREAM AREA(ACRES) = 0.76
TOTAL STREAM AREA(ACRES) = 0.76
PEAK FLOW RATE(CFS) AT CONFLUENCE = 2.57

** CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	0.75	10.92	3.100	0.30(0.03)	0.10	0.3	10.00
2	2.57	7.68	3.783	0.30(0.03)	0.10	0.8	12.00

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	3.21	7.68	3.783	0.30(0.03)	0.10	0.9	12.00
2	2.85	10.92	3.100	0.30(0.03)	0.10	1.0	10.00

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE(CFS) = 3.21 Tc(MIN.) = 7.68
EFFECTIVE AREA(ACRES) = 0.95 AREA-AVERAGED Fm(INCH/HR) = 0.03
AREA-AVERAGED Fp(INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.10
TOTAL AREA(ACRES) = 1.0
LONGEST FLOWPATH FROM NODE 10.00 TO NODE 11.00 = 475.00 FEET.

FLOW PROCESS FROM NODE 11.00 TO NODE 13.00 IS CODE = 62

>>>>COMPUTE STREET FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>(STREET TABLE SECTION # 1 USED)<<<<<

UPSTREAM ELEVATION(FEET) = 165.10 DOWNSTREAM ELEVATION(FEET) = 164.20
STREET LENGTH(FEET) = 455.00 CURB HEIGHT(INCHES) = 8.0
STREET HALFWIDTH(FEET) = 24.00

DISTANCE FROM CROWN TO CROSSFALL GRADEBREAK(FEET) = 19.00
INSIDE STREET CROSSFALL(DECIMAL) = 0.020
OUTSIDE STREET CROSSFALL(DECIMAL) = 0.020

SPECIFIED NUMBER OF HALFSTREETS CARRYING RUNOFF = 1
STREET PARKWAY CROSSFALL(DECIMAL) = 0.020
Manning's FRICTION FACTOR for Streetflow Section(curbs-to-curbs) = 0.0150
Manning's FRICTION FACTOR for Back-of-Walk Flow Section = 0.0200

**TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 3.65
STREETFLOW MODEL RESULTS USING ESTIMATED FLOW:
STREET FLOW DEPTH(FEET) = 0.48

HALFSTREET FLOOD WIDTH (FEET) = 15.95
 AVERAGE FLOW VELOCITY (FEET/SEC.) = 1.34
 PRODUCT OF DEPTH&VELOCITY (FT*FT/SEC.) = 0.64
 STREET FLOW TRAVEL TIME (MIN.) = 5.67 Tc (MIN.) = 13.35
 * 25 YEAR RAINFALL INTENSITY (INCH/HR) = 2.767
 SUBAREA LOSS RATE DATA (AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
COMMERCIAL	B	0.36	0.30	0.100	56

 SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp (INCH/HR) = 0.30
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.100
 SUBAREA AREA (ACRES) = 0.36 SUBAREA RUNOFF (CFS) = 0.89
 EFFECTIVE AREA (ACRES) = 1.31 AREA-AVERAGED Fm (INCH/HR) = 0.03
 AREA-AVERAGED Fp (INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.10
 TOTAL AREA (ACRES) = 1.4 PEAK FLOW RATE (CFS) = 3.23

END OF SUBAREA STREET FLOW HYDRAULICS:
 DEPTH (FEET) = 0.46 HALFSTREET FLOOD WIDTH (FEET) = 15.21
 FLOW VELOCITY (FEET/SEC.) = 1.29 DEPTH*VELOCITY (FT*FT/SEC.) = 0.60
 LONGEST FLOWPATH FROM NODE 10.00 TO NODE 13.00 = 930.00 FEET.

 FLOW PROCESS FROM NODE 13.00 TO NODE 13.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

=====
 TOTAL NUMBER OF STREAMS = 2
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
 TIME OF CONCENTRATION (MIN.) = 13.35
 RAINFALL INTENSITY (INCH/HR) = 2.77
 AREA-AVERAGED Fm (INCH/HR) = 0.03
 AREA-AVERAGED Fp (INCH/HR) = 0.30
 AREA-AVERAGED Ap = 0.10
 EFFECTIVE STREAM AREA (ACRES) = 1.31
 TOTAL STREAM AREA (ACRES) = 1.39
 PEAK FLOW RATE (CFS) AT CONFLUENCE = 3.23

 FLOW PROCESS FROM NODE 14.00 TO NODE 15.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
 >>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

=====
 INITIAL SUBAREA FLOW-LENGTH (FEET) = 125.00
 ELEVATION DATA: UPSTREAM (FEET) = 164.60 DOWNSTREAM (FEET) = 164.20

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$
 SUBAREA ANALYSIS USED MINIMUM Tc (MIN.) = 6.616
 * 25 YEAR RAINFALL INTENSITY (INCH/HR) = 4.117

SUBAREA Tc AND LOSS RATE DATA (AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN	Tc (MIN.)
COMMERCIAL	B	0.15	0.30	0.100	56	6.62

 SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp (INCH/HR) = 0.30

SUBAREA AVERAGE PERVIOUS AREA FRACTION, $A_p = 0.100$
 SUBAREA RUNOFF (CFS) = 0.55
 TOTAL AREA (ACRES) = 0.15 PEAK FLOW RATE (CFS) = 0.55

 FLOW PROCESS FROM NODE 15.00 TO NODE 13.00 IS CODE = 41

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
 >>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM (FEET) = 159.30 DOWNSTREAM (FEET) = 159.20
 FLOW LENGTH (FEET) = 50.00 MANNING'S N = 0.013
 DEPTH OF FLOW IN 18.0 INCH PIPE IS 4.2 INCHES
 PIPE-FLOW VELOCITY (FEET/SEC.) = 1.76
 GIVEN PIPE DIAMETER (INCH) = 18.00 NUMBER OF PIPES = 1
 PIPE-FLOW (CFS) = 0.55
 PIPE TRAVEL TIME (MIN.) = 0.47 T_c (MIN.) = 7.09
 LONGEST FLOWPATH FROM NODE 14.00 TO NODE 13.00 = 175.00 FEET.

 FLOW PROCESS FROM NODE 13.00 TO NODE 13.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
 >>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<

=====

TOTAL NUMBER OF STREAMS = 2
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
 TIME OF CONCENTRATION (MIN.) = 7.09
 RAINFALL INTENSITY (INCH/HR) = 3.96
 AREA-AVERAGED F_m (INCH/HR) = 0.03
 AREA-AVERAGED F_p (INCH/HR) = 0.30
 AREA-AVERAGED $A_p = 0.10$
 EFFECTIVE STREAM AREA (ACRES) = 0.15
 TOTAL STREAM AREA (ACRES) = 0.15
 PEAK FLOW RATE (CFS) AT CONFLUENCE = 0.55

** CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	T_c (MIN.)	Intensity (INCH/HR)	$F_p(F_m)$ (INCH/HR)	A_p	A_e (ACRES)	HEADWATER NODE
1	3.23	13.35	2.767	0.30 (0.03)	0.10	1.3	12.00
1	3.00	16.78	2.431	0.30 (0.03)	0.10	1.4	10.00
2	0.55	7.09	3.959	0.30 (0.03)	0.10	0.2	14.00

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
 CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	T_c (MIN.)	Intensity (INCH/HR)	$F_p(F_m)$ (INCH/HR)	A_p	A_e (ACRES)	HEADWATER NODE
1	3.01	7.09	3.959	0.30 (0.03)	0.10	0.8	14.00
2	3.61	13.35	2.767	0.30 (0.03)	0.10	1.5	12.00
3	3.34	16.78	2.431	0.30 (0.03)	0.10	1.5	10.00

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE (CFS) = 3.61 Tc (MIN.) = 13.35
 EFFECTIVE AREA (ACRES) = 1.46 AREA-AVERAGED Fm (INCH/HR) = 0.03
 AREA-AVERAGED Fp (INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.10
 TOTAL AREA (ACRES) = 1.5
 LONGEST FLOWPATH FROM NODE 10.00 TO NODE 13.00 = 930.00 FEET.

FLOW PROCESS FROM NODE 13.00 TO NODE 16.00 IS CODE = 41

 >>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
 >>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM (FEET) = 159.20 DOWNSTREAM (FEET) = 151.55
 FLOW LENGTH (FEET) = 235.00 MANNING'S N = 0.013
 DEPTH OF FLOW IN 18.0 INCH PIPE IS 5.4 INCHES
 PIPE-FLOW VELOCITY (FEET/SEC.) = 8.11
 GIVEN PIPE DIAMETER (INCH) = 18.00 NUMBER OF PIPES = 1
 PIPE-FLOW (CFS) = 3.61
 PIPE TRAVEL TIME (MIN.) = 0.48 Tc (MIN.) = 13.84
 LONGEST FLOWPATH FROM NODE 10.00 TO NODE 16.00 = 1165.00 FEET.

FLOW PROCESS FROM NODE 16.00 TO NODE 16.00 IS CODE = 10

 >>>>MAIN-STREAM MEMORY COPIED ONTO MEMORY BANK # 1 <<<<<

FLOW PROCESS FROM NODE 17.00 TO NODE 18.00 IS CODE = 21

 >>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
 >>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

=====

INITIAL SUBAREA FLOW-LENGTH (FEET) = 150.00
 ELEVATION DATA: UPSTREAM (FEET) = 166.90 DOWNSTREAM (FEET) = 166.50

Tc = K * [(LENGTH** 3.00) / (ELEVATION CHANGE)] ** 0.20
 SUBAREA ANALYSIS USED MINIMUM Tc (MIN.) = 8.741
 * 25 YEAR RAINFALL INTENSITY (INCH/HR) = 3.516
 SUBAREA Tc AND LOSS RATE DATA (AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN	Tc (MIN.)
CONDOMINIUMS	B	0.36	0.30	0.350	56	8.74

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp (INCH/HR) = 0.30
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.350
 SUBAREA RUNOFF (CFS) = 1.11
 TOTAL AREA (ACRES) = 0.36 PEAK FLOW RATE (CFS) = 1.11

FLOW PROCESS FROM NODE 18.00 TO NODE 19.00 IS CODE = 31

 >>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
 >>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 153.00 DOWNSTREAM(FEET) = 151.60
 FLOW LENGTH(FEET) = 65.00 MANNING'S N = 0.013
 ESTIMATED PIPE DIAMETER(INCH) INCREASED TO 12.000
 DEPTH OF FLOW IN 12.0 INCH PIPE IS 3.8 INCHES
 PIPE-FLOW VELOCITY(FEET/SEC.) = 5.19
 ESTIMATED PIPE DIAMETER(INCH) = 12.000 NUMBER OF PIPES = 1
 PIPE-FLOW(CFS) = 1.11
 PIPE TRAVEL TIME(MIN.) = 0.21 Tc(MIN.) = 8.95
 LONGEST FLOWPATH FROM NODE 17.00 TO NODE 19.00 = 215.00 FEET.

FLOW PROCESS FROM NODE 19.00 TO NODE 19.00 IS CODE = 1

 >>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

=====

TOTAL NUMBER OF STREAMS = 2
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
 TIME OF CONCENTRATION(MIN.) = 8.95
 RAINFALL INTENSITY(INCH/HR) = 3.47
 AREA-AVERAGED Fm(INCH/HR) = 0.11
 AREA-AVERAGED Fp(INCH/HR) = 0.30
 AREA-AVERAGED Ap = 0.35
 EFFECTIVE STREAM AREA(ACRES) = 0.36
 TOTAL STREAM AREA(ACRES) = 0.36
 PEAK FLOW RATE(CFS) AT CONFLUENCE = 1.11

FLOW PROCESS FROM NODE 20.00 TO NODE 21.00 IS CODE = 21

 >>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
 >>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

=====

INITIAL SUBAREA FLOW-LENGTH(FEET) = 150.00
 ELEVATION DATA: UPSTREAM(FEET) = 167.30 DOWNSTREAM(FEET) = 166.80

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] * 0.20$
 SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) = 8.359
 * 25 YEAR RAINFALL INTENSITY(INCH/HR) = 3.606
 SUBAREA Tc AND LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN	Tc (MIN.)
CONDOMINIUMS	A	0.50	0.40	0.350	32	8.36

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.40
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.350
 SUBAREA RUNOFF(CFS) = 1.56
 TOTAL AREA(ACRES) = 0.50 PEAK FLOW RATE(CFS) = 1.56

FLOW PROCESS FROM NODE 21.00 TO NODE 19.00 IS CODE = 31

 >>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
 >>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 153.00 DOWNSTREAM(FEET) = 151.60

FLOW LENGTH (FEET) = 45.00 MANNING'S N = 0.013
 ESTIMATED PIPE DIAMETER (INCH) INCREASED TO 12.000
 DEPTH OF FLOW IN 12.0 INCH PIPE IS 4.1 INCHES
 PIPE-FLOW VELOCITY (FEET/SEC.) = 6.52
 ESTIMATED PIPE DIAMETER (INCH) = 12.000 NUMBER OF PIPES = 1
 PIPE-FLOW (CFS) = 1.56
 PIPE TRAVEL TIME (MIN.) = 0.12 Tc (MIN.) = 8.47
 LONGEST FLOWPATH FROM NODE 20.00 TO NODE 19.00 = 195.00 FEET.

 FLOW PROCESS FROM NODE 19.00 TO NODE 19.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
 >>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<

=====
 TOTAL NUMBER OF STREAMS = 2
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
 TIME OF CONCENTRATION (MIN.) = 8.47
 RAINFALL INTENSITY (INCH/HR) = 3.58
 AREA-AVERAGED Fm (INCH/HR) = 0.14
 AREA-AVERAGED Fp (INCH/HR) = 0.40
 AREA-AVERAGED Ap = 0.35
 EFFECTIVE STREAM AREA (ACRES) = 0.50
 TOTAL STREAM AREA (ACRES) = 0.50
 PEAK FLOW RATE (CFS) AT CONFLUENCE = 1.56

** CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	1.11	8.95	3.470	0.30 (0.11)	0.35	0.4	17.00
2	1.56	8.47	3.578	0.40 (0.14)	0.35	0.5	20.00

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
 CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	2.64	8.47	3.578	0.36 (0.13)	0.35	0.8	20.00
2	2.62	8.95	3.470	0.36 (0.13)	0.35	0.9	17.00

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE (CFS) = 2.64 Tc (MIN.) = 8.47
 EFFECTIVE AREA (ACRES) = 0.84 AREA-AVERAGED Fm (INCH/HR) = 0.13
 AREA-AVERAGED Fp (INCH/HR) = 0.36 AREA-AVERAGED Ap = 0.35
 TOTAL AREA (ACRES) = 0.9
 LONGEST FLOWPATH FROM NODE 17.00 TO NODE 19.00 = 215.00 FEET.

 FLOW PROCESS FROM NODE 19.00 TO NODE 19.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

=====
 MAINLINE Tc (MIN.) = 8.47

* 25 YEAR RAINFALL INTENSITY (INCH/HR) = 3.578

SUBAREA LOSS RATE DATA (AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
CONDOMINIUMS	B	0.42	0.30	0.350	56

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp (INCH/HR) = 0.30
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.350
SUBAREA AREA (ACRES) = 0.42 SUBAREA RUNOFF (CFS) = 1.31
EFFECTIVE AREA (ACRES) = 1.26 AREA-AVERAGED Fm (INCH/HR) = 0.12
AREA-AVERAGED Fp (INCH/HR) = 0.34 AREA-AVERAGED Ap = 0.35
TOTAL AREA (ACRES) = 1.3 PEAK FLOW RATE (CFS) = 3.93

FLOW PROCESS FROM NODE 19.00 TO NODE 16.00 IS CODE = 41

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM (FEET) = 151.60 DOWNSTREAM (FEET) = 151.55
FLOW LENGTH (FEET) = 30.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 18.0 INCH PIPE IS 13.9 INCHES
PIPE-FLOW VELOCITY (FEET/SEC.) = 2.69
GIVEN PIPE DIAMETER (INCH) = 18.00 NUMBER OF PIPES = 1
PIPE-FLOW (CFS) = 3.93
PIPE TRAVEL TIME (MIN.) = 0.19 Tc (MIN.) = 8.66
LONGEST FLOWPATH FROM NODE 17.00 TO NODE 16.00 = 245.00 FEET.

FLOW PROCESS FROM NODE 16.00 TO NODE 16.00 IS CODE = 11

>>>>CONFLUENCE MEMORY BANK # 1 WITH THE MAIN-STREAM MEMORY<<<<<

** MAIN STREAM CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp (Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	3.93	8.66	3.535	0.34 (0.12)	0.35	1.3	20.00
2	3.86	9.14	3.429	0.34 (0.12)	0.35	1.3	17.00

LONGEST FLOWPATH FROM NODE 17.00 TO NODE 16.00 = 245.00 FEET.

** MEMORY BANK # 1 CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp (Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	3.01	7.60	3.806	0.30 (0.03)	0.10	0.8	14.00
2	3.61	13.84	2.711	0.30 (0.03)	0.10	1.5	12.00
3	3.34	17.28	2.391	0.30 (0.03)	0.10	1.5	10.00

LONGEST FLOWPATH FROM NODE 10.00 TO NODE 16.00 = 1165.00 FEET.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp (Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	6.73	7.60	3.806	0.33 (0.08)	0.24	2.0	14.00
2	7.04	8.66	3.535	0.33 (0.08)	0.24	2.2	20.00
3	7.02	9.14	3.429	0.33 (0.08)	0.24	2.3	17.00

4	6.63	13.84	2.711	0.33 (0.07)	0.22	2.7	12.00
5	5.99	17.28	2.391	0.33 (0.07)	0.21	2.8	10.00
TOTAL AREA (ACRES) =			2.8				

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE (CFS) = 7.04 Tc (MIN.) = 8.660
EFFECTIVE AREA (ACRES) = 2.21 AREA-AVERAGED Fm (INCH/HR) = 0.08
AREA-AVERAGED Fp (INCH/HR) = 0.33 AREA-AVERAGED Ap = 0.24
TOTAL AREA (ACRES) = 2.8
LONGEST FLOWPATH FROM NODE 10.00 TO NODE 16.00 = 1165.00 FEET.

FLOW PROCESS FROM NODE 16.00 TO NODE 22.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<

=====

ELEVATION DATA: UPSTREAM (FEET) = 151.55 DOWNSTREAM (FEET) = 151.30
FLOW LENGTH (FEET) = 115.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 21.0 INCH PIPE IS 16.8 INCHES
PIPE-FLOW VELOCITY (FEET/SEC.) = 3.41
ESTIMATED PIPE DIAMETER (INCH) = 21.00 NUMBER OF PIPES = 1
PIPE-FLOW (CFS) = 7.04
PIPE TRAVEL TIME (MIN.) = 0.56 Tc (MIN.) = 9.22
LONGEST FLOWPATH FROM NODE 10.00 TO NODE 22.00 = 1280.00 FEET.

FLOW PROCESS FROM NODE 22.00 TO NODE 22.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

=====

MAINLINE Tc (MIN.) = 9.22
* 25 YEAR RAINFALL INTENSITY (INCH/HR) = 3.411
SUBAREA LOSS RATE DATA (AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
CONDOMINIUMS	A	0.31	0.40	0.350	32

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp (INCH/HR) = 0.40
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.350
SUBAREA AREA (ACRES) = 0.31 SUBAREA RUNOFF (CFS) = 0.91
EFFECTIVE AREA (ACRES) = 2.52 AREA-AVERAGED Fm (INCH/HR) = 0.09
AREA-AVERAGED Fp (INCH/HR) = 0.34 AREA-AVERAGED Ap = 0.26
TOTAL AREA (ACRES) = 3.1 PEAK FLOW RATE (CFS) = 7.54

FLOW PROCESS FROM NODE 22.00 TO NODE 22.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

=====

TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION (MIN.) = 9.22
RAINFALL INTENSITY (INCH/HR) = 3.41
AREA-AVERAGED Fm (INCH/HR) = 0.09

AREA-AVERAGED F_p (INCH/HR) = 0.34
 AREA-AVERAGED A_p = 0.26
 EFFECTIVE STREAM AREA (ACRES) = 2.52
 TOTAL STREAM AREA (ACRES) = 3.13
 PEAK FLOW RATE (CFS) AT CONFLUENCE = 7.54

FLOW PROCESS FROM NODE 23.00 TO NODE 22.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
 >>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

INITIAL SUBAREA FLOW-LENGTH (FEET) = 225.00
 ELEVATION DATA: UPSTREAM (FEET) = 166.30 DOWNSTREAM (FEET) = 165.50

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$
 SUBAREA ANALYSIS USED MINIMUM T_c (MIN.) = 9.705
 * 25 YEAR RAINFALL INTENSITY (INCH/HR) = 3.314

SUBAREA T_c AND LOSS RATE DATA (AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	F_p (INCH/HR)	A_p (DECIMAL)	SCS CN	T_c (MIN.)
CONDOMINIUMS	B	0.71	0.30	0.350	56	9.70

SUBAREA AVERAGE PERVIOUS LOSS RATE, F_p (INCH/HR) = 0.30
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, A_p = 0.350
 SUBAREA RUNOFF (CFS) = 2.05
 TOTAL AREA (ACRES) = 0.71 PEAK FLOW RATE (CFS) = 2.05

FLOW PROCESS FROM NODE 22.00 TO NODE 22.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
 >>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<

TOTAL NUMBER OF STREAMS = 2
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
 TIME OF CONCENTRATION (MIN.) = 9.70
 RAINFALL INTENSITY (INCH/HR) = 3.31
 AREA-AVERAGED F_m (INCH/HR) = 0.11
 AREA-AVERAGED F_p (INCH/HR) = 0.30
 AREA-AVERAGED A_p = 0.35
 EFFECTIVE STREAM AREA (ACRES) = 0.71
 TOTAL STREAM AREA (ACRES) = 0.71
 PEAK FLOW RATE (CFS) AT CONFLUENCE = 2.05

** CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	T_c (MIN.)	Intensity (INCH/HR)	F_p (F_m) (INCH/HR)	A_p	A_e (ACRES)	HEADWATER NODE
1	7.26	8.16	3.655	0.35 (0.09)	0.26	2.3	14.00
1	7.54	9.22	3.411	0.34 (0.09)	0.26	2.5	20.00
1	7.52	9.70	3.315	0.34 (0.09)	0.25	2.6	17.00
1	7.06	14.40	2.651	0.34 (0.08)	0.23	3.0	12.00
1	6.40	17.85	2.347	0.34 (0.08)	0.23	3.1	10.00
2	2.05	9.70	3.314	0.30 (0.11)	0.35	0.7	23.00

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
 CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	9.17	8.16	3.655	0.33(0.09)	0.28	2.9	14.00
2	9.55	9.22	3.411	0.33(0.09)	0.28	3.2	20.00
3	9.57	9.70	3.315	0.33(0.09)	0.27	3.3	17.00
4	9.57	9.70	3.314	0.33(0.09)	0.27	3.3	23.00
5	8.69	14.40	2.651	0.33(0.08)	0.25	3.8	12.00
6	7.83	17.85	2.347	0.33(0.08)	0.25	3.8	10.00

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE (CFS) = 9.57 Tc (MIN.) = 9.70
 EFFECTIVE AREA (ACRES) = 3.30 AREA-AVERAGED Fm (INCH/HR) = 0.09
 AREA-AVERAGED Fp (INCH/HR) = 0.33 AREA-AVERAGED Ap = 0.27
 TOTAL AREA (ACRES) = 3.8
 LONGEST FLOWPATH FROM NODE 10.00 TO NODE 22.00 = 1280.00 FEET.

FLOW PROCESS FROM NODE 22.00 TO NODE 22.00 IS CODE = 10

>>>>MAIN-STREAM MEMORY COPIED ONTO MEMORY BANK # 2 <<<<<

FLOW PROCESS FROM NODE 24.00 TO NODE 25.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
 >>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

INITIAL SUBAREA FLOW-LENGTH (FEET) = 270.00
 ELEVATION DATA: UPSTREAM (FEET) = 166.50 DOWNSTREAM (FEET) = 165.50

Tc = K * [(LENGTH** 3.00) / (ELEVATION CHANGE)] ** 0.20
 SUBAREA ANALYSIS USED MINIMUM Tc (MIN.) = 10.354
 * 25 YEAR RAINFALL INTENSITY (INCH/HR) = 3.195
 SUBAREA Tc AND LOSS RATE DATA (AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN	Tc (MIN.)
CONDOMINIUMS	B	0.51	0.30	0.350	56	10.35

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp (INCH/HR) = 0.30
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.350
 SUBAREA RUNOFF (CFS) = 1.42
 TOTAL AREA (ACRES) = 0.51 PEAK FLOW RATE (CFS) = 1.42

FLOW PROCESS FROM NODE 25.00 TO NODE 25.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

TOTAL NUMBER OF STREAMS = 2
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:

TIME OF CONCENTRATION(MIN.) = 10.35
 RAINFALL INTENSITY(INCH/HR) = 3.19
 AREA-AVERAGED Fm(INCH/HR) = 0.11
 AREA-AVERAGED Fp(INCH/HR) = 0.30
 AREA-AVERAGED Ap = 0.35
 EFFECTIVE STREAM AREA(ACRES) = 0.51
 TOTAL STREAM AREA(ACRES) = 0.51
 PEAK FLOW RATE(CFS) AT CONFLUENCE = 1.42

 FLOW PROCESS FROM NODE 26.00 TO NODE 25.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
 >>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

INITIAL SUBAREA FLOW-LENGTH(FEET) = 155.00
 ELEVATION DATA: UPSTREAM(FEET) = 166.50 DOWNSTREAM(FEET) = 165.50

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$
 SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) = 7.422
 * 25 YEAR RAINFALL INTENSITY(INCH/HR) = 3.857

SUBAREA Tc AND LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN	Tc (MIN.)
CONDOMINIUMS	B	0.30	0.30	0.350	56	7.42

 SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.30
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.350
 SUBAREA RUNOFF(CFS) = 1.01
 TOTAL AREA(ACRES) = 0.30 PEAK FLOW RATE(CFS) = 1.01

 FLOW PROCESS FROM NODE 25.00 TO NODE 25.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
 >>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<

TOTAL NUMBER OF STREAMS = 2
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
 TIME OF CONCENTRATION(MIN.) = 7.42
 RAINFALL INTENSITY(INCH/HR) = 3.86
 AREA-AVERAGED Fm(INCH/HR) = 0.10
 AREA-AVERAGED Fp(INCH/HR) = 0.30
 AREA-AVERAGED Ap = 0.35
 EFFECTIVE STREAM AREA(ACRES) = 0.30
 TOTAL STREAM AREA(ACRES) = 0.30
 PEAK FLOW RATE(CFS) AT CONFLUENCE = 1.01

** CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	1.42	10.35	3.195	0.30 (0.11)	0.35	0.5	24.00
2	1.01	7.42	3.857	0.30 (0.10)	0.35	0.3	26.00

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO

CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	2.25	7.42	3.857	0.30 (0.10)	0.35	0.7	26.00
2	2.25	10.35	3.195	0.30 (0.11)	0.35	0.8	24.00

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE (CFS) = 2.25 Tc (MIN.) = 10.35
EFFECTIVE AREA (ACRES) = 0.81 AREA-AVERAGED Fm (INCH/HR) = 0.11
AREA-AVERAGED Fp (INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.35
TOTAL AREA (ACRES) = 0.8
LONGEST FLOWPATH FROM NODE 24.00 TO NODE 25.00 = 270.00 FEET.

FLOW PROCESS FROM NODE 25.00 TO NODE 28.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<

=====

ELEVATION DATA: UPSTREAM (FEET) = 161.00 DOWNSTREAM (FEET) = 158.00
FLOW LENGTH (FEET) = 175.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 12.0 INCH PIPE IS 6.0 INCHES
PIPE-FLOW VELOCITY (FEET/SEC.) = 5.77
ESTIMATED PIPE DIAMETER (INCH) = 12.00 NUMBER OF PIPES = 1
PIPE-FLOW (CFS) = 2.25
PIPE TRAVEL TIME (MIN.) = 0.51 Tc (MIN.) = 10.86
LONGEST FLOWPATH FROM NODE 24.00 TO NODE 28.00 = 445.00 FEET.

FLOW PROCESS FROM NODE 28.00 TO NODE 28.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

=====

TOTAL NUMBER OF STREAMS = 3
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION (MIN.) = 10.86
RAINFALL INTENSITY (INCH/HR) = 3.11
AREA-AVERAGED Fm (INCH/HR) = 0.11
AREA-AVERAGED Fp (INCH/HR) = 0.30
AREA-AVERAGED Ap = 0.35
EFFECTIVE STREAM AREA (ACRES) = 0.81
TOTAL STREAM AREA (ACRES) = 0.81
PEAK FLOW RATE (CFS) AT CONFLUENCE = 2.25

FLOW PROCESS FROM NODE 27.00 TO NODE 28.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

=====

INITIAL SUBAREA FLOW-LENGTH (FEET) = 180.00
ELEVATION DATA: UPSTREAM (FEET) = 166.50 DOWNSTREAM (FEET) = 165.40

Tc = K*[(LENGTH** 3.00)/(ELEVATION CHANGE)]**0.20

SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) = 7.965

* 25 YEAR RAINFALL INTENSITY(INCH/HR) = 3.706

SUBAREA Tc AND LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN	Tc (MIN.)
CONDOMINIUMS	B	0.18	0.30	0.350	56	7.97

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.30

SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.350

SUBAREA RUNOFF(CFS) = 0.58

TOTAL AREA(ACRES) = 0.18 PEAK FLOW RATE(CFS) = 0.58

FLOW PROCESS FROM NODE 28.00 TO NODE 28.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

TOTAL NUMBER OF STREAMS = 3

CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:

TIME OF CONCENTRATION(MIN.) = 7.97

RAINFALL INTENSITY(INCH/HR) = 3.71

AREA-AVERAGED Fm(INCH/HR) = 0.11

AREA-AVERAGED Fp(INCH/HR) = 0.30

AREA-AVERAGED Ap = 0.35

EFFECTIVE STREAM AREA(ACRES) = 0.18

TOTAL STREAM AREA(ACRES) = 0.18

PEAK FLOW RATE(CFS) AT CONFLUENCE = 0.58

FLOW PROCESS FROM NODE 29.00 TO NODE 28.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<

>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

INITIAL SUBAREA FLOW-LENGTH(FEET) = 230.00

ELEVATION DATA: UPSTREAM(FEET) = 166.50 DOWNSTREAM(FEET) = 165.40

Tc = K*[(LENGTH** 3.00)/(ELEVATION CHANGE)]**0.20

SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) = 9.227

* 25 YEAR RAINFALL INTENSITY(INCH/HR) = 3.410

SUBAREA Tc AND LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN	Tc (MIN.)
CONDOMINIUMS	B	0.48	0.30	0.350	56	9.23

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.30

SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.350

SUBAREA RUNOFF(CFS) = 1.43

TOTAL AREA(ACRES) = 0.48 PEAK FLOW RATE(CFS) = 1.43

FLOW PROCESS FROM NODE 28.00 TO NODE 28.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

>>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<

=====

TOTAL NUMBER OF STREAMS = 3
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 3 ARE:
TIME OF CONCENTRATION(MIN.) = 9.23
RAINFALL INTENSITY(INCH/HR) = 3.41
AREA-AVERAGED Fm(INCH/HR) = 0.11
AREA-AVERAGED Fp(INCH/HR) = 0.30
AREA-AVERAGED Ap = 0.35
EFFECTIVE STREAM AREA(ACRES) = 0.48
TOTAL STREAM AREA(ACRES) = 0.48
PEAK FLOW RATE(CFS) AT CONFLUENCE = 1.43

** CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	2.25	7.93	3.716	0.30(0.10)	0.35	0.7	26.00
1	2.25	10.86	3.110	0.30(0.11)	0.35	0.8	24.00
2	0.58	7.97	3.706	0.30(0.11)	0.35	0.2	27.00
3	1.43	9.23	3.410	0.30(0.11)	0.35	0.5	29.00

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
CONFLUENCE FORMULA USED FOR 3 STREAMS.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	4.17	7.93	3.716	0.30(0.10)	0.35	1.3	26.00
2	4.17	7.97	3.706	0.30(0.11)	0.35	1.3	27.00
3	4.22	9.23	3.410	0.30(0.11)	0.35	1.4	29.00
4	4.04	10.86	3.110	0.30(0.11)	0.35	1.5	24.00

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE(CFS) = 4.22 Tc(MIN.) = 9.23
EFFECTIVE AREA(ACRES) = 1.39 AREA-AVERAGED Fm(INCH/HR) = 0.10
AREA-AVERAGED Fp(INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.35
TOTAL AREA(ACRES) = 1.5
LONGEST FLOWPATH FROM NODE 24.00 TO NODE 28.00 = 445.00 FEET.

FLOW PROCESS FROM NODE 28.00 TO NODE 22.00 IS CODE = 31

>>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<

>>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 158.00 DOWNSTREAM(FEET) = 151.30
FLOW LENGTH(FEET) = 250.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 12.0 INCH PIPE IS 7.7 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 7.93
ESTIMATED PIPE DIAMETER(INCH) = 12.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 4.22
PIPE TRAVEL TIME(MIN.) = 0.53 Tc(MIN.) = 9.75
LONGEST FLOWPATH FROM NODE 24.00 TO NODE 22.00 = 695.00 FEET.

FLOW PROCESS FROM NODE 22.00 TO NODE 22.00 IS CODE = 11

>>>>CONFLUENCE MEMORY BANK # 2 WITH THE MAIN-STREAM MEMORY<<<<<

** MAIN STREAM CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	4.17	8.45	3.584	0.30(0.10)	0.35	1.3	26.00
2	4.17	8.49	3.574	0.30(0.11)	0.35	1.3	27.00
3	4.22	9.75	3.305	0.30(0.11)	0.35	1.4	29.00
4	4.04	11.39	3.027	0.30(0.11)	0.35	1.5	24.00
LONGEST FLOWPATH FROM NODE				24.00 TO NODE	22.00 =	695.00 FEET.	

** MEMORY BANK # 2 CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	9.17	8.16	3.655	0.33(0.09)	0.28	2.9	14.00
2	9.55	9.22	3.411	0.33(0.09)	0.28	3.2	20.00
3	9.57	9.70	3.315	0.33(0.09)	0.27	3.3	17.00
4	9.57	9.70	3.314	0.33(0.09)	0.27	3.3	23.00
5	8.69	14.40	2.651	0.33(0.08)	0.25	3.8	12.00
6	7.83	17.85	2.347	0.33(0.08)	0.25	3.8	10.00
LONGEST FLOWPATH FROM NODE				10.00 TO NODE	22.00 =	1280.00 FEET.	

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	13.28	8.16	3.655	0.32(0.10)	0.30	4.1	14.00
2	13.44	8.45	3.584	0.32(0.10)	0.30	4.2	26.00
3	13.46	8.49	3.574	0.32(0.10)	0.30	4.2	27.00
4	13.75	9.22	3.411	0.32(0.10)	0.30	4.5	20.00
5	13.78	9.70	3.315	0.32(0.10)	0.30	4.7	17.00
6	13.78	9.70	3.314	0.32(0.10)	0.30	4.7	23.00
7	13.78	9.75	3.305	0.32(0.10)	0.30	4.7	29.00
8	13.29	11.39	3.027	0.32(0.09)	0.29	4.9	24.00
9	12.21	14.40	2.651	0.32(0.09)	0.28	5.2	12.00
10	10.93	17.85	2.347	0.32(0.09)	0.28	5.3	10.00
TOTAL AREA (ACRES) =				5.3			

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE (CFS) = 13.78 Tc (MIN.) = 9.705
EFFECTIVE AREA (ACRES) = 4.68 AREA-AVERAGED Fm (INCH/HR) = 0.10
AREA-AVERAGED Fp (INCH/HR) = 0.32 AREA-AVERAGED Ap = 0.29
TOTAL AREA (ACRES) = 5.3
LONGEST FLOWPATH FROM NODE 10.00 TO NODE 22.00 = 1280.00 FEET.

FLOW PROCESS FROM NODE 22.00 TO NODE 33.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<

>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 151.30 DOWNSTREAM(FEET) = 150.50
 FLOW LENGTH(FEET) = 425.00 MANNING'S N = 0.013
 DEPTH OF FLOW IN 30.0 INCH PIPE IS 20.2 INCHES
 PIPE-FLOW VELOCITY(FEET/SEC.) = 3.92
 ESTIMATED PIPE DIAMETER(INCH) = 30.00 NUMBER OF PIPES = 1
 PIPE-FLOW(CFS) = 13.78
 PIPE TRAVEL TIME(MIN.) = 1.81 Tc(MIN.) = 11.51
 LONGEST FLOWPATH FROM NODE 10.00 TO NODE 33.00 = 1705.00 FEET.

 FLOW PROCESS FROM NODE 33.00 TO NODE 33.00 IS CODE = 10

 >>>>MAIN-STREAM MEMORY COPIED ONTO MEMORY BANK # 3 <<<<<
 =====

 FLOW PROCESS FROM NODE 30.00 TO NODE 31.00 IS CODE = 21

 >>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
 >>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<
 =====

INITIAL SUBAREA FLOW-LENGTH(FEET) = 150.00
 ELEVATION DATA: UPSTREAM(FEET) = 166.50 DOWNSTREAM(FEET) = 165.50

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$
 SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) = 7.277
 * 25 YEAR RAINFALL INTENSITY(INCH/HR) = 3.901

SUBAREA Tc AND LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN	Tc (MIN.)
CONDOMINIUMS	B	0.29	0.30	0.350	56	7.28

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.30

SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.350

SUBAREA RUNOFF(CFS) = 0.99

TOTAL AREA(ACRES) = 0.29 PEAK FLOW RATE(CFS) = 0.99

 FLOW PROCESS FROM NODE 31.00 TO NODE 31.00 IS CODE = 81

 >>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<
 =====

MAINLINE Tc(MIN.) = 7.28

* 25 YEAR RAINFALL INTENSITY(INCH/HR) = 3.901

SUBAREA LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
CONDOMINIUMS	B	0.65	0.30	0.350	56

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.30

SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.350

SUBAREA AREA(ACRES) = 0.65 SUBAREA RUNOFF(CFS) = 2.22

EFFECTIVE AREA(ACRES) = 0.94 AREA-AVERAGED Fm(INCH/HR) = 0.11

AREA-AVERAGED Fp(INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.35

TOTAL AREA(ACRES) = 0.9 PEAK FLOW RATE(CFS) = 3.21

FLOW PROCESS FROM NODE 31.00 TO NODE 33.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<

ELEVATION DATA: UPSTREAM(FEET) = 157.00 DOWNSTREAM(FEET) = 150.50
FLOW LENGTH(FEET) = 380.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 12.0 INCH PIPE IS 7.4 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 6.28
ESTIMATED PIPE DIAMETER(INCH) = 12.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 3.21
PIPE TRAVEL TIME(MIN.) = 1.01 Tc(MIN.) = 8.29
LONGEST FLOWPATH FROM NODE 30.00 TO NODE 33.00 = 530.00 FEET.

FLOW PROCESS FROM NODE 33.00 TO NODE 33.00 IS CODE = 11

>>>>CONFLUENCE MEMORY BANK # 3 WITH THE MAIN-STREAM MEMORY<<<<<

** MAIN STREAM CONFLUENCE DATA **

STREAM Q Tc Intensity Fp(Fm) Ap Ae HEADWATER
NUMBER (CFS) (MIN.) (INCH/HR) (INCH/HR) (ACRES) NODE
1 3.21 8.29 3.624 0.30(0.11) 0.35 0.9 30.00
LONGEST FLOWPATH FROM NODE 30.00 TO NODE 33.00 = 530.00 FEET.

** MEMORY BANK # 3 CONFLUENCE DATA **

STREAM Q Tc Intensity Fp(Fm) Ap Ae HEADWATER
NUMBER (CFS) (MIN.) (INCH/HR) (INCH/HR) (ACRES) NODE
1 13.28 9.98 3.262 0.32(0.10) 0.30 4.1 14.00
2 13.44 10.27 3.210 0.32(0.10) 0.30 4.2 26.00
3 13.46 10.31 3.203 0.32(0.10) 0.30 4.2 27.00
4 13.75 11.03 3.083 0.32(0.10) 0.30 4.5 20.00
5 13.78 11.50 3.010 0.32(0.10) 0.30 4.7 17.00
6 13.78 11.51 3.009 0.32(0.10) 0.30 4.7 23.00
7 13.78 11.56 3.002 0.32(0.10) 0.30 4.7 29.00
8 13.29 13.21 2.784 0.32(0.09) 0.29 4.9 24.00
9 12.21 16.29 2.472 0.32(0.09) 0.28 5.2 12.00
10 10.93 19.77 2.215 0.32(0.09) 0.28 5.3 10.00
LONGEST FLOWPATH FROM NODE 10.00 TO NODE 33.00 = 1705.00 FEET.

** PEAK FLOW RATE TABLE **

STREAM Q Tc Intensity Fp(Fm) Ap Ae HEADWATER
NUMBER (CFS) (MIN.) (INCH/HR) (INCH/HR) (ACRES) NODE
1 15.50 8.29 3.624 0.32(0.10) 0.31 4.3 30.00
2 16.16 9.98 3.262 0.32(0.10) 0.31 5.0 14.00
3 16.28 10.27 3.210 0.32(0.10) 0.31 5.1 26.00
4 16.29 10.31 3.203 0.32(0.10) 0.31 5.2 27.00
5 16.46 11.03 3.083 0.32(0.10) 0.31 5.5 20.00
6 16.43 11.50 3.010 0.32(0.10) 0.31 5.6 17.00
7 16.43 11.51 3.009 0.32(0.10) 0.31 5.6 23.00
8 16.42 11.56 3.002 0.32(0.10) 0.31 5.6 29.00
9 15.73 13.21 2.784 0.32(0.10) 0.30 5.9 24.00

10	14.36	16.29	2.472	0.32 (0.09)	0.29	6.2	12.00
11	12.85	19.77	2.215	0.32 (0.09)	0.29	6.2	10.00
TOTAL AREA (ACRES) =			6.2				

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE (CFS) = 16.46 Tc (MIN.) = 11.030
EFFECTIVE AREA (ACRES) = 5.47 AREA-AVERAGED Fm (INCH/HR) = 0.10
AREA-AVERAGED Fp (INCH/HR) = 0.32 AREA-AVERAGED Ap = 0.31
TOTAL AREA (ACRES) = 6.2
LONGEST FLOWPATH FROM NODE 10.00 TO NODE 33.00 = 1705.00 FEET.

FLOW PROCESS FROM NODE 16.00 TO NODE 16.00 IS CODE = 12

>>>>CLEAR MEMORY BANK # 1 <<<<<

FLOW PROCESS FROM NODE 22.00 TO NODE 22.00 IS CODE = 12

>>>>CLEAR MEMORY BANK # 2 <<<<<

FLOW PROCESS FROM NODE 33.00 TO NODE 33.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

TOTAL NUMBER OF STREAMS = 5
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION (MIN.) = 11.03
RAINFALL INTENSITY (INCH/HR) = 3.08
AREA-AVERAGED Fm (INCH/HR) = 0.10
AREA-AVERAGED Fp (INCH/HR) = 0.32
AREA-AVERAGED Ap = 0.31
EFFECTIVE STREAM AREA (ACRES) = 5.47
TOTAL STREAM AREA (ACRES) = 6.25
PEAK FLOW RATE (CFS) AT CONFLUENCE = 16.46

FLOW PROCESS FROM NODE 30.00 TO NODE 33.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

INITIAL SUBAREA FLOW-LENGTH (FEET) = 185.00
ELEVATION DATA: UPSTREAM (FEET) = 166.60 DOWNSTREAM (FEET) = 166.00

$$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$$

SUBAREA ANALYSIS USED MINIMUM Tc (MIN.) = 9.141
* 25 YEAR RAINFALL INTENSITY (INCH/HR) = 3.428

SUBAREA Tc AND LOSS RATE DATA (AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN	Tc (MIN.)
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CONDOMINIUMS B 0.67 0.30 0.350 56 9.14
 SUBAREA AVERAGE PERVIOUS LOSS RATE, F_p (INCH/HR) = 0.30
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, A_p = 0.350
 SUBAREA RUNOFF (CFS) = 2.00
 TOTAL AREA (ACRES) = 0.67 PEAK FLOW RATE (CFS) = 2.00

FLOW PROCESS FROM NODE 33.00 TO NODE 33.00 IS CODE = 1

 >>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

=====

TOTAL NUMBER OF STREAMS = 5
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
 TIME OF CONCENTRATION (MIN.) = 9.14
 RAINFALL INTENSITY (INCH/HR) = 3.43
 AREA-AVERAGED F_m (INCH/HR) = 0.11
 AREA-AVERAGED F_p (INCH/HR) = 0.30
 AREA-AVERAGED A_p = 0.35
 EFFECTIVE STREAM AREA (ACRES) = 0.67
 TOTAL STREAM AREA (ACRES) = 0.67
 PEAK FLOW RATE (CFS) AT CONFLUENCE = 2.00

FLOW PROCESS FROM NODE 34.00 TO NODE 35.00 IS CODE = 21

 >>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
 >>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

=====

INITIAL SUBAREA FLOW-LENGTH (FEET) = 180.00
 ELEVATION DATA: UPSTREAM (FEET) = 166.20 DOWNSTREAM (FEET) = 166.10

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$
 SUBAREA ANALYSIS USED MINIMUM T_c (MIN.) = 12.867
 * 25 YEAR RAINFALL INTENSITY (INCH/HR) = 2.825
 SUBAREA T_c AND LOSS RATE DATA (AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	F_p (INCH/HR)	A_p (DECIMAL)	SCS CN	T_c (MIN.)
CONDOMINIUMS	B	0.50	0.30	0.350	56	12.87

SUBAREA AVERAGE PERVIOUS LOSS RATE, F_p (INCH/HR) = 0.30
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, A_p = 0.350
 SUBAREA RUNOFF (CFS) = 1.22
 TOTAL AREA (ACRES) = 0.50 PEAK FLOW RATE (CFS) = 1.22

FLOW PROCESS FROM NODE 33.00 TO NODE 33.00 IS CODE = 1

 >>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

=====

TOTAL NUMBER OF STREAMS = 5
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 3 ARE:
 TIME OF CONCENTRATION (MIN.) = 12.87
 RAINFALL INTENSITY (INCH/HR) = 2.83
 AREA-AVERAGED F_m (INCH/HR) = 0.11
 AREA-AVERAGED F_p (INCH/HR) = 0.30

AREA-AVERAGED $A_p = 0.35$
EFFECTIVE STREAM AREA (ACRES) = 0.50
TOTAL STREAM AREA (ACRES) = 0.50
PEAK FLOW RATE (CFS) AT CONFLUENCE = 1.22

FLOW PROCESS FROM NODE 36.00 TO NODE 37.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<
=====

INITIAL SUBAREA FLOW-LENGTH (FEET) = 150.00
ELEVATION DATA: UPSTREAM (FEET) = 166.70 DOWNSTREAM (FEET) = 166.40

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$
SUBAREA ANALYSIS USED MINIMUM T_c (MIN.) = 9.258
* 25 YEAR RAINFALL INTENSITY (INCH/HR) = 3.404

SUBAREA T_c AND LOSS RATE DATA (AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	F_p (INCH/HR)	A_p (DECIMAL)	SCS CN	T_c (MIN.)
CONDOMINIUMS	B	0.29	0.30	0.350	56	9.26

SUBAREA AVERAGE PERVIOUS LOSS RATE, F_p (INCH/HR) = 0.30

SUBAREA AVERAGE PERVIOUS AREA FRACTION, $A_p = 0.350$

SUBAREA RUNOFF (CFS) = 0.86

TOTAL AREA (ACRES) = 0.29 PEAK FLOW RATE (CFS) = 0.86

FLOW PROCESS FROM NODE 33.00 TO NODE 33.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
=====

TOTAL NUMBER OF STREAMS = 5
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 4 ARE:

TIME OF CONCENTRATION (MIN.) = 9.26

RAINFALL INTENSITY (INCH/HR) = 3.40

AREA-AVERAGED F_m (INCH/HR) = 0.11

AREA-AVERAGED F_p (INCH/HR) = 0.30

AREA-AVERAGED $A_p = 0.35$

EFFECTIVE STREAM AREA (ACRES) = 0.29

TOTAL STREAM AREA (ACRES) = 0.29

PEAK FLOW RATE (CFS) AT CONFLUENCE = 0.86

FLOW PROCESS FROM NODE 38.00 TO NODE 33.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<
=====

INITIAL SUBAREA FLOW-LENGTH (FEET) = 150.00
ELEVATION DATA: UPSTREAM (FEET) = 166.40 DOWNSTREAM (FEET) = 166.00

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$

SUBAREA ANALYSIS USED MINIMUM T_c (MIN.) = 8.741

* 25 YEAR RAINFALL INTENSITY (INCH/HR) = 3.516

SUBAREA Tc AND LOSS RATE DATA (AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN	Tc (MIN.)
CONDOMINIUMS	B	0.18	0.30	0.350	56	8.74

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp (INCH/HR) = 0.30
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.350
 SUBAREA RUNOFF (CFS) = 0.55
 TOTAL AREA (ACRES) = 0.18 PEAK FLOW RATE (CFS) = 0.55

FLOW PROCESS FROM NODE 33.00 TO NODE 33.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
 >>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<

=====

TOTAL NUMBER OF STREAMS = 5
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 5 ARE:
 TIME OF CONCENTRATION (MIN.) = 8.74
 RAINFALL INTENSITY (INCH/HR) = 3.52
 AREA-AVERAGED Fm (INCH/HR) = 0.11
 AREA-AVERAGED Fp (INCH/HR) = 0.30
 AREA-AVERAGED Ap = 0.35
 EFFECTIVE STREAM AREA (ACRES) = 0.18
 TOTAL STREAM AREA (ACRES) = 0.18
 PEAK FLOW RATE (CFS) AT CONFLUENCE = 0.55

** CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	15.50	8.29	3.624	0.32 (0.10)	0.31	4.3	30.00
1	16.16	9.98	3.262	0.32 (0.10)	0.31	5.0	14.00
1	16.28	10.27	3.210	0.32 (0.10)	0.31	5.1	26.00
1	16.29	10.31	3.203	0.32 (0.10)	0.31	5.2	27.00
1	16.46	11.03	3.083	0.32 (0.10)	0.31	5.5	20.00
1	16.43	11.50	3.010	0.32 (0.10)	0.31	5.6	17.00
1	16.43	11.51	3.009	0.32 (0.10)	0.31	5.6	23.00
1	16.42	11.56	3.002	0.32 (0.10)	0.31	5.6	29.00
1	15.73	13.21	2.784	0.32 (0.10)	0.30	5.9	24.00
1	14.36	16.29	2.472	0.32 (0.09)	0.29	6.2	12.00
1	12.85	19.77	2.215	0.32 (0.09)	0.29	6.2	10.00
2	2.00	9.14	3.428	0.30 (0.11)	0.35	0.7	30.00
3	1.22	12.87	2.825	0.30 (0.11)	0.35	0.5	34.00
4	0.86	9.26	3.404	0.30 (0.11)	0.35	0.3	36.00
5	0.55	8.74	3.516	0.30 (0.11)	0.35	0.2	38.00

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
 CONFLUENCE FORMULA USED FOR 5 STREAMS.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	19.80	8.29	3.624	0.31 (0.10)	0.32	5.7	30.00
2	20.08	8.74	3.516	0.31 (0.10)	0.32	5.9	38.00
3	20.29	9.14	3.428	0.31 (0.10)	0.32	6.2	30.00

4	20.33	9.26	3.404	0.31	(0.10)	0.32	6.2	36.00
5	20.50	9.98	3.262	0.31	(0.10)	0.32	6.5	14.00
6	20.58	10.27	3.210	0.31	(0.10)	0.32	6.7	26.00
7	20.58	10.31	3.203	0.31	(0.10)	0.32	6.7	27.00
8	20.67	11.03	3.083	0.31	(0.10)	0.32	7.0	20.00
9	20.58	11.50	3.010	0.31	(0.10)	0.32	7.2	17.00
10	20.58	11.51	3.009	0.31	(0.10)	0.32	7.2	23.00
11	20.56	11.56	3.002	0.31	(0.10)	0.32	7.2	29.00
12	19.89	12.87	2.825	0.31	(0.10)	0.31	7.5	34.00
13	19.69	13.21	2.784	0.31	(0.10)	0.31	7.5	24.00
14	17.86	16.29	2.472	0.31	(0.09)	0.30	7.8	12.00
15	15.97	19.77	2.215	0.31	(0.09)	0.30	7.9	10.00

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE (CFS) = 20.67 Tc (MIN.) = 11.03
EFFECTIVE AREA (ACRES) = 7.04 AREA-AVERAGED Fm (INCH/HR) = 0.10
AREA-AVERAGED Fp (INCH/HR) = 0.31 AREA-AVERAGED Ap = 0.32
TOTAL AREA (ACRES) = 7.9
LONGEST FLOWPATH FROM NODE 10.00 TO NODE 33.00 = 1705.00 FEET.

FLOW PROCESS FROM NODE 33.00 TO NODE 38.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<

=====

ELEVATION DATA: UPSTREAM (FEET) = 150.50 DOWNSTREAM (FEET) = 150.20
FLOW LENGTH (FEET) = 150.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 33.0 INCH PIPE IS 24.4 INCHES
PIPE-FLOW VELOCITY (FEET/SEC.) = 4.39
ESTIMATED PIPE DIAMETER (INCH) = 33.00 NUMBER OF PIPES = 1
PIPE-FLOW (CFS) = 20.67
PIPE TRAVEL TIME (MIN.) = 0.57 Tc (MIN.) = 11.60
LONGEST FLOWPATH FROM NODE 10.00 TO NODE 38.00 = 1855.00 FEET.

FLOW PROCESS FROM NODE 38.00 TO NODE 38.00 IS CODE = 10

>>>>MAIN-STREAM MEMORY COPIED ONTO MEMORY BANK # 1 <<<<<

FLOW PROCESS FROM NODE 39.00 TO NODE 40.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

=====

INITIAL SUBAREA FLOW-LENGTH (FEET) = 300.00
ELEVATION DATA: UPSTREAM (FEET) = 168.00 DOWNSTREAM (FEET) = 166.90

Tc = K * [(LENGTH** 3.00) / (ELEVATION CHANGE)]**0.20
SUBAREA ANALYSIS USED MINIMUM Tc (MIN.) = 10.822
* 25 YEAR RAINFALL INTENSITY (INCH/HR) = 3.116
SUBAREA Tc AND LOSS RATE DATA (AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN	Tc (MIN.)
CONDOMINIUMS	B	0.80	0.30	0.350	56	10.82

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp (INCH/HR) = 0.30
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.350
 SUBAREA RUNOFF (CFS) = 2.17
 TOTAL AREA (ACRES) = 0.80 PEAK FLOW RATE (CFS) = 2.17

 FLOW PROCESS FROM NODE 40.00 TO NODE 41.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
 >>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<

=====

ELEVATION DATA: UPSTREAM (FEET) = 154.00 DOWNSTREAM (FEET) = 151.10
 FLOW LENGTH (FEET) = 40.00 MANNING'S N = 0.013
 ESTIMATED PIPE DIAMETER (INCH) INCREASED TO 12.000
 DEPTH OF FLOW IN 12.0 INCH PIPE IS 3.9 INCHES
 PIPE-FLOW VELOCITY (FEET/SEC.) = 9.69
 ESTIMATED PIPE DIAMETER (INCH) = 12.00 NUMBER OF PIPES = 1
 PIPE-FLOW (CFS) = 2.17
 PIPE TRAVEL TIME (MIN.) = 0.07 Tc (MIN.) = 10.89
 LONGEST FLOWPATH FROM NODE 39.00 TO NODE 41.00 = 340.00 FEET.

 FLOW PROCESS FROM NODE 41.00 TO NODE 41.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

=====

MAINLINE Tc (MIN.) = 10.89
 * 25 YEAR RAINFALL INTENSITY (INCH/HR) = 3.105
 SUBAREA LOSS RATE DATA (AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
CONDOMINIUMS	B	0.45	0.30	0.350	56

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp (INCH/HR) = 0.30
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.350
 SUBAREA AREA (ACRES) = 0.45 SUBAREA RUNOFF (CFS) = 1.21
 EFFECTIVE AREA (ACRES) = 1.25 AREA-AVERAGED Fm (INCH/HR) = 0.10
 AREA-AVERAGED Fp (INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.35
 TOTAL AREA (ACRES) = 1.2 PEAK FLOW RATE (CFS) = 3.37

 FLOW PROCESS FROM NODE 41.00 TO NODE 42.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
 >>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<

=====

ELEVATION DATA: UPSTREAM (FEET) = 151.10 DOWNSTREAM (FEET) = 150.60
 FLOW LENGTH (FEET) = 270.00 MANNING'S N = 0.013
 DEPTH OF FLOW IN 18.0 INCH PIPE IS 11.8 INCHES
 PIPE-FLOW VELOCITY (FEET/SEC.) = 2.75
 ESTIMATED PIPE DIAMETER (INCH) = 18.00 NUMBER OF PIPES = 1
 PIPE-FLOW (CFS) = 3.37

PIPE TRAVEL TIME(MIN.) = 1.64 Tc(MIN.) = 12.53
LONGEST FLOWPATH FROM NODE 39.00 TO NODE 42.00 = 610.00 FEET.

FLOW PROCESS FROM NODE 42.00 TO NODE 42.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

=====

MAINLINE Tc(MIN.) = 12.53
* 25 YEAR RAINFALL INTENSITY(INCH/HR) = 2.868
SUBAREA LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
CONDOMINIUMS	B	1.15	0.30	0.350	56

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.30
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.350
SUBAREA AREA(ACRES) = 1.15 SUBAREA RUNOFF(CFS) = 2.86
EFFECTIVE AREA(ACRES) = 2.40 AREA-AVERAGED Fm(INCH/HR) = 0.10
AREA-AVERAGED Fp(INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.35
TOTAL AREA(ACRES) = 2.4 PEAK FLOW RATE(CFS) = 5.97

FLOW PROCESS FROM NODE 42.00 TO NODE 42.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

=====

TOTAL NUMBER OF STREAMS = 3
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION(MIN.) = 12.53
RAINFALL INTENSITY(INCH/HR) = 2.87
AREA-AVERAGED Fm(INCH/HR) = 0.10
AREA-AVERAGED Fp(INCH/HR) = 0.30
AREA-AVERAGED Ap = 0.35
EFFECTIVE STREAM AREA(ACRES) = 2.40
TOTAL STREAM AREA(ACRES) = 2.40
PEAK FLOW RATE(CFS) AT CONFLUENCE = 5.97

FLOW PROCESS FROM NODE 43.00 TO NODE 44.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

=====

INITIAL SUBAREA FLOW-LENGTH(FEET) = 180.00
ELEVATION DATA: UPSTREAM(FEET) = 168.00 DOWNSTREAM(FEET) = 166.20

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$
SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) = 7.218
* 25 YEAR RAINFALL INTENSITY(INCH/HR) = 3.919
SUBAREA Tc AND LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN	Tc (MIN.)
CONDOMINIUMS	B	0.44	0.30	0.350	56	7.22

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.30

SUBAREA AVERAGE PERVIOUS AREA FRACTION, $A_p = 0.350$
SUBAREA RUNOFF (CFS) = 1.51
TOTAL AREA (ACRES) = 0.44 PEAK FLOW RATE (CFS) = 1.51

FLOW PROCESS FROM NODE 44.00 TO NODE 42.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<

=====

ELEVATION DATA: UPSTREAM (FEET) = 153.00 DOWNSTREAM (FEET) = 150.60
FLOW LENGTH (FEET) = 75.00 MANNING'S N = 0.013
ESTIMATED PIPE DIAMETER (INCH) INCREASED TO 12.000
DEPTH OF FLOW IN 12.0 INCH PIPE IS 4.0 INCHES
PIPE-FLOW VELOCITY (FEET/SEC.) = 6.52
ESTIMATED PIPE DIAMETER (INCH) = 12.000 NUMBER OF PIPES = 1
PIPE-FLOW (CFS) = 1.51
PIPE TRAVEL TIME (MIN.) = 0.19 T_c (MIN.) = 7.41
LONGEST FLOWPATH FROM NODE 43.00 TO NODE 42.00 = 255.00 FEET.

FLOW PROCESS FROM NODE 42.00 TO NODE 42.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

=====

TOTAL NUMBER OF STREAMS = 3
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
TIME OF CONCENTRATION (MIN.) = 7.41
RAINFALL INTENSITY (INCH/HR) = 3.86
AREA-AVERAGED F_m (INCH/HR) = 0.10
AREA-AVERAGED F_p (INCH/HR) = 0.30
AREA-AVERAGED $A_p = 0.35$
EFFECTIVE STREAM AREA (ACRES) = 0.44
TOTAL STREAM AREA (ACRES) = 0.44
PEAK FLOW RATE (CFS) AT CONFLUENCE = 1.51

FLOW PROCESS FROM NODE 45.00 TO NODE 46.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

=====

INITIAL SUBAREA FLOW-LENGTH (FEET) = 180.00
ELEVATION DATA: UPSTREAM (FEET) = 168.00 DOWNSTREAM (FEET) = 166.40

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$

SUBAREA ANALYSIS USED MINIMUM T_c (MIN.) = 7.390

* 25 YEAR RAINFALL INTENSITY (INCH/HR) = 3.867

SUBAREA T_c AND LOSS RATE DATA (AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	F_p (INCH/HR)	A_p (DECIMAL)	SCS CN	T_c (MIN.)
CONDOMINIUMS	B	0.45	0.30	0.350	56	7.39

SUBAREA AVERAGE PERVIOUS LOSS RATE, F_p (INCH/HR) = 0.30

SUBAREA AVERAGE PERVIOUS AREA FRACTION, $A_p = 0.350$

SUBAREA RUNOFF(CFS) = 1.52
 TOTAL AREA(ACRES) = 0.45 PEAK FLOW RATE(CFS) = 1.52

FLOW PROCESS FROM NODE 46.00 TO NODE 42.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
 >>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 153.00 DOWNSTREAM(FEET) = 150.60
 FLOW LENGTH(FEET) = 75.00 MANNING'S N = 0.013
 ESTIMATED PIPE DIAMETER(INCH) INCREASED TO 12.000
 DEPTH OF FLOW IN 12.0 INCH PIPE IS 4.0 INCHES
 PIPE-FLOW VELOCITY(FEET/SEC.) = 6.55
 ESTIMATED PIPE DIAMETER(INCH) = 12.000 NUMBER OF PIPES = 1
 PIPE-FLOW(CFS) = 1.52
 PIPE TRAVEL TIME(MIN.) = 0.19 Tc(MIN.) = 7.58
 LONGEST FLOWPATH FROM NODE 45.00 TO NODE 42.00 = 255.00 FEET.

FLOW PROCESS FROM NODE 42.00 TO NODE 42.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
 >>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<

=====

TOTAL NUMBER OF STREAMS = 3
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 3 ARE:
 TIME OF CONCENTRATION(MIN.) = 7.58
 RAINFALL INTENSITY(INCH/HR) = 3.81
 AREA-AVERAGED Fm(INCH/HR) = 0.10
 AREA-AVERAGED Fp(INCH/HR) = 0.30
 AREA-AVERAGED Ap = 0.35
 EFFECTIVE STREAM AREA(ACRES) = 0.45
 TOTAL STREAM AREA(ACRES) = 0.45
 PEAK FLOW RATE(CFS) AT CONFLUENCE = 1.52

** CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	5.97	12.53	2.868	0.30 (0.10)	0.35	2.4	39.00
2	1.51	7.41	3.861	0.30 (0.11)	0.35	0.4	43.00
3	1.52	7.58	3.811	0.30 (0.10)	0.35	0.4	45.00

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
 CONFLUENCE FORMULA USED FOR 3 STREAMS.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	7.82	7.41	3.861	0.30 (0.10)	0.35	2.3	43.00
2	7.86	7.58	3.811	0.30 (0.10)	0.35	2.3	45.00
3	8.21	12.53	2.868	0.30 (0.10)	0.35	3.3	39.00

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE (CFS) = 8.21 Tc (MIN.) = 12.53
 EFFECTIVE AREA (ACRES) = 3.29 AREA-AVERAGED Fm (INCH/HR) = 0.10
 AREA-AVERAGED Fp (INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.35
 TOTAL AREA (ACRES) = 3.3
 LONGEST FLOWPATH FROM NODE 39.00 TO NODE 42.00 = 610.00 FEET.

 FLOW PROCESS FROM NODE 42.00 TO NODE 38.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
 >>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<

=====

ELEVATION DATA: UPSTREAM (FEET) = 150.60 DOWNSTREAM (FEET) = 150.20
 FLOW LENGTH (FEET) = 190.00 MANNING'S N = 0.013
 DEPTH OF FLOW IN 24.0 INCH PIPE IS 16.4 INCHES
 PIPE-FLOW VELOCITY (FEET/SEC.) = 3.59
 ESTIMATED PIPE DIAMETER (INCH) = 24.00 NUMBER OF PIPES = 1
 PIPE-FLOW (CFS) = 8.21
 PIPE TRAVEL TIME (MIN.) = 0.88 Tc (MIN.) = 13.41
 LONGEST FLOWPATH FROM NODE 39.00 TO NODE 38.00 = 800.00 FEET.

 FLOW PROCESS FROM NODE 38.00 TO NODE 38.00 IS CODE = 11

>>>>CONFLUENCE MEMORY BANK # 1 WITH THE MAIN-STREAM MEMORY<<<<<

** MAIN STREAM CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp (Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	7.82	8.30	3.621	0.30 (0.10)	0.35	2.3	43.00
2	7.86	8.47	3.579	0.30 (0.10)	0.35	2.3	45.00
3	8.21	13.41	2.759	0.30 (0.10)	0.35	3.3	39.00

LONGEST FLOWPATH FROM NODE 39.00 TO NODE 38.00 = 800.00 FEET.

** MEMORY BANK # 1 CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp (Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	19.80	8.86	3.490	0.31 (0.10)	0.32	5.7	30.00
2	20.08	9.31	3.392	0.31 (0.10)	0.32	5.9	38.00
3	20.29	9.71	3.313	0.31 (0.10)	0.32	6.2	30.00
4	20.33	9.83	3.290	0.31 (0.10)	0.32	6.2	36.00
5	20.50	10.55	3.161	0.31 (0.10)	0.32	6.5	14.00
6	20.58	10.84	3.113	0.31 (0.10)	0.32	6.7	26.00
7	20.58	10.88	3.107	0.31 (0.10)	0.32	6.7	27.00
8	20.67	11.60	2.996	0.31 (0.10)	0.32	7.0	20.00
9	20.58	12.07	2.929	0.31 (0.10)	0.32	7.2	17.00
10	20.58	12.08	2.928	0.31 (0.10)	0.32	7.2	23.00
11	20.56	12.13	2.921	0.31 (0.10)	0.32	7.2	29.00
12	19.89	13.44	2.756	0.31 (0.10)	0.31	7.5	34.00
13	19.69	13.78	2.717	0.31 (0.10)	0.31	7.5	24.00
14	17.86	16.89	2.422	0.31 (0.09)	0.30	7.8	12.00
15	15.97	20.38	2.178	0.31 (0.09)	0.30	7.9	10.00

LONGEST FLOWPATH FROM NODE 10.00 TO NODE 38.00 = 1855.00 FEET.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	27.09	8.30	3.621	0.31 (0.10)	0.33	7.6	43.00
2	27.29	8.47	3.579	0.31 (0.10)	0.33	7.8	45.00
3	27.69	8.86	3.490	0.31 (0.10)	0.33	8.1	30.00
4	27.99	9.31	3.392	0.31 (0.10)	0.33	8.4	38.00
5	28.24	9.71	3.313	0.31 (0.10)	0.33	8.7	30.00
6	28.28	9.83	3.290	0.31 (0.10)	0.33	8.8	36.00
7	28.51	10.55	3.161	0.31 (0.10)	0.33	9.3	14.00
8	28.61	10.84	3.113	0.31 (0.10)	0.33	9.5	26.00
9	28.61	10.88	3.107	0.31 (0.10)	0.33	9.5	27.00
10	28.75	11.60	2.996	0.31 (0.10)	0.33	10.0	20.00
11	28.70	12.07	2.929	0.31 (0.10)	0.33	10.2	17.00
12	28.70	12.08	2.928	0.31 (0.10)	0.33	10.2	23.00
13	28.68	12.13	2.921	0.31 (0.10)	0.33	10.3	29.00
14	28.12	13.41	2.759	0.31 (0.10)	0.32	10.7	39.00
15	28.10	13.44	2.756	0.31 (0.10)	0.32	10.8	34.00
16	27.77	13.78	2.717	0.31 (0.10)	0.32	10.8	24.00
17	25.03	16.89	2.422	0.31 (0.10)	0.32	11.1	12.00
18	22.38	20.38	2.178	0.31 (0.10)	0.32	11.2	10.00
TOTAL AREA (ACRES) =			11.2				

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE (CFS) = 28.75 Tc (MIN.) = 11.599
 EFFECTIVE AREA (ACRES) = 9.98 AREA-AVERAGED Fm (INCH/HR) = 0.10
 AREA-AVERAGED Fp (INCH/HR) = 0.31 AREA-AVERAGED Ap = 0.33
 TOTAL AREA (ACRES) = 11.2
 LONGEST FLOWPATH FROM NODE 10.00 TO NODE 38.00 = 1855.00 FEET.

 FLOW PROCESS FROM NODE 38.00 TO NODE 38.00 IS CODE = 12

>>>>CLEAR MEMORY BANK # 1 <<<<<

 FLOW PROCESS FROM NODE 38.00 TO NODE 47.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
 >>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<

=====

ELEVATION DATA: UPSTREAM (FEET) = 150.20 DOWNSTREAM (FEET) = 149.90
 FLOW LENGTH (FEET) = 150.00 MANNING'S N = 0.013
 DEPTH OF FLOW IN 36.0 INCH PIPE IS 29.2 INCHES
 PIPE-FLOW VELOCITY (FEET/SEC.) = 4.69
 ESTIMATED PIPE DIAMETER (INCH) = 36.00 NUMBER OF PIPES = 1
 PIPE-FLOW (CFS) = 28.75
 PIPE TRAVEL TIME (MIN.) = 0.53 Tc (MIN.) = 12.13
 LONGEST FLOWPATH FROM NODE 10.00 TO NODE 47.00 = 2005.00 FEET.

 FLOW PROCESS FROM NODE 47.00 TO NODE 47.00 IS CODE = 10

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>>>>MAIN-STREAM MEMORY COPIED ONTO MEMORY BANK # 1 <<<<<
=====

FLOW PROCESS FROM NODE 32.00 TO NODE 48.00 IS CODE = 21
=====

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<
=====

INITIAL SUBAREA FLOW-LENGTH (FEET) = 300.00
ELEVATION DATA: UPSTREAM (FEET) = 166.30 DOWNSTREAM (FEET) = 166.00

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$

SUBAREA ANALYSIS USED MINIMUM T_c (MIN.) = 11.850

* 25 YEAR RAINFALL INTENSITY (INCH/HR) = 2.960

SUBAREA T_c AND LOSS RATE DATA (AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	F_p (INCH/HR)	A_p (DECIMAL)	SCS CN	T_c (MIN.)
COMMERCIAL	B	2.62	0.30	0.100	56	11.85

SUBAREA AVERAGE PERVIOUS LOSS RATE, F_p (INCH/HR) = 0.30

SUBAREA AVERAGE PERVIOUS AREA FRACTION, A_p = 0.100

SUBAREA RUNOFF (CFS) = 6.91

TOTAL AREA (ACRES) = 2.62 PEAK FLOW RATE (CFS) = 6.91

FLOW PROCESS FROM NODE 48.00 TO NODE 49.00 IS CODE = 61
=====

>>>>COMPUTE STREET FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>(STANDARD CURB SECTION USED)<<<<<
=====

UPSTREAM ELEVATION (FEET) = 166.00 DOWNSTREAM ELEVATION (FEET) = 165.60
STREET LENGTH (FEET) = 485.00 CURB HEIGHT (INCHES) = 6.0
STREET HALFWIDTH (FEET) = 25.00

DISTANCE FROM CROWN TO CROSSFALL GRADEBREAK (FEET) = 10.00

INSIDE STREET CROSSFALL (DECIMAL) = 0.020

OUTSIDE STREET CROSSFALL (DECIMAL) = 0.020

SPECIFIED NUMBER OF HALFSTREETS CARRYING RUNOFF = 2

STREET PARKWAY CROSSFALL (DECIMAL) = 0.020

Manning's FRICTION FACTOR for Streetflow Section (curb-to-curb) = 0.0150

Manning's FRICTION FACTOR for Back-of-Walk Flow Section = 0.0200

**TRAVEL TIME COMPUTED USING ESTIMATED FLOW (CFS) = 11.77

STREETFLOW MODEL RESULTS USING ESTIMATED FLOW:

STREET FLOW DEPTH (FEET) = 0.59

HALFSTREET FLOOD WIDTH (FEET) = 27.48

AVERAGE FLOW VELOCITY (FEET/SEC.) = 1.04

PRODUCT OF DEPTH&VELOCITY (FT*FT/SEC.) = 0.61

STREET FLOW TRAVEL TIME (MIN.) = 7.75 T_c (MIN.) = 19.60

* 25 YEAR RAINFALL INTENSITY (INCH/HR) = 2.226

SUBAREA LOSS RATE DATA (AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	F_p (INCH/HR)	A_p (DECIMAL)	SCS CN
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LAND USE	GROUP	(ACRES)	(INCH/HR)	(DECIMAL)	CN
COMMERCIAL	B	4.89	0.30	0.100	56
SUBAREA AVERAGE PERVIOUS LOSS RATE, F_p (INCH/HR) = 0.30					
SUBAREA AVERAGE PERVIOUS AREA FRACTION, A_p = 0.100					
SUBAREA AREA (ACRES) =		4.89	SUBAREA RUNOFF (CFS) =		9.67
EFFECTIVE AREA (ACRES) =		7.51	AREA-AVERAGED F_m (INCH/HR) =		0.03
AREA-AVERAGED F_p (INCH/HR) =		0.30	AREA-AVERAGED A_p =		0.10
TOTAL AREA (ACRES) =		7.5	PEAK FLOW RATE (CFS) =		14.84

END OF SUBAREA STREET FLOW HYDRAULICS:

DEPTH (FEET) = 0.63 HALFSTREET FLOOD WIDTH (FEET) = 31.40
FLOW VELOCITY (FEET/SEC.) = 1.09 DEPTH*VELOCITY (FT*FT/SEC.) = 0.68
*NOTE: INITIAL SUBAREA NOMOGRAPH WITH SUBAREA PARAMETERS,
AND L = 485.0 FT WITH ELEVATION-DROP = 0.4 FT, IS 11.3 CFS,
WHICH EXCEEDS THE TOP-OF-CURB STREET CAPACITY AT NODE 49.00
LONGEST FLOWPATH FROM NODE 32.00 TO NODE 49.00 = 785.00 FEET.

FLOW PROCESS FROM NODE 49.00 TO NODE 50.00 IS CODE = 51

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<<
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM (FEET) = 165.60 DOWNSTREAM (FEET) = 165.40
CHANNEL LENGTH THRU SUBAREA (FEET) = 180.00 CHANNEL SLOPE = 0.0011
CHANNEL BASE (FEET) = 5.00 "Z" FACTOR = 0.020
MANNING'S FACTOR = 0.015 MAXIMUM DEPTH (FEET) = 2.00
CHANNEL FLOW THRU SUBAREA (CFS) = 14.84
FLOW VELOCITY (FEET/SEC.) = 2.74 FLOW DEPTH (FEET) = 1.08
TRAVEL TIME (MIN.) = 1.09 T_c (MIN.) = 20.70
LONGEST FLOWPATH FROM NODE 32.00 TO NODE 50.00 = 965.00 FEET.

FLOW PROCESS FROM NODE 50.00 TO NODE 50.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

=====

TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION (MIN.) = 20.70
RAINFALL INTENSITY (INCH/HR) = 2.16
AREA-AVERAGED F_m (INCH/HR) = 0.03
AREA-AVERAGED F_p (INCH/HR) = 0.30
AREA-AVERAGED A_p = 0.10
EFFECTIVE STREAM AREA (ACRES) = 7.51
TOTAL STREAM AREA (ACRES) = 7.51
PEAK FLOW RATE (CFS) AT CONFLUENCE = 14.84

FLOW PROCESS FROM NODE 50.00 TO NODE 50.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

INITIAL SUBAREA FLOW-LENGTH (FEET) = 665.00
 ELEVATION DATA: UPSTREAM (FEET) = 167.20 DOWNSTREAM (FEET) = 165.40

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$
 SUBAREA ANALYSIS USED MINIMUM T_c (MIN.) = 13.351
 * 25 YEAR RAINFALL INTENSITY (INCH/HR) = 2.767

SUBAREA T_c AND LOSS RATE DATA (AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	F_p (INCH/HR)	A_p (DECIMAL)	SCS CN	T_c (MIN.)
COMMERCIAL	B	6.31	0.30	0.100	56	13.35

SUBAREA AVERAGE PERVIOUS LOSS RATE, F_p (INCH/HR) = 0.30

SUBAREA AVERAGE PERVIOUS AREA FRACTION, A_p = 0.100

SUBAREA RUNOFF (CFS) = 15.54

TOTAL AREA (ACRES) = 6.31 PEAK FLOW RATE (CFS) = 15.54

FLOW PROCESS FROM NODE 50.00 TO NODE 50.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<<
 >>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<<

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TOTAL NUMBER OF STREAMS = 2
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
 TIME OF CONCENTRATION (MIN.) = 13.35
 RAINFALL INTENSITY (INCH/HR) = 2.77
 AREA-AVERAGED F_m (INCH/HR) = 0.03
 AREA-AVERAGED F_p (INCH/HR) = 0.30
 AREA-AVERAGED A_p = 0.10
 EFFECTIVE STREAM AREA (ACRES) = 6.31
 TOTAL STREAM AREA (ACRES) = 6.31
 PEAK FLOW RATE (CFS) AT CONFLUENCE = 15.54

** CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	T_c (MIN.)	Intensity (INCH/HR)	F_p (F_m) (INCH/HR)	A_p	A_e (ACRES)	HEADWATER NODE
1	14.84	20.70	2.159	0.30 (0.03)	0.10	7.5	32.00
2	15.54	13.35	2.767	0.30 (0.03)	0.10	6.3	50.00

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
 CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	T_c (MIN.)	Intensity (INCH/HR)	F_p (F_m) (INCH/HR)	A_p	A_e (ACRES)	HEADWATER NODE
1	27.85	13.35	2.767	0.30 (0.03)	0.10	11.2	50.00
2	26.93	20.70	2.159	0.30 (0.03)	0.10	13.8	32.00

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE (CFS) = 27.85 T_c (MIN.) = 13.35
 EFFECTIVE AREA (ACRES) = 11.15 AREA-AVERAGED F_m (INCH/HR) = 0.03
 AREA-AVERAGED F_p (INCH/HR) = 0.30 AREA-AVERAGED A_p = 0.10
 TOTAL AREA (ACRES) = 13.8
 LONGEST FLOWPATH FROM NODE 32.00 TO NODE 50.00 = 965.00 FEET.

FLOW PROCESS FROM NODE 50.00 TO NODE 47.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<

ELEVATION DATA: UPSTREAM(FEET) = 162.40 DOWNSTREAM(FEET) = 149.90
FLOW LENGTH(FEET) = 185.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 21.0 INCH PIPE IS 12.9 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 18.03
ESTIMATED PIPE DIAMETER(INCH) = 21.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 27.85
PIPE TRAVEL TIME(MIN.) = 0.17 Tc(MIN.) = 13.52
LONGEST FLOWPATH FROM NODE 32.00 TO NODE 47.00 = 1150.00 FEET.

FLOW PROCESS FROM NODE 47.00 TO NODE 47.00 IS CODE = 11

>>>>CONFLUENCE MEMORY BANK # 1 WITH THE MAIN-STREAM MEMORY<<<<<

** MAIN STREAM CONFLUENCE DATA **

Table with 8 columns: STREAM NUMBER, Q (CFS), Tc (MIN.), Intensity (INCH/HR), Fp(Fm) (INCH/HR), Ap, Ae (ACRES), HEADWATER NODE. Contains 2 rows of data and a longest flowpath summary.

** MEMORY BANK # 1 CONFLUENCE DATA **

Table with 8 columns: STREAM NUMBER, Q (CFS), Tc (MIN.), Intensity (INCH/HR), Fp(Fm) (INCH/HR), Ap, Ae (ACRES), HEADWATER NODE. Contains 18 rows of data and a longest flowpath summary.

** PEAK FLOW RATE TABLE **

Table with 8 columns: STREAM NUMBER, Q (CFS), Tc (MIN.), Intensity (INCH/HR), Fp(Fm) (INCH/HR), Ap, Ae (ACRES), HEADWATER NODE.

1	50.30	8.84	3.495	0.31 (0.07)	0.22	14.9	43.00
2	50.69	9.01	3.457	0.31 (0.07)	0.22	15.2	45.00
3	51.51	9.39	3.376	0.31 (0.07)	0.22	15.8	30.00
4	52.31	9.85	3.287	0.31 (0.07)	0.22	16.6	38.00
5	52.97	10.24	3.214	0.31 (0.07)	0.22	17.2	30.00
6	53.14	10.36	3.193	0.31 (0.07)	0.22	17.4	36.00
7	54.09	11.09	3.074	0.31 (0.07)	0.21	18.4	14.00
8	54.47	11.37	3.030	0.31 (0.07)	0.21	18.9	26.00
9	54.51	11.41	3.024	0.31 (0.07)	0.21	18.9	27.00
10	55.34	12.13	2.921	0.31 (0.07)	0.21	20.0	20.00
11	55.73	12.61	2.858	0.31 (0.06)	0.21	20.6	17.00
12	55.74	12.61	2.857	0.31 (0.06)	0.21	20.7	23.00
13	55.76	12.66	2.851	0.31 (0.06)	0.21	20.7	29.00
14	56.16	13.52	2.747	0.31 (0.06)	0.21	21.7	50.00
15	55.92	13.95	2.699	0.31 (0.06)	0.21	22.1	39.00
16	55.89	13.97	2.696	0.31 (0.06)	0.21	22.1	34.00
17	55.52	14.32	2.660	0.31 (0.06)	0.21	22.2	24.00
18	52.39	17.44	2.379	0.31 (0.06)	0.20	23.7	12.00
19	49.37	20.87	2.149	0.31 (0.06)	0.20	25.0	32.00
20	49.26	20.94	2.144	0.31 (0.06)	0.20	25.0	10.00
TOTAL AREA (ACRES) =			25.0				

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE (CFS) = 56.16 Tc (MIN.) = 13.522
EFFECTIVE AREA (ACRES) = 21.74 AREA-AVERAGED Fm (INCH/HR) = 0.06
AREA-AVERAGED Fp (INCH/HR) = 0.31 AREA-AVERAGED Ap = 0.22
TOTAL AREA (ACRES) = 25.0
LONGEST FLOWPATH FROM NODE 10.00 TO NODE 47.00 = 2005.00 FEET.

FLOW PROCESS FROM NODE 47.00 TO NODE 47.00 IS CODE = 12

>>>>CLEAR MEMORY BANK # 1 <<<<<

FLOW PROCESS FROM NODE 47.00 TO NODE 47.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

TOTAL NUMBER OF STREAMS = 2

CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:

TIME OF CONCENTRATION (MIN.) = 13.52
RAINFALL INTENSITY (INCH/HR) = 2.75
AREA-AVERAGED Fm (INCH/HR) = 0.06
AREA-AVERAGED Fp (INCH/HR) = 0.31
AREA-AVERAGED Ap = 0.21
EFFECTIVE STREAM AREA (ACRES) = 21.74
TOTAL STREAM AREA (ACRES) = 25.00
PEAK FLOW RATE (CFS) AT CONFLUENCE = 56.16

FLOW PROCESS FROM NODE 51.00 TO NODE 52.00 IS CODE = 21

>>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

=====

INITIAL SUBAREA FLOW-LENGTH (FEET) = 140.00
ELEVATION DATA: UPSTREAM (FEET) = 166.70 DOWNSTREAM (FEET) = 166.30

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$
SUBAREA ANALYSIS USED MINIMUM T_c (MIN.) = 8.386
* 25 YEAR RAINFALL INTENSITY (INCH/HR) = 3.600

SUBAREA T_c AND LOSS RATE DATA (AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	F_p (INCH/HR)	A_p (DECIMAL)	SCS CN	T_c (MIN.)
CONDOMINIUMS	B	0.64	0.30	0.350	56	8.39

SUBAREA AVERAGE PERVIOUS LOSS RATE, F_p (INCH/HR) = 0.30

SUBAREA AVERAGE PERVIOUS AREA FRACTION, A_p = 0.350

SUBAREA RUNOFF (CFS) = 2.01

TOTAL AREA (ACRES) = 0.64 PEAK FLOW RATE (CFS) = 2.01

FLOW PROCESS FROM NODE 52.00 TO NODE 47.00 IS CODE = 31

>>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<

>>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<

=====

ELEVATION DATA: UPSTREAM (FEET) = 156.00 DOWNSTREAM (FEET) = 149.90

FLOW LENGTH (FEET) = 40.00 MANNING'S N = 0.013

ESTIMATED PIPE DIAMETER (INCH) INCREASED TO 12.000

DEPTH OF FLOW IN 12.0 INCH PIPE IS 3.1 INCHES

PIPE-FLOW VELOCITY (FEET/SEC.) = 12.40

ESTIMATED PIPE DIAMETER (INCH) = 12.000 NUMBER OF PIPES = 1

PIPE-FLOW (CFS) = 2.01

PIPE TRAVEL TIME (MIN.) = 0.05 T_c (MIN.) = 8.44

LONGEST FLOWPATH FROM NODE 51.00 TO NODE 47.00 = 180.00 FEET.

FLOW PROCESS FROM NODE 47.00 TO NODE 47.00 IS CODE = 1

>>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

>>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<

=====

TOTAL NUMBER OF STREAMS = 2

CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:

TIME OF CONCENTRATION (MIN.) = 8.44

RAINFALL INTENSITY (INCH/HR) = 3.59

AREA-AVERAGED F_m (INCH/HR) = 0.10

AREA-AVERAGED F_p (INCH/HR) = 0.30

AREA-AVERAGED A_p = 0.35

EFFECTIVE STREAM AREA (ACRES) = 0.64

TOTAL STREAM AREA (ACRES) = 0.64

PEAK FLOW RATE (CFS) AT CONFLUENCE = 2.01

** CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	T_c (MIN.)	Intensity (INCH/HR)	F_p (F_m) (INCH/HR)	A_p	A_e (ACRES)	HEADWATER NODE
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1	50.30	8.84	3.495	0.31 (0.07)	0.22	14.9	43.00
1	50.69	9.01	3.457	0.31 (0.07)	0.22	15.2	45.00
1	51.51	9.39	3.376	0.31 (0.07)	0.22	15.8	30.00
1	52.31	9.85	3.287	0.31 (0.07)	0.22	16.6	38.00
1	52.97	10.24	3.214	0.31 (0.07)	0.22	17.2	30.00
1	53.14	10.36	3.193	0.31 (0.07)	0.22	17.4	36.00
1	54.09	11.09	3.074	0.31 (0.07)	0.21	18.4	14.00
1	54.47	11.37	3.030	0.31 (0.07)	0.21	18.9	26.00
1	54.51	11.41	3.024	0.31 (0.07)	0.21	18.9	27.00
1	55.34	12.13	2.921	0.31 (0.07)	0.21	20.0	20.00
1	55.73	12.61	2.858	0.31 (0.06)	0.21	20.6	17.00
1	55.74	12.61	2.857	0.31 (0.06)	0.21	20.7	23.00
1	55.76	12.66	2.851	0.31 (0.06)	0.21	20.7	29.00
1	56.16	13.52	2.747	0.31 (0.06)	0.21	21.7	50.00
1	55.92	13.95	2.699	0.31 (0.06)	0.21	22.1	39.00
1	55.89	13.97	2.696	0.31 (0.06)	0.21	22.1	34.00
1	55.52	14.32	2.660	0.31 (0.06)	0.21	22.2	24.00
1	52.39	17.44	2.379	0.31 (0.06)	0.20	23.7	12.00
1	49.37	20.87	2.149	0.31 (0.06)	0.20	25.0	32.00
1	49.26	20.94	2.144	0.31 (0.06)	0.20	25.0	10.00
2	2.01	8.44	3.587	0.30 (0.10)	0.35	0.6	51.00

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	51.34	8.44	3.587	0.31 (0.07)	0.22	14.9	51.00
2	52.26	8.84	3.495	0.31 (0.07)	0.22	15.5	43.00
3	52.63	9.01	3.457	0.31 (0.07)	0.22	15.8	45.00
4	53.41	9.39	3.376	0.31 (0.07)	0.22	16.5	30.00
5	54.15	9.85	3.287	0.31 (0.07)	0.22	17.2	38.00
6	54.77	10.24	3.214	0.31 (0.07)	0.22	17.8	30.00
7	54.92	10.36	3.193	0.31 (0.07)	0.22	18.0	36.00
8	55.80	11.09	3.074	0.31 (0.07)	0.22	19.1	14.00
9	56.16	11.37	3.030	0.31 (0.07)	0.22	19.5	26.00
10	56.20	11.41	3.024	0.31 (0.07)	0.22	19.6	27.00
11	56.97	12.13	2.921	0.31 (0.07)	0.22	20.6	20.00
12	57.32	12.61	2.858	0.31 (0.07)	0.22	21.3	17.00
13	57.33	12.61	2.857	0.31 (0.07)	0.22	21.3	23.00
14	57.35	12.66	2.851	0.31 (0.07)	0.22	21.3	29.00
15	57.69	13.52	2.747	0.31 (0.07)	0.21	22.4	50.00
16	57.42	13.95	2.699	0.31 (0.07)	0.21	22.7	39.00
17	57.39	13.97	2.696	0.31 (0.07)	0.21	22.7	34.00
18	57.00	14.32	2.660	0.31 (0.06)	0.21	22.9	24.00
19	53.70	17.44	2.379	0.31 (0.06)	0.21	24.3	12.00
20	50.55	20.87	2.149	0.31 (0.06)	0.20	25.6	32.00
21	50.44	20.94	2.144	0.31 (0.06)	0.20	25.6	10.00

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE (CFS) = 57.69 Tc (MIN.) = 13.52
EFFECTIVE AREA (ACRES) = 22.38 AREA-AVERAGED Fm (INCH/HR) = 0.07
AREA-AVERAGED Fp (INCH/HR) = 0.31 AREA-AVERAGED Ap = 0.21

TOTAL AREA (ACRES) = 25.6
LONGEST FLOWPATH FROM NODE 10.00 TO NODE 47.00 = 2005.00 FEET.

FLOW PROCESS FROM NODE 47.00 TO NODE 53.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<

=====

ELEVATION DATA: UPSTREAM (FEET) = 149.80 DOWNSTREAM (FEET) = 149.70
FLOW LENGTH (FEET) = 100.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 54.0 INCH PIPE IS 42.1 INCHES
PIPE-FLOW VELOCITY (FEET/SEC.) = 4.34
ESTIMATED PIPE DIAMETER (INCH) = 54.00 NUMBER OF PIPES = 1
PIPE-FLOW (CFS) = 57.69
PIPE TRAVEL TIME (MIN.) = 0.38 Tc (MIN.) = 13.91
LONGEST FLOWPATH FROM NODE 10.00 TO NODE 53.00 = 2105.00 FEET.

FLOW PROCESS FROM NODE 53.00 TO NODE 53.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

=====

TOTAL NUMBER OF STREAMS = 3
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION (MIN.) = 13.91
RAINFALL INTENSITY (INCH/HR) = 2.70
AREA-AVERAGED Fm (INCH/HR) = 0.07
AREA-AVERAGED Fp (INCH/HR) = 0.31
AREA-AVERAGED Ap = 0.21
EFFECTIVE STREAM AREA (ACRES) = 22.38
TOTAL STREAM AREA (ACRES) = 25.64
PEAK FLOW RATE (CFS) AT CONFLUENCE = 57.69

FLOW PROCESS FROM NODE 38.00 TO NODE 54.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

=====

INITIAL SUBAREA FLOW-LENGTH (FEET) = 265.00
ELEVATION DATA: UPSTREAM (FEET) = 166.40 DOWNSTREAM (FEET) = 165.80

Tc = K * [(LENGTH** 3.00) / (ELEVATION CHANGE)] ** 0.20
SUBAREA ANALYSIS USED MINIMUM Tc (MIN.) = 11.340
* 25 YEAR RAINFALL INTENSITY (INCH/HR) = 3.034
SUBAREA Tc AND LOSS RATE DATA (AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN	Tc (MIN.)
CONDOMINIUMS	B	0.80	0.30	0.350	56	11.34

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp (INCH/HR) = 0.30
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.350
SUBAREA RUNOFF (CFS) = 2.11
TOTAL AREA (ACRES) = 0.80 PEAK FLOW RATE (CFS) = 2.11

FLOW PROCESS FROM NODE 54.00 TO NODE 53.00 IS CODE = 41

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) =	149.80	DOWNSTREAM(FEET) =	149.70
FLOW LENGTH(FEET) =	70.00	MANNING'S N =	0.013
DEPTH OF FLOW IN 18.0 INCH PIPE IS	9.5 INCHES		
PIPE-FLOW VELOCITY(FEET/SEC.) =	2.24		
GIVEN PIPE DIAMETER(INCH) =	18.00	NUMBER OF PIPES =	1
PIPE-FLOW(CFS) =	2.11		
PIPE TRAVEL TIME(MIN.) =	0.52	Tc(MIN.) =	11.86
LONGEST FLOWPATH FROM NODE	38.00 TO NODE	53.00 =	335.00 FEET.

FLOW PROCESS FROM NODE 53.00 TO NODE 53.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

=====

TOTAL NUMBER OF STREAMS =	3
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:	
TIME OF CONCENTRATION(MIN.) =	11.86
RAINFALL INTENSITY(INCH/HR) =	2.96
AREA-AVERAGED Fm(INCH/HR) =	0.11
AREA-AVERAGED Fp(INCH/HR) =	0.30
AREA-AVERAGED Ap =	0.35
EFFECTIVE STREAM AREA(ACRES) =	0.80
TOTAL STREAM AREA(ACRES) =	0.80
PEAK FLOW RATE(CFS) AT CONFLUENCE =	2.11

FLOW PROCESS FROM NODE 55.00 TO NODE 56.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

=====

INITIAL SUBAREA FLOW-LENGTH(FEET) =	130.00		
ELEVATION DATA: UPSTREAM(FEET) =	166.50	DOWNSTREAM(FEET) =	166.30

$$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$$

SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) = 9.214

* 25 YEAR RAINFALL INTENSITY(INCH/HR) = 3.413

SUBAREA Tc AND LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN	Tc (MIN.)
CONDOMINIUMS	B	0.19	0.30	0.350	56	9.21

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.30

SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.350

SUBAREA RUNOFF(CFS) = 0.57

TOTAL AREA(ACRES) = 0.19 PEAK FLOW RATE(CFS) = 0.57

FLOW PROCESS FROM NODE 56.00 TO NODE 53.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<

ELEVATION DATA: UPSTREAM(FEET) = 156.00 DOWNSTREAM(FEET) = 149.70
FLOW LENGTH(FEET) = 40.00 MANNING'S N = 0.013
ESTIMATED PIPE DIAMETER(INCH) INCREASED TO 12.000
DEPTH OF FLOW IN 12.0 INCH PIPE IS 1.7 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 8.66
ESTIMATED PIPE DIAMETER(INCH) = 12.000 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 0.57
PIPE TRAVEL TIME(MIN.) = 0.08 Tc(MIN.) = 9.29
LONGEST FLOWPATH FROM NODE 55.00 TO NODE 53.00 = 170.00 FEET.

FLOW PROCESS FROM NODE 53.00 TO NODE 53.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<

TOTAL NUMBER OF STREAMS = 3
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 3 ARE:
TIME OF CONCENTRATION(MIN.) = 9.29
RAINFALL INTENSITY(INCH/HR) = 3.40
AREA-AVERAGED Fm(INCH/HR) = 0.11
AREA-AVERAGED Fp(INCH/HR) = 0.30
AREA-AVERAGED Ap = 0.35
EFFECTIVE STREAM AREA(ACRES) = 0.19
TOTAL STREAM AREA(ACRES) = 0.19
PEAK FLOW RATE(CFS) AT CONFLUENCE = 0.57

** CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	51.34	8.84	3.494	0.31 (0.07)	0.22	14.9	51.00
1	52.26	9.22	3.411	0.31 (0.07)	0.22	15.5	43.00
1	52.63	9.39	3.376	0.31 (0.07)	0.22	15.8	45.00
1	53.41	9.78	3.300	0.31 (0.07)	0.22	16.5	30.00
1	54.15	10.23	3.216	0.31 (0.07)	0.22	17.2	38.00
1	54.77	10.63	3.147	0.31 (0.07)	0.22	17.8	30.00
1	54.92	10.75	3.128	0.31 (0.07)	0.22	18.0	36.00
1	55.80	11.47	3.015	0.31 (0.07)	0.22	19.1	14.00
1	56.16	11.76	2.973	0.31 (0.07)	0.22	19.5	26.00
1	56.20	11.80	2.968	0.31 (0.07)	0.22	19.6	27.00
1	56.97	12.52	2.870	0.31 (0.07)	0.22	20.6	20.00
1	57.32	12.99	2.810	0.31 (0.07)	0.22	21.3	17.00
1	57.33	13.00	2.809	0.31 (0.07)	0.22	21.3	23.00
1	57.35	13.05	2.803	0.31 (0.07)	0.22	21.3	29.00
1	57.69	13.91	2.704	0.31 (0.07)	0.21	22.4	50.00
1	57.42	14.33	2.658	0.31 (0.07)	0.21	22.7	39.00
1	57.39	14.36	2.655	0.31 (0.07)	0.21	22.7	34.00
1	57.00	14.70	2.620	0.31 (0.06)	0.21	22.9	24.00
1	53.70	17.82	2.349	0.31 (0.06)	0.21	24.3	12.00

1	50.55	21.27	2.126	0.31 (0.06)	0.20	25.6	32.00
1	50.44	21.34	2.122	0.31 (0.06)	0.20	25.6	10.00
2	2.11	11.86	2.958	0.30 (0.11)	0.35	0.8	38.00
3	0.57	9.29	3.397	0.30 (0.11)	0.35	0.2	55.00

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
CONFLUENCE FORMULA USED FOR 3 STREAMS.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp (Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	53.77	8.84	3.494	0.31 (0.07)	0.23	15.7	51.00
2	54.72	9.22	3.411	0.31 (0.07)	0.23	16.4	43.00
3	54.88	9.29	3.397	0.31 (0.07)	0.23	16.5	55.00
4	55.11	9.39	3.376	0.31 (0.07)	0.23	16.7	45.00
5	55.90	9.78	3.300	0.31 (0.07)	0.23	17.3	30.00
6	56.67	10.23	3.216	0.31 (0.07)	0.23	18.1	38.00
7	57.31	10.63	3.147	0.31 (0.07)	0.23	18.7	30.00
8	57.47	10.75	3.128	0.31 (0.07)	0.23	18.9	36.00
9	58.38	11.47	3.015	0.31 (0.07)	0.23	20.0	14.00
10	58.75	11.76	2.973	0.31 (0.07)	0.22	20.5	26.00
11	58.80	11.80	2.968	0.31 (0.07)	0.22	20.5	27.00
12	58.87	11.86	2.958	0.31 (0.07)	0.22	20.7	38.00
13	59.49	12.52	2.870	0.31 (0.07)	0.22	21.6	20.00
14	59.79	12.99	2.810	0.31 (0.07)	0.22	22.3	17.00
15	59.79	13.00	2.809	0.31 (0.07)	0.22	22.3	23.00
16	59.81	13.05	2.803	0.31 (0.07)	0.22	22.3	29.00
17	60.05	13.91	2.704	0.31 (0.07)	0.22	23.4	50.00
18	59.74	14.33	2.658	0.31 (0.07)	0.22	23.7	39.00
19	59.71	14.36	2.655	0.31 (0.07)	0.22	23.7	34.00
20	59.29	14.70	2.620	0.31 (0.07)	0.22	23.9	24.00
21	55.75	17.82	2.349	0.31 (0.06)	0.21	25.3	12.00
22	52.39	21.27	2.126	0.31 (0.06)	0.21	26.6	32.00
23	52.28	21.34	2.122	0.31 (0.06)	0.21	26.6	10.00

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE (CFS) = 60.05 Tc (MIN.) = 13.91
EFFECTIVE AREA (ACRES) = 23.37 AREA-AVERAGED Fm (INCH/HR) = 0.07
AREA-AVERAGED Fp (INCH/HR) = 0.31 AREA-AVERAGED Ap = 0.22
TOTAL AREA (ACRES) = 26.6
LONGEST FLOWPATH FROM NODE 10.00 TO NODE 53.00 = 2105.00 FEET.

FLOW PROCESS FROM NODE 53.00 TO NODE 57.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<

=====

ELEVATION DATA: UPSTREAM (FEET) = 149.70 DOWNSTREAM (FEET) = 149.10
FLOW LENGTH (FEET) = 280.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 48.0 INCH PIPE IS 36.5 INCHES
PIPE-FLOW VELOCITY (FEET/SEC.) = 5.86
ESTIMATED PIPE DIAMETER (INCH) = 48.00 NUMBER OF PIPES = 1
PIPE-FLOW (CFS) = 60.05

PIPE TRAVEL TIME(MIN.) = 0.80 Tc(MIN.) = 14.70
LONGEST FLOWPATH FROM NODE 10.00 TO NODE 57.00 = 2385.00 FEET.

FLOW PROCESS FROM NODE 57.00 TO NODE 57.00 IS CODE = 10

>>>>MAIN-STREAM MEMORY COPIED ONTO MEMORY BANK # 1 <<<<<
=====

FLOW PROCESS FROM NODE 58.00 TO NODE 57.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<
=====

INITIAL SUBAREA FLOW-LENGTH(FEET) = 240.00
ELEVATION DATA: UPSTREAM(FEET) = 166.50 DOWNSTREAM(FEET) = 165.60

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$

SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) = 9.853

* 25 YEAR RAINFALL INTENSITY(INCH/HR) = 3.286

SUBAREA Tc AND LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN	Tc (MIN.)
CONDOMINIUMS	B	0.46	0.30	0.350	56	9.85

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.30

SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.350

SUBAREA RUNOFF(CFS) = 1.32

TOTAL AREA(ACRES) = 0.46 PEAK FLOW RATE(CFS) = 1.32

FLOW PROCESS FROM NODE 57.00 TO NODE 57.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
=====

TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:

TIME OF CONCENTRATION(MIN.) = 9.85

RAINFALL INTENSITY(INCH/HR) = 3.29

AREA-AVERAGED Fm(INCH/HR) = 0.11

AREA-AVERAGED Fp(INCH/HR) = 0.30

AREA-AVERAGED Ap = 0.35

EFFECTIVE STREAM AREA(ACRES) = 0.46

TOTAL STREAM AREA(ACRES) = 0.46

PEAK FLOW RATE(CFS) AT CONFLUENCE = 1.32

FLOW PROCESS FROM NODE 59.00 TO NODE 57.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<
=====

INITIAL SUBAREA FLOW-LENGTH(FEET) = 265.00
ELEVATION DATA: UPSTREAM(FEET) = 166.80 DOWNSTREAM(FEET) = 165.80

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$
 SUBAREA ANALYSIS USED MINIMUM T_c (MIN.) = 10.239
 * 25 YEAR RAINFALL INTENSITY (INCH/HR) = 3.215
 SUBAREA T_c AND LOSS RATE DATA (AMC II):
 DEVELOPMENT TYPE/ LAND USE SCS SOIL GROUP AREA (ACRES) F_p (INCH/HR) A_p (DECIMAL) SCS CN T_c (MIN.)
 CONDOMINIUMS B 0.57 0.30 0.350 56 10.24
 SUBAREA AVERAGE PERVIOUS LOSS RATE, F_p (INCH/HR) = 0.30
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, A_p = 0.350
 SUBAREA RUNOFF (CFS) = 1.60
 TOTAL AREA (ACRES) = 0.57 PEAK FLOW RATE (CFS) = 1.60

 FLOW PROCESS FROM NODE 57.00 TO NODE 57.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<<
 >>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<<

=====
 TOTAL NUMBER OF STREAMS = 2
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
 TIME OF CONCENTRATION (MIN.) = 10.24
 RAINFALL INTENSITY (INCH/HR) = 3.22
 AREA-AVERAGED F_m (INCH/HR) = 0.11
 AREA-AVERAGED F_p (INCH/HR) = 0.30
 AREA-AVERAGED A_p = 0.35
 EFFECTIVE STREAM AREA (ACRES) = 0.57
 TOTAL STREAM AREA (ACRES) = 0.57
 PEAK FLOW RATE (CFS) AT CONFLUENCE = 1.60

** CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	T_c (MIN.)	Intensity (INCH/HR)	F_p (F_m) (INCH/HR)	A_p	A_e (ACRES)	HEADWATER NODE
1	1.32	9.85	3.286	0.30 (0.11)	0.35	0.5	58.00
2	1.60	10.24	3.215	0.30 (0.11)	0.35	0.6	59.00

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
 CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	T_c (MIN.)	Intensity (INCH/HR)	F_p (F_m) (INCH/HR)	A_p	A_e (ACRES)	HEADWATER NODE
1	2.89	9.85	3.286	0.30 (0.11)	0.35	1.0	58.00
2	2.88	10.24	3.215	0.30 (0.11)	0.35	1.0	59.00

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
 PEAK FLOW RATE (CFS) = 2.89 T_c (MIN.) = 9.85
 EFFECTIVE AREA (ACRES) = 1.01 AREA-AVERAGED F_m (INCH/HR) = 0.11
 AREA-AVERAGED F_p (INCH/HR) = 0.30 AREA-AVERAGED A_p = 0.35
 TOTAL AREA (ACRES) = 1.0
 LONGEST FLOWPATH FROM NODE 59.00 TO NODE 57.00 = 265.00 FEET.

 FLOW PROCESS FROM NODE 57.00 TO NODE 57.00 IS CODE = 11

=====
 >>>>CONFLUENCE MEMORY BANK # 1 WITH THE MAIN-STREAM MEMORY<<<<<
 =====

** MAIN STREAM CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp (Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	2.89	9.85	3.286	0.30 (0.11)	0.35	1.0	58.00
2	2.88	10.24	3.215	0.30 (0.11)	0.35	1.0	59.00
LONGEST FLOWPATH FROM NODE			59.00	TO NODE	57.00 =	265.00 FEET.	

** MEMORY BANK # 1 CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp (Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	53.77	9.67	3.321	0.31 (0.07)	0.23	15.7	51.00
2	54.72	10.03	3.253	0.31 (0.07)	0.23	16.4	43.00
3	54.88	10.10	3.240	0.31 (0.07)	0.23	16.5	55.00
4	55.11	10.20	3.222	0.31 (0.07)	0.23	16.7	45.00
5	55.90	10.58	3.155	0.31 (0.07)	0.23	17.3	30.00
6	56.67	11.04	3.082	0.31 (0.07)	0.23	18.1	38.00
7	57.31	11.43	3.021	0.31 (0.07)	0.23	18.7	30.00
8	57.47	11.55	3.003	0.31 (0.07)	0.23	18.9	36.00
9	58.38	12.27	2.902	0.31 (0.07)	0.23	20.0	14.00
10	58.75	12.56	2.865	0.31 (0.07)	0.22	20.5	26.00
11	58.80	12.59	2.860	0.31 (0.07)	0.22	20.5	27.00
12	58.87	12.66	2.851	0.31 (0.07)	0.22	20.7	38.00
13	59.49	13.31	2.771	0.31 (0.07)	0.22	21.6	20.00
14	59.79	13.79	2.717	0.31 (0.07)	0.22	22.3	17.00
15	59.79	13.80	2.716	0.31 (0.07)	0.22	22.3	23.00
16	59.81	13.84	2.711	0.31 (0.07)	0.22	22.3	29.00
17	60.05	14.70	2.620	0.31 (0.07)	0.22	23.4	50.00
18	59.74	15.13	2.578	0.31 (0.07)	0.22	23.7	39.00
19	59.71	15.15	2.575	0.31 (0.07)	0.22	23.7	34.00
20	59.29	15.50	2.543	0.31 (0.07)	0.22	23.9	24.00
21	55.75	18.63	2.291	0.31 (0.06)	0.21	25.3	12.00
22	52.39	22.10	2.080	0.31 (0.06)	0.21	26.6	32.00
23	52.28	22.17	2.076	0.31 (0.06)	0.21	26.6	10.00
LONGEST FLOWPATH FROM NODE			10.00	TO NODE	57.00 =	2385.00 FEET.	

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp (Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	56.63	9.67	3.321	0.30 (0.07)	0.24	16.6	51.00
2	57.14	9.85	3.286	0.30 (0.07)	0.24	17.0	58.00
3	57.61	10.03	3.253	0.30 (0.07)	0.24	17.4	43.00
4	57.76	10.10	3.240	0.30 (0.07)	0.24	17.5	55.00
5	57.99	10.20	3.222	0.30 (0.07)	0.24	17.7	45.00
6	58.07	10.24	3.215	0.30 (0.07)	0.24	17.8	59.00
7	58.73	10.58	3.155	0.31 (0.07)	0.23	18.4	30.00
8	59.42	11.04	3.082	0.31 (0.07)	0.23	19.1	38.00
9	60.01	11.43	3.021	0.31 (0.07)	0.23	19.8	30.00
10	60.15	11.55	3.003	0.31 (0.07)	0.23	20.0	36.00
11	60.98	12.27	2.902	0.31 (0.07)	0.23	21.1	14.00
12	61.31	12.56	2.865	0.31 (0.07)	0.23	21.5	26.00

13	61.35	12.59	2.860	0.31	(0.07)	0.23	21.6	27.00
14	61.42	12.66	2.851	0.31	(0.07)	0.23	21.7	38.00
15	61.96	13.31	2.771	0.31	(0.07)	0.23	22.7	20.00
16	62.21	13.79	2.717	0.31	(0.07)	0.23	23.3	17.00
17	62.21	13.80	2.716	0.31	(0.07)	0.23	23.3	23.00
18	62.23	13.84	2.711	0.31	(0.07)	0.23	23.4	29.00
19	62.38	14.70	2.620	0.31	(0.07)	0.22	24.4	50.00
20	62.04	15.13	2.578	0.31	(0.07)	0.22	24.7	39.00
21	62.00	15.15	2.575	0.31	(0.07)	0.22	24.7	34.00
22	61.55	15.50	2.543	0.31	(0.07)	0.22	24.9	24.00
23	57.78	18.63	2.291	0.30	(0.07)	0.22	26.3	12.00
24	54.22	22.10	2.080	0.30	(0.06)	0.21	27.7	32.00
25	54.10	22.17	2.076	0.30	(0.06)	0.21	27.7	10.00
TOTAL AREA (ACRES) =			27.7					

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE (CFS) = 62.38 Tc (MIN.) = 14.703
EFFECTIVE AREA (ACRES) = 24.40 AREA-AVERAGED Fm (INCH/HR) = 0.07
AREA-AVERAGED Fp (INCH/HR) = 0.31 AREA-AVERAGED Ap = 0.22
TOTAL AREA (ACRES) = 27.7
LONGEST FLOWPATH FROM NODE 10.00 TO NODE 57.00 = 2385.00 FEET.

FLOW PROCESS FROM NODE 57.00 TO NODE 57.00 IS CODE = 12

>>>>CLEAR MEMORY BANK # 1 <<<<<

FLOW PROCESS FROM NODE 57.00 TO NODE 57.00 IS CODE = 10

>>>>MAIN-STREAM MEMORY COPIED ONTO MEMORY BANK # 1 <<<<<

FLOW PROCESS FROM NODE 60.00 TO NODE 61.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

INITIAL SUBAREA FLOW-LENGTH (FEET) = 200.00
ELEVATION DATA: UPSTREAM (FEET) = 167.00 DOWNSTREAM (FEET) = 166.20

Tc = K * [(LENGTH** 3.00) / (ELEVATION CHANGE)] ** 0.20
SUBAREA ANALYSIS USED MINIMUM Tc (MIN.) = 9.043
* 25 YEAR RAINFALL INTENSITY (INCH/HR) = 3.449
SUBAREA Tc AND LOSS RATE DATA (AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN	Tc (MIN.)
CONDOMINIUMS	B	0.27	0.30	0.350	56	9.04

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp (INCH/HR) = 0.30
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.350
SUBAREA RUNOFF (CFS) = 0.81
TOTAL AREA (ACRES) = 0.27 PEAK FLOW RATE (CFS) = 0.81

FLOW PROCESS FROM NODE 61.00 TO NODE 62.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) =	166.20	DOWNSTREAM(FEET) =	165.30
FLOW LENGTH(FEET) =	210.00	MANNING'S N =	0.013
ESTIMATED PIPE DIAMETER(INCH) INCREASED TO	12.000		
DEPTH OF FLOW IN 12.0 INCH PIPE IS	5.0	INCHES	
PIPE-FLOW VELOCITY(FEET/SEC.) =	2.65		
ESTIMATED PIPE DIAMETER(INCH) =	12.00	NUMBER OF PIPES =	1
PIPE-FLOW(CFS) =	0.81		
PIPE TRAVEL TIME(MIN.) =	1.32	Tc(MIN.) =	10.36
LONGEST FLOWPATH FROM NODE 60.00 TO NODE 62.00 =			410.00 FEET.

FLOW PROCESS FROM NODE 62.00 TO NODE 62.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

=====

TOTAL NUMBER OF STREAMS =	3
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:	
TIME OF CONCENTRATION(MIN.) =	10.36
RAINFALL INTENSITY(INCH/HR) =	3.19
AREA-AVERAGED Fm(INCH/HR) =	0.10
AREA-AVERAGED Fp(INCH/HR) =	0.30
AREA-AVERAGED Ap =	0.35
EFFECTIVE STREAM AREA(ACRES) =	0.27
TOTAL STREAM AREA(ACRES) =	0.27
PEAK FLOW RATE(CFS) AT CONFLUENCE =	0.81

FLOW PROCESS FROM NODE 63.00 TO NODE 62.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

=====

INITIAL SUBAREA FLOW-LENGTH(FEET) =	270.00		
ELEVATION DATA: UPSTREAM(FEET) =	166.50	DOWNSTREAM(FEET) =	165.30

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$
SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) = 9.984
* 25 YEAR RAINFALL INTENSITY(INCH/HR) = 3.261

SUBAREA Tc AND LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN	Tc (MIN.)
CONDOMINIUMS	B	0.70	0.30	0.350	56	9.98
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.30						
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.350						
SUBAREA RUNOFF(CFS) = 1.99						
TOTAL AREA(ACRES) = 0.70 PEAK FLOW RATE(CFS) = 1.99						

FLOW PROCESS FROM NODE 62.00 TO NODE 62.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

TOTAL NUMBER OF STREAMS = 3
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
TIME OF CONCENTRATION(MIN.) = 9.98
RAINFALL INTENSITY(INCH/HR) = 3.26
AREA-AVERAGED Fm(INCH/HR) = 0.11
AREA-AVERAGED Fp(INCH/HR) = 0.30
AREA-AVERAGED Ap = 0.35
EFFECTIVE STREAM AREA(ACRES) = 0.70
TOTAL STREAM AREA(ACRES) = 0.70
PEAK FLOW RATE(CFS) AT CONFLUENCE = 1.99

FLOW PROCESS FROM NODE 64.00 TO NODE 62.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

INITIAL SUBAREA FLOW-LENGTH(FEET) = 295.00
ELEVATION DATA: UPSTREAM(FEET) = 166.50 DOWNSTREAM(FEET) = 165.30

Tc = K*[(LENGTH** 3.00)/(ELEVATION CHANGE)]**0.20
SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) = 20.647
* 25 YEAR RAINFALL INTENSITY(INCH/HR) = 2.162
SUBAREA Tc AND LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ LAND USE SCS SOIL GROUP AREA(ACRES) Fp(INCH/HR) Ap(DECIMAL) SCS CN Tc(MIN.)
AGRICULTURAL FAIR COVER
"PASTURE, IRRIGATED" B 0.27 0.30 1.000 65 20.65
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.30
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 1.000
SUBAREA RUNOFF(CFS) = 0.45
TOTAL AREA(ACRES) = 0.27 PEAK FLOW RATE(CFS) = 0.45

FLOW PROCESS FROM NODE 62.00 TO NODE 62.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<

TOTAL NUMBER OF STREAMS = 3
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 3 ARE:
TIME OF CONCENTRATION(MIN.) = 20.65
RAINFALL INTENSITY(INCH/HR) = 2.16
AREA-AVERAGED Fm(INCH/HR) = 0.30
AREA-AVERAGED Fp(INCH/HR) = 0.30
AREA-AVERAGED Ap = 1.00
EFFECTIVE STREAM AREA(ACRES) = 0.27
TOTAL STREAM AREA(ACRES) = 0.27
PEAK FLOW RATE(CFS) AT CONFLUENCE = 0.45

** CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	0.81	10.36	3.193	0.30 (0.10)	0.35	0.3	60.00
2	1.99	9.98	3.261	0.30 (0.11)	0.35	0.7	63.00
3	0.45	20.65	2.162	0.30 (0.30)	1.00	0.3	64.00

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
CONFLUENCE FORMULA USED FOR 3 STREAMS.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	3.14	9.98	3.261	0.30 (0.13)	0.43	1.1	63.00
2	3.11	10.36	3.193	0.30 (0.13)	0.43	1.1	60.00
3	2.29	20.65	2.162	0.30 (0.15)	0.49	1.2	64.00

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE (CFS) = 3.14 Tc (MIN.) = 9.98
 EFFECTIVE AREA (ACRES) = 1.09 AREA-AVERAGED Fm (INCH/HR) = 0.13
 AREA-AVERAGED Fp (INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.43
 TOTAL AREA (ACRES) = 1.2
 LONGEST FLOWPATH FROM NODE 60.00 TO NODE 62.00 = 410.00 FEET.

FLOW PROCESS FROM NODE 62.00 TO NODE 57.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
 >>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW) <<<<<

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ELEVATION DATA: UPSTREAM (FEET) = 156.00 DOWNSTREAM (FEET) = 149.10
 FLOW LENGTH (FEET) = 245.00 MANNING'S N = 0.013
 DEPTH OF FLOW IN 12.0 INCH PIPE IS 6.3 INCHES
 PIPE-FLOW VELOCITY (FEET/SEC.) = 7.55
 ESTIMATED PIPE DIAMETER (INCH) = 12.00 NUMBER OF PIPES = 1
 PIPE-FLOW (CFS) = 3.14
 PIPE TRAVEL TIME (MIN.) = 0.54 Tc (MIN.) = 10.52
 LONGEST FLOWPATH FROM NODE 60.00 TO NODE 57.00 = 655.00 FEET.

FLOW PROCESS FROM NODE 57.00 TO NODE 57.00 IS CODE = 11

>>>>CONFLUENCE MEMORY BANK # 1 WITH THE MAIN-STREAM MEMORY<<<<<

** MAIN STREAM CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	3.14	10.52	3.165	0.30 (0.13)	0.43	1.1	63.00
2	3.11	10.91	3.102	0.30 (0.13)	0.43	1.1	60.00
3	2.29	21.23	2.128	0.30 (0.15)	0.49	1.2	64.00

LONGEST FLOWPATH FROM NODE 60.00 TO NODE 57.00 = 655.00 FEET.

** MEMORY BANK # 1 CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp (Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	56.63	9.67	3.321	0.30 (0.07)	0.24	16.6	51.00
2	57.14	9.85	3.286	0.30 (0.07)	0.24	17.0	58.00
3	57.61	10.03	3.253	0.30 (0.07)	0.24	17.4	43.00
4	57.76	10.10	3.240	0.30 (0.07)	0.24	17.5	55.00
5	57.99	10.20	3.222	0.30 (0.07)	0.24	17.7	45.00
6	58.07	10.24	3.215	0.30 (0.07)	0.24	17.8	59.00
7	58.73	10.58	3.155	0.31 (0.07)	0.23	18.4	30.00
8	59.42	11.04	3.082	0.31 (0.07)	0.23	19.1	38.00
9	60.01	11.43	3.021	0.31 (0.07)	0.23	19.8	30.00
10	60.15	11.55	3.003	0.31 (0.07)	0.23	20.0	36.00
11	60.98	12.27	2.902	0.31 (0.07)	0.23	21.1	14.00
12	61.31	12.56	2.865	0.31 (0.07)	0.23	21.5	26.00
13	61.35	12.59	2.860	0.31 (0.07)	0.23	21.6	27.00
14	61.42	12.66	2.851	0.31 (0.07)	0.23	21.7	38.00
15	61.96	13.31	2.771	0.31 (0.07)	0.23	22.7	20.00
16	62.21	13.79	2.717	0.31 (0.07)	0.23	23.3	17.00
17	62.21	13.80	2.716	0.31 (0.07)	0.23	23.3	23.00
18	62.23	13.84	2.711	0.31 (0.07)	0.23	23.4	29.00
19	62.38	14.70	2.620	0.31 (0.07)	0.22	24.4	50.00
20	62.04	15.13	2.578	0.31 (0.07)	0.22	24.7	39.00
21	62.00	15.15	2.575	0.31 (0.07)	0.22	24.7	34.00
22	61.55	15.50	2.543	0.31 (0.07)	0.22	24.9	24.00
23	57.78	18.63	2.291	0.30 (0.07)	0.22	26.3	12.00
24	54.22	22.10	2.080	0.30 (0.06)	0.21	27.7	32.00
25	54.10	22.17	2.076	0.30 (0.06)	0.21	27.7	10.00
LONGEST FLOWPATH FROM NODE			10.00	TO NODE	57.00 =	2385.00	FEET.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp (Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	59.66	9.67	3.321	0.30 (0.08)	0.25	17.7	51.00
2	60.19	9.85	3.286	0.30 (0.08)	0.25	18.0	58.00
3	60.68	10.03	3.253	0.30 (0.08)	0.25	18.4	43.00
4	60.85	10.10	3.240	0.30 (0.08)	0.25	18.6	55.00
5	61.09	10.20	3.222	0.30 (0.08)	0.25	18.8	45.00
6	61.17	10.24	3.215	0.30 (0.08)	0.25	18.8	59.00
7	61.75	10.52	3.165	0.30 (0.07)	0.25	19.4	63.00
8	61.86	10.58	3.155	0.30 (0.07)	0.25	19.5	30.00
9	62.34	10.91	3.102	0.30 (0.07)	0.25	20.0	60.00
10	62.53	11.04	3.082	0.30 (0.07)	0.24	20.2	38.00
11	63.08	11.43	3.021	0.30 (0.07)	0.24	20.9	30.00
12	63.21	11.55	3.003	0.30 (0.07)	0.24	21.1	36.00
13	63.98	12.27	2.902	0.30 (0.07)	0.24	22.2	14.00
14	64.29	12.56	2.865	0.30 (0.07)	0.24	22.6	26.00
15	64.33	12.59	2.860	0.30 (0.07)	0.24	22.7	27.00
16	64.39	12.66	2.851	0.30 (0.07)	0.24	22.8	38.00
17	64.88	13.31	2.771	0.30 (0.07)	0.24	23.8	20.00
18	65.09	13.79	2.717	0.30 (0.07)	0.24	24.4	17.00
19	65.09	13.80	2.716	0.30 (0.07)	0.24	24.5	23.00
20	65.10	13.84	2.711	0.30 (0.07)	0.24	24.5	29.00
21	65.19	14.70	2.620	0.30 (0.07)	0.23	25.6	50.00

22	64.81	15.13	2.578	0.30	(0.07)	0.23	25.9	39.00
23	64.78	15.15	2.575	0.30	(0.07)	0.23	25.9	34.00
24	64.30	15.50	2.543	0.30	(0.07)	0.23	26.1	24.00
25	60.27	18.63	2.291	0.30	(0.07)	0.23	27.5	12.00
26	57.40	21.23	2.128	0.30	(0.07)	0.22	28.6	64.00
27	56.46	22.10	2.080	0.30	(0.07)	0.22	28.9	32.00
28	56.33	22.17	2.076	0.30	(0.07)	0.22	28.9	10.00
TOTAL AREA (ACRES) =			28.9					

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE (CFS) = 65.19 Tc (MIN.) = 14.703
EFFECTIVE AREA (ACRES) = 25.56 AREA-AVERAGED Fm (INCH/HR) = 0.07
AREA-AVERAGED Fp (INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.23
TOTAL AREA (ACRES) = 28.9
LONGEST FLOWPATH FROM NODE 10.00 TO NODE 57.00 = 2385.00 FEET.

FLOW PROCESS FROM NODE 57.00 TO NODE 57.00 IS CODE = 12

>>>>CLEAR MEMORY BANK # 1 <<<<<

FLOW PROCESS FROM NODE 57.00 TO NODE 57.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION (MIN.) = 14.70
RAINFALL INTENSITY (INCH/HR) = 2.62
AREA-AVERAGED Fm (INCH/HR) = 0.07
AREA-AVERAGED Fp (INCH/HR) = 0.30
AREA-AVERAGED Ap = 0.23
EFFECTIVE STREAM AREA (ACRES) = 25.56
TOTAL STREAM AREA (ACRES) = 28.90
PEAK FLOW RATE (CFS) AT CONFLUENCE = 65.19

FLOW PROCESS FROM NODE 65.00 TO NODE 66.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<

>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

INITIAL SUBAREA FLOW-LENGTH (FEET) = 135.00
ELEVATION DATA: UPSTREAM (FEET) = 166.70 DOWNSTREAM (FEET) = 166.30

$$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$$

SUBAREA ANALYSIS USED MINIMUM Tc (MIN.) = 8.205

* 25 YEAR RAINFALL INTENSITY (INCH/HR) = 3.644

SUBAREA Tc AND LOSS RATE DATA (AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN	Tc (MIN.)
CONDOMINIUMS	B	0.19	0.30	0.350	56	8.21

SUBAREA AVERAGE PERVIOUS LOSS RATE, F_p (INCH/HR) = 0.30
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, A_p = 0.350
 SUBAREA RUNOFF (CFS) = 0.61
 TOTAL AREA (ACRES) = 0.19 PEAK FLOW RATE (CFS) = 0.61

FLOW PROCESS FROM NODE 66.00 TO NODE 57.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
 >>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<

=====

ELEVATION DATA: UPSTREAM (FEET) = 156.00 DOWNSTREAM (FEET) = 149.10
 FLOW LENGTH (FEET) = 45.00 MANNING'S N = 0.013
 ESTIMATED PIPE DIAMETER (INCH) INCREASED TO 12.000
 DEPTH OF FLOW IN 12.0 INCH PIPE IS 1.7 INCHES
 PIPE-FLOW VELOCITY (FEET/SEC.) = 8.70
 ESTIMATED PIPE DIAMETER (INCH) = 12.000 NUMBER OF PIPES = 1
 PIPE-FLOW (CFS) = 0.61
 PIPE TRAVEL TIME (MIN.) = 0.09 T_c (MIN.) = 8.29
 LONGEST FLOWPATH FROM NODE 65.00 TO NODE 57.00 = 180.00 FEET.

FLOW PROCESS FROM NODE 57.00 TO NODE 57.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
 >>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<

=====

TOTAL NUMBER OF STREAMS = 2
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
 TIME OF CONCENTRATION (MIN.) = 8.29
 RAINFALL INTENSITY (INCH/HR) = 3.62
 AREA-AVERAGED F_m (INCH/HR) = 0.11
 AREA-AVERAGED F_p (INCH/HR) = 0.30
 AREA-AVERAGED A_p = 0.35
 EFFECTIVE STREAM AREA (ACRES) = 0.19
 TOTAL STREAM AREA (ACRES) = 0.19
 PEAK FLOW RATE (CFS) AT CONFLUENCE = 0.61

** CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	T_c (MIN.)	Intensity (INCH/HR)	F_p (F_m) (INCH/HR)	A_p	A_e (ACRES)	HEADWATER NODE
1	59.66	9.67	3.321	0.30 (0.08)	0.25	17.7	51.00
1	60.19	9.85	3.286	0.30 (0.08)	0.25	18.0	58.00
1	60.68	10.03	3.253	0.30 (0.08)	0.25	18.4	43.00
1	60.85	10.10	3.240	0.30 (0.08)	0.25	18.6	55.00
1	61.09	10.20	3.222	0.30 (0.08)	0.25	18.8	45.00
1	61.17	10.24	3.215	0.30 (0.08)	0.25	18.8	59.00
1	61.75	10.52	3.165	0.30 (0.07)	0.25	19.4	63.00
1	61.86	10.58	3.155	0.30 (0.07)	0.25	19.5	30.00
1	62.34	10.91	3.102	0.30 (0.07)	0.25	20.0	60.00
1	62.53	11.04	3.082	0.30 (0.07)	0.24	20.2	38.00
1	63.08	11.43	3.021	0.30 (0.07)	0.24	20.9	30.00
1	63.21	11.55	3.003	0.30 (0.07)	0.24	21.1	36.00
1	63.98	12.27	2.902	0.30 (0.07)	0.24	22.2	14.00

1	64.29	12.56	2.865	0.30 (0.07)	0.24	22.6	26.00
1	64.33	12.59	2.860	0.30 (0.07)	0.24	22.7	27.00
1	64.39	12.66	2.851	0.30 (0.07)	0.24	22.8	38.00
1	64.88	13.31	2.771	0.30 (0.07)	0.24	23.8	20.00
1	65.09	13.79	2.717	0.30 (0.07)	0.24	24.4	17.00
1	65.09	13.80	2.716	0.30 (0.07)	0.24	24.5	23.00
1	65.10	13.84	2.711	0.30 (0.07)	0.24	24.5	29.00
1	65.19	14.70	2.620	0.30 (0.07)	0.23	25.6	50.00
1	64.81	15.13	2.578	0.30 (0.07)	0.23	25.9	39.00
1	64.78	15.15	2.575	0.30 (0.07)	0.23	25.9	34.00
1	64.30	15.50	2.543	0.30 (0.07)	0.23	26.1	24.00
1	60.27	18.63	2.291	0.30 (0.07)	0.23	27.5	12.00
1	57.40	21.23	2.128	0.30 (0.07)	0.22	28.6	64.00
1	56.46	22.10	2.080	0.30 (0.07)	0.22	28.9	32.00
1	56.33	22.17	2.076	0.30 (0.07)	0.22	28.9	10.00
2	0.61	8.29	3.623	0.30 (0.11)	0.35	0.2	65.00

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	56.53	8.29	3.623	0.30 (0.08)	0.25	15.3	65.00
2	60.21	9.67	3.321	0.30 (0.08)	0.25	17.8	51.00
3	60.74	9.85	3.286	0.30 (0.08)	0.25	18.2	58.00
4	61.22	10.03	3.253	0.30 (0.08)	0.25	18.6	43.00
5	61.38	10.10	3.240	0.30 (0.08)	0.25	18.7	55.00
6	61.62	10.20	3.222	0.30 (0.08)	0.25	18.9	45.00
7	61.71	10.24	3.215	0.30 (0.08)	0.25	19.0	59.00
8	62.28	10.52	3.165	0.30 (0.08)	0.25	19.5	63.00
9	62.39	10.58	3.155	0.30 (0.08)	0.25	19.6	30.00
10	62.85	10.91	3.102	0.30 (0.07)	0.25	20.2	60.00
11	63.04	11.04	3.082	0.30 (0.07)	0.25	20.4	38.00
12	63.58	11.43	3.021	0.30 (0.07)	0.24	21.1	30.00
13	63.71	11.55	3.003	0.30 (0.07)	0.24	21.3	36.00
14	64.46	12.27	2.902	0.30 (0.07)	0.24	22.4	14.00
15	64.76	12.56	2.865	0.30 (0.07)	0.24	22.8	26.00
16	64.80	12.59	2.860	0.30 (0.07)	0.24	22.9	27.00
17	64.86	12.66	2.851	0.30 (0.07)	0.24	23.0	38.00
18	65.34	13.31	2.771	0.30 (0.07)	0.24	24.0	20.00
19	65.54	13.79	2.717	0.30 (0.07)	0.24	24.6	17.00
20	65.54	13.80	2.716	0.30 (0.07)	0.24	24.6	23.00
21	65.55	13.84	2.711	0.30 (0.07)	0.24	24.7	29.00
22	65.63	14.70	2.620	0.30 (0.07)	0.24	25.7	50.00
23	65.24	15.13	2.578	0.30 (0.07)	0.24	26.1	39.00
24	65.20	15.15	2.575	0.30 (0.07)	0.24	26.1	34.00
25	64.72	15.50	2.543	0.30 (0.07)	0.23	26.3	24.00
26	60.65	18.63	2.291	0.30 (0.07)	0.23	27.7	12.00
27	57.75	21.23	2.128	0.30 (0.07)	0.23	28.8	64.00
28	56.80	22.10	2.080	0.30 (0.07)	0.22	29.1	32.00
29	56.67	22.17	2.076	0.30 (0.07)	0.22	29.1	10.00

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE (CFS) = 65.63 Tc (MIN.) = 14.70
 EFFECTIVE AREA (ACRES) = 25.75 AREA-AVERAGED Fm (INCH/HR) = 0.07
 AREA-AVERAGED Fp (INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.24
 TOTAL AREA (ACRES) = 29.1
 LONGEST FLOWPATH FROM NODE 10.00 TO NODE 57.00 = 2385.00 FEET.

 FLOW PROCESS FROM NODE 57.00 TO NODE 67.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
 >>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<

=====

ELEVATION DATA: UPSTREAM (FEET) = 149.10 DOWNSTREAM (FEET) = 148.50
 FLOW LENGTH (FEET) = 275.00 MANNING'S N = 0.013
 DEPTH OF FLOW IN 51.0 INCH PIPE IS 36.2 INCHES
 PIPE-FLOW VELOCITY (FEET/SEC.) = 6.09
 ESTIMATED PIPE DIAMETER (INCH) = 51.00 NUMBER OF PIPES = 1
 PIPE-FLOW (CFS) = 65.63
 PIPE TRAVEL TIME (MIN.) = 0.75 Tc (MIN.) = 15.46
 LONGEST FLOWPATH FROM NODE 10.00 TO NODE 67.00 = 2660.00 FEET.

 FLOW PROCESS FROM NODE 67.00 TO NODE 67.00 IS CODE = 10

>>>>MAIN-STREAM MEMORY COPIED ONTO MEMORY BANK # 1 <<<<<

 FLOW PROCESS FROM NODE 68.00 TO NODE 69.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
 >>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

=====

INITIAL SUBAREA FLOW-LENGTH (FEET) = 365.00
 ELEVATION DATA: UPSTREAM (FEET) = 168.20 DOWNSTREAM (FEET) = 167.30

$$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$$

SUBAREA ANALYSIS USED MINIMUM Tc (MIN.) = 11.404

* 25 YEAR RAINFALL INTENSITY (INCH/HR) = 3.025

SUBAREA Tc AND LOSS RATE DATA (AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN	Tc (MIN.)
APARTMENTS	B	1.00	0.30	0.200	56	11.40

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp (INCH/HR) = 0.30

SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.200

SUBAREA RUNOFF (CFS) = 2.67

TOTAL AREA (ACRES) = 1.00 PEAK FLOW RATE (CFS) = 2.67

 FLOW PROCESS FROM NODE 69.00 TO NODE 70.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
 >>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 162.00 DOWNSTREAM(FEET) = 159.00
 FLOW LENGTH(FEET) = 185.00 MANNING'S N = 0.013
 DEPTH OF FLOW IN 12.0 INCH PIPE IS 6.7 INCHES
 PIPE-FLOW VELOCITY(FEET/SEC.) = 5.89
 ESTIMATED PIPE DIAMETER(INCH) = 12.00 NUMBER OF PIPES = 1
 PIPE-FLOW(CFS) = 2.67
 PIPE TRAVEL TIME(MIN.) = 0.52 Tc(MIN.) = 11.93
 LONGEST FLOWPATH FROM NODE 68.00 TO NODE 70.00 = 550.00 FEET.

 FLOW PROCESS FROM NODE 70.00 TO NODE 71.00 IS CODE = 1

 >>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

=====

TOTAL NUMBER OF STREAMS = 2
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
 TIME OF CONCENTRATION(MIN.) = 11.93
 RAINFALL INTENSITY(INCH/HR) = 2.95
 AREA-AVERAGED Fm(INCH/HR) = 0.06
 AREA-AVERAGED Fp(INCH/HR) = 0.30
 AREA-AVERAGED Ap = 0.20
 EFFECTIVE STREAM AREA(ACRES) = 1.00
 TOTAL STREAM AREA(ACRES) = 1.00
 PEAK FLOW RATE(CFS) AT CONFLUENCE = 2.67

 FLOW PROCESS FROM NODE 71.00 TO NODE 70.00 IS CODE = 21

 >>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
 >>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

=====

INITIAL SUBAREA FLOW-LENGTH(FEET) = 250.00
 ELEVATION DATA: UPSTREAM(FEET) = 167.20 DOWNSTREAM(FEET) = 166.80

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$
 SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) = 10.688
 * 25 YEAR RAINFALL INTENSITY(INCH/HR) = 3.138
 SUBAREA Tc AND LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN	Tc (MIN.)
APARTMENTS	B	0.66	0.30	0.200	56	10.69

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.30
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.200
 SUBAREA RUNOFF(CFS) = 1.83
 TOTAL AREA(ACRES) = 0.66 PEAK FLOW RATE(CFS) = 1.83

 FLOW PROCESS FROM NODE 70.00 TO NODE 70.00 IS CODE = 1

 >>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
 >>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<

=====

TOTAL NUMBER OF STREAMS = 2
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:

TIME OF CONCENTRATION (MIN.) = 10.69
 RAINFALL INTENSITY (INCH/HR) = 3.14
 AREA-AVERAGED Fm (INCH/HR) = 0.06
 AREA-AVERAGED Fp (INCH/HR) = 0.30
 AREA-AVERAGED Ap = 0.20
 EFFECTIVE STREAM AREA (ACRES) = 0.66
 TOTAL STREAM AREA (ACRES) = 0.66
 PEAK FLOW RATE (CFS) AT CONFLUENCE = 1.83

** CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp (Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	2.67	11.93	2.949	0.30 (0.06)	0.20	1.0	68.00
2	1.83	10.69	3.138	0.30 (0.06)	0.20	0.7	71.00

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
 CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp (Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	4.38	10.69	3.138	0.30 (0.06)	0.20	1.6	71.00
2	4.38	11.93	2.949	0.30 (0.06)	0.20	1.7	68.00

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE (CFS) = 4.38 Tc (MIN.) = 11.93
 EFFECTIVE AREA (ACRES) = 1.66 AREA-AVERAGED Fm (INCH/HR) = 0.06
 AREA-AVERAGED Fp (INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.20
 TOTAL AREA (ACRES) = 1.7
 LONGEST FLOWPATH FROM NODE 68.00 TO NODE 70.00 = 550.00 FEET.

FLOW PROCESS FROM NODE 33.00 TO NODE 33.00 IS CODE = 12

>>>>CLEAR MEMORY BANK # 3 <<<<<

FLOW PROCESS FROM NODE 70.00 TO NODE 70.00 IS CODE = 10

>>>>MAIN-STREAM MEMORY COPIED ONTO MEMORY BANK # 3 <<<<<

FLOW PROCESS FROM NODE 72.00 TO NODE 73.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<

>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

INITIAL SUBAREA FLOW-LENGTH (FEET) = 230.00
 ELEVATION DATA: UPSTREAM (FEET) = 168.00 DOWNSTREAM (FEET) = 166.80

Tc = K * [(LENGTH** 3.00) / (ELEVATION CHANGE)]**0.20

SUBAREA ANALYSIS USED MINIMUM Tc (MIN.) = 8.161

* 25 YEAR RAINFALL INTENSITY (INCH/HR) = 3.656
 SUBAREA Tc AND LOSS RATE DATA (AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN	Tc (MIN.)
APARTMENTS	B	0.28	0.30	0.200	56	8.16

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp (INCH/HR) = 0.30
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.200
 SUBAREA RUNOFF (CFS) = 0.91
 TOTAL AREA (ACRES) = 0.28 PEAK FLOW RATE (CFS) = 0.91

 FLOW PROCESS FROM NODE 73.00 TO NODE 70.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
 >>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<

=====

ELEVATION DATA: UPSTREAM (FEET) = 160.00 DOWNSTREAM (FEET) = 159.00
 FLOW LENGTH (FEET) = 270.00 MANNING'S N = 0.013
 ESTIMATED PIPE DIAMETER (INCH) INCREASED TO 12.000
 DEPTH OF FLOW IN 12.0 INCH PIPE IS 5.5 INCHES
 PIPE-FLOW VELOCITY (FEET/SEC.) = 2.59
 ESTIMATED PIPE DIAMETER (INCH) = 12.000 NUMBER OF PIPES = 1
 PIPE-FLOW (CFS) = 0.91
 PIPE TRAVEL TIME (MIN.) = 1.74 Tc (MIN.) = 9.90
 LONGEST FLOWPATH FROM NODE 72.00 TO NODE 70.00 = 500.00 FEET.

 FLOW PROCESS FROM NODE 70.00 TO NODE 70.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

=====

TOTAL NUMBER OF STREAMS = 2
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
 TIME OF CONCENTRATION (MIN.) = 9.90
 RAINFALL INTENSITY (INCH/HR) = 3.28
 AREA-AVERAGED Fm (INCH/HR) = 0.06
 AREA-AVERAGED Fp (INCH/HR) = 0.30
 AREA-AVERAGED Ap = 0.20
 EFFECTIVE STREAM AREA (ACRES) = 0.28
 TOTAL STREAM AREA (ACRES) = 0.28
 PEAK FLOW RATE (CFS) AT CONFLUENCE = 0.91

 FLOW PROCESS FROM NODE 72.00 TO NODE 70.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
 >>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

=====

INITIAL SUBAREA FLOW-LENGTH (FEET) = 390.00
 ELEVATION DATA: UPSTREAM (FEET) = 167.20 DOWNSTREAM (FEET) = 166.80

Tc = K * [(LENGTH** 3.00) / (ELEVATION CHANGE)]**0.20
 SUBAREA ANALYSIS USED MINIMUM Tc (MIN.) = 13.956
 * 25 YEAR RAINFALL INTENSITY (INCH/HR) = 2.698

SUBAREA Tc AND LOSS RATE DATA (AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN	Tc (MIN.)
APARTMENTS	B	1.49	0.30	0.200	56	13.96

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp (INCH/HR) = 0.30
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.200
 SUBAREA RUNOFF (CFS) = 3.54
 TOTAL AREA (ACRES) = 1.49 PEAK FLOW RATE (CFS) = 3.54

 FLOW PROCESS FROM NODE 70.00 TO NODE 70.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
 >>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<

=====

TOTAL NUMBER OF STREAMS = 2
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
 TIME OF CONCENTRATION (MIN.) = 13.96
 RAINFALL INTENSITY (INCH/HR) = 2.70
 AREA-AVERAGED Fm (INCH/HR) = 0.06
 AREA-AVERAGED Fp (INCH/HR) = 0.30
 AREA-AVERAGED Ap = 0.20
 EFFECTIVE STREAM AREA (ACRES) = 1.49
 TOTAL STREAM AREA (ACRES) = 1.49
 PEAK FLOW RATE (CFS) AT CONFLUENCE = 3.54

** CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp (Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	0.91	9.90	3.277	0.30 (0.06)	0.20	0.3	72.00
2	3.54	13.96	2.698	0.30 (0.06)	0.20	1.5	72.00

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
 CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp (Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	3.97	9.90	3.277	0.30 (0.06)	0.20	1.3	72.00
2	4.28	13.96	2.698	0.30 (0.06)	0.20	1.8	72.00

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE (CFS) = 4.28 Tc (MIN.) = 13.96
 EFFECTIVE AREA (ACRES) = 1.77 AREA-AVERAGED Fm (INCH/HR) = 0.06
 AREA-AVERAGED Fp (INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.20
 TOTAL AREA (ACRES) = 1.8
 LONGEST FLOWPATH FROM NODE 72.00 TO NODE 70.00 = 500.00 FEET.

 FLOW PROCESS FROM NODE 70.00 TO NODE 70.00 IS CODE = 11

>>>>CONFLUENCE MEMORY BANK # 3 WITH THE MAIN-STREAM MEMORY<<<<<

** MAIN STREAM CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	3.97	9.90	3.277	0.30 (0.06)	0.20	1.3	72.00
2	4.28	13.96	2.698	0.30 (0.06)	0.20	1.8	72.00
LONGEST FLOWPATH FROM NODE			72.00	TO NODE	70.00 =	500.00	FEET.

** MEMORY BANK # 3 CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	4.38	10.69	3.138	0.30 (0.06)	0.20	1.6	71.00
2	4.38	11.93	2.949	0.30 (0.06)	0.20	1.7	68.00
LONGEST FLOWPATH FROM NODE			68.00	TO NODE	70.00 =	550.00	FEET.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	8.20	9.90	3.277	0.30 (0.06)	0.20	2.8	72.00
2	8.40	10.69	3.138	0.30 (0.06)	0.20	3.0	71.00
3	8.51	11.93	2.949	0.30 (0.06)	0.20	3.2	68.00
4	8.28	13.96	2.698	0.30 (0.06)	0.20	3.4	72.00
TOTAL AREA (ACRES) =			3.4				

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE (CFS) = 8.51 Tc (MIN.) = 11.928
 EFFECTIVE AREA (ACRES) = 3.21 AREA-AVERAGED Fm (INCH/HR) = 0.06
 AREA-AVERAGED Fp (INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.20
 TOTAL AREA (ACRES) = 3.4
 LONGEST FLOWPATH FROM NODE 68.00 TO NODE 70.00 = 550.00 FEET.

FLOW PROCESS FROM NODE 70.00 TO NODE 70.00 IS CODE = 12

>>>>CLEAR MEMORY BANK # 3 <<<<<

FLOW PROCESS FROM NODE 70.00 TO NODE 67.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<

>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<

ELEVATION DATA: UPSTREAM (FEET) = 155.00 DOWNSTREAM (FEET) = 148.50
 FLOW LENGTH (FEET) = 30.00 MANNING'S N = 0.013
 DEPTH OF FLOW IN 12.0 INCH PIPE IS 6.2 INCHES
 PIPE-FLOW VELOCITY (FEET/SEC.) = 20.84
 ESTIMATED PIPE DIAMETER (INCH) = 12.00 NUMBER OF PIPES = 1
 PIPE-FLOW (CFS) = 8.51
 PIPE TRAVEL TIME (MIN.) = 0.02 Tc (MIN.) = 11.95
 LONGEST FLOWPATH FROM NODE 68.00 TO NODE 67.00 = 580.00 FEET.

FLOW PROCESS FROM NODE 67.00 TO NODE 67.00 IS CODE = 11

>>>>CONFLUENCE MEMORY BANK # 1 WITH THE MAIN-STREAM MEMORY<<<<<

=====
 ** MAIN STREAM CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	8.20	9.93	3.272	0.30 (0.06)	0.20	2.8	72.00
2	8.40	10.71	3.134	0.30 (0.06)	0.20	3.0	71.00
3	8.51	11.95	2.946	0.30 (0.06)	0.20	3.2	68.00
4	8.28	13.98	2.695	0.30 (0.06)	0.20	3.4	72.00
LONGEST FLOWPATH FROM NODE			68.00	TO NODE	67.00 =	580.00 FEET.	

** MEMORY BANK # 1 CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	56.53	9.07	3.443	0.30 (0.08)	0.25	15.3	65.00
2	60.21	10.44	3.179	0.30 (0.08)	0.25	17.8	51.00
3	60.74	10.63	3.148	0.30 (0.08)	0.25	18.2	58.00
4	61.22	10.81	3.118	0.30 (0.08)	0.25	18.6	43.00
5	61.38	10.87	3.108	0.30 (0.08)	0.25	18.7	55.00
6	61.62	10.97	3.091	0.30 (0.08)	0.25	18.9	45.00
7	61.71	11.01	3.085	0.30 (0.08)	0.25	19.0	59.00
8	62.28	11.30	3.041	0.30 (0.08)	0.25	19.5	63.00
9	62.39	11.36	3.032	0.30 (0.08)	0.25	19.6	30.00
10	62.85	11.68	2.984	0.30 (0.07)	0.25	20.2	60.00
11	63.04	11.81	2.966	0.30 (0.07)	0.25	20.4	38.00
12	63.58	12.21	2.911	0.30 (0.07)	0.24	21.1	30.00
13	63.71	12.32	2.895	0.30 (0.07)	0.24	21.3	36.00
14	64.46	13.04	2.804	0.30 (0.07)	0.24	22.4	14.00
15	64.76	13.33	2.769	0.30 (0.07)	0.24	22.8	26.00
16	64.80	13.37	2.765	0.30 (0.07)	0.24	22.9	27.00
17	64.86	13.43	2.757	0.30 (0.07)	0.24	23.0	38.00
18	65.34	14.09	2.684	0.30 (0.07)	0.24	24.0	20.00
19	65.54	14.54	2.636	0.30 (0.07)	0.24	24.6	17.00
20	65.54	14.55	2.635	0.30 (0.07)	0.24	24.6	23.00
21	65.55	14.60	2.630	0.30 (0.07)	0.24	24.7	29.00
22	65.63	15.46	2.547	0.30 (0.07)	0.24	25.7	50.00
23	65.24	15.90	2.506	0.30 (0.07)	0.24	26.1	39.00
24	65.20	15.93	2.504	0.30 (0.07)	0.24	26.1	34.00
25	64.72	16.27	2.474	0.30 (0.07)	0.23	26.3	24.00
26	60.65	19.40	2.239	0.30 (0.07)	0.23	27.7	12.00
27	57.75	22.01	2.085	0.30 (0.07)	0.23	28.8	64.00
28	56.80	22.88	2.040	0.30 (0.07)	0.22	29.1	32.00
29	56.67	22.95	2.036	0.30 (0.07)	0.22	29.1	10.00
LONGEST FLOWPATH FROM NODE			10.00	TO NODE	67.00 =	2660.00 FEET.	

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	64.42	9.07	3.443	0.30 (0.07)	0.24	17.9	65.00
2	67.02	9.93	3.272	0.30 (0.07)	0.24	19.7	72.00
3	68.55	10.44	3.179	0.30 (0.07)	0.24	20.8	51.00
4	69.12	10.63	3.148	0.30 (0.07)	0.24	21.2	58.00
5	69.37	10.71	3.134	0.30 (0.07)	0.24	21.4	71.00

6	69.63	10.81	3.118	0.30	(0.07)	0.24	21.6	43.00
7	69.80	10.87	3.108	0.30	(0.07)	0.24	21.7	55.00
8	70.05	10.97	3.091	0.30	(0.07)	0.24	22.0	45.00
9	70.14	11.01	3.085	0.30	(0.07)	0.24	22.0	59.00
10	70.73	11.30	3.041	0.30	(0.07)	0.24	22.6	63.00
11	70.84	11.36	3.032	0.30	(0.07)	0.24	22.7	30.00
12	71.34	11.68	2.984	0.30	(0.07)	0.24	23.4	60.00
13	71.53	11.81	2.966	0.30	(0.07)	0.24	23.6	38.00
14	71.74	11.95	2.946	0.30	(0.07)	0.24	23.9	68.00
15	72.06	12.21	2.911	0.30	(0.07)	0.24	24.3	30.00
16	72.18	12.32	2.895	0.30	(0.07)	0.24	24.5	36.00
17	72.85	13.04	2.804	0.30	(0.07)	0.24	25.7	14.00
18	73.12	13.33	2.769	0.30	(0.07)	0.24	26.2	26.00
19	73.15	13.37	2.765	0.30	(0.07)	0.24	26.3	27.00
20	73.21	13.43	2.757	0.30	(0.07)	0.24	26.4	38.00
21	73.54	13.98	2.695	0.30	(0.07)	0.24	27.2	72.00
22	73.58	14.09	2.684	0.30	(0.07)	0.24	27.4	20.00
23	73.64	14.54	2.636	0.30	(0.07)	0.23	28.1	17.00
24	73.64	14.55	2.635	0.30	(0.07)	0.23	28.1	23.00
25	73.63	14.60	2.630	0.30	(0.07)	0.23	28.1	29.00
26	73.44	15.46	2.547	0.30	(0.07)	0.23	29.2	50.00
27	72.93	15.90	2.506	0.30	(0.07)	0.23	29.5	39.00
28	72.88	15.93	2.504	0.30	(0.07)	0.23	29.5	34.00
29	72.30	16.27	2.474	0.30	(0.07)	0.23	29.7	24.00
30	67.50	19.40	2.239	0.30	(0.07)	0.23	31.2	12.00
31	64.11	22.01	2.085	0.30	(0.07)	0.22	32.2	64.00
32	63.02	22.88	2.040	0.30	(0.07)	0.22	32.5	32.00
33	62.88	22.95	2.036	0.30	(0.07)	0.22	32.5	10.00
TOTAL AREA (ACRES) =			32.5					

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE (CFS) = 73.64 Tc (MIN.) = 14.550
EFFECTIVE AREA (ACRES) = 28.07 AREA-AVERAGED Fm (INCH/HR) = 0.07
AREA-AVERAGED Fp (INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.23
TOTAL AREA (ACRES) = 32.5
LONGEST FLOWPATH FROM NODE 10.00 TO NODE 67.00 = 2660.00 FEET.

FLOW PROCESS FROM NODE 67.00 TO NODE 74.00 IS CODE = 31

>>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<

=====

ELEVATION DATA: UPSTREAM (FEET) = 148.50 DOWNSTREAM (FEET) = 148.00
FLOW LENGTH (FEET) = 280.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 54.0 INCH PIPE IS 40.4 INCHES
PIPE-FLOW VELOCITY (FEET/SEC.) = 5.77
ESTIMATED PIPE DIAMETER (INCH) = 54.00 NUMBER OF PIPES = 1
PIPE-FLOW (CFS) = 73.64
PIPE TRAVEL TIME (MIN.) = 0.81 Tc (MIN.) = 15.36
LONGEST FLOWPATH FROM NODE 10.00 TO NODE 74.00 = 2940.00 FEET.

FLOW PROCESS FROM NODE 74.00 TO NODE 74.00 IS CODE = 10

=====
>>>>MAIN-STREAM MEMORY COPIED ONTO MEMORY BANK # 3 <<<<<
=====

FLOW PROCESS FROM NODE 75.00 TO NODE 76.00 IS CODE = 21
=====

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<
=====

INITIAL SUBAREA FLOW-LENGTH(FEET) = 275.00
ELEVATION DATA: UPSTREAM(FEET) = 167.00 DOWNSTREAM(FEET) = 165.30

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$

SUBAREA ANALYSIS USED MINIMUM T_c (MIN.) = 9.415

* 25 YEAR RAINFALL INTENSITY(INCH/HR) = 3.371

SUBAREA T_c AND LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	F_p (INCH/HR)	A_p (DECIMAL)	SCS CN	T_c (MIN.)
CONDOMINIUMS	B	0.70	0.30	0.350	56	9.41

SUBAREA AVERAGE PERVIOUS LOSS RATE, F_p (INCH/HR) = 0.30

SUBAREA AVERAGE PERVIOUS AREA FRACTION, A_p = 0.350

SUBAREA RUNOFF(CFS) = 2.06

TOTAL AREA(ACRES) = 0.70 PEAK FLOW RATE(CFS) = 2.06

FLOW PROCESS FROM NODE 76.00 TO NODE 76.00 IS CODE = 1
=====

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
=====

TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION(MIN.) = 9.41
RAINFALL INTENSITY(INCH/HR) = 3.37
AREA-AVERAGED F_m (INCH/HR) = 0.11
AREA-AVERAGED F_p (INCH/HR) = 0.30
AREA-AVERAGED A_p = 0.35
EFFECTIVE STREAM AREA(ACRES) = 0.70
TOTAL STREAM AREA(ACRES) = 0.70
PEAK FLOW RATE(CFS) AT CONFLUENCE = 2.06

FLOW PROCESS FROM NODE 77.00 TO NODE 76.00 IS CODE = 21
=====

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<
=====

INITIAL SUBAREA FLOW-LENGTH(FEET) = 255.00
ELEVATION DATA: UPSTREAM(FEET) = 167.00 DOWNSTREAM(FEET) = 166.30

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$

SUBAREA ANALYSIS USED MINIMUM T_c (MIN.) = 10.745

* 25 YEAR RAINFALL INTENSITY(INCH/HR) = 3.129

SUBAREA T_c AND LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ LAND USE SCS SOIL GROUP AREA (ACRES) Fp (INCH/HR) Ap (DECIMAL) SCS CN Tc (MIN.)

CONDOMINIUMS B 0.51 0.30 0.350 56 10.75

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp (INCH/HR) = 0.30

SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.350

SUBAREA RUNOFF (CFS) = 1.39

TOTAL AREA (ACRES) = 0.51 PEAK FLOW RATE (CFS) = 1.39

FLOW PROCESS FROM NODE 76.00 TO NODE 76.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<

=====

TOTAL NUMBER OF STREAMS = 2

CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:

TIME OF CONCENTRATION (MIN.) = 10.75

RAINFALL INTENSITY (INCH/HR) = 3.13

AREA-AVERAGED Fm (INCH/HR) = 0.11

AREA-AVERAGED Fp (INCH/HR) = 0.30

AREA-AVERAGED Ap = 0.35

EFFECTIVE STREAM AREA (ACRES) = 0.51

TOTAL STREAM AREA (ACRES) = 0.51

PEAK FLOW RATE (CFS) AT CONFLUENCE = 1.39

** CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp (Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	2.06	9.41	3.371	0.30 (0.11)	0.35	0.7	75.00
2	1.39	10.75	3.129	0.30 (0.11)	0.35	0.5	77.00

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO

CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp (Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	3.37	9.41	3.371	0.30 (0.11)	0.35	1.1	75.00
2	3.29	10.75	3.129	0.30 (0.10)	0.35	1.2	77.00

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE (CFS) = 3.37 Tc (MIN.) = 9.41

EFFECTIVE AREA (ACRES) = 1.15 AREA-AVERAGED Fm (INCH/HR) = 0.10

AREA-AVERAGED Fp (INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.35

TOTAL AREA (ACRES) = 1.2

LONGEST FLOWPATH FROM NODE 75.00 TO NODE 76.00 = 275.00 FEET.

FLOW PROCESS FROM NODE 76.00 TO NODE 79.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<

>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<

=====

ELEVATION DATA: UPSTREAM (FEET) = 154.00 DOWNSTREAM (FEET) = 152.00

FLOW LENGTH(FEET) = 300.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 15.0 INCH PIPE IS 8.9 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 4.47
ESTIMATED PIPE DIAMETER(INCH) = 15.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 3.37
PIPE TRAVEL TIME(MIN.) = 1.12 Tc(MIN.) = 10.53
LONGEST FLOWPATH FROM NODE 75.00 TO NODE 79.00 = 575.00 FEET.

FLOW PROCESS FROM NODE 79.00 TO NODE 79.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

=====

TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION(MIN.) = 10.53
RAINFALL INTENSITY(INCH/HR) = 3.16
AREA-AVERAGED Fm(INCH/HR) = 0.10
AREA-AVERAGED Fp(INCH/HR) = 0.30
AREA-AVERAGED Ap = 0.35
EFFECTIVE STREAM AREA(ACRES) = 1.15
TOTAL STREAM AREA(ACRES) = 1.21
PEAK FLOW RATE(CFS) AT CONFLUENCE = 3.37

FLOW PROCESS FROM NODE 78.00 TO NODE 79.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

=====

INITIAL SUBAREA FLOW-LENGTH(FEET) = 200.00
ELEVATION DATA: UPSTREAM(FEET) = 165.90 DOWNSTREAM(FEET) = 165.30

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$
SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) = 9.578
* 25 YEAR RAINFALL INTENSITY(INCH/HR) = 3.339
SUBAREA Tc AND LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN	Tc (MIN.)
CONDOMINIUMS	B	0.40	0.30	0.350	56	9.58

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.30
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.350
SUBAREA RUNOFF(CFS) = 1.16
TOTAL AREA(ACRES) = 0.40 PEAK FLOW RATE(CFS) = 1.16

FLOW PROCESS FROM NODE 79.00 TO NODE 79.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<

=====

TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
TIME OF CONCENTRATION(MIN.) = 9.58

RAINFALL INTENSITY (INCH/HR) = 3.34
 AREA-AVERAGED Fm (INCH/HR) = 0.11
 AREA-AVERAGED Fp (INCH/HR) = 0.30
 AREA-AVERAGED Ap = 0.35
 EFFECTIVE STREAM AREA (ACRES) = 0.40
 TOTAL STREAM AREA (ACRES) = 0.40
 PEAK FLOW RATE (CFS) AT CONFLUENCE = 1.16

** CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp (Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	3.37	10.53	3.164	0.30 (0.11)	0.35	1.1	75.00
1	3.29	11.87	2.957	0.30 (0.10)	0.35	1.2	77.00
2	1.16	9.58	3.339	0.30 (0.11)	0.35	0.4	78.00

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
 CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp (Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	4.41	9.58	3.339	0.30 (0.10)	0.35	1.4	78.00
2	4.47	10.53	3.164	0.30 (0.11)	0.35	1.5	75.00
3	4.32	11.87	2.957	0.30 (0.10)	0.35	1.6	77.00

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE (CFS) = 4.47 Tc (MIN.) = 10.53
 EFFECTIVE AREA (ACRES) = 1.55 AREA-AVERAGED Fm (INCH/HR) = 0.11
 AREA-AVERAGED Fp (INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.35
 TOTAL AREA (ACRES) = 1.6
 LONGEST FLOWPATH FROM NODE 75.00 TO NODE 79.00 = 575.00 FEET.

FLOW PROCESS FROM NODE 79.00 TO NODE 74.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
 >>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<

=====
 ELEVATION DATA: UPSTREAM (FEET) = 152.00 DOWNSTREAM (FEET) = 148.00
 FLOW LENGTH (FEET) = 50.00 MANNING'S N = 0.013
 ESTIMATED PIPE DIAMETER (INCH) INCREASED TO 12.000
 DEPTH OF FLOW IN 12.0 INCH PIPE IS 5.7 INCHES
 PIPE-FLOW VELOCITY (FEET/SEC.) = 12.21
 ESTIMATED PIPE DIAMETER (INCH) = 12.00 NUMBER OF PIPES = 1
 PIPE-FLOW (CFS) = 4.47
 PIPE TRAVEL TIME (MIN.) = 0.07 Tc (MIN.) = 10.60
 LONGEST FLOWPATH FROM NODE 75.00 TO NODE 74.00 = 625.00 FEET.

FLOW PROCESS FROM NODE 74.00 TO NODE 74.00 IS CODE = 11

>>>>CONFLUENCE MEMORY BANK # 3 WITH THE MAIN-STREAM MEMORY<<<<<

=====

** MAIN STREAM CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	4.41	9.65	3.325	0.30 (0.10)	0.35	1.4	78.00
2	4.47	10.60	3.152	0.30 (0.11)	0.35	1.5	75.00
3	4.32	11.94	2.947	0.30 (0.10)	0.35	1.6	77.00
LONGEST FLOWPATH FROM NODE				75.00 TO NODE	74.00 =	625.00 FEET.	

** MEMORY BANK # 3 CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	64.42	9.91	3.275	0.30 (0.07)	0.24	17.9	65.00
2	67.02	10.76	3.126	0.30 (0.07)	0.24	19.7	72.00
3	68.55	11.28	3.044	0.30 (0.07)	0.24	20.8	51.00
4	69.12	11.46	3.016	0.30 (0.07)	0.24	21.2	58.00
5	69.37	11.55	3.004	0.30 (0.07)	0.24	21.4	71.00
6	69.63	11.62	2.993	0.30 (0.07)	0.24	21.6	43.00
7	69.80	11.69	2.983	0.30 (0.07)	0.24	21.7	55.00
8	70.05	11.79	2.969	0.30 (0.07)	0.24	22.0	45.00
9	70.14	11.83	2.963	0.30 (0.07)	0.24	22.0	59.00
10	70.73	12.11	2.924	0.30 (0.07)	0.24	22.6	63.00
11	70.84	12.17	2.915	0.30 (0.07)	0.24	22.7	30.00
12	71.34	12.49	2.873	0.30 (0.07)	0.24	23.4	60.00
13	71.53	12.62	2.856	0.30 (0.07)	0.24	23.6	38.00
14	71.74	12.76	2.838	0.30 (0.07)	0.24	23.9	68.00
15	72.06	13.02	2.807	0.30 (0.07)	0.24	24.3	30.00
16	72.18	13.13	2.793	0.30 (0.07)	0.24	24.5	36.00
17	72.85	13.85	2.710	0.30 (0.07)	0.24	25.7	14.00
18	73.12	14.14	2.678	0.30 (0.07)	0.24	26.2	26.00
19	73.15	14.18	2.674	0.30 (0.07)	0.24	26.3	27.00
20	73.21	14.24	2.667	0.30 (0.07)	0.24	26.4	38.00
21	73.54	14.79	2.611	0.30 (0.07)	0.24	27.2	72.00
22	73.58	14.90	2.600	0.30 (0.07)	0.24	27.4	20.00
23	73.64	15.35	2.556	0.30 (0.07)	0.23	28.1	17.00
24	73.64	15.36	2.556	0.30 (0.07)	0.23	28.1	23.00
25	73.63	15.41	2.551	0.30 (0.07)	0.23	28.1	29.00
26	73.44	16.27	2.474	0.30 (0.07)	0.23	29.2	50.00
27	72.93	16.71	2.437	0.30 (0.07)	0.23	29.5	39.00
28	72.88	16.74	2.434	0.30 (0.07)	0.23	29.5	34.00
29	72.30	17.08	2.406	0.30 (0.07)	0.23	29.7	24.00
30	67.50	20.24	2.186	0.30 (0.07)	0.23	31.2	12.00
31	64.11	22.85	2.041	0.30 (0.07)	0.22	32.2	64.00
32	63.02	23.72	1.998	0.30 (0.07)	0.22	32.5	32.00
33	62.88	23.79	1.995	0.30 (0.07)	0.22	32.5	10.00
LONGEST FLOWPATH FROM NODE				10.00 TO NODE	74.00 =	2940.00 FEET.	

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	68.10	9.65	3.325	0.30 (0.08)	0.25	18.8	78.00
2	68.85	9.91	3.275	0.30 (0.08)	0.25	19.3	65.00
3	71.01	10.60	3.152	0.30 (0.08)	0.25	20.9	75.00
4	71.48	10.76	3.126	0.30 (0.08)	0.25	21.2	72.00
5	72.94	11.28	3.044	0.30 (0.08)	0.25	22.3	51.00

6	73.50	11.46	3.016	0.30	(0.08)	0.25	22.8	58.00
7	73.74	11.55	3.004	0.30	(0.08)	0.25	23.0	71.00
8	73.99	11.62	2.993	0.30	(0.08)	0.25	23.2	43.00
9	74.15	11.69	2.983	0.30	(0.08)	0.25	23.3	55.00
10	74.39	11.79	2.969	0.30	(0.08)	0.25	23.6	45.00
11	74.47	11.83	2.963	0.30	(0.08)	0.25	23.7	59.00
12	74.69	11.94	2.947	0.30	(0.08)	0.25	23.9	77.00
13	75.01	12.11	2.924	0.30	(0.08)	0.25	24.2	63.00
14	75.12	12.17	2.915	0.30	(0.08)	0.25	24.4	30.00
15	75.54	12.49	2.873	0.30	(0.08)	0.25	25.0	60.00
16	75.71	12.62	2.856	0.30	(0.07)	0.25	25.2	38.00
17	75.89	12.76	2.838	0.30	(0.07)	0.25	25.5	68.00
18	76.16	13.02	2.807	0.30	(0.07)	0.25	25.9	30.00
19	76.26	13.13	2.793	0.30	(0.07)	0.25	26.1	36.00
20	76.81	13.85	2.710	0.30	(0.07)	0.24	27.3	14.00
21	77.03	14.14	2.678	0.30	(0.07)	0.24	27.8	26.00
22	77.06	14.18	2.674	0.30	(0.07)	0.24	27.9	27.00
23	77.10	14.24	2.667	0.30	(0.07)	0.24	28.0	38.00
24	77.35	14.79	2.611	0.30	(0.07)	0.24	28.9	72.00
25	77.38	14.90	2.600	0.30	(0.07)	0.24	29.0	20.00
26	77.36	15.35	2.556	0.30	(0.07)	0.24	29.7	17.00
27	77.36	15.36	2.556	0.30	(0.07)	0.24	29.7	23.00
28	77.35	15.41	2.551	0.30	(0.07)	0.24	29.7	29.00
29	77.04	16.27	2.474	0.30	(0.07)	0.24	30.8	50.00
30	76.47	16.71	2.437	0.30	(0.07)	0.24	31.1	39.00
31	76.42	16.74	2.434	0.30	(0.07)	0.24	31.1	34.00
32	75.80	17.08	2.406	0.30	(0.07)	0.24	31.3	24.00
33	70.66	20.24	2.186	0.30	(0.07)	0.23	32.8	12.00
34	67.05	22.85	2.041	0.30	(0.07)	0.23	33.8	64.00
35	65.90	23.72	1.998	0.30	(0.07)	0.23	34.1	32.00
36	65.76	23.79	1.995	0.30	(0.07)	0.23	34.1	10.00
TOTAL AREA (ACRES) =			34.1					

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE (CFS) = 77.38 Tc (MIN.) = 14.896
EFFECTIVE AREA (ACRES) = 29.02 AREA-AVERAGED Fm (INCH/HR) = 0.07
AREA-AVERAGED Fp (INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.24
TOTAL AREA (ACRES) = 34.1
LONGEST FLOWPATH FROM NODE 10.00 TO NODE 74.00 = 2940.00 FEET.

FLOW PROCESS FROM NODE 74.00 TO NODE 74.00 IS CODE = 12

>>>>CLEAR MEMORY BANK # 3 <<<<<

FLOW PROCESS FROM NODE 74.00 TO NODE 74.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION (MIN.) = 14.90

RAINFALL INTENSITY (INCH/HR) = 2.60
 AREA-AVERAGED Fm (INCH/HR) = 0.07
 AREA-AVERAGED Fp (INCH/HR) = 0.30
 AREA-AVERAGED Ap = 0.24
 EFFECTIVE STREAM AREA (ACRES) = 29.02
 TOTAL STREAM AREA (ACRES) = 34.13
 PEAK FLOW RATE (CFS) AT CONFLUENCE = 77.38

 FLOW PROCESS FROM NODE 59.00 TO NODE 74.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
 >>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

=====
 INITIAL SUBAREA FLOW-LENGTH (FEET) = 300.00
 ELEVATION DATA: UPSTREAM (FEET) = 166.80 DOWNSTREAM (FEET) = 165.00

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$
 SUBAREA ANALYSIS USED MINIMUM Tc (MIN.) = 9.807
 * 25 YEAR RAINFALL INTENSITY (INCH/HR) = 3.295

SUBAREA Tc AND LOSS RATE DATA (AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN	Tc (MIN.)
CONDOMINIUMS	B	0.41	0.30	0.350	56	9.81

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp (INCH/HR) = 0.30

SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.350

SUBAREA RUNOFF (CFS) = 1.18

TOTAL AREA (ACRES) = 0.41 PEAK FLOW RATE (CFS) = 1.18

 FLOW PROCESS FROM NODE 74.00 TO NODE 74.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
 >>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<

=====
 TOTAL NUMBER OF STREAMS = 2
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:

TIME OF CONCENTRATION (MIN.) = 9.81
 RAINFALL INTENSITY (INCH/HR) = 3.29
 AREA-AVERAGED Fm (INCH/HR) = 0.11
 AREA-AVERAGED Fp (INCH/HR) = 0.30
 AREA-AVERAGED Ap = 0.35
 EFFECTIVE STREAM AREA (ACRES) = 0.41
 TOTAL STREAM AREA (ACRES) = 0.41
 PEAK FLOW RATE (CFS) AT CONFLUENCE = 1.18

** CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp (Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	68.10	9.65	3.325	0.30 (0.08)	0.25	18.8	78.00
1	68.85	9.91	3.275	0.30 (0.08)	0.25	19.3	65.00
1	71.01	10.60	3.152	0.30 (0.08)	0.25	20.9	75.00
1	71.48	10.76	3.126	0.30 (0.08)	0.25	21.2	72.00
1	72.94	11.28	3.044	0.30 (0.08)	0.25	22.3	51.00

1	73.50	11.46	3.016	0.30	(0.08)	0.25	22.8	58.00
1	73.74	11.55	3.004	0.30	(0.08)	0.25	23.0	71.00
1	73.99	11.62	2.993	0.30	(0.08)	0.25	23.2	43.00
1	74.15	11.69	2.983	0.30	(0.08)	0.25	23.3	55.00
1	74.39	11.79	2.969	0.30	(0.08)	0.25	23.6	45.00
1	74.47	11.83	2.963	0.30	(0.08)	0.25	23.7	59.00
1	74.69	11.94	2.947	0.30	(0.08)	0.25	23.9	77.00
1	75.01	12.11	2.924	0.30	(0.08)	0.25	24.2	63.00
1	75.12	12.17	2.915	0.30	(0.08)	0.25	24.4	30.00
1	75.54	12.49	2.873	0.30	(0.08)	0.25	25.0	60.00
1	75.71	12.62	2.856	0.30	(0.07)	0.25	25.2	38.00
1	75.89	12.76	2.838	0.30	(0.07)	0.25	25.5	68.00
1	76.16	13.02	2.807	0.30	(0.07)	0.25	25.9	30.00
1	76.26	13.13	2.793	0.30	(0.07)	0.25	26.1	36.00
1	76.81	13.85	2.710	0.30	(0.07)	0.24	27.3	14.00
1	77.03	14.14	2.678	0.30	(0.07)	0.24	27.8	26.00
1	77.06	14.18	2.674	0.30	(0.07)	0.24	27.9	27.00
1	77.10	14.24	2.667	0.30	(0.07)	0.24	28.0	38.00
1	77.35	14.79	2.611	0.30	(0.07)	0.24	28.9	72.00
1	77.38	14.90	2.600	0.30	(0.07)	0.24	29.0	20.00
1	77.36	15.35	2.556	0.30	(0.07)	0.24	29.7	17.00
1	77.36	15.36	2.556	0.30	(0.07)	0.24	29.7	23.00
1	77.35	15.41	2.551	0.30	(0.07)	0.24	29.7	29.00
1	77.04	16.27	2.474	0.30	(0.07)	0.24	30.8	50.00
1	76.47	16.71	2.437	0.30	(0.07)	0.24	31.1	39.00
1	76.42	16.74	2.434	0.30	(0.07)	0.24	31.1	34.00
1	75.80	17.08	2.406	0.30	(0.07)	0.24	31.3	24.00
1	70.66	20.24	2.186	0.30	(0.07)	0.23	32.8	12.00
1	67.05	22.85	2.041	0.30	(0.07)	0.23	33.8	64.00
1	65.90	23.72	1.998	0.30	(0.07)	0.23	34.1	32.00
1	65.76	23.79	1.995	0.30	(0.07)	0.23	34.1	10.00
2	1.18	9.81	3.295	0.30	(0.11)	0.35	0.4	59.00

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE	
1	69.26	9.65	3.325	0.30	(0.08)	0.25	19.2	78.00
2	69.72	9.81	3.295	0.30	(0.08)	0.25	19.5	59.00
3	70.02	9.91	3.275	0.30	(0.08)	0.25	19.8	65.00
4	72.13	10.60	3.152	0.30	(0.08)	0.25	21.3	75.00
5	72.59	10.76	3.126	0.30	(0.08)	0.25	21.6	72.00
6	74.03	11.28	3.044	0.30	(0.08)	0.25	22.7	51.00
7	74.57	11.46	3.016	0.30	(0.08)	0.25	23.2	58.00
8	74.81	11.55	3.004	0.30	(0.08)	0.25	23.4	71.00
9	75.06	11.62	2.993	0.30	(0.08)	0.25	23.6	43.00
10	75.21	11.69	2.983	0.30	(0.08)	0.25	23.8	55.00
11	75.44	11.79	2.969	0.30	(0.08)	0.25	24.0	45.00
12	75.52	11.83	2.963	0.30	(0.08)	0.25	24.1	59.00
13	75.74	11.94	2.947	0.30	(0.08)	0.25	24.3	77.00
14	76.05	12.11	2.924	0.30	(0.08)	0.25	24.6	63.00
15	76.15	12.17	2.915	0.30	(0.08)	0.25	24.8	30.00

16	76.56	12.49	2.873	0.30	(0.08)	0.25	25.4	60.00
17	76.73	12.62	2.856	0.30	(0.08)	0.25	25.6	38.00
18	76.90	12.76	2.838	0.30	(0.08)	0.25	25.9	68.00
19	77.16	13.02	2.807	0.30	(0.08)	0.25	26.3	30.00
20	77.25	13.13	2.793	0.30	(0.08)	0.25	26.5	36.00
21	77.77	13.85	2.710	0.30	(0.07)	0.25	27.7	14.00
22	77.98	14.14	2.678	0.30	(0.07)	0.24	28.2	26.00
23	78.01	14.18	2.674	0.30	(0.07)	0.24	28.3	27.00
24	78.04	14.24	2.667	0.30	(0.07)	0.24	28.4	38.00
25	78.28	14.79	2.611	0.30	(0.07)	0.24	29.3	72.00
26	78.30	14.90	2.600	0.30	(0.07)	0.24	29.4	20.00
27	78.27	15.35	2.556	0.30	(0.07)	0.24	30.1	17.00
28	78.27	15.36	2.556	0.30	(0.07)	0.24	30.1	23.00
29	78.25	15.41	2.551	0.30	(0.07)	0.24	30.2	29.00
30	77.92	16.27	2.474	0.30	(0.07)	0.24	31.2	50.00
31	77.33	16.71	2.437	0.30	(0.07)	0.24	31.5	39.00
32	77.28	16.74	2.434	0.30	(0.07)	0.24	31.5	34.00
33	76.65	17.08	2.406	0.30	(0.07)	0.24	31.7	24.00
34	71.43	20.24	2.186	0.30	(0.07)	0.23	33.2	12.00
35	67.77	22.85	2.041	0.30	(0.07)	0.23	34.2	64.00
36	66.60	23.72	1.998	0.30	(0.07)	0.23	34.5	32.00
37	66.45	23.79	1.995	0.30	(0.07)	0.23	34.5	10.00

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE (CFS) = 78.30 Tc (MIN.) = 14.90
EFFECTIVE AREA (ACRES) = 29.43 AREA-AVERAGED Fm (INCH/HR) = 0.07
AREA-AVERAGED Fp (INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.24
TOTAL AREA (ACRES) = 34.5
LONGEST FLOWPATH FROM NODE 10.00 TO NODE 74.00 = 2940.00 FEET.

FLOW PROCESS FROM NODE 80.00 TO NODE 81.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<

>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

INITIAL SUBAREA FLOW-LENGTH (FEET) = 290.00

ELEVATION DATA: UPSTREAM (FEET) = 166.40 DOWNSTREAM (FEET) = 165.90

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$

SUBAREA ANALYSIS USED MINIMUM Tc (MIN.) = 10.484

* 25 YEAR RAINFALL INTENSITY (INCH/HR) = 3.172

SUBAREA Tc AND LOSS RATE DATA (AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN	Tc (MIN.)
COMMERCIAL	B	0.76	0.30	0.100	56	10.48

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp (INCH/HR) = 0.30

SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.100

SUBAREA RUNOFF (CFS) = 2.15

TOTAL AREA (ACRES) = 0.76 PEAK FLOW RATE (CFS) = 2.15

FLOW PROCESS FROM NODE 81.00 TO NODE 82.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 153.00 DOWNSTREAM(FEET) = 147.60
FLOW LENGTH(FEET) = 320.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 12.0 INCH PIPE IS 5.8 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 5.67
ESTIMATED PIPE DIAMETER(INCH) = 12.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 2.15
PIPE TRAVEL TIME(MIN.) = 0.94 Tc(MIN.) = 11.42
LONGEST FLOWPATH FROM NODE 80.00 TO NODE 82.00 = 610.00 FEET.

FLOW PROCESS FROM NODE 82.00 TO NODE 82.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

=====

TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION(MIN.) = 11.42
RAINFALL INTENSITY(INCH/HR) = 3.02
AREA-AVERAGED Fm(INCH/HR) = 0.03
AREA-AVERAGED Fp(INCH/HR) = 0.30
AREA-AVERAGED Ap = 0.10
EFFECTIVE STREAM AREA(ACRES) = 0.76
TOTAL STREAM AREA(ACRES) = 0.76
PEAK FLOW RATE(CFS) AT CONFLUENCE = 2.15

FLOW PROCESS FROM NODE 81.00 TO NODE 82.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

=====

INITIAL SUBAREA FLOW-LENGTH(FEET) = 320.00
ELEVATION DATA: UPSTREAM(FEET) = 165.90 DOWNSTREAM(FEET) = 165.40

Tc = K*[(LENGTH** 3.00)/(ELEVATION CHANGE)]**0.20
SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) = 11.122
* 25 YEAR RAINFALL INTENSITY(INCH/HR) = 3.068
SUBAREA Tc AND LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN	Tc (MIN.)
COMMERCIAL	B	0.59	0.30	0.100	56	11.12

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.30
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.100
SUBAREA RUNOFF(CFS) = 1.61
TOTAL AREA(ACRES) = 0.59 PEAK FLOW RATE(CFS) = 1.61

FLOW PROCESS FROM NODE 82.00 TO NODE 82.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<

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=====
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
TIME OF CONCENTRATION(MIN.) = 11.12
RAINFALL INTENSITY(INCH/HR) = 3.07
AREA-AVERAGED Fm(INCH/HR) = 0.03
AREA-AVERAGED Fp(INCH/HR) = 0.30
AREA-AVERAGED Ap = 0.10
EFFECTIVE STREAM AREA(ACRES) = 0.59
TOTAL STREAM AREA(ACRES) = 0.59
PEAK FLOW RATE(CFS) AT CONFLUENCE = 1.61

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** CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	2.15	11.42	3.022	0.30(0.03)	0.10	0.8	80.00
2	1.61	11.12	3.068	0.30(0.03)	0.10	0.6	81.00

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	3.74	11.12	3.068	0.30(0.03)	0.10	1.3	81.00
2	3.74	11.42	3.022	0.30(0.03)	0.10	1.3	80.00

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

```

PEAK FLOW RATE(CFS) = 3.74 Tc(MIN.) = 11.42
EFFECTIVE AREA(ACRES) = 1.35 AREA-AVERAGED Fm(INCH/HR) = 0.03
AREA-AVERAGED Fp(INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.10
TOTAL AREA(ACRES) = 1.3
LONGEST FLOWPATH FROM NODE 80.00 TO NODE 82.00 = 610.00 FEET.

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END OF STUDY SUMMARY:

```

TOTAL AREA(ACRES) = 1.3 TC(MIN.) = 11.42
EFFECTIVE AREA(ACRES) = 1.35 AREA-AVERAGED Fm(INCH/HR) = 0.03
AREA-AVERAGED Fp(INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.100
PEAK FLOW RATE(CFS) = 3.74

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** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	3.74	11.12	3.068	0.30(0.03)	0.10	1.3	81.00
2	3.74	11.42	3.022	0.30(0.03)	0.10	1.3	80.00

END OF RATIONAL METHOD ANALYSIS

RATIONAL METHOD HYDROLOGY COMPUTER PROGRAM PACKAGE
(Reference: 1986 ORANGE COUNTY HYDROLOGY CRITERION)
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Ver. 20.0 Release Date: 06/01/2013 License ID 1654

Analysis prepared by:

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949-474-1960

***** DESCRIPTION OF STUDY *****

* FREEMAN *
* PROPOSED CONDITION *
* 100 YEAR ANALYSIS *

FILE NAME: FRPR100.DAT
TIME/DATE OF STUDY: 09:23 05/29/2017

=====

USER SPECIFIED HYDROLOGY AND HYDRAULIC MODEL INFORMATION:

=====

--*TIME-OF-CONCENTRATION MODEL*--

USER SPECIFIED STORM EVENT (YEAR) = 100.00
SPECIFIED MINIMUM PIPE SIZE (INCH) = 12.00
SPECIFIED PERCENT OF GRADIENTS (DECIMAL) TO USE FOR FRICTION SLOPE = 0.95
DATA BANK RAINFALL USED
ANTECEDENT MOISTURE CONDITION (AMC) III ASSUMED FOR RATIONAL METHOD

USER-DEFINED STREET-SECTIONS FOR COUPLED PIPEFLOW AND STREETFLOW MODEL

NO.	HALF-	CROWN TO	STREET-CROSSFALL:			CURB	GUTTER-GEOMETRIES:			MANNING
	WIDTH	CROSSFALL	IN-	/	OUT-/PARK-	HEIGHT	WIDTH	LIP	HIKE	FACTOR
	(FT)	(FT)	SIDE	/	SIDE/ WAY	(FT)	(FT)	(FT)	(FT)	(n)
1	24.0	19.0	0.020	/	0.020/0.020	0.67	2.00	0.0313	0.167	0.0150
2	18.0	13.0	0.020	/	0.020/0.020	0.50	1.50	0.0313	0.125	0.0150

GLOBAL STREET FLOW-DEPTH CONSTRAINTS:

1. Relative Flow-Depth = 0.50 FEET
as (Maximum Allowable Street Flow Depth) - (Top-of-Curb)
 2. (Depth)*(Velocity) Constraint = 6.0 (FT*FT/S)
- *SIZE PIPE WITH A FLOW CAPACITY GREATER THAN
OR EQUAL TO THE UPSTREAM TRIBUTARY PIPE.*
*USER-SPECIFIED MINIMUM TOPOGRAPHIC SLOPE ADJUSTMENT NOT SELECTED

FLOW PROCESS FROM NODE 10.00 TO NODE 32.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

INITIAL SUBAREA FLOW-LENGTH (FEET) = 265.00
ELEVATION DATA: UPSTREAM (FEET) = 167.50 DOWNSTREAM (FEET) = 166.30

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$
SUBAREA ANALYSIS USED MINIMUM T_c (MIN.) = 8.337
* 100 YEAR RAINFALL INTENSITY (INCH/HR) = 4.616

SUBAREA T_c AND LOSS RATE DATA (AMC III):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	F_p (INCH/HR)	A_p (DECIMAL)	SCS CN	T_c (MIN.)
COMMERCIAL	B	0.15	0.30	0.100	76	8.34

SUBAREA AVERAGE PERVIOUS LOSS RATE, F_p (INCH/HR) = 0.30

SUBAREA AVERAGE PERVIOUS AREA FRACTION, A_p = 0.100

SUBAREA RUNOFF (CFS) = 0.62

TOTAL AREA (ACRES) = 0.15 PEAK FLOW RATE (CFS) = 0.62

FLOW PROCESS FROM NODE 32.00 TO NODE 11.00 IS CODE = 61

>>>>COMPUTE STREET FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>(STANDARD CURB SECTION USED)<<<<<

UPSTREAM ELEVATION (FEET) = 166.30 DOWNSTREAM ELEVATION (FEET) = 165.10
STREET LENGTH (FEET) = 210.00 CURB HEIGHT (INCHES) = 6.0
STREET HALFWIDTH (FEET) = 25.00

DISTANCE FROM CROWN TO CROSSFALL GRADEBREAK (FEET) = 10.00

INSIDE STREET CROSSFALL (DECIMAL) = 0.020

OUTSIDE STREET CROSSFALL (DECIMAL) = 0.020

SPECIFIED NUMBER OF HALFSTREETS CARRYING RUNOFF = 1

STREET PARKWAY CROSSFALL (DECIMAL) = 0.020

Manning's FRICTION FACTOR for Streetflow Section (curb-to-curb) = 0.0150

Manning's FRICTION FACTOR for Back-of-Walk Flow Section = 0.0200

**TRAVEL TIME COMPUTED USING ESTIMATED FLOW (CFS) = 0.83

STREETFLOW MODEL RESULTS USING ESTIMATED FLOW:

STREET FLOW DEPTH (FEET) = 0.26

HALFSTREET FLOOD WIDTH (FEET) = 6.85

AVERAGE FLOW VELOCITY (FEET/SEC.) = 1.42

PRODUCT OF DEPTH&VELOCITY (FT*FT/SEC.) = 0.37

STREET FLOW TRAVEL TIME (MIN.) = 2.47 T_c (MIN.) = 10.81

* 100 YEAR RAINFALL INTENSITY (INCH/HR) = 3.978

SUBAREA LOSS RATE DATA (AMC III):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	F_p (INCH/HR)	A_p (DECIMAL)	SCS CN
COMMERCIAL	B	0.12	0.30	0.100	76

SUBAREA AVERAGE PERVIOUS LOSS RATE, F_p (INCH/HR) = 0.30

SUBAREA AVERAGE PERVIOUS AREA FRACTION, A_p = 0.100

SUBAREA AREA (ACRES) = 0.12 SUBAREA RUNOFF (CFS) = 0.43

EFFECTIVE AREA (ACRES) = 0.27 AREA-AVERAGED Fm (INCH/HR) = 0.03
AREA-AVERAGED Fp (INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.10
TOTAL AREA (ACRES) = 0.3 PEAK FLOW RATE (CFS) = 0.96

END OF SUBAREA STREET FLOW HYDRAULICS:

DEPTH (FEET) = 0.27 HALFSTREET FLOOD WIDTH (FEET) = 7.33
FLOW VELOCITY (FEET/SEC.) = 1.46 DEPTH*VELOCITY (FT*FT/SEC.) = 0.40
LONGEST FLOWPATH FROM NODE 10.00 TO NODE 11.00 = 475.00 FEET.

FLOW PROCESS FROM NODE 11.00 TO NODE 11.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

=====

TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION (MIN.) = 10.81
RAINFALL INTENSITY (INCH/HR) = 3.98
AREA-AVERAGED Fm (INCH/HR) = 0.03
AREA-AVERAGED Fp (INCH/HR) = 0.30
AREA-AVERAGED Ap = 0.10
EFFECTIVE STREAM AREA (ACRES) = 0.27
TOTAL STREAM AREA (ACRES) = 0.27
PEAK FLOW RATE (CFS) AT CONFLUENCE = 0.96

FLOW PROCESS FROM NODE 12.00 TO NODE 11.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<

>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

=====

INITIAL SUBAREA FLOW-LENGTH (FEET) = 210.00
ELEVATION DATA: UPSTREAM (FEET) = 166.00 DOWNSTREAM (FEET) = 165.10

$$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$$

SUBAREA ANALYSIS USED MINIMUM Tc (MIN.) = 7.680

* 100 YEAR RAINFALL INTENSITY (INCH/HR) = 4.838

SUBAREA Tc AND LOSS RATE DATA (AMC III):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN	Tc (MIN.)
COMMERCIAL	B	0.76	0.30	0.100	76	7.68

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp (INCH/HR) = 0.30

SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.100

SUBAREA RUNOFF (CFS) = 3.29

TOTAL AREA (ACRES) = 0.76 PEAK FLOW RATE (CFS) = 3.29

FLOW PROCESS FROM NODE 11.00 TO NODE 11.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<

=====

TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:

TIME OF CONCENTRATION (MIN.) = 7.68
 RAINFALL INTENSITY (INCH/HR) = 4.84
 AREA-AVERAGED F_m (INCH/HR) = 0.03
 AREA-AVERAGED F_p (INCH/HR) = 0.30
 AREA-AVERAGED A_p = 0.10
 EFFECTIVE STREAM AREA (ACRES) = 0.76
 TOTAL STREAM AREA (ACRES) = 0.76
 PEAK FLOW RATE (CFS) AT CONFLUENCE = 3.29

** CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp (Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	0.96	10.81	3.978	0.30 (0.03)	0.10	0.3	10.00
2	3.29	7.68	4.838	0.30 (0.03)	0.10	0.8	12.00

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
 CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp (Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	4.12	7.68	4.838	0.30 (0.03)	0.10	1.0	12.00
2	3.66	10.81	3.978	0.30 (0.03)	0.10	1.0	10.00

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE (CFS) = 4.12 Tc (MIN.) = 7.68
 EFFECTIVE AREA (ACRES) = 0.95 AREA-AVERAGED F_m (INCH/HR) = 0.03
 AREA-AVERAGED F_p (INCH/HR) = 0.30 AREA-AVERAGED A_p = 0.10
 TOTAL AREA (ACRES) = 1.0
 LONGEST FLOWPATH FROM NODE 10.00 TO NODE 11.00 = 475.00 FEET.

FLOW PROCESS FROM NODE 11.00 TO NODE 13.00 IS CODE = 62

>>>>COMPUTE STREET FLOW TRAVEL TIME THRU SUBAREA<<<<<
 >>>>(STREET TABLE SECTION # 1 USED)<<<<<

UPSTREAM ELEVATION (FEET) = 165.10 DOWNSTREAM ELEVATION (FEET) = 164.20
 STREET LENGTH (FEET) = 455.00 CURB HEIGHT (INCHES) = 8.0
 STREET HALFWIDTH (FEET) = 24.00

DISTANCE FROM CROWN TO CROSSFALL GRADEBREAK (FEET) = 19.00
 INSIDE STREET CROSSFALL (DECIMAL) = 0.020
 OUTSIDE STREET CROSSFALL (DECIMAL) = 0.020

SPECIFIED NUMBER OF HALFSTREETS CARRYING RUNOFF = 1
 STREET PARKWAY CROSSFALL (DECIMAL) = 0.020
 Manning's FRICTION FACTOR for Streetflow Section (curb-to-curb) = 0.0150
 Manning's FRICTION FACTOR for Back-of-Walk Flow Section = 0.0200

**TRAVEL TIME COMPUTED USING ESTIMATED FLOW (CFS) = 4.69
 STREETFLOW MODEL RESULTS USING ESTIMATED FLOW:
 STREET FLOW DEPTH (FEET) = 0.51
 HALFSTREET FLOOD WIDTH (FEET) = 17.65

AVERAGE FLOW VELOCITY (FEET/SEC.) = 1.42
 PRODUCT OF DEPTH&VELOCITY (FT*FT/SEC.) = 0.73
 STREET FLOW TRAVEL TIME (MIN.) = 5.34 Tc (MIN.) = 13.02
 * 100 YEAR RAINFALL INTENSITY (INCH/HR) = 3.575

SUBAREA LOSS RATE DATA (AMC III):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
COMMERCIAL	B	0.36	0.30	0.100	76

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp (INCH/HR) = 0.30
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.100
 SUBAREA AREA (ACRES) = 0.36 SUBAREA RUNOFF (CFS) = 1.15
 EFFECTIVE AREA (ACRES) = 1.31 AREA-AVERAGED Fm (INCH/HR) = 0.03
 AREA-AVERAGED Fp (INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.10
 TOTAL AREA (ACRES) = 1.4 PEAK FLOW RATE (CFS) = 4.19

END OF SUBAREA STREET FLOW HYDRAULICS:

DEPTH (FEET) = 0.50 HALFSTREET FLOOD WIDTH (FEET) = 16.91
 FLOW VELOCITY (FEET/SEC.) = 1.37 DEPTH*VELOCITY (FT*FT/SEC.) = 0.68
 LONGEST FLOWPATH FROM NODE 10.00 TO NODE 13.00 = 930.00 FEET.

FLOW PROCESS FROM NODE 13.00 TO NODE 13.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

TOTAL NUMBER OF STREAMS = 2
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
 TIME OF CONCENTRATION (MIN.) = 13.02
 RAINFALL INTENSITY (INCH/HR) = 3.58
 AREA-AVERAGED Fm (INCH/HR) = 0.03
 AREA-AVERAGED Fp (INCH/HR) = 0.30
 AREA-AVERAGED Ap = 0.10
 EFFECTIVE STREAM AREA (ACRES) = 1.31
 TOTAL STREAM AREA (ACRES) = 1.39
 PEAK FLOW RATE (CFS) AT CONFLUENCE = 4.19

FLOW PROCESS FROM NODE 14.00 TO NODE 15.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<

>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

INITIAL SUBAREA FLOW-LENGTH (FEET) = 125.00
 ELEVATION DATA: UPSTREAM (FEET) = 164.60 DOWNSTREAM (FEET) = 164.20

Tc = K * [(LENGTH** 3.00) / (ELEVATION CHANGE)] ** 0.20
 SUBAREA ANALYSIS USED MINIMUM Tc (MIN.) = 6.616
 * 100 YEAR RAINFALL INTENSITY (INCH/HR) = 5.270

SUBAREA Tc AND LOSS RATE DATA (AMC III):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN	Tc (MIN.)
COMMERCIAL	B	0.15	0.30	0.100	76	6.62

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp (INCH/HR) = 0.30
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.100

SUBAREA RUNOFF(CFS) = 0.71
 TOTAL AREA(ACRES) = 0.15 PEAK FLOW RATE(CFS) = 0.71

FLOW PROCESS FROM NODE 15.00 TO NODE 13.00 IS CODE = 41

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
 >>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 159.30 DOWNSTREAM(FEET) = 159.20
 FLOW LENGTH(FEET) = 50.00 MANNING'S N = 0.013
 DEPTH OF FLOW IN 18.0 INCH PIPE IS 4.8 INCHES
 PIPE-FLOW VELOCITY(FEET/SEC.) = 1.88
 GIVEN PIPE DIAMETER(INCH) = 18.00 NUMBER OF PIPES = 1
 PIPE-FLOW(CFS) = 0.71
 PIPE TRAVEL TIME(MIN.) = 0.44 Tc(MIN.) = 7.06
 LONGEST FLOWPATH FROM NODE 14.00 TO NODE 13.00 = 175.00 FEET.

FLOW PROCESS FROM NODE 13.00 TO NODE 13.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
 >>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<

=====

TOTAL NUMBER OF STREAMS = 2
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
 TIME OF CONCENTRATION(MIN.) = 7.06
 RAINFALL INTENSITY(INCH/HR) = 5.08
 AREA-AVERAGED Fm(INCH/HR) = 0.03
 AREA-AVERAGED Fp(INCH/HR) = 0.30
 AREA-AVERAGED Ap = 0.10
 EFFECTIVE STREAM AREA(ACRES) = 0.15
 TOTAL STREAM AREA(ACRES) = 0.15
 PEAK FLOW RATE(CFS) AT CONFLUENCE = 0.71

** CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	4.19	13.02	3.575	0.30 (0.03)	0.10	1.3	12.00
1	3.89	16.31	3.142	0.30 (0.03)	0.10	1.4	10.00
2	0.71	7.06	5.077	0.30 (0.03)	0.10	0.2	14.00

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
 CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	3.94	7.06	5.077	0.30 (0.03)	0.10	0.9	14.00
2	4.68	13.02	3.575	0.30 (0.03)	0.10	1.5	12.00
3	4.33	16.31	3.142	0.30 (0.03)	0.10	1.5	10.00

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
 PEAK FLOW RATE(CFS) = 4.68 Tc(MIN.) = 13.02

EFFECTIVE AREA (ACRES) = 1.46 AREA-AVERAGED Fm (INCH/HR) = 0.03
AREA-AVERAGED Fp (INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.10
TOTAL AREA (ACRES) = 1.5
LONGEST FLOWPATH FROM NODE 10.00 TO NODE 13.00 = 930.00 FEET.

FLOW PROCESS FROM NODE 13.00 TO NODE 16.00 IS CODE = 41

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM (FEET) = 159.20 DOWNSTREAM (FEET) = 151.55
FLOW LENGTH (FEET) = 235.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 18.0 INCH PIPE IS 6.2 INCHES
PIPE-FLOW VELOCITY (FEET/SEC.) = 8.73
GIVEN PIPE DIAMETER (INCH) = 18.00 NUMBER OF PIPES = 1
PIPE-FLOW (CFS) = 4.68
PIPE TRAVEL TIME (MIN.) = 0.45 Tc (MIN.) = 13.47
LONGEST FLOWPATH FROM NODE 10.00 TO NODE 16.00 = 1165.00 FEET.

FLOW PROCESS FROM NODE 16.00 TO NODE 16.00 IS CODE = 10

>>>>MAIN-STREAM MEMORY COPIED ONTO MEMORY BANK # 1 <<<<<

FLOW PROCESS FROM NODE 17.00 TO NODE 18.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

=====

INITIAL SUBAREA FLOW-LENGTH (FEET) = 150.00
ELEVATION DATA: UPSTREAM (FEET) = 166.90 DOWNSTREAM (FEET) = 166.50

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$
SUBAREA ANALYSIS USED MINIMUM Tc (MIN.) = 8.741
* 100 YEAR RAINFALL INTENSITY (INCH/HR) = 4.493
SUBAREA Tc AND LOSS RATE DATA (AMC III):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN	Tc (MIN.)
CONDOMINIUMS	B	0.36	0.30	0.350	76	8.74

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp (INCH/HR) = 0.30
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.350
SUBAREA RUNOFF (CFS) = 1.42
TOTAL AREA (ACRES) = 0.36 PEAK FLOW RATE (CFS) = 1.42

FLOW PROCESS FROM NODE 18.00 TO NODE 19.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<

=====

ELEVATION DATA: UPSTREAM (FEET) = 153.00 DOWNSTREAM (FEET) = 151.60

FLOW LENGTH (FEET) = 65.00 MANNING'S N = 0.013
 ESTIMATED PIPE DIAMETER (INCH) INCREASED TO 12.000
 DEPTH OF FLOW IN 12.0 INCH PIPE IS 4.3 INCHES
 PIPE-FLOW VELOCITY (FEET/SEC.) = 5.56
 ESTIMATED PIPE DIAMETER (INCH) = 12.000 NUMBER OF PIPES = 1
 PIPE-FLOW (CFS) = 1.42
 PIPE TRAVEL TIME (MIN.) = 0.19 Tc (MIN.) = 8.94
 LONGEST FLOWPATH FROM NODE 17.00 TO NODE 19.00 = 215.00 FEET.

FLOW PROCESS FROM NODE 19.00 TO NODE 19.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

TOTAL NUMBER OF STREAMS = 2
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
 TIME OF CONCENTRATION (MIN.) = 8.94
 RAINFALL INTENSITY (INCH/HR) = 4.44
 AREA-AVERAGED Fm (INCH/HR) = 0.11
 AREA-AVERAGED Fp (INCH/HR) = 0.30
 AREA-AVERAGED Ap = 0.35
 EFFECTIVE STREAM AREA (ACRES) = 0.36
 TOTAL STREAM AREA (ACRES) = 0.36
 PEAK FLOW RATE (CFS) AT CONFLUENCE = 1.42

FLOW PROCESS FROM NODE 20.00 TO NODE 21.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<

>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

INITIAL SUBAREA FLOW-LENGTH (FEET) = 150.00
 ELEVATION DATA: UPSTREAM (FEET) = 167.30 DOWNSTREAM (FEET) = 166.80

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$

SUBAREA ANALYSIS USED MINIMUM Tc (MIN.) = 8.359

* 100 YEAR RAINFALL INTENSITY (INCH/HR) = 4.609

SUBAREA Tc AND LOSS RATE DATA (AMC III):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN	Tc (MIN.)
CONDOMINIUMS	A	0.50	0.40	0.350	52	8.36

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp (INCH/HR) = 0.40

SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.350

SUBAREA RUNOFF (CFS) = 2.01

TOTAL AREA (ACRES) = 0.50 PEAK FLOW RATE (CFS) = 2.01

FLOW PROCESS FROM NODE 21.00 TO NODE 19.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<

>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<

ELEVATION DATA: UPSTREAM (FEET) = 153.00 DOWNSTREAM (FEET) = 151.60

FLOW LENGTH (FEET) = 45.00 MANNING'S N = 0.013

ESTIMATED PIPE DIAMETER(INCH) INCREASED TO 12.000
 DEPTH OF FLOW IN 12.0 INCH PIPE IS 4.7 INCHES
 PIPE-FLOW VELOCITY(FEET/SEC.) = 6.99
 ESTIMATED PIPE DIAMETER(INCH) = 12.00 NUMBER OF PIPES = 1
 PIPE-FLOW(CFS) = 2.01
 PIPE TRAVEL TIME(MIN.) = 0.11 Tc(MIN.) = 8.47
 LONGEST FLOWPATH FROM NODE 20.00 TO NODE 19.00 = 195.00 FEET.

 FLOW PROCESS FROM NODE 19.00 TO NODE 19.00 IS CODE = 1

 >>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
 >>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<

=====

TOTAL NUMBER OF STREAMS = 2
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
 TIME OF CONCENTRATION(MIN.) = 8.47
 RAINFALL INTENSITY(INCH/HR) = 4.58
 AREA-AVERAGED Fm(INCH/HR) = 0.14
 AREA-AVERAGED Fp(INCH/HR) = 0.40
 AREA-AVERAGED Ap = 0.35
 EFFECTIVE STREAM AREA(ACRES) = 0.50
 TOTAL STREAM AREA(ACRES) = 0.50
 PEAK FLOW RATE(CFS) AT CONFLUENCE = 2.01

** CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	1.42	8.94	4.436	0.30 (0.11)	0.35	0.4	17.00
2	2.01	8.47	4.575	0.40 (0.14)	0.35	0.5	20.00

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
 CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	3.40	8.47	4.575	0.36 (0.13)	0.35	0.8	20.00
2	3.37	8.94	4.436	0.36 (0.13)	0.35	0.9	17.00

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE(CFS) = 3.40 Tc(MIN.) = 8.47
 EFFECTIVE AREA(ACRES) = 0.84 AREA-AVERAGED Fm(INCH/HR) = 0.13
 AREA-AVERAGED Fp(INCH/HR) = 0.36 AREA-AVERAGED Ap = 0.35
 TOTAL AREA(ACRES) = 0.9
 LONGEST FLOWPATH FROM NODE 17.00 TO NODE 19.00 = 215.00 FEET.

 FLOW PROCESS FROM NODE 19.00 TO NODE 19.00 IS CODE = 81

 >>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

=====

MAINLINE Tc(MIN.) = 8.47
 * 100 YEAR RAINFALL INTENSITY(INCH/HR) = 4.575

SUBAREA LOSS RATE DATA(AMC III):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
CONDOMINIUMS	B	0.42	0.30	0.350	76

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.30
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.350
 SUBAREA AREA(ACRES) = 0.42 SUBAREA RUNOFF(CFS) = 1.69
 EFFECTIVE AREA(ACRES) = 1.26 AREA-AVERAGED Fm(INCH/HR) = 0.12
 AREA-AVERAGED Fp(INCH/HR) = 0.34 AREA-AVERAGED Ap = 0.35
 TOTAL AREA(ACRES) = 1.3 PEAK FLOW RATE(CFS) = 5.06

 FLOW PROCESS FROM NODE 19.00 TO NODE 16.00 IS CODE = 41

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
 >>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 151.60 DOWNSTREAM(FEET) = 151.55
 FLOW LENGTH(FEET) = 30.00 MANNING'S N = 0.013
 ASSUME FULL-FLOWING PIPELINE
 PIPE-FLOW VELOCITY(FEET/SEC.) = 2.86
 PIPE FLOW VELOCITY = (TOTAL FLOW)/(PIPE CROSS SECTION AREA)
 GIVEN PIPE DIAMETER(INCH) = 18.00 NUMBER OF PIPES = 1
 PIPE-FLOW(CFS) = 5.06
 PIPE TRAVEL TIME(MIN.) = 0.17 Tc(MIN.) = 8.64
 LONGEST FLOWPATH FROM NODE 17.00 TO NODE 16.00 = 245.00 FEET.

 FLOW PROCESS FROM NODE 16.00 TO NODE 16.00 IS CODE = 11

>>>>CONFLUENCE MEMORY BANK # 1 WITH THE MAIN-STREAM MEMORY<<<<<

** MAIN STREAM CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	5.06	8.64	4.522	0.34(0.12)	0.35	1.3	20.00
2	4.97	9.11	4.387	0.34(0.12)	0.35	1.3	17.00

LONGEST FLOWPATH FROM NODE 17.00 TO NODE 16.00 = 245.00 FEET.

** MEMORY BANK # 1 CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	3.94	7.53	4.893	0.30(0.03)	0.10	0.9	14.00
2	4.68	13.47	3.507	0.30(0.03)	0.10	1.5	12.00
3	4.33	16.77	3.092	0.30(0.03)	0.10	1.5	10.00

LONGEST FLOWPATH FROM NODE 10.00 TO NODE 16.00 = 1165.00 FEET.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	8.72	7.53	4.893	0.33(0.08)	0.24	2.0	14.00
2	9.14	8.64	4.522	0.33(0.08)	0.24	2.2	20.00
3	9.11	9.11	4.387	0.33(0.08)	0.24	2.3	17.00

4	8.63	13.47	3.507	0.33 (0.07)	0.22	2.7	12.00
5	7.79	16.77	3.092	0.33 (0.07)	0.21	2.8	10.00
TOTAL AREA (ACRES) =			2.8				

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE (CFS) = 9.14 Tc (MIN.) = 8.641
EFFECTIVE AREA (ACRES) = 2.23 AREA-AVERAGED Fm (INCH/HR) = 0.08
AREA-AVERAGED Fp (INCH/HR) = 0.33 AREA-AVERAGED Ap = 0.24
TOTAL AREA (ACRES) = 2.8
LONGEST FLOWPATH FROM NODE 10.00 TO NODE 16.00 = 1165.00 FEET.

FLOW PROCESS FROM NODE 16.00 TO NODE 22.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<

=====

ELEVATION DATA: UPSTREAM (FEET) = 151.55 DOWNSTREAM (FEET) = 151.30
FLOW LENGTH (FEET) = 115.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 24.0 INCH PIPE IS 17.6 INCHES
PIPE-FLOW VELOCITY (FEET/SEC.) = 3.70
ESTIMATED PIPE DIAMETER (INCH) = 24.00 NUMBER OF PIPES = 1
PIPE-FLOW (CFS) = 9.14
PIPE TRAVEL TIME (MIN.) = 0.52 Tc (MIN.) = 9.16
LONGEST FLOWPATH FROM NODE 10.00 TO NODE 22.00 = 1280.00 FEET.

FLOW PROCESS FROM NODE 22.00 TO NODE 22.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

=====

MAINLINE Tc (MIN.) = 9.16
* 100 YEAR RAINFALL INTENSITY (INCH/HR) = 4.374
SUBAREA LOSS RATE DATA (AMC III):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
CONDOMINIUMS	A	0.31	0.40	0.350	52

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp (INCH/HR) = 0.40
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.350
SUBAREA AREA (ACRES) = 0.31 SUBAREA RUNOFF (CFS) = 1.18
EFFECTIVE AREA (ACRES) = 2.54 AREA-AVERAGED Fm (INCH/HR) = 0.09
AREA-AVERAGED Fp (INCH/HR) = 0.34 AREA-AVERAGED Ap = 0.25
TOTAL AREA (ACRES) = 3.1 PEAK FLOW RATE (CFS) = 9.82

FLOW PROCESS FROM NODE 22.00 TO NODE 22.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

=====

TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION (MIN.) = 9.16
RAINFALL INTENSITY (INCH/HR) = 4.37
AREA-AVERAGED Fm (INCH/HR) = 0.09

AREA-AVERAGED F_p (INCH/HR) = 0.34
 AREA-AVERAGED A_p = 0.25
 EFFECTIVE STREAM AREA (ACRES) = 2.54
 TOTAL STREAM AREA (ACRES) = 3.13
 PEAK FLOW RATE (CFS) AT CONFLUENCE = 9.82

FLOW PROCESS FROM NODE 23.00 TO NODE 22.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
 >>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

INITIAL SUBAREA FLOW-LENGTH (FEET) = 225.00
 ELEVATION DATA: UPSTREAM (FEET) = 166.30 DOWNSTREAM (FEET) = 165.50

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$
 SUBAREA ANALYSIS USED MINIMUM T_c (MIN.) = 9.705
 * 100 YEAR RAINFALL INTENSITY (INCH/HR) = 4.231
 SUBAREA T_c AND LOSS RATE DATA (AMC III):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	F_p (INCH/HR)	A_p (DECIMAL)	SCS CN	T_c (MIN.)
CONDOMINIUMS	B	0.71	0.30	0.350	76	9.70

SUBAREA AVERAGE PERVIOUS LOSS RATE, F_p (INCH/HR) = 0.30
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, A_p = 0.350
 SUBAREA RUNOFF (CFS) = 2.64
 TOTAL AREA (ACRES) = 0.71 PEAK FLOW RATE (CFS) = 2.64

FLOW PROCESS FROM NODE 22.00 TO NODE 22.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
 >>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<

TOTAL NUMBER OF STREAMS = 2
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
 TIME OF CONCENTRATION (MIN.) = 9.70
 RAINFALL INTENSITY (INCH/HR) = 4.23
 AREA-AVERAGED F_m (INCH/HR) = 0.11
 AREA-AVERAGED F_p (INCH/HR) = 0.30
 AREA-AVERAGED A_p = 0.35
 EFFECTIVE STREAM AREA (ACRES) = 0.71
 TOTAL STREAM AREA (ACRES) = 0.71
 PEAK FLOW RATE (CFS) AT CONFLUENCE = 2.64

** CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	T_c (MIN.)	Intensity (INCH/HR)	F_p (F_m) (INCH/HR)	A_p	A_e (ACRES)	HEADWATER NODE
1	9.44	8.05	4.709	0.35 (0.09)	0.26	2.3	14.00
1	9.82	9.16	4.374	0.34 (0.09)	0.25	2.5	20.00
1	9.78	9.63	4.250	0.34 (0.09)	0.25	2.6	17.00
1	9.21	13.99	3.431	0.34 (0.08)	0.23	3.1	12.00
1	8.34	17.31	3.038	0.34 (0.08)	0.23	3.1	10.00
2	2.64	9.70	4.231	0.30 (0.11)	0.35	0.7	23.00

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
 CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	11.88	8.05	4.709	0.33(0.09)	0.27	2.9	14.00
2	12.39	9.16	4.374	0.33(0.09)	0.27	3.2	20.00
3	12.41	9.63	4.250	0.33(0.09)	0.27	3.3	17.00
4	12.41	9.70	4.231	0.33(0.09)	0.27	3.3	23.00
5	11.33	13.99	3.431	0.33(0.08)	0.25	3.8	12.00
6	10.21	17.31	3.038	0.33(0.08)	0.25	3.8	10.00

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE (CFS) = 12.41 Tc (MIN.) = 9.63
 EFFECTIVE AREA (ACRES) = 3.32 AREA-AVERAGED Fm (INCH/HR) = 0.09
 AREA-AVERAGED Fp (INCH/HR) = 0.33 AREA-AVERAGED Ap = 0.27
 TOTAL AREA (ACRES) = 3.8
 LONGEST FLOWPATH FROM NODE 10.00 TO NODE 22.00 = 1280.00 FEET.

FLOW PROCESS FROM NODE 22.00 TO NODE 22.00 IS CODE = 10

>>>>MAIN-STREAM MEMORY COPIED ONTO MEMORY BANK # 2 <<<<<

FLOW PROCESS FROM NODE 24.00 TO NODE 25.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<

>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

INITIAL SUBAREA FLOW-LENGTH (FEET) = 270.00

ELEVATION DATA: UPSTREAM (FEET) = 166.50 DOWNSTREAM (FEET) = 165.50

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$

SUBAREA ANALYSIS USED MINIMUM Tc (MIN.) = 10.354

* 100 YEAR RAINFALL INTENSITY (INCH/HR) = 4.077

SUBAREA Tc AND LOSS RATE DATA (AMC III):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN	Tc (MIN.)
CONDOMINIUMS	B	0.51	0.30	0.350	76	10.35

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp (INCH/HR) = 0.30

SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.350

SUBAREA RUNOFF (CFS) = 1.82

TOTAL AREA (ACRES) = 0.51 PEAK FLOW RATE (CFS) = 1.82

FLOW PROCESS FROM NODE 25.00 TO NODE 25.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

TOTAL NUMBER OF STREAMS = 2

CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:

TIME OF CONCENTRATION(MIN.) = 10.35
 RAINFALL INTENSITY(INCH/HR) = 4.08
 AREA-AVERAGED Fm(INCH/HR) = 0.11
 AREA-AVERAGED Fp(INCH/HR) = 0.30
 AREA-AVERAGED Ap = 0.35
 EFFECTIVE STREAM AREA(ACRES) = 0.51
 TOTAL STREAM AREA(ACRES) = 0.51
 PEAK FLOW RATE(CFS) AT CONFLUENCE = 1.82

 FLOW PROCESS FROM NODE 26.00 TO NODE 25.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
 >>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

INITIAL SUBAREA FLOW-LENGTH(FEET) = 155.00
 ELEVATION DATA: UPSTREAM(FEET) = 166.50 DOWNSTREAM(FEET) = 165.50

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$
 SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) = 7.422
 * 100 YEAR RAINFALL INTENSITY(INCH/HR) = 4.934

SUBAREA Tc AND LOSS RATE DATA(AMC III):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN	Tc (MIN.)
CONDOMINIUMS	B	0.30	0.30	0.350	76	7.42

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.30
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.350
 SUBAREA RUNOFF(CFS) = 1.30
 TOTAL AREA(ACRES) = 0.30 PEAK FLOW RATE(CFS) = 1.30

 FLOW PROCESS FROM NODE 25.00 TO NODE 25.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
 >>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<

TOTAL NUMBER OF STREAMS = 2
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
 TIME OF CONCENTRATION(MIN.) = 7.42
 RAINFALL INTENSITY(INCH/HR) = 4.93
 AREA-AVERAGED Fm(INCH/HR) = 0.10
 AREA-AVERAGED Fp(INCH/HR) = 0.30
 AREA-AVERAGED Ap = 0.35
 EFFECTIVE STREAM AREA(ACRES) = 0.30
 TOTAL STREAM AREA(ACRES) = 0.30
 PEAK FLOW RATE(CFS) AT CONFLUENCE = 1.30

** CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	1.82	10.35	4.077	0.30(0.11)	0.35	0.5	24.00
2	1.30	7.42	4.934	0.30(0.10)	0.35	0.3	26.00

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO

CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	2.89	7.42	4.934	0.30 (0.10)	0.35	0.7	26.00
2	2.90	10.35	4.077	0.30 (0.11)	0.35	0.8	24.00

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE (CFS) = 2.90 Tc (MIN.) = 10.35
EFFECTIVE AREA (ACRES) = 0.81 AREA-AVERAGED Fm (INCH/HR) = 0.11
AREA-AVERAGED Fp (INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.35
TOTAL AREA (ACRES) = 0.8
LONGEST FLOWPATH FROM NODE 24.00 TO NODE 25.00 = 270.00 FEET.

FLOW PROCESS FROM NODE 25.00 TO NODE 28.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<

=====

ELEVATION DATA: UPSTREAM (FEET) = 161.00 DOWNSTREAM (FEET) = 158.00
FLOW LENGTH (FEET) = 175.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 12.0 INCH PIPE IS 7.0 INCHES
PIPE-FLOW VELOCITY (FEET/SEC.) = 6.13
ESTIMATED PIPE DIAMETER (INCH) = 12.00 NUMBER OF PIPES = 1
PIPE-FLOW (CFS) = 2.90
PIPE TRAVEL TIME (MIN.) = 0.48 Tc (MIN.) = 10.83
LONGEST FLOWPATH FROM NODE 24.00 TO NODE 28.00 = 445.00 FEET.

FLOW PROCESS FROM NODE 28.00 TO NODE 28.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

=====

TOTAL NUMBER OF STREAMS = 3
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION (MIN.) = 10.83
RAINFALL INTENSITY (INCH/HR) = 3.97
AREA-AVERAGED Fm (INCH/HR) = 0.11
AREA-AVERAGED Fp (INCH/HR) = 0.30
AREA-AVERAGED Ap = 0.35
EFFECTIVE STREAM AREA (ACRES) = 0.81
TOTAL STREAM AREA (ACRES) = 0.81
PEAK FLOW RATE (CFS) AT CONFLUENCE = 2.90

FLOW PROCESS FROM NODE 27.00 TO NODE 28.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

=====

INITIAL SUBAREA FLOW-LENGTH (FEET) = 180.00
ELEVATION DATA: UPSTREAM (FEET) = 166.50 DOWNSTREAM (FEET) = 165.40

$$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$$

SUBAREA ANALYSIS USED MINIMUM T_c (MIN.) = 7.965

* 100 YEAR RAINFALL INTENSITY (INCH/HR) = 4.738

SUBAREA T_c AND LOSS RATE DATA (AMC III):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	F_p (INCH/HR)	A_p (DECIMAL)	SCS CN	T_c (MIN.)
CONDOMINIUMS	B	0.18	0.30	0.350	76	7.97

SUBAREA AVERAGE PERVIOUS LOSS RATE, F_p (INCH/HR) = 0.30

SUBAREA AVERAGE PERVIOUS AREA FRACTION, A_p = 0.350

SUBAREA RUNOFF (CFS) = 0.75

TOTAL AREA (ACRES) = 0.18 PEAK FLOW RATE (CFS) = 0.75

FLOW PROCESS FROM NODE 28.00 TO NODE 28.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

=====

TOTAL NUMBER OF STREAMS = 3

CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:

TIME OF CONCENTRATION (MIN.) = 7.97

RAINFALL INTENSITY (INCH/HR) = 4.74

AREA-AVERAGED F_m (INCH/HR) = 0.11

AREA-AVERAGED F_p (INCH/HR) = 0.30

AREA-AVERAGED A_p = 0.35

EFFECTIVE STREAM AREA (ACRES) = 0.18

TOTAL STREAM AREA (ACRES) = 0.18

PEAK FLOW RATE (CFS) AT CONFLUENCE = 0.75

FLOW PROCESS FROM NODE 29.00 TO NODE 28.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<

>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

=====

INITIAL SUBAREA FLOW-LENGTH (FEET) = 230.00

ELEVATION DATA: UPSTREAM (FEET) = 166.50 DOWNSTREAM (FEET) = 165.40

$$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$$

SUBAREA ANALYSIS USED MINIMUM T_c (MIN.) = 9.227

* 100 YEAR RAINFALL INTENSITY (INCH/HR) = 4.355

SUBAREA T_c AND LOSS RATE DATA (AMC III):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	F_p (INCH/HR)	A_p (DECIMAL)	SCS CN	T_c (MIN.)
CONDOMINIUMS	B	0.48	0.30	0.350	76	9.23

SUBAREA AVERAGE PERVIOUS LOSS RATE, F_p (INCH/HR) = 0.30

SUBAREA AVERAGE PERVIOUS AREA FRACTION, A_p = 0.350

SUBAREA RUNOFF (CFS) = 1.84

TOTAL AREA (ACRES) = 0.48 PEAK FLOW RATE (CFS) = 1.84

FLOW PROCESS FROM NODE 28.00 TO NODE 28.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

>>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<

=====

TOTAL NUMBER OF STREAMS = 3
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 3 ARE:
TIME OF CONCENTRATION(MIN.) = 9.23
RAINFALL INTENSITY(INCH/HR) = 4.36
AREA-AVERAGED Fm(INCH/HR) = 0.11
AREA-AVERAGED Fp(INCH/HR) = 0.30
AREA-AVERAGED Ap = 0.35
EFFECTIVE STREAM AREA(ACRES) = 0.48
TOTAL STREAM AREA(ACRES) = 0.48
PEAK FLOW RATE(CFS) AT CONFLUENCE = 1.84

** CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	2.89	7.90	4.762	0.30(0.10)	0.35	0.7	26.00
1	2.90	10.83	3.973	0.30(0.11)	0.35	0.8	24.00
2	0.75	7.97	4.738	0.30(0.11)	0.35	0.2	27.00
3	1.84	9.23	4.355	0.30(0.11)	0.35	0.5	29.00

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
CONFLUENCE FORMULA USED FOR 3 STREAMS.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	5.36	7.90	4.762	0.30(0.10)	0.35	1.3	26.00
2	5.37	7.97	4.738	0.30(0.10)	0.35	1.3	27.00
3	5.43	9.23	4.355	0.30(0.11)	0.35	1.4	29.00
4	5.19	10.83	3.973	0.30(0.11)	0.35	1.5	24.00

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE(CFS) = 5.43 Tc(MIN.) = 9.23
EFFECTIVE AREA(ACRES) = 1.39 AREA-AVERAGED Fm(INCH/HR) = 0.11
AREA-AVERAGED Fp(INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.35
TOTAL AREA(ACRES) = 1.5
LONGEST FLOWPATH FROM NODE 24.00 TO NODE 28.00 = 445.00 FEET.

FLOW PROCESS FROM NODE 28.00 TO NODE 22.00 IS CODE = 31

>>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<

>>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 158.00 DOWNSTREAM(FEET) = 151.30
FLOW LENGTH(FEET) = 250.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 12.0 INCH PIPE IS 9.4 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 8.24
ESTIMATED PIPE DIAMETER(INCH) = 12.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 5.43
PIPE TRAVEL TIME(MIN.) = 0.51 Tc(MIN.) = 9.73
LONGEST FLOWPATH FROM NODE 24.00 TO NODE 22.00 = 695.00 FEET.

FLOW PROCESS FROM NODE 22.00 TO NODE 22.00 IS CODE = 11

>>>>CONFLUENCE MEMORY BANK # 2 WITH THE MAIN-STREAM MEMORY<<<<<

** MAIN STREAM CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	5.36	8.40	4.595	0.30(0.10)	0.35	1.3	26.00
2	5.37	8.47	4.574	0.30(0.10)	0.35	1.3	27.00
3	5.43	9.73	4.224	0.30(0.11)	0.35	1.4	29.00
4	5.19	11.34	3.871	0.30(0.11)	0.35	1.5	24.00
LONGEST FLOWPATH FROM NODE				24.00 TO NODE	22.00 =	695.00 FEET.	

** MEMORY BANK # 2 CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	11.88	8.05	4.709	0.33(0.09)	0.27	2.9	14.00
2	12.39	9.16	4.374	0.33(0.09)	0.27	3.2	20.00
3	12.41	9.63	4.250	0.33(0.09)	0.27	3.3	17.00
4	12.41	9.70	4.231	0.33(0.09)	0.27	3.3	23.00
5	11.33	13.99	3.431	0.33(0.08)	0.25	3.8	12.00
6	10.21	17.31	3.038	0.33(0.08)	0.25	3.8	10.00
LONGEST FLOWPATH FROM NODE				10.00 TO NODE	22.00 =	1280.00 FEET.	

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	17.15	8.05	4.709	0.32(0.10)	0.30	4.1	14.00
2	17.41	8.40	4.595	0.32(0.10)	0.30	4.2	26.00
3	17.45	8.47	4.574	0.32(0.10)	0.30	4.3	27.00
4	17.79	9.16	4.374	0.32(0.10)	0.30	4.5	20.00
5	17.83	9.63	4.250	0.32(0.09)	0.30	4.7	17.00
6	17.84	9.70	4.231	0.32(0.09)	0.30	4.7	23.00
7	17.83	9.73	4.224	0.32(0.09)	0.30	4.7	29.00
8	17.19	11.34	3.871	0.32(0.09)	0.29	5.0	24.00
9	15.92	13.99	3.431	0.32(0.09)	0.28	5.2	12.00
10	14.26	17.31	3.038	0.32(0.09)	0.28	5.3	10.00
TOTAL AREA (ACRES) =				5.3			

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE (CFS) = 17.84 Tc (MIN.) = 9.705
EFFECTIVE AREA (ACRES) = 4.72 AREA-AVERAGED Fm (INCH/HR) = 0.09
AREA-AVERAGED Fp (INCH/HR) = 0.32 AREA-AVERAGED Ap = 0.29
TOTAL AREA (ACRES) = 5.3
LONGEST FLOWPATH FROM NODE 10.00 TO NODE 22.00 = 1280.00 FEET.

FLOW PROCESS FROM NODE 22.00 TO NODE 33.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<

>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW) <<<<<

ELEVATION DATA: UPSTREAM(FEET) = 151.30 DOWNSTREAM(FEET) = 150.50
 FLOW LENGTH(FEET) = 425.00 MANNING'S N = 0.013
 DEPTH OF FLOW IN 33.0 INCH PIPE IS 22.3 INCHES
 PIPE-FLOW VELOCITY(FEET/SEC.) = 4.18
 ESTIMATED PIPE DIAMETER(INCH) = 33.00 NUMBER OF PIPES = 1
 PIPE-FLOW(CFS) = 17.84
 PIPE TRAVEL TIME(MIN.) = 1.69 Tc(MIN.) = 11.40
 LONGEST FLOWPATH FROM NODE 10.00 TO NODE 33.00 = 1705.00 FEET.

 FLOW PROCESS FROM NODE 33.00 TO NODE 33.00 IS CODE = 10

>>>>MAIN-STREAM MEMORY COPIED ONTO MEMORY BANK # 3 <<<<<

 FLOW PROCESS FROM NODE 30.00 TO NODE 31.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
 >>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

INITIAL SUBAREA FLOW-LENGTH(FEET) = 150.00
 ELEVATION DATA: UPSTREAM(FEET) = 166.50 DOWNSTREAM(FEET) = 165.50

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$
 SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) = 7.277
 * 100 YEAR RAINFALL INTENSITY(INCH/HR) = 4.990

SUBAREA Tc AND LOSS RATE DATA(AMC III):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN	Tc (MIN.)
CONDOMINIUMS	B	0.29	0.30	0.350	76	7.28

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.30

SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.350

SUBAREA RUNOFF(CFS) = 1.28

TOTAL AREA(ACRES) = 0.29 PEAK FLOW RATE(CFS) = 1.28

 FLOW PROCESS FROM NODE 31.00 TO NODE 31.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

MAINLINE Tc(MIN.) = 7.28
 * 100 YEAR RAINFALL INTENSITY(INCH/HR) = 4.990

SUBAREA LOSS RATE DATA(AMC III):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
CONDOMINIUMS	B	0.65	0.30	0.350	76

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.30

SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.350

SUBAREA AREA(ACRES) = 0.65 SUBAREA RUNOFF(CFS) = 2.86

EFFECTIVE AREA(ACRES) = 0.94 AREA-AVERAGED Fm(INCH/HR) = 0.11

AREA-AVERAGED Fp(INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.35

TOTAL AREA(ACRES) = 0.9 PEAK FLOW RATE(CFS) = 4.13

FLOW PROCESS FROM NODE 31.00 TO NODE 33.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<

ELEVATION DATA: UPSTREAM(FEET) = 157.00 DOWNSTREAM(FEET) = 150.50
FLOW LENGTH(FEET) = 380.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 12.0 INCH PIPE IS 9.0 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 6.55
ESTIMATED PIPE DIAMETER(INCH) = 12.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 4.13
PIPE TRAVEL TIME(MIN.) = 0.97 Tc(MIN.) = 8.24
LONGEST FLOWPATH FROM NODE 30.00 TO NODE 33.00 = 530.00 FEET.

FLOW PROCESS FROM NODE 33.00 TO NODE 33.00 IS CODE = 11

>>>>CONFLUENCE MEMORY BANK # 3 WITH THE MAIN-STREAM MEMORY<<<<<

** MAIN STREAM CONFLUENCE DATA **

Table with 8 columns: STREAM NUMBER, Q (CFS), Tc (MIN.), Intensity (INCH/HR), Fp(Fm) (INCH/HR), Ap, Ae (ACRES), HEADWATER NODE. Row 1: 1, 4.13, 8.24, 4.646, 0.30(0.11), 0.35, 0.9, 30.00. LONGEST FLOWPATH FROM NODE 30.00 TO NODE 33.00 = 530.00 FEET.

** MEMORY BANK # 3 CONFLUENCE DATA **

Table with 8 columns: STREAM NUMBER, Q (CFS), Tc (MIN.), Intensity (INCH/HR), Fp(Fm) (INCH/HR), Ap, Ae (ACRES), HEADWATER NODE. Rows 1-10 with values ranging from 17.15 to 14.26 CFS and 9.81 to 19.10 min. LONGEST FLOWPATH FROM NODE 10.00 TO NODE 33.00 = 1705.00 FEET.

** PEAK FLOW RATE TABLE **

Table with 8 columns: STREAM NUMBER, Q (CFS), Tc (MIN.), Intensity (INCH/HR), Fp(Fm) (INCH/HR), Ap, Ae (ACRES), HEADWATER NODE. Rows 1-9 with values ranging from 20.09 to 20.34 CFS and 8.24 to 13.10 min. LONGEST FLOWPATH FROM NODE 10.00 TO NODE 33.00 = 1705.00 FEET.

10	18.74	15.76	3.205	0.32 (0.09)	0.29	6.2	12.00
11	16.77	19.10	2.870	0.32 (0.09)	0.29	6.2	10.00
TOTAL AREA (ACRES) =			6.2				

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE (CFS) = 21.31 Tc (MIN.) = 10.855
EFFECTIVE AREA (ACRES) = 5.49 AREA-AVERAGED Fm (INCH/HR) = 0.10
AREA-AVERAGED Fp (INCH/HR) = 0.32 AREA-AVERAGED Ap = 0.31
TOTAL AREA (ACRES) = 6.2
LONGEST FLOWPATH FROM NODE 10.00 TO NODE 33.00 = 1705.00 FEET.

FLOW PROCESS FROM NODE 16.00 TO NODE 16.00 IS CODE = 12

>>>>CLEAR MEMORY BANK # 1 <<<<<

FLOW PROCESS FROM NODE 22.00 TO NODE 22.00 IS CODE = 12

>>>>CLEAR MEMORY BANK # 2 <<<<<

FLOW PROCESS FROM NODE 33.00 TO NODE 33.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

TOTAL NUMBER OF STREAMS = 5
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION (MIN.) = 10.85
RAINFALL INTENSITY (INCH/HR) = 3.97
AREA-AVERAGED Fm (INCH/HR) = 0.10
AREA-AVERAGED Fp (INCH/HR) = 0.32
AREA-AVERAGED Ap = 0.31
EFFECTIVE STREAM AREA (ACRES) = 5.49
TOTAL STREAM AREA (ACRES) = 6.25
PEAK FLOW RATE (CFS) AT CONFLUENCE = 21.31

FLOW PROCESS FROM NODE 30.00 TO NODE 33.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

INITIAL SUBAREA FLOW-LENGTH (FEET) = 185.00
ELEVATION DATA: UPSTREAM (FEET) = 166.60 DOWNSTREAM (FEET) = 166.00

$$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$$

SUBAREA ANALYSIS USED MINIMUM Tc (MIN.) = 9.141
* 100 YEAR RAINFALL INTENSITY (INCH/HR) = 4.379

SUBAREA Tc AND LOSS RATE DATA (AMC III):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN	Tc (MIN.)
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CONDOMINIUMS B 0.67 0.30 0.350 76 9.14
 SUBAREA AVERAGE PERVIOUS LOSS RATE, F_p (INCH/HR) = 0.30
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, A_p = 0.350
 SUBAREA RUNOFF (CFS) = 2.58
 TOTAL AREA (ACRES) = 0.67 PEAK FLOW RATE (CFS) = 2.58

FLOW PROCESS FROM NODE 33.00 TO NODE 33.00 IS CODE = 1

 >>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

=====

TOTAL NUMBER OF STREAMS = 5
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
 TIME OF CONCENTRATION (MIN.) = 9.14
 RAINFALL INTENSITY (INCH/HR) = 4.38
 AREA-AVERAGED F_m (INCH/HR) = 0.11
 AREA-AVERAGED F_p (INCH/HR) = 0.30
 AREA-AVERAGED A_p = 0.35
 EFFECTIVE STREAM AREA (ACRES) = 0.67
 TOTAL STREAM AREA (ACRES) = 0.67
 PEAK FLOW RATE (CFS) AT CONFLUENCE = 2.58

FLOW PROCESS FROM NODE 34.00 TO NODE 35.00 IS CODE = 21

 >>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
 >>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

=====

INITIAL SUBAREA FLOW-LENGTH (FEET) = 180.00
 ELEVATION DATA: UPSTREAM (FEET) = 166.20 DOWNSTREAM (FEET) = 166.10

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$
 SUBAREA ANALYSIS USED MINIMUM T_c (MIN.) = 12.867
 * 100 YEAR RAINFALL INTENSITY (INCH/HR) = 3.600
 SUBAREA T_c AND LOSS RATE DATA (AMC III):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	F_p (INCH/HR)	A_p (DECIMAL)	SCS CN	T_c (MIN.)
CONDOMINIUMS	B	0.50	0.30	0.350	76	12.87

SUBAREA AVERAGE PERVIOUS LOSS RATE, F_p (INCH/HR) = 0.30
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, A_p = 0.350
 SUBAREA RUNOFF (CFS) = 1.57
 TOTAL AREA (ACRES) = 0.50 PEAK FLOW RATE (CFS) = 1.57

FLOW PROCESS FROM NODE 33.00 TO NODE 33.00 IS CODE = 1

 >>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

=====

TOTAL NUMBER OF STREAMS = 5
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 3 ARE:
 TIME OF CONCENTRATION (MIN.) = 12.87
 RAINFALL INTENSITY (INCH/HR) = 3.60
 AREA-AVERAGED F_m (INCH/HR) = 0.11
 AREA-AVERAGED F_p (INCH/HR) = 0.30

AREA-AVERAGED $A_p = 0.35$
EFFECTIVE STREAM AREA (ACRES) = 0.50
TOTAL STREAM AREA (ACRES) = 0.50
PEAK FLOW RATE (CFS) AT CONFLUENCE = 1.57

FLOW PROCESS FROM NODE 36.00 TO NODE 37.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<
=====

INITIAL SUBAREA FLOW-LENGTH (FEET) = 150.00
ELEVATION DATA: UPSTREAM (FEET) = 166.70 DOWNSTREAM (FEET) = 166.40

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$
SUBAREA ANALYSIS USED MINIMUM T_c (MIN.) = 9.258
* 100 YEAR RAINFALL INTENSITY (INCH/HR) = 4.347

SUBAREA T_c AND LOSS RATE DATA (AMC III):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	F_p (INCH/HR)	A_p (DECIMAL)	SCS CN	T_c (MIN.)
CONDOMINIUMS	B	0.29	0.30	0.350	76	9.26

SUBAREA AVERAGE PERVIOUS LOSS RATE, F_p (INCH/HR) = 0.30

SUBAREA AVERAGE PERVIOUS AREA FRACTION, $A_p = 0.350$

SUBAREA RUNOFF (CFS) = 1.11

TOTAL AREA (ACRES) = 0.29 PEAK FLOW RATE (CFS) = 1.11

FLOW PROCESS FROM NODE 33.00 TO NODE 33.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
=====

TOTAL NUMBER OF STREAMS = 5
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 4 ARE:

TIME OF CONCENTRATION (MIN.) = 9.26

RAINFALL INTENSITY (INCH/HR) = 4.35

AREA-AVERAGED F_m (INCH/HR) = 0.11

AREA-AVERAGED F_p (INCH/HR) = 0.30

AREA-AVERAGED $A_p = 0.35$

EFFECTIVE STREAM AREA (ACRES) = 0.29

TOTAL STREAM AREA (ACRES) = 0.29

PEAK FLOW RATE (CFS) AT CONFLUENCE = 1.11

FLOW PROCESS FROM NODE 38.00 TO NODE 33.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<
=====

INITIAL SUBAREA FLOW-LENGTH (FEET) = 150.00
ELEVATION DATA: UPSTREAM (FEET) = 166.40 DOWNSTREAM (FEET) = 166.00

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$

SUBAREA ANALYSIS USED MINIMUM T_c (MIN.) = 8.741

* 100 YEAR RAINFALL INTENSITY (INCH/HR) = 4.493

SUBAREA Tc AND LOSS RATE DATA (AMC III):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN	Tc (MIN.)
CONDOMINIUMS	B	0.18	0.30	0.350	76	8.74

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp (INCH/HR) = 0.30
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.350
 SUBAREA RUNOFF (CFS) = 0.71
 TOTAL AREA (ACRES) = 0.18 PEAK FLOW RATE (CFS) = 0.71

FLOW PROCESS FROM NODE 33.00 TO NODE 33.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
 >>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<

=====

TOTAL NUMBER OF STREAMS = 5
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 5 ARE:
 TIME OF CONCENTRATION (MIN.) = 8.74
 RAINFALL INTENSITY (INCH/HR) = 4.49
 AREA-AVERAGED Fm (INCH/HR) = 0.11
 AREA-AVERAGED Fp (INCH/HR) = 0.30
 AREA-AVERAGED Ap = 0.35
 EFFECTIVE STREAM AREA (ACRES) = 0.18
 TOTAL STREAM AREA (ACRES) = 0.18
 PEAK FLOW RATE (CFS) AT CONFLUENCE = 0.71

** CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	20.09	8.24	4.646	0.32 (0.10)	0.31	4.4	30.00
1	20.88	9.81	4.205	0.32 (0.10)	0.31	5.0	14.00
1	21.07	10.11	4.134	0.32 (0.10)	0.31	5.2	26.00
1	21.10	10.17	4.119	0.32 (0.10)	0.31	5.2	27.00
1	21.31	10.85	3.968	0.32 (0.10)	0.31	5.5	20.00
1	21.26	11.33	3.873	0.32 (0.10)	0.30	5.6	17.00
1	21.25	11.40	3.859	0.32 (0.10)	0.30	5.7	23.00
1	21.24	11.43	3.853	0.32 (0.10)	0.30	5.7	29.00
1	20.34	13.10	3.564	0.32 (0.09)	0.30	5.9	24.00
1	18.74	15.76	3.205	0.32 (0.09)	0.29	6.2	12.00
1	16.77	19.10	2.870	0.32 (0.09)	0.29	6.2	10.00
2	2.58	9.14	4.379	0.30 (0.11)	0.35	0.7	30.00
3	1.57	12.87	3.600	0.30 (0.11)	0.35	0.5	34.00
4	1.11	9.26	4.347	0.30 (0.11)	0.35	0.3	36.00
5	0.71	8.74	4.493	0.30 (0.11)	0.35	0.2	38.00

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
 CONFLUENCE FORMULA USED FOR 5 STREAMS.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	25.62	8.24	4.646	0.31 (0.10)	0.32	5.7	30.00
2	26.00	8.74	4.493	0.31 (0.10)	0.32	6.0	38.00
3	26.28	9.14	4.379	0.31 (0.10)	0.32	6.2	30.00

4	26.33	9.26	4.347	0.31	(0.10)	0.32	6.3	36.00
5	26.50	9.81	4.205	0.31	(0.10)	0.32	6.5	14.00
6	26.63	10.11	4.134	0.31	(0.10)	0.32	6.7	26.00
7	26.64	10.17	4.119	0.31	(0.10)	0.32	6.7	27.00
8	26.74	10.85	3.968	0.31	(0.10)	0.32	7.0	20.00
9	26.62	11.33	3.873	0.31	(0.10)	0.31	7.2	17.00
10	26.60	11.40	3.859	0.31	(0.10)	0.31	7.2	23.00
11	26.58	11.43	3.853	0.31	(0.10)	0.31	7.2	29.00
12	25.63	12.87	3.600	0.31	(0.10)	0.31	7.5	34.00
13	25.45	13.10	3.564	0.31	(0.10)	0.31	7.5	24.00
14	23.32	15.76	3.205	0.31	(0.09)	0.30	7.8	12.00
15	20.86	19.10	2.870	0.31	(0.09)	0.30	7.9	10.00

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE (CFS) = 26.74 Tc (MIN.) = 10.85
EFFECTIVE AREA (ACRES) = 7.05 AREA-AVERAGED Fm (INCH/HR) = 0.10
AREA-AVERAGED Fp (INCH/HR) = 0.31 AREA-AVERAGED Ap = 0.32
TOTAL AREA (ACRES) = 7.9
LONGEST FLOWPATH FROM NODE 10.00 TO NODE 33.00 = 1705.00 FEET.

FLOW PROCESS FROM NODE 33.00 TO NODE 38.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<

=====

ELEVATION DATA: UPSTREAM (FEET) = 150.50 DOWNSTREAM (FEET) = 150.20
FLOW LENGTH (FEET) = 150.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 36.0 INCH PIPE IS 27.2 INCHES
PIPE-FLOW VELOCITY (FEET/SEC.) = 4.67
ESTIMATED PIPE DIAMETER (INCH) = 36.00 NUMBER OF PIPES = 1
PIPE-FLOW (CFS) = 26.74
PIPE TRAVEL TIME (MIN.) = 0.54 Tc (MIN.) = 11.39
LONGEST FLOWPATH FROM NODE 10.00 TO NODE 38.00 = 1855.00 FEET.

FLOW PROCESS FROM NODE 38.00 TO NODE 38.00 IS CODE = 10

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FLOW PROCESS FROM NODE 39.00 TO NODE 40.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

=====

INITIAL SUBAREA FLOW-LENGTH (FEET) = 300.00
ELEVATION DATA: UPSTREAM (FEET) = 168.00 DOWNSTREAM (FEET) = 166.90

Tc = K * [(LENGTH** 3.00) / (ELEVATION CHANGE)]**0.20
SUBAREA ANALYSIS USED MINIMUM Tc (MIN.) = 10.822
* 100 YEAR RAINFALL INTENSITY (INCH/HR) = 3.975
SUBAREA Tc AND LOSS RATE DATA (AMC III):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN	Tc (MIN.)
CONDOMINIUMS	B	0.80	0.30	0.350	76	10.82

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp (INCH/HR) = 0.30
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.350
 SUBAREA RUNOFF (CFS) = 2.79
 TOTAL AREA (ACRES) = 0.80 PEAK FLOW RATE (CFS) = 2.79

 FLOW PROCESS FROM NODE 40.00 TO NODE 41.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
 >>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<

=====

ELEVATION DATA: UPSTREAM (FEET) = 154.00 DOWNSTREAM (FEET) = 151.10
 FLOW LENGTH (FEET) = 40.00 MANNING'S N = 0.013
 ESTIMATED PIPE DIAMETER (INCH) INCREASED TO 12.000
 DEPTH OF FLOW IN 12.0 INCH PIPE IS 4.5 INCHES
 PIPE-FLOW VELOCITY (FEET/SEC.) = 10.38
 ESTIMATED PIPE DIAMETER (INCH) = 12.00 NUMBER OF PIPES = 1
 PIPE-FLOW (CFS) = 2.79
 PIPE TRAVEL TIME (MIN.) = 0.06 Tc (MIN.) = 10.89
 LONGEST FLOWPATH FROM NODE 39.00 TO NODE 41.00 = 340.00 FEET.

 FLOW PROCESS FROM NODE 41.00 TO NODE 41.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

=====

MAINLINE Tc (MIN.) = 10.89
 * 100 YEAR RAINFALL INTENSITY (INCH/HR) = 3.962
 SUBAREA LOSS RATE DATA (AMC III):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
CONDOMINIUMS	B	0.45	0.30	0.350	76

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp (INCH/HR) = 0.30
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.350
 SUBAREA AREA (ACRES) = 0.45 SUBAREA RUNOFF (CFS) = 1.56
 EFFECTIVE AREA (ACRES) = 1.25 AREA-AVERAGED Fm (INCH/HR) = 0.10
 AREA-AVERAGED Fp (INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.35
 TOTAL AREA (ACRES) = 1.2 PEAK FLOW RATE (CFS) = 4.34

 FLOW PROCESS FROM NODE 41.00 TO NODE 42.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
 >>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<

=====

ELEVATION DATA: UPSTREAM (FEET) = 151.10 DOWNSTREAM (FEET) = 150.60
 FLOW LENGTH (FEET) = 270.00 MANNING'S N = 0.013
 DEPTH OF FLOW IN 18.0 INCH PIPE IS 14.5 INCHES
 PIPE-FLOW VELOCITY (FEET/SEC.) = 2.84
 ESTIMATED PIPE DIAMETER (INCH) = 18.00 NUMBER OF PIPES = 1
 PIPE-FLOW (CFS) = 4.34

PIPE TRAVEL TIME(MIN.) = 1.58 Tc(MIN.) = 12.47
LONGEST FLOWPATH FROM NODE 39.00 TO NODE 42.00 = 610.00 FEET.

FLOW PROCESS FROM NODE 42.00 TO NODE 42.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

MAINLINE Tc(MIN.) = 12.47

* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 3.665

SUBAREA LOSS RATE DATA(AMC III):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
CONDOMINIUMS	B	1.15	0.30	0.350	76

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.30

SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.350

SUBAREA AREA(ACRES) = 1.15 SUBAREA RUNOFF(CFS) = 3.68

EFFECTIVE AREA(ACRES) = 2.40 AREA-AVERAGED Fm(INCH/HR) = 0.10

AREA-AVERAGED Fp(INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.35

TOTAL AREA(ACRES) = 2.4 PEAK FLOW RATE(CFS) = 7.69

FLOW PROCESS FROM NODE 42.00 TO NODE 42.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

TOTAL NUMBER OF STREAMS = 3

CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:

TIME OF CONCENTRATION(MIN.) = 12.47

RAINFALL INTENSITY(INCH/HR) = 3.67

AREA-AVERAGED Fm(INCH/HR) = 0.10

AREA-AVERAGED Fp(INCH/HR) = 0.30

AREA-AVERAGED Ap = 0.35

EFFECTIVE STREAM AREA(ACRES) = 2.40

TOTAL STREAM AREA(ACRES) = 2.40

PEAK FLOW RATE(CFS) AT CONFLUENCE = 7.69

FLOW PROCESS FROM NODE 43.00 TO NODE 44.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<

>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

INITIAL SUBAREA FLOW-LENGTH(FEET) = 180.00

ELEVATION DATA: UPSTREAM(FEET) = 168.00 DOWNSTREAM(FEET) = 166.20

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$

SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) = 7.218

* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 5.013

SUBAREA Tc AND LOSS RATE DATA(AMC III):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN	Tc (MIN.)
CONDOMINIUMS	B	0.44	0.30	0.350	76	7.22

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.30

SUBAREA AVERAGE PERVIOUS AREA FRACTION, $A_p = 0.350$
 SUBAREA RUNOFF(CFS) = 1.94
 TOTAL AREA(ACRES) = 0.44 PEAK FLOW RATE(CFS) = 1.94

FLOW PROCESS FROM NODE 44.00 TO NODE 42.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
 >>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 153.00 DOWNSTREAM(FEET) = 150.60
 FLOW LENGTH(FEET) = 75.00 MANNING'S N = 0.013
 ESTIMATED PIPE DIAMETER(INCH) INCREASED TO 12.000
 DEPTH OF FLOW IN 12.0 INCH PIPE IS 4.6 INCHES
 PIPE-FLOW VELOCITY(FEET/SEC.) = 6.98
 ESTIMATED PIPE DIAMETER(INCH) = 12.000 NUMBER OF PIPES = 1
 PIPE-FLOW(CFS) = 1.94
 PIPE TRAVEL TIME(MIN.) = 0.18 T_c (MIN.) = 7.40
 LONGEST FLOWPATH FROM NODE 43.00 TO NODE 42.00 = 255.00 FEET.

FLOW PROCESS FROM NODE 42.00 TO NODE 42.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

=====

TOTAL NUMBER OF STREAMS = 3
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
 TIME OF CONCENTRATION(MIN.) = 7.40
 RAINFALL INTENSITY(INCH/HR) = 4.94
 AREA-AVERAGED F_m (INCH/HR) = 0.10
 AREA-AVERAGED F_p (INCH/HR) = 0.30
 AREA-AVERAGED $A_p = 0.35$
 EFFECTIVE STREAM AREA(ACRES) = 0.44
 TOTAL STREAM AREA(ACRES) = 0.44
 PEAK FLOW RATE(CFS) AT CONFLUENCE = 1.94

FLOW PROCESS FROM NODE 45.00 TO NODE 46.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
 >>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

=====

INITIAL SUBAREA FLOW-LENGTH(FEET) = 180.00
 ELEVATION DATA: UPSTREAM(FEET) = 168.00 DOWNSTREAM(FEET) = 166.40

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$
 SUBAREA ANALYSIS USED MINIMUM T_c (MIN.) = 7.390
 * 100 YEAR RAINFALL INTENSITY(INCH/HR) = 4.946

SUBAREA T_c AND LOSS RATE DATA(AMC III):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	F_p (INCH/HR)	A_p (DECIMAL)	SCS CN	T_c (MIN.)
CONDOMINIUMS	B	0.45	0.30	0.350	76	7.39

SUBAREA AVERAGE PERVIOUS LOSS RATE, F_p (INCH/HR) = 0.30

SUBAREA AVERAGE PERVIOUS AREA FRACTION, $A_p = 0.350$

SUBAREA RUNOFF(CFS) = 1.96
 TOTAL AREA(ACRES) = 0.45 PEAK FLOW RATE(CFS) = 1.96

FLOW PROCESS FROM NODE 46.00 TO NODE 42.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
 >>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 153.00 DOWNSTREAM(FEET) = 150.60
 FLOW LENGTH(FEET) = 75.00 MANNING'S N = 0.013
 ESTIMATED PIPE DIAMETER(INCH) INCREASED TO 12.000
 DEPTH OF FLOW IN 12.0 INCH PIPE IS 4.6 INCHES
 PIPE-FLOW VELOCITY(FEET/SEC.) = 7.02
 ESTIMATED PIPE DIAMETER(INCH) = 12.000 NUMBER OF PIPES = 1
 PIPE-FLOW(CFS) = 1.96
 PIPE TRAVEL TIME(MIN.) = 0.18 Tc(MIN.) = 7.57
 LONGEST FLOWPATH FROM NODE 45.00 TO NODE 42.00 = 255.00 FEET.

FLOW PROCESS FROM NODE 42.00 TO NODE 42.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
 >>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<

=====

TOTAL NUMBER OF STREAMS = 3
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 3 ARE:
 TIME OF CONCENTRATION(MIN.) = 7.57
 RAINFALL INTENSITY(INCH/HR) = 4.88
 AREA-AVERAGED Fm(INCH/HR) = 0.10
 AREA-AVERAGED Fp(INCH/HR) = 0.30
 AREA-AVERAGED Ap = 0.35
 EFFECTIVE STREAM AREA(ACRES) = 0.45
 TOTAL STREAM AREA(ACRES) = 0.45
 PEAK FLOW RATE(CFS) AT CONFLUENCE = 1.96

** CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	7.69	12.47	3.665	0.30 (0.10)	0.35	2.4	39.00
2	1.94	7.40	4.944	0.30 (0.11)	0.35	0.4	43.00
3	1.96	7.57	4.879	0.30 (0.10)	0.35	0.4	45.00

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
 CONFLUENCE FORMULA USED FOR 3 STREAMS.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	10.09	7.40	4.944	0.30 (0.11)	0.35	2.3	43.00
2	10.14	7.57	4.879	0.30 (0.10)	0.35	2.3	45.00
3	10.58	12.47	3.665	0.30 (0.10)	0.35	3.3	39.00

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE (CFS) = 10.58 Tc (MIN.) = 12.47
 EFFECTIVE AREA (ACRES) = 3.29 AREA-AVERAGED Fm (INCH/HR) = 0.10
 AREA-AVERAGED Fp (INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.35
 TOTAL AREA (ACRES) = 3.3
 LONGEST FLOWPATH FROM NODE 39.00 TO NODE 42.00 = 610.00 FEET.

 FLOW PROCESS FROM NODE 42.00 TO NODE 38.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
 >>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<

=====
 ELEVATION DATA: UPSTREAM (FEET) = 150.60 DOWNSTREAM (FEET) = 150.20
 FLOW LENGTH (FEET) = 190.00 MANNING'S N = 0.013
 DEPTH OF FLOW IN 27.0 INCH PIPE IS 17.7 INCHES
 PIPE-FLOW VELOCITY (FEET/SEC.) = 3.84
 ESTIMATED PIPE DIAMETER (INCH) = 27.00 NUMBER OF PIPES = 1
 PIPE-FLOW (CFS) = 10.58
 PIPE TRAVEL TIME (MIN.) = 0.83 Tc (MIN.) = 13.29
 LONGEST FLOWPATH FROM NODE 39.00 TO NODE 38.00 = 800.00 FEET.

 FLOW PROCESS FROM NODE 38.00 TO NODE 38.00 IS CODE = 11

>>>>CONFLUENCE MEMORY BANK # 1 WITH THE MAIN-STREAM MEMORY<<<<<

** MAIN STREAM CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp (Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	10.09	8.26	4.641	0.30 (0.11)	0.35	2.3	43.00
2	10.14	8.40	4.596	0.30 (0.10)	0.35	2.3	45.00
3	10.58	13.29	3.533	0.30 (0.10)	0.35	3.3	39.00

LONGEST FLOWPATH FROM NODE 39.00 TO NODE 38.00 = 800.00 FEET.

** MEMORY BANK # 1 CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp (Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	25.62	8.78	4.480	0.31 (0.10)	0.32	5.7	30.00
2	26.00	9.28	4.342	0.31 (0.10)	0.32	6.0	38.00
3	26.28	9.68	4.238	0.31 (0.10)	0.32	6.2	30.00
4	26.33	9.79	4.209	0.31 (0.10)	0.32	6.3	36.00
5	26.50	10.35	4.078	0.31 (0.10)	0.32	6.5	14.00
6	26.63	10.64	4.014	0.31 (0.10)	0.32	6.7	26.00
7	26.64	10.71	3.999	0.31 (0.10)	0.32	6.7	27.00
8	26.74	11.39	3.860	0.31 (0.10)	0.32	7.0	20.00
9	26.62	11.86	3.772	0.31 (0.10)	0.31	7.2	17.00
10	26.60	11.94	3.758	0.31 (0.10)	0.31	7.2	23.00
11	26.58	11.96	3.753	0.31 (0.10)	0.31	7.2	29.00
12	25.63	13.41	3.516	0.31 (0.10)	0.31	7.5	34.00
13	25.45	13.64	3.482	0.31 (0.10)	0.31	7.5	24.00
14	23.32	16.31	3.143	0.31 (0.09)	0.30	7.8	12.00
15	20.86	19.67	2.823	0.31 (0.09)	0.30	7.9	10.00

LONGEST FLOWPATH FROM NODE 10.00 TO NODE 38.00 = 1855.00 FEET.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	35.06	8.26	4.641	0.31 (0.10)	0.33	7.7	43.00
2	35.29	8.40	4.596	0.31 (0.10)	0.33	7.8	45.00
3	35.79	8.78	4.480	0.31 (0.10)	0.33	8.1	30.00
4	36.22	9.28	4.342	0.31 (0.10)	0.33	8.5	38.00
5	36.53	9.68	4.238	0.31 (0.10)	0.33	8.8	30.00
6	36.59	9.79	4.209	0.31 (0.10)	0.33	8.9	36.00
7	36.81	10.35	4.078	0.31 (0.10)	0.33	9.2	14.00
8	36.97	10.64	4.014	0.31 (0.10)	0.33	9.5	26.00
9	36.99	10.71	3.999	0.31 (0.10)	0.33	9.5	27.00
10	37.15	11.39	3.860	0.31 (0.10)	0.33	10.0	20.00
11	37.07	11.86	3.772	0.31 (0.10)	0.33	10.2	17.00
12	37.06	11.94	3.758	0.31 (0.10)	0.32	10.3	23.00
13	37.05	11.96	3.753	0.31 (0.10)	0.32	10.3	29.00
14	36.28	13.29	3.533	0.31 (0.10)	0.32	10.8	39.00
15	36.16	13.41	3.516	0.31 (0.10)	0.32	10.8	34.00
16	35.87	13.64	3.482	0.31 (0.10)	0.32	10.8	24.00
17	32.70	16.31	3.143	0.31 (0.10)	0.32	11.1	12.00
18	29.25	19.67	2.823	0.31 (0.10)	0.32	11.2	10.00
TOTAL AREA (ACRES) =			11.2				

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE (CFS) = 37.15 Tc (MIN.) = 11.391
 EFFECTIVE AREA (ACRES) = 9.97 AREA-AVERAGED Fm (INCH/HR) = 0.10
 AREA-AVERAGED Fp (INCH/HR) = 0.31 AREA-AVERAGED Ap = 0.33
 TOTAL AREA (ACRES) = 11.2
 LONGEST FLOWPATH FROM NODE 10.00 TO NODE 38.00 = 1855.00 FEET.

 FLOW PROCESS FROM NODE 38.00 TO NODE 38.00 IS CODE = 12

>>>>CLEAR MEMORY BANK # 1 <<<<<

 FLOW PROCESS FROM NODE 38.00 TO NODE 47.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
 >>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<

=====

ELEVATION DATA: UPSTREAM (FEET) = 150.20 DOWNSTREAM (FEET) = 149.90
 FLOW LENGTH (FEET) = 150.00 MANNING'S N = 0.013
 DEPTH OF FLOW IN 42.0 INCH PIPE IS 29.7 INCHES
 PIPE-FLOW VELOCITY (FEET/SEC.) = 5.11
 ESTIMATED PIPE DIAMETER (INCH) = 42.00 NUMBER OF PIPES = 1
 PIPE-FLOW (CFS) = 37.15
 PIPE TRAVEL TIME (MIN.) = 0.49 Tc (MIN.) = 11.88
 LONGEST FLOWPATH FROM NODE 10.00 TO NODE 47.00 = 2005.00 FEET.

 FLOW PROCESS FROM NODE 47.00 TO NODE 47.00 IS CODE = 10

>>>>MAIN-STREAM MEMORY COPIED ONTO MEMORY BANK # 1 <<<<<
=====

FLOW PROCESS FROM NODE 32.00 TO NODE 48.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<
=====

INITIAL SUBAREA FLOW-LENGTH(FEET) = 300.00
ELEVATION DATA: UPSTREAM(FEET) = 166.30 DOWNSTREAM(FEET) = 166.00

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$
SUBAREA ANALYSIS USED MINIMUM T_c (MIN.) = 11.850
* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 3.774
SUBAREA T_c AND LOSS RATE DATA(AMC III):
DEVELOPMENT TYPE/ SCS SOIL AREA F_p A_p SCS T_c
LAND USE GROUP (ACRES) (INCH/HR) (DECIMAL) CN (MIN.)
COMMERCIAL B 2.62 0.30 0.100 76 11.85
SUBAREA AVERAGE PERVIOUS LOSS RATE, F_p (INCH/HR) = 0.30
SUBAREA AVERAGE PERVIOUS AREA FRACTION, A_p = 0.100
SUBAREA RUNOFF(CFS) = 8.83
TOTAL AREA(ACRES) = 2.62 PEAK FLOW RATE(CFS) = 8.83

FLOW PROCESS FROM NODE 48.00 TO NODE 49.00 IS CODE = 61

>>>>COMPUTE STREET FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>(STANDARD CURB SECTION USED)<<<<<
=====

UPSTREAM ELEVATION(FEET) = 166.00 DOWNSTREAM ELEVATION(FEET) = 165.60
STREET LENGTH(FEET) = 485.00 CURB HEIGHT(INCHES) = 6.0
STREET HALFWIDTH(FEET) = 25.00

DISTANCE FROM CROWN TO CROSSFALL GRADEBREAK(FEET) = 10.00
INSIDE STREET CROSSFALL(DECIMAL) = 0.020
OUTSIDE STREET CROSSFALL(DECIMAL) = 0.020

SPECIFIED NUMBER OF HALFSTREETS CARRYING RUNOFF = 2
STREET PARKWAY CROSSFALL(DECIMAL) = 0.020
Manning's FRICTION FACTOR for Streetflow Section(curbs-to-curbs) = 0.0150
Manning's FRICTION FACTOR for Back-of-Walk Flow Section = 0.0200

**TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 15.10
STREET FLOWING FULL
STREETFLOW MODEL RESULTS USING ESTIMATED FLOW:
STREET FLOW DEPTH(FEET) = 0.63
HALFSTREET FLOOD WIDTH(FEET) = 31.53
AVERAGE FLOW VELOCITY(FEET/SEC.) = 1.09
PRODUCT OF DEPTH&VELOCITY(FT*FT/SEC.) = 0.69
STREET FLOW TRAVEL TIME(MIN.) = 7.39 T_c (MIN.) = 19.24
* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 2.859
SUBAREA LOSS RATE DATA(AMC III):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
COMMERCIAL	B	4.89	0.30	0.100	76
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp (INCH/HR) = 0.30					
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.100					
SUBAREA AREA (ACRES) =		4.89	SUBAREA RUNOFF (CFS) =		12.45
EFFECTIVE AREA (ACRES) =		7.51	AREA-AVERAGED Fm (INCH/HR) =		0.03
AREA-AVERAGED Fp (INCH/HR) =		0.30	AREA-AVERAGED Ap =		0.10
TOTAL AREA (ACRES) =		7.5	PEAK FLOW RATE (CFS) =		19.12

END OF SUBAREA STREET FLOW HYDRAULICS:

DEPTH (FEET) = 0.67 HALFSTREET FLOOD WIDTH (FEET) = 33.42
FLOW VELOCITY (FEET/SEC.) = 1.18 DEPTH*VELOCITY (FT*FT/SEC.) = 0.79
*NOTE: INITIAL SUBAREA NOMOGRAPH WITH SUBAREA PARAMETERS,
AND L = 485.0 FT WITH ELEVATION-DROP = 0.4 FT, IS 14.4 CFS,
WHICH EXCEEDS THE TOP-OF-CURB STREET CAPACITY AT NODE 49.00
LONGEST FLOWPATH FROM NODE 32.00 TO NODE 49.00 = 785.00 FEET.

FLOW PROCESS FROM NODE 49.00 TO NODE 50.00 IS CODE = 51

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<<
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM (FEET) = 165.60 DOWNSTREAM (FEET) = 165.40
CHANNEL LENGTH THRU SUBAREA (FEET) = 180.00 CHANNEL SLOPE = 0.0011
CHANNEL BASE (FEET) = 5.00 "Z" FACTOR = 0.020
MANNING'S FACTOR = 0.015 MAXIMUM DEPTH (FEET) = 2.00
CHANNEL FLOW THRU SUBAREA (CFS) = 19.12
FLOW VELOCITY (FEET/SEC.) = 2.96 FLOW DEPTH (FEET) = 1.28
TRAVEL TIME (MIN.) = 1.01 Tc (MIN.) = 20.25
LONGEST FLOWPATH FROM NODE 32.00 TO NODE 50.00 = 965.00 FEET.

FLOW PROCESS FROM NODE 50.00 TO NODE 50.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

=====

TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION (MIN.) = 20.25
RAINFALL INTENSITY (INCH/HR) = 2.78
AREA-AVERAGED Fm (INCH/HR) = 0.03
AREA-AVERAGED Fp (INCH/HR) = 0.30
AREA-AVERAGED Ap = 0.10
EFFECTIVE STREAM AREA (ACRES) = 7.51
TOTAL STREAM AREA (ACRES) = 7.51
PEAK FLOW RATE (CFS) AT CONFLUENCE = 19.12

FLOW PROCESS FROM NODE 50.00 TO NODE 50.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

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=====
INITIAL SUBAREA FLOW-LENGTH (FEET) = 665.00
ELEVATION DATA: UPSTREAM (FEET) = 167.20 DOWNSTREAM (FEET) = 165.40

Tc = K * [(LENGTH** 3.00) / (ELEVATION CHANGE)]**0.20
SUBAREA ANALYSIS USED MINIMUM Tc (MIN.) = 13.351
* 100 YEAR RAINFALL INTENSITY (INCH/HR) = 3.524
SUBAREA Tc AND LOSS RATE DATA (AMC III):
DEVELOPMENT TYPE/      SCS SOIL  AREA      Fp        Ap        SCS  Tc
LAND USE                GROUP   (ACRES)  (INCH/HR) (DECIMAL) CN (MIN.)
COMMERCIAL              B       6.31     0.30     0.100    76   13.35
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp (INCH/HR) = 0.30
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.100
SUBAREA RUNOFF (CFS) = 19.84
TOTAL AREA (ACRES) = 6.31 PEAK FLOW RATE (CFS) = 19.84

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*****
FLOW PROCESS FROM NODE 50.00 TO NODE 50.00 IS CODE = 1
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>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<
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TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
TIME OF CONCENTRATION (MIN.) = 13.35
RAINFALL INTENSITY (INCH/HR) = 3.52
AREA-AVERAGED Fm (INCH/HR) = 0.03
AREA-AVERAGED Fp (INCH/HR) = 0.30
AREA-AVERAGED Ap = 0.10
EFFECTIVE STREAM AREA (ACRES) = 6.31
TOTAL STREAM AREA (ACRES) = 6.31
PEAK FLOW RATE (CFS) AT CONFLUENCE = 19.84

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** CONFLUENCE DATA **

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STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp (Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	19.12	20.25	2.776	0.30 (0.03)	0.10	7.5	32.00
2	19.84	13.35	3.524	0.30 (0.03)	0.10	6.3	50.00

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RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
CONFLUENCE FORMULA USED FOR 2 STREAMS.

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** PEAK FLOW RATE TABLE **

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STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp (Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	35.89	13.35	3.524	0.30 (0.03)	0.10	11.3	50.00
2	34.71	20.25	2.776	0.30 (0.03)	0.10	13.8	32.00

```

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
PEAK FLOW RATE (CFS) = 35.89 Tc (MIN.) = 13.35
EFFECTIVE AREA (ACRES) = 11.26 AREA-AVERAGED Fm (INCH/HR) = 0.03
AREA-AVERAGED Fp (INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.10
TOTAL AREA (ACRES) = 13.8
LONGEST FLOWPATH FROM NODE 32.00 TO NODE 50.00 = 965.00 FEET.

```

 FLOW PROCESS FROM NODE 50.00 TO NODE 47.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
 >>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW) <<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 162.40 DOWNSTREAM(FEET) = 149.90
 FLOW LENGTH(FEET) = 185.00 MANNING'S N = 0.013
 DEPTH OF FLOW IN 21.0 INCH PIPE IS 15.5 INCHES
 PIPE-FLOW VELOCITY(FEET/SEC.) = 18.87
 ESTIMATED PIPE DIAMETER(INCH) = 21.00 NUMBER OF PIPES = 1
 PIPE-FLOW(CFS) = 35.89
 PIPE TRAVEL TIME(MIN.) = 0.16 Tc(MIN.) = 13.51
 LONGEST FLOWPATH FROM NODE 32.00 TO NODE 47.00 = 1150.00 FEET.

 FLOW PROCESS FROM NODE 47.00 TO NODE 47.00 IS CODE = 11

>>>>CONFLUENCE MEMORY BANK # 1 WITH THE MAIN-STREAM MEMORY<<<<<

** MAIN STREAM CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	35.89	13.51	3.500	0.30(0.03)	0.10	11.3	50.00
2	34.71	20.41	2.763	0.30(0.03)	0.10	13.8	32.00
LONGEST FLOWPATH FROM NODE 32.00 TO NODE 47.00 = 1150.00 FEET.							

** MEMORY BANK # 1 CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	35.06	8.77	4.485	0.31(0.10)	0.33	7.7	43.00
2	35.29	8.91	4.445	0.31(0.10)	0.33	7.8	45.00
3	35.79	9.29	4.339	0.31(0.10)	0.33	8.1	30.00
4	36.22	9.77	4.215	0.31(0.10)	0.33	8.5	38.00
5	36.53	10.17	4.120	0.31(0.10)	0.33	8.8	30.00
6	36.59	10.29	4.093	0.31(0.10)	0.33	8.9	36.00
7	36.81	10.84	3.972	0.31(0.10)	0.33	9.2	14.00
8	36.97	11.13	3.912	0.31(0.10)	0.33	9.5	26.00
9	36.99	11.20	3.898	0.31(0.10)	0.33	9.5	27.00
10	37.15	11.88	3.768	0.31(0.10)	0.33	10.0	20.00
11	37.07	12.35	3.685	0.31(0.10)	0.33	10.2	17.00
12	37.06	12.42	3.673	0.31(0.10)	0.32	10.3	23.00
13	37.05	12.45	3.668	0.31(0.10)	0.32	10.3	29.00
14	36.28	13.78	3.460	0.31(0.10)	0.32	10.8	39.00
15	36.16	13.90	3.444	0.31(0.10)	0.32	10.8	34.00
16	35.87	14.14	3.410	0.31(0.10)	0.32	10.8	24.00
17	32.70	16.82	3.088	0.31(0.10)	0.32	11.1	12.00
18	29.25	20.19	2.781	0.31(0.10)	0.32	11.2	10.00
LONGEST FLOWPATH FROM NODE 10.00 TO NODE 47.00 = 2005.00 FEET.							

** PEAK FLOW RATE TABLE **

STREAM	Q	Tc	Intensity	Fp(Fm)	Ap	Ae	HEADWATER
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NUMBER	(CFS)	(MIN.)	(INCH/HR)	(INCH/HR)		(ACRES)	NODE
1	64.95	8.77	4.485	0.31 (0.07)	0.22	15.0	43.00
2	65.38	8.91	4.445	0.31 (0.07)	0.22	15.2	45.00
3	66.42	9.29	4.339	0.31 (0.07)	0.22	15.9	30.00
4	67.51	9.77	4.215	0.31 (0.07)	0.22	16.6	38.00
5	68.36	10.17	4.120	0.31 (0.07)	0.22	17.3	30.00
6	68.57	10.29	4.093	0.31 (0.07)	0.22	17.5	36.00
7	69.50	10.84	3.972	0.31 (0.07)	0.21	18.3	14.00
8	70.03	11.13	3.912	0.31 (0.07)	0.21	18.8	26.00
9	70.14	11.20	3.898	0.31 (0.07)	0.21	18.9	27.00
10	71.13	11.88	3.768	0.31 (0.07)	0.21	19.9	20.00
11	71.62	12.35	3.685	0.31 (0.07)	0.21	20.5	17.00
12	71.69	12.42	3.673	0.31 (0.06)	0.21	20.6	23.00
13	71.71	12.45	3.668	0.31 (0.06)	0.21	20.7	29.00
14	72.32	13.51	3.500	0.31 (0.06)	0.21	21.9	50.00
15	72.12	13.78	3.460	0.31 (0.06)	0.21	22.1	39.00
16	71.98	13.90	3.444	0.31 (0.06)	0.21	22.2	34.00
17	71.65	14.14	3.410	0.31 (0.06)	0.21	22.3	24.00
18	68.02	16.82	3.088	0.31 (0.06)	0.20	23.6	12.00
19	64.00	20.19	2.781	0.31 (0.06)	0.20	24.9	10.00
20	63.77	20.41	2.763	0.31 (0.06)	0.20	25.0	32.00
TOTAL AREA (ACRES) =			25.0				

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE (CFS) = 72.32 Tc (MIN.) = 13.514
EFFECTIVE AREA (ACRES) = 21.94 AREA-AVERAGED Fm (INCH/HR) = 0.06
AREA-AVERAGED Fp (INCH/HR) = 0.31 AREA-AVERAGED Ap = 0.22
TOTAL AREA (ACRES) = 25.0
LONGEST FLOWPATH FROM NODE 10.00 TO NODE 47.00 = 2005.00 FEET.

FLOW PROCESS FROM NODE 47.00 TO NODE 47.00 IS CODE = 12

>>>>CLEAR MEMORY BANK # 1 <<<<<

FLOW PROCESS FROM NODE 47.00 TO NODE 47.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION (MIN.) = 13.51
RAINFALL INTENSITY (INCH/HR) = 3.50
AREA-AVERAGED Fm (INCH/HR) = 0.06
AREA-AVERAGED Fp (INCH/HR) = 0.31
AREA-AVERAGED Ap = 0.21
EFFECTIVE STREAM AREA (ACRES) = 21.94
TOTAL STREAM AREA (ACRES) = 25.00
PEAK FLOW RATE (CFS) AT CONFLUENCE = 72.32

FLOW PROCESS FROM NODE 51.00 TO NODE 52.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<
=====

INITIAL SUBAREA FLOW-LENGTH (FEET) = 140.00
ELEVATION DATA: UPSTREAM (FEET) = 166.70 DOWNSTREAM (FEET) = 166.30

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$
SUBAREA ANALYSIS USED MINIMUM T_c (MIN.) = 8.386
* 100 YEAR RAINFALL INTENSITY (INCH/HR) = 4.600

SUBAREA T_c AND LOSS RATE DATA (AMC III):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	F_p (INCH/HR)	A_p (DECIMAL)	SCS CN	T_c (MIN.)
CONDOMINIUMS	B	0.64	0.30	0.350	76	8.39

SUBAREA AVERAGE PERVIOUS LOSS RATE, F_p (INCH/HR) = 0.30

SUBAREA AVERAGE PERVIOUS AREA FRACTION, A_p = 0.350

SUBAREA RUNOFF (CFS) = 2.59

TOTAL AREA (ACRES) = 0.64 PEAK FLOW RATE (CFS) = 2.59

FLOW PROCESS FROM NODE 52.00 TO NODE 47.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<
=====

ELEVATION DATA: UPSTREAM (FEET) = 156.00 DOWNSTREAM (FEET) = 149.90
FLOW LENGTH (FEET) = 40.00 MANNING'S N = 0.013
ESTIMATED PIPE DIAMETER (INCH) INCREASED TO 12.000
DEPTH OF FLOW IN 12.0 INCH PIPE IS 3.6 INCHES
PIPE-FLOW VELOCITY (FEET/SEC.) = 13.30
ESTIMATED PIPE DIAMETER (INCH) = 12.000 NUMBER OF PIPES = 1
PIPE-FLOW (CFS) = 2.59
PIPE TRAVEL TIME (MIN.) = 0.05 T_c (MIN.) = 8.44
LONGEST FLOWPATH FROM NODE 51.00 TO NODE 47.00 = 180.00 FEET.

FLOW PROCESS FROM NODE 47.00 TO NODE 47.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<
=====

TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
TIME OF CONCENTRATION (MIN.) = 8.44
RAINFALL INTENSITY (INCH/HR) = 4.58
AREA-AVERAGED F_m (INCH/HR) = 0.10
AREA-AVERAGED F_p (INCH/HR) = 0.30
AREA-AVERAGED A_p = 0.35
EFFECTIVE STREAM AREA (ACRES) = 0.64
TOTAL STREAM AREA (ACRES) = 0.64
PEAK FLOW RATE (CFS) AT CONFLUENCE = 2.59

** CONFLUENCE DATA **

STREAM	Q	T_c	Intensity	F_p (F_m)	A_p	A_e	HEADWATER
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NUMBER	(CFS)	(MIN.)	(INCH/HR)	(INCH/HR)		(ACRES)	NODE
1	64.95	8.77	4.485	0.31 (0.07)	0.22	15.0	43.00
1	65.38	8.91	4.445	0.31 (0.07)	0.22	15.2	45.00
1	66.42	9.29	4.339	0.31 (0.07)	0.22	15.9	30.00
1	67.51	9.77	4.215	0.31 (0.07)	0.22	16.6	38.00
1	68.36	10.17	4.120	0.31 (0.07)	0.22	17.3	30.00
1	68.57	10.29	4.093	0.31 (0.07)	0.22	17.5	36.00
1	69.50	10.84	3.972	0.31 (0.07)	0.21	18.3	14.00
1	70.03	11.13	3.912	0.31 (0.07)	0.21	18.8	26.00
1	70.14	11.20	3.898	0.31 (0.07)	0.21	18.9	27.00
1	71.13	11.88	3.768	0.31 (0.07)	0.21	19.9	20.00
1	71.62	12.35	3.685	0.31 (0.07)	0.21	20.5	17.00
1	71.69	12.42	3.673	0.31 (0.06)	0.21	20.6	23.00
1	71.71	12.45	3.668	0.31 (0.06)	0.21	20.7	29.00
1	72.32	13.51	3.500	0.31 (0.06)	0.21	21.9	50.00
1	72.12	13.78	3.460	0.31 (0.06)	0.21	22.1	39.00
1	71.98	13.90	3.444	0.31 (0.06)	0.21	22.2	34.00
1	71.65	14.14	3.410	0.31 (0.06)	0.21	22.3	24.00
1	68.02	16.82	3.088	0.31 (0.06)	0.20	23.6	12.00
1	64.00	20.19	2.781	0.31 (0.06)	0.20	24.9	10.00
1	63.77	20.41	2.763	0.31 (0.06)	0.20	25.0	32.00
2	2.59	8.44	4.585	0.30 (0.10)	0.35	0.6	51.00

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	66.51	8.44	4.585	0.31 (0.07)	0.22	15.1	51.00
2	67.48	8.77	4.485	0.31 (0.07)	0.22	15.6	43.00
3	67.89	8.91	4.445	0.31 (0.07)	0.22	15.9	45.00
4	68.87	9.29	4.339	0.31 (0.07)	0.22	16.5	30.00
5	69.88	9.77	4.215	0.31 (0.07)	0.22	17.3	38.00
6	70.68	10.17	4.120	0.31 (0.07)	0.22	17.9	30.00
7	70.88	10.29	4.093	0.31 (0.07)	0.22	18.1	36.00
8	71.74	10.84	3.972	0.31 (0.07)	0.22	18.9	14.00
9	72.24	11.13	3.912	0.31 (0.07)	0.22	19.4	26.00
10	72.33	11.20	3.898	0.31 (0.07)	0.22	19.5	27.00
11	73.25	11.88	3.768	0.31 (0.07)	0.22	20.5	20.00
12	73.69	12.35	3.685	0.31 (0.07)	0.22	21.2	17.00
13	73.76	12.42	3.673	0.31 (0.07)	0.22	21.3	23.00
14	73.77	12.45	3.668	0.31 (0.07)	0.22	21.3	29.00
15	74.29	13.51	3.500	0.31 (0.07)	0.21	22.6	50.00
16	74.06	13.78	3.460	0.31 (0.07)	0.21	22.8	39.00
17	73.91	13.90	3.444	0.31 (0.06)	0.21	22.8	34.00
18	73.56	14.14	3.410	0.31 (0.06)	0.21	23.0	24.00
19	69.75	16.82	3.088	0.31 (0.06)	0.21	24.2	12.00
20	65.55	20.19	2.781	0.31 (0.06)	0.20	25.6	10.00
21	65.30	20.41	2.763	0.31 (0.06)	0.20	25.6	32.00

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE (CFS) = 74.29 Tc (MIN.) = 13.51
EFFECTIVE AREA (ACRES) = 22.58 AREA-AVERAGED Fm (INCH/HR) = 0.07

AREA-AVERAGED F_p (INCH/HR) = 0.31 AREA-AVERAGED A_p = 0.21
TOTAL AREA (ACRES) = 25.6
LONGEST FLOWPATH FROM NODE 10.00 TO NODE 47.00 = 2005.00 FEET.

FLOW PROCESS FROM NODE 47.00 TO NODE 53.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 149.80 DOWNSTREAM(FEET) = 149.70
FLOW LENGTH(FEET) = 100.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 60.0 INCH PIPE IS 45.6 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 4.64
ESTIMATED PIPE DIAMETER(INCH) = 60.00 NUMBER OF PIPES = 1
PIPE-FLOW (CFS) = 74.29
PIPE TRAVEL TIME (MIN.) = 0.36 T_c (MIN.) = 13.87
LONGEST FLOWPATH FROM NODE 10.00 TO NODE 53.00 = 2105.00 FEET.

FLOW PROCESS FROM NODE 53.00 TO NODE 53.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

=====

TOTAL NUMBER OF STREAMS = 3
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION (MIN.) = 13.87
RAINFALL INTENSITY (INCH/HR) = 3.45
AREA-AVERAGED F_m (INCH/HR) = 0.07
AREA-AVERAGED F_p (INCH/HR) = 0.31
AREA-AVERAGED A_p = 0.21
EFFECTIVE STREAM AREA (ACRES) = 22.58
TOTAL STREAM AREA (ACRES) = 25.64
PEAK FLOW RATE (CFS) AT CONFLUENCE = 74.29

FLOW PROCESS FROM NODE 38.00 TO NODE 54.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

=====

INITIAL SUBAREA FLOW-LENGTH (FEET) = 265.00
ELEVATION DATA: UPSTREAM(FEET) = 166.40 DOWNSTREAM(FEET) = 165.80

$$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$$

SUBAREA ANALYSIS USED MINIMUM T_c (MIN.) = 11.340
* 100 YEAR RAINFALL INTENSITY (INCH/HR) = 3.870

SUBAREA T_c AND LOSS RATE DATA (AMC III):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	F_p (INCH/HR)	A_p (DECIMAL)	SCS CN	T_c (MIN.)
CONDOMINIUMS	B	0.80	0.30	0.350	76	11.34

SUBAREA AVERAGE PERVIOUS LOSS RATE, F_p (INCH/HR) = 0.30

SUBAREA AVERAGE PERVIOUS AREA FRACTION, A_p = 0.350

SUBAREA RUNOFF (CFS) = 2.71

TOTAL AREA (ACRES) = 0.80 PEAK FLOW RATE (CFS) = 2.71

FLOW PROCESS FROM NODE 54.00 TO NODE 53.00 IS CODE = 41

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM (FEET) = 149.80 DOWNSTREAM (FEET) = 149.70
FLOW LENGTH (FEET) = 70.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 18.0 INCH PIPE IS 11.1 INCHES
PIPE-FLOW VELOCITY (FEET/SEC.) = 2.37
GIVEN PIPE DIAMETER (INCH) = 18.00 NUMBER OF PIPES = 1
PIPE-FLOW (CFS) = 2.71
PIPE TRAVEL TIME (MIN.) = 0.49 Tc (MIN.) = 11.83
LONGEST FLOWPATH FROM NODE 38.00 TO NODE 53.00 = 335.00 FEET.

FLOW PROCESS FROM NODE 53.00 TO NODE 53.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

=====

TOTAL NUMBER OF STREAMS = 3
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
TIME OF CONCENTRATION (MIN.) = 11.83
RAINFALL INTENSITY (INCH/HR) = 3.78
AREA-AVERAGED Fm (INCH/HR) = 0.11
AREA-AVERAGED Fp (INCH/HR) = 0.30
AREA-AVERAGED Ap = 0.35
EFFECTIVE STREAM AREA (ACRES) = 0.80
TOTAL STREAM AREA (ACRES) = 0.80
PEAK FLOW RATE (CFS) AT CONFLUENCE = 2.71

FLOW PROCESS FROM NODE 55.00 TO NODE 56.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

=====

INITIAL SUBAREA FLOW-LENGTH (FEET) = 130.00
ELEVATION DATA: UPSTREAM (FEET) = 166.50 DOWNSTREAM (FEET) = 166.30

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$

SUBAREA ANALYSIS USED MINIMUM Tc (MIN.) = 9.214

* 100 YEAR RAINFALL INTENSITY (INCH/HR) = 4.359

SUBAREA Tc AND LOSS RATE DATA (AMC III):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN	Tc (MIN.)
CONDOMINIUMS	B	0.19	0.30	0.350	76	9.21

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp (INCH/HR) = 0.30

SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.350

SUBAREA RUNOFF (CFS) = 0.73

TOTAL AREA (ACRES) = 0.19 PEAK FLOW RATE (CFS) = 0.73

FLOW PROCESS FROM NODE 56.00 TO NODE 53.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<

ELEVATION DATA: UPSTREAM(FEET) = 156.00 DOWNSTREAM(FEET) = 149.70
FLOW LENGTH(FEET) = 40.00 MANNING'S N = 0.013
ESTIMATED PIPE DIAMETER(INCH) INCREASED TO 12.000
DEPTH OF FLOW IN 12.0 INCH PIPE IS 1.9 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 9.25
ESTIMATED PIPE DIAMETER(INCH) = 12.000 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 0.73
PIPE TRAVEL TIME(MIN.) = 0.07 Tc(MIN.) = 9.29
LONGEST FLOWPATH FROM NODE 55.00 TO NODE 53.00 = 170.00 FEET.

FLOW PROCESS FROM NODE 53.00 TO NODE 53.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<

TOTAL NUMBER OF STREAMS = 3
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 3 ARE:
TIME OF CONCENTRATION(MIN.) = 9.29
RAINFALL INTENSITY(INCH/HR) = 4.34
AREA-AVERAGED Fm(INCH/HR) = 0.11
AREA-AVERAGED Fp(INCH/HR) = 0.30
AREA-AVERAGED Ap = 0.35
EFFECTIVE STREAM AREA(ACRES) = 0.19
TOTAL STREAM AREA(ACRES) = 0.19
PEAK FLOW RATE(CFS) AT CONFLUENCE = 0.73

** CONFLUENCE DATA **

Table with 8 columns: STREAM NUMBER, Q (CFS), Tc (MIN.), Intensity (INCH/HR), Fp(Fm) (INCH/HR), Ap, Ae (ACRES), HEADWATER NODE. It lists 20 rows of data for different stream numbers and their respective flow characteristics.

1	69.75	17.19	3.050	0.31 (0.06)	0.21	24.2	12.00
1	65.55	20.56	2.752	0.31 (0.06)	0.20	25.6	10.00
1	65.30	20.79	2.735	0.31 (0.06)	0.20	25.6	32.00
2	2.71	11.83	3.777	0.30 (0.11)	0.35	0.8	38.00
3	0.73	9.29	4.339	0.30 (0.11)	0.35	0.2	55.00

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
CONFLUENCE FORMULA USED FOR 3 STREAMS.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp (Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	69.62	8.81	4.473	0.31 (0.07)	0.23	15.8	51.00
2	70.64	9.14	4.380	0.31 (0.07)	0.23	16.4	43.00
3	71.06	9.28	4.342	0.31 (0.07)	0.23	16.7	45.00
4	71.09	9.29	4.339	0.31 (0.07)	0.23	16.7	55.00
5	72.07	9.66	4.243	0.31 (0.07)	0.23	17.3	30.00
6	73.12	10.14	4.126	0.31 (0.07)	0.23	18.2	38.00
7	73.94	10.53	4.038	0.31 (0.07)	0.23	18.8	30.00
8	74.14	10.65	4.013	0.31 (0.07)	0.23	19.0	36.00
9	75.04	11.20	3.898	0.31 (0.07)	0.23	19.9	14.00
10	75.56	11.49	3.841	0.31 (0.07)	0.23	20.4	26.00
11	75.66	11.56	3.828	0.31 (0.07)	0.22	20.5	27.00
12	76.04	11.83	3.777	0.31 (0.07)	0.22	20.9	38.00
13	76.53	12.24	3.704	0.31 (0.07)	0.22	21.5	20.00
14	76.89	12.71	3.625	0.31 (0.07)	0.22	22.2	17.00
15	76.95	12.78	3.613	0.31 (0.07)	0.22	22.3	23.00
16	76.96	12.81	3.609	0.31 (0.07)	0.22	22.3	29.00
17	77.33	13.87	3.448	0.31 (0.07)	0.22	23.6	50.00
18	77.07	14.14	3.410	0.31 (0.07)	0.22	23.8	39.00
19	76.90	14.26	3.394	0.31 (0.07)	0.22	23.8	34.00
20	76.53	14.50	3.362	0.31 (0.07)	0.22	24.0	24.00
21	72.43	17.19	3.050	0.31 (0.06)	0.21	25.2	12.00
22	67.95	20.56	2.752	0.31 (0.06)	0.21	26.5	10.00
23	67.70	20.79	2.735	0.31 (0.06)	0.21	26.6	32.00

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE (CFS) = 77.33 Tc (MIN.) = 13.87
EFFECTIVE AREA (ACRES) = 23.57 AREA-AVERAGED Fm (INCH/HR) = 0.07
AREA-AVERAGED Fp (INCH/HR) = 0.31 AREA-AVERAGED Ap = 0.22
TOTAL AREA (ACRES) = 26.6
LONGEST FLOWPATH FROM NODE 10.00 TO NODE 53.00 = 2105.00 FEET.

FLOW PROCESS FROM NODE 53.00 TO NODE 57.00 IS CODE = 31

>>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<

=====

ELEVATION DATA: UPSTREAM (FEET) = 149.70 DOWNSTREAM (FEET) = 149.10
FLOW LENGTH (FEET) = 280.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 54.0 INCH PIPE IS 39.0 INCHES
PIPE-FLOW VELOCITY (FEET/SEC.) = 6.29
ESTIMATED PIPE DIAMETER (INCH) = 54.00 NUMBER OF PIPES = 1

PIPE-FLOW (CFS) = 77.33
PIPE TRAVEL TIME (MIN.) = 0.74 Tc (MIN.) = 14.62
LONGEST FLOWPATH FROM NODE 10.00 TO NODE 57.00 = 2385.00 FEET.

FLOW PROCESS FROM NODE 57.00 TO NODE 57.00 IS CODE = 10

>>>>MAIN-STREAM MEMORY COPIED ONTO MEMORY BANK # 1 <<<<<
=====

FLOW PROCESS FROM NODE 58.00 TO NODE 57.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<
=====

INITIAL SUBAREA FLOW-LENGTH (FEET) = 240.00
ELEVATION DATA: UPSTREAM (FEET) = 166.50 DOWNSTREAM (FEET) = 165.60

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$
SUBAREA ANALYSIS USED MINIMUM Tc (MIN.) = 9.853
* 100 YEAR RAINFALL INTENSITY (INCH/HR) = 4.195

SUBAREA Tc AND LOSS RATE DATA (AMC III):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN	Tc (MIN.)
CONDOMINIUMS	B	0.46	0.30	0.350	76	9.85

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp (INCH/HR) = 0.30

SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.350

SUBAREA RUNOFF (CFS) = 1.69

TOTAL AREA (ACRES) = 0.46 PEAK FLOW RATE (CFS) = 1.69

FLOW PROCESS FROM NODE 57.00 TO NODE 57.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
=====

TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:

TIME OF CONCENTRATION (MIN.) = 9.85

RAINFALL INTENSITY (INCH/HR) = 4.19

AREA-AVERAGED Fm (INCH/HR) = 0.11

AREA-AVERAGED Fp (INCH/HR) = 0.30

AREA-AVERAGED Ap = 0.35

EFFECTIVE STREAM AREA (ACRES) = 0.46

TOTAL STREAM AREA (ACRES) = 0.46

PEAK FLOW RATE (CFS) AT CONFLUENCE = 1.69

FLOW PROCESS FROM NODE 59.00 TO NODE 57.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<
=====

INITIAL SUBAREA FLOW-LENGTH (FEET) = 265.00

ELEVATION DATA: UPSTREAM(FEET) = 166.80 DOWNSTREAM(FEET) = 165.80

Tc = K*[(LENGTH** 3.00)/(ELEVATION CHANGE)]**0.20

SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) = 10.239

* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 4.103

SUBAREA Tc AND LOSS RATE DATA(AMC III):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN	Tc (MIN.)
CONDOMINIUMS	B	0.57	0.30	0.350	76	10.24

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.30
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.350
SUBAREA RUNOFF(CFS) = 2.05
TOTAL AREA(ACRES) = 0.57 PEAK FLOW RATE(CFS) = 2.05

FLOW PROCESS FROM NODE 57.00 TO NODE 57.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<<
>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<<

=====

TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
TIME OF CONCENTRATION(MIN.) = 10.24
RAINFALL INTENSITY(INCH/HR) = 4.10
AREA-AVERAGED Fm(INCH/HR) = 0.11
AREA-AVERAGED Fp(INCH/HR) = 0.30
AREA-AVERAGED Ap = 0.35
EFFECTIVE STREAM AREA(ACRES) = 0.57
TOTAL STREAM AREA(ACRES) = 0.57
PEAK FLOW RATE(CFS) AT CONFLUENCE = 2.05

** CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	1.69	9.85	4.195	0.30(0.11)	0.35	0.5	58.00
2	2.05	10.24	4.103	0.30(0.11)	0.35	0.6	59.00

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	3.71	9.85	4.195	0.30(0.11)	0.35	1.0	58.00
2	3.71	10.24	4.103	0.30(0.11)	0.35	1.0	59.00

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE(CFS) = 3.71 Tc(MIN.) = 9.85
EFFECTIVE AREA(ACRES) = 1.01 AREA-AVERAGED Fm(INCH/HR) = 0.11
AREA-AVERAGED Fp(INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.35
TOTAL AREA(ACRES) = 1.0
LONGEST FLOWPATH FROM NODE 59.00 TO NODE 57.00 = 265.00 FEET.

FLOW PROCESS FROM NODE 57.00 TO NODE 57.00 IS CODE = 11

=====
 >>>>>CONFLUENCE MEMORY BANK # 1 WITH THE MAIN-STREAM MEMORY<<<<<<
 =====

** MAIN STREAM CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp (Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	3.71	9.85	4.195	0.30 (0.11)	0.35	1.0	58.00
2	3.71	10.24	4.103	0.30 (0.11)	0.35	1.0	59.00
LONGEST FLOWPATH FROM NODE				59.00 TO NODE	57.00 =	265.00 FEET.	

** MEMORY BANK # 1 CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp (Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	69.62	9.57	4.264	0.31 (0.07)	0.23	15.8	51.00
2	70.64	9.90	4.183	0.31 (0.07)	0.23	16.4	43.00
3	71.06	10.04	4.149	0.31 (0.07)	0.23	16.7	45.00
4	71.09	10.05	4.147	0.31 (0.07)	0.23	16.7	55.00
5	72.07	10.42	4.062	0.31 (0.07)	0.23	17.3	30.00
6	73.12	10.90	3.958	0.31 (0.07)	0.23	18.2	38.00
7	73.94	11.29	3.880	0.31 (0.07)	0.23	18.8	30.00
8	74.14	11.41	3.857	0.31 (0.07)	0.23	19.0	36.00
9	75.04	11.96	3.754	0.31 (0.07)	0.23	19.9	14.00
10	75.56	12.25	3.702	0.31 (0.07)	0.23	20.4	26.00
11	75.66	12.32	3.691	0.31 (0.07)	0.22	20.5	27.00
12	76.04	12.59	3.644	0.31 (0.07)	0.22	20.9	38.00
13	76.53	12.98	3.581	0.31 (0.07)	0.22	21.5	20.00
14	76.89	13.45	3.509	0.31 (0.07)	0.22	22.2	17.00
15	76.95	13.53	3.498	0.31 (0.07)	0.22	22.3	23.00
16	76.96	13.55	3.494	0.31 (0.07)	0.22	22.3	29.00
17	77.33	14.62	3.346	0.31 (0.07)	0.22	23.6	50.00
18	77.07	14.89	3.311	0.31 (0.07)	0.22	23.8	39.00
19	76.90	15.00	3.297	0.31 (0.07)	0.22	23.8	34.00
20	76.53	15.24	3.267	0.31 (0.07)	0.22	24.0	24.00
21	72.43	17.95	2.975	0.31 (0.06)	0.21	25.2	12.00
22	67.95	21.33	2.695	0.31 (0.06)	0.21	26.5	10.00
23	67.70	21.56	2.678	0.31 (0.06)	0.21	26.6	32.00
LONGEST FLOWPATH FROM NODE				10.00 TO NODE	57.00 =	2385.00 FEET.	

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp (Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	73.29	9.57	4.264	0.31 (0.07)	0.24	16.8	51.00
2	74.20	9.85	4.195	0.31 (0.07)	0.24	17.3	58.00
3	74.35	9.90	4.183	0.31 (0.07)	0.24	17.4	43.00
4	74.77	10.04	4.149	0.31 (0.07)	0.24	17.7	45.00
5	74.80	10.05	4.147	0.31 (0.07)	0.24	17.7	55.00
6	75.29	10.24	4.103	0.31 (0.07)	0.24	18.1	59.00
7	75.74	10.42	4.062	0.31 (0.07)	0.23	18.4	30.00
8	76.69	10.90	3.958	0.31 (0.07)	0.23	19.2	38.00
9	77.44	11.29	3.880	0.31 (0.07)	0.23	19.9	30.00
10	77.62	11.41	3.857	0.31 (0.07)	0.23	20.0	36.00
11	78.42	11.96	3.754	0.31 (0.07)	0.23	20.9	14.00

12	78.89	12.25	3.702	0.31	(0.07)	0.23	21.4	26.00
13	78.98	12.32	3.691	0.31	(0.07)	0.23	21.5	27.00
14	79.32	12.59	3.644	0.31	(0.07)	0.23	21.9	38.00
15	79.75	12.98	3.581	0.31	(0.07)	0.23	22.5	20.00
16	80.05	13.45	3.509	0.31	(0.07)	0.23	23.2	17.00
17	80.09	13.53	3.498	0.31	(0.07)	0.23	23.3	23.00
18	80.10	13.55	3.494	0.31	(0.07)	0.23	23.3	29.00
19	80.33	14.62	3.346	0.31	(0.07)	0.22	24.6	50.00
20	80.04	14.89	3.311	0.31	(0.07)	0.22	24.8	39.00
21	79.86	15.00	3.297	0.31	(0.07)	0.22	24.9	34.00
22	79.46	15.24	3.267	0.31	(0.07)	0.22	25.0	24.00
23	75.09	17.95	2.975	0.30	(0.07)	0.22	26.2	12.00
24	70.35	21.33	2.695	0.30	(0.06)	0.21	27.6	10.00
25	70.08	21.56	2.678	0.30	(0.06)	0.21	27.7	32.00
TOTAL AREA (ACRES) =			27.7					

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE (CFS) = 80.33 Tc (MIN.) = 14.616
EFFECTIVE AREA (ACRES) = 24.60 AREA-AVERAGED Fm (INCH/HR) = 0.07
AREA-AVERAGED Fp (INCH/HR) = 0.31 AREA-AVERAGED Ap = 0.22
TOTAL AREA (ACRES) = 27.7
LONGEST FLOWPATH FROM NODE 10.00 TO NODE 57.00 = 2385.00 FEET.

FLOW PROCESS FROM NODE 57.00 TO NODE 57.00 IS CODE = 12

>>>>CLEAR MEMORY BANK # 1 <<<<<

FLOW PROCESS FROM NODE 57.00 TO NODE 57.00 IS CODE = 10

>>>>MAIN-STREAM MEMORY COPIED ONTO MEMORY BANK # 1 <<<<<

FLOW PROCESS FROM NODE 60.00 TO NODE 61.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

INITIAL SUBAREA FLOW-LENGTH (FEET) = 200.00
ELEVATION DATA: UPSTREAM (FEET) = 167.00 DOWNSTREAM (FEET) = 166.20

Tc = K * [(LENGTH** 3.00) / (ELEVATION CHANGE)] ** 0.20
SUBAREA ANALYSIS USED MINIMUM Tc (MIN.) = 9.043
* 100 YEAR RAINFALL INTENSITY (INCH/HR) = 4.406
SUBAREA Tc AND LOSS RATE DATA (AMC III):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN	Tc (MIN.)
CONDOMINIUMS	B	0.27	0.30	0.350	76	9.04

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp (INCH/HR) = 0.30
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.350
SUBAREA RUNOFF (CFS) = 1.05

TOTAL AREA (ACRES) = 0.27 PEAK FLOW RATE (CFS) = 1.05

FLOW PROCESS FROM NODE 61.00 TO NODE 62.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<

=====

ELEVATION DATA: UPSTREAM (FEET) = 166.20 DOWNSTREAM (FEET) = 165.30
FLOW LENGTH (FEET) = 210.00 MANNING'S N = 0.013
ESTIMATED PIPE DIAMETER (INCH) INCREASED TO 12.000
DEPTH OF FLOW IN 12.0 INCH PIPE IS 5.7 INCHES
PIPE-FLOW VELOCITY (FEET/SEC.) = 2.84
ESTIMATED PIPE DIAMETER (INCH) = 12.000 NUMBER OF PIPES = 1
PIPE-FLOW (CFS) = 1.05
PIPE TRAVEL TIME (MIN.) = 1.23 Tc (MIN.) = 10.28
LONGEST FLOWPATH FROM NODE 60.00 TO NODE 62.00 = 410.00 FEET.

FLOW PROCESS FROM NODE 62.00 TO NODE 62.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

=====

TOTAL NUMBER OF STREAMS = 3
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION (MIN.) = 10.28
RAINFALL INTENSITY (INCH/HR) = 4.10
AREA-AVERAGED Fm (INCH/HR) = 0.10
AREA-AVERAGED Fp (INCH/HR) = 0.30
AREA-AVERAGED Ap = 0.35
EFFECTIVE STREAM AREA (ACRES) = 0.27
TOTAL STREAM AREA (ACRES) = 0.27
PEAK FLOW RATE (CFS) AT CONFLUENCE = 1.05

FLOW PROCESS FROM NODE 63.00 TO NODE 62.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

=====

INITIAL SUBAREA FLOW-LENGTH (FEET) = 270.00
ELEVATION DATA: UPSTREAM (FEET) = 166.50 DOWNSTREAM (FEET) = 165.30

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$

SUBAREA ANALYSIS USED MINIMUM Tc (MIN.) = 9.984

* 100 YEAR RAINFALL INTENSITY (INCH/HR) = 4.163

SUBAREA Tc AND LOSS RATE DATA (AMC III):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN	Tc (MIN.)
CONDOMINIUMS	B	0.70	0.30	0.350	76	9.98

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp (INCH/HR) = 0.30

SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.350

SUBAREA RUNOFF (CFS) = 2.56

TOTAL AREA (ACRES) = 0.70 PEAK FLOW RATE (CFS) = 2.56

FLOW PROCESS FROM NODE 62.00 TO NODE 62.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

=====

TOTAL NUMBER OF STREAMS = 3
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
TIME OF CONCENTRATION(MIN.) = 9.98
RAINFALL INTENSITY(INCH/HR) = 4.16
AREA-AVERAGED Fm(INCH/HR) = 0.11
AREA-AVERAGED Fp(INCH/HR) = 0.30
AREA-AVERAGED Ap = 0.35
EFFECTIVE STREAM AREA(ACRES) = 0.70
TOTAL STREAM AREA(ACRES) = 0.70
PEAK FLOW RATE(CFS) AT CONFLUENCE = 2.56

FLOW PROCESS FROM NODE 64.00 TO NODE 62.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

=====

INITIAL SUBAREA FLOW-LENGTH(FEET) = 295.00
ELEVATION DATA: UPSTREAM(FEET) = 166.50 DOWNSTREAM(FEET) = 165.30

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$
SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) = 20.647
* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 2.745

SUBAREA Tc AND LOSS RATE DATA(AMC III):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN	Tc (MIN.)
AGRICULTURAL FAIR COVER "PASTURE, IRRIGATED"	B	0.27	0.30	1.000	83	20.65

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.30
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 1.000
SUBAREA RUNOFF(CFS) = 0.59
TOTAL AREA(ACRES) = 0.27 PEAK FLOW RATE(CFS) = 0.59

FLOW PROCESS FROM NODE 62.00 TO NODE 62.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<

=====

TOTAL NUMBER OF STREAMS = 3
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 3 ARE:
TIME OF CONCENTRATION(MIN.) = 20.65
RAINFALL INTENSITY(INCH/HR) = 2.75
AREA-AVERAGED Fm(INCH/HR) = 0.30
AREA-AVERAGED Fp(INCH/HR) = 0.30
AREA-AVERAGED Ap = 1.00
EFFECTIVE STREAM AREA(ACRES) = 0.27
TOTAL STREAM AREA(ACRES) = 0.27

PEAK FLOW RATE (CFS) AT CONFLUENCE = 0.59

** CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp (Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	1.05	10.28	4.095	0.30 (0.10)	0.35	0.3	60.00
2	2.56	9.98	4.163	0.30 (0.11)	0.35	0.7	63.00
3	0.59	20.65	2.745	0.30 (0.30)	1.00	0.3	64.00

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
CONFLUENCE FORMULA USED FOR 3 STREAMS.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp (Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	4.04	9.98	4.163	0.30 (0.13)	0.43	1.1	63.00
2	4.02	10.28	4.095	0.30 (0.13)	0.43	1.1	60.00
3	2.95	20.65	2.745	0.30 (0.15)	0.49	1.2	64.00

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE (CFS) = 4.04 Tc (MIN.) = 9.98
EFFECTIVE AREA (ACRES) = 1.09 AREA-AVERAGED Fm (INCH/HR) = 0.13
AREA-AVERAGED Fp (INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.43
TOTAL AREA (ACRES) = 1.2
LONGEST FLOWPATH FROM NODE 60.00 TO NODE 62.00 = 410.00 FEET.

FLOW PROCESS FROM NODE 62.00 TO NODE 57.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<

=====

ELEVATION DATA: UPSTREAM (FEET) = 156.00 DOWNSTREAM (FEET) = 149.10
FLOW LENGTH (FEET) = 245.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 12.0 INCH PIPE IS 7.4 INCHES
PIPE-FLOW VELOCITY (FEET/SEC.) = 8.02
ESTIMATED PIPE DIAMETER (INCH) = 12.00 NUMBER OF PIPES = 1
PIPE-FLOW (CFS) = 4.04
PIPE TRAVEL TIME (MIN.) = 0.51 Tc (MIN.) = 10.49
LONGEST FLOWPATH FROM NODE 60.00 TO NODE 57.00 = 655.00 FEET.

FLOW PROCESS FROM NODE 57.00 TO NODE 57.00 IS CODE = 11

>>>>CONFLUENCE MEMORY BANK # 1 WITH THE MAIN-STREAM MEMORY<<<<<

** MAIN STREAM CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp (Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	4.04	10.49	4.046	0.30 (0.13)	0.43	1.1	63.00
2	4.02	10.79	3.983	0.30 (0.13)	0.43	1.1	60.00
3	2.95	21.20	2.704	0.30 (0.15)	0.49	1.2	64.00

LONGEST FLOWPATH FROM NODE 60.00 TO NODE 57.00 = 655.00 FEET.

** MEMORY BANK # 1 CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp (Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	73.29	9.57	4.264	0.31 (0.07)	0.24	16.8	51.00
2	74.20	9.85	4.195	0.31 (0.07)	0.24	17.3	58.00
3	74.35	9.90	4.183	0.31 (0.07)	0.24	17.4	43.00
4	74.77	10.04	4.149	0.31 (0.07)	0.24	17.7	45.00
5	74.80	10.05	4.147	0.31 (0.07)	0.24	17.7	55.00
6	75.29	10.24	4.103	0.31 (0.07)	0.24	18.1	59.00
7	75.74	10.42	4.062	0.31 (0.07)	0.23	18.4	30.00
8	76.69	10.90	3.958	0.31 (0.07)	0.23	19.2	38.00
9	77.44	11.29	3.880	0.31 (0.07)	0.23	19.9	30.00
10	77.62	11.41	3.857	0.31 (0.07)	0.23	20.0	36.00
11	78.42	11.96	3.754	0.31 (0.07)	0.23	20.9	14.00
12	78.89	12.25	3.702	0.31 (0.07)	0.23	21.4	26.00
13	78.98	12.32	3.691	0.31 (0.07)	0.23	21.5	27.00
14	79.32	12.59	3.644	0.31 (0.07)	0.23	21.9	38.00
15	79.75	12.98	3.581	0.31 (0.07)	0.23	22.5	20.00
16	80.05	13.45	3.509	0.31 (0.07)	0.23	23.2	17.00
17	80.09	13.53	3.498	0.31 (0.07)	0.23	23.3	23.00
18	80.10	13.55	3.494	0.31 (0.07)	0.23	23.3	29.00
19	80.33	14.62	3.346	0.31 (0.07)	0.22	24.6	50.00
20	80.04	14.89	3.311	0.31 (0.07)	0.22	24.8	39.00
21	79.86	15.00	3.297	0.31 (0.07)	0.22	24.9	34.00
22	79.46	15.24	3.267	0.31 (0.07)	0.22	25.0	24.00
23	75.09	17.95	2.975	0.30 (0.07)	0.22	26.2	12.00
24	70.35	21.33	2.695	0.30 (0.06)	0.21	27.6	10.00
25	70.08	21.56	2.678	0.30 (0.06)	0.21	27.7	32.00
LONGEST FLOWPATH FROM NODE			10.00	TO NODE	57.00 =	2385.00	FEET.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp (Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	77.18	9.57	4.264	0.30 (0.08)	0.25	17.8	51.00
2	78.14	9.85	4.195	0.30 (0.07)	0.25	18.4	58.00
3	78.30	9.90	4.183	0.30 (0.07)	0.25	18.5	43.00
4	78.75	10.04	4.149	0.30 (0.07)	0.25	18.7	45.00
5	78.77	10.05	4.147	0.30 (0.07)	0.25	18.8	55.00
6	79.30	10.24	4.103	0.30 (0.07)	0.25	19.1	59.00
7	79.77	10.42	4.062	0.30 (0.07)	0.25	19.5	30.00
8	79.92	10.49	4.046	0.30 (0.07)	0.25	19.6	63.00
9	80.48	10.79	3.983	0.30 (0.07)	0.24	20.1	60.00
10	80.70	10.90	3.958	0.30 (0.07)	0.24	20.3	38.00
11	81.40	11.29	3.880	0.30 (0.07)	0.24	21.0	30.00
12	81.57	11.41	3.857	0.30 (0.07)	0.24	21.2	36.00
13	82.32	11.96	3.754	0.30 (0.07)	0.24	22.0	14.00
14	82.76	12.25	3.702	0.30 (0.07)	0.24	22.5	26.00
15	82.84	12.32	3.691	0.30 (0.07)	0.24	22.6	27.00
16	83.16	12.59	3.644	0.30 (0.07)	0.24	23.1	38.00
17	83.54	12.98	3.581	0.31 (0.07)	0.24	23.7	20.00
18	83.79	13.45	3.509	0.30 (0.07)	0.24	24.3	17.00
19	83.83	13.53	3.498	0.30 (0.07)	0.24	24.4	23.00
20	83.84	13.55	3.494	0.30 (0.07)	0.24	24.5	29.00

21	83.96	14.62	3.346	0.30	(0.07)	0.23	25.8	50.00
22	83.64	14.89	3.311	0.30	(0.07)	0.23	26.0	39.00
23	83.45	15.00	3.297	0.30	(0.07)	0.23	26.0	34.00
24	83.02	15.24	3.267	0.30	(0.07)	0.23	26.1	24.00
25	78.37	17.95	2.975	0.30	(0.07)	0.23	27.4	12.00
26	73.49	21.20	2.704	0.30	(0.07)	0.22	28.8	64.00
27	73.29	21.33	2.695	0.30	(0.07)	0.22	28.8	10.00
28	73.00	21.56	2.678	0.30	(0.07)	0.22	28.9	32.00
TOTAL AREA (ACRES) =			28.9					

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE (CFS) = 83.96 Tc (MIN.) = 14.616
EFFECTIVE AREA (ACRES) = 25.75 AREA-AVERAGED Fm (INCH/HR) = 0.07
AREA-AVERAGED Fp (INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.23
TOTAL AREA (ACRES) = 28.9
LONGEST FLOWPATH FROM NODE 10.00 TO NODE 57.00 = 2385.00 FEET.

FLOW PROCESS FROM NODE 57.00 TO NODE 57.00 IS CODE = 12

>>>>CLEAR MEMORY BANK # 1 <<<<<
=====

FLOW PROCESS FROM NODE 57.00 TO NODE 57.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
=====

TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION (MIN.) = 14.62
RAINFALL INTENSITY (INCH/HR) = 3.35
AREA-AVERAGED Fm (INCH/HR) = 0.07
AREA-AVERAGED Fp (INCH/HR) = 0.30
AREA-AVERAGED Ap = 0.23
EFFECTIVE STREAM AREA (ACRES) = 25.75
TOTAL STREAM AREA (ACRES) = 28.90
PEAK FLOW RATE (CFS) AT CONFLUENCE = 83.96

FLOW PROCESS FROM NODE 65.00 TO NODE 66.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<
=====

INITIAL SUBAREA FLOW-LENGTH (FEET) = 135.00
ELEVATION DATA: UPSTREAM (FEET) = 166.70 DOWNSTREAM (FEET) = 166.30

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$
SUBAREA ANALYSIS USED MINIMUM Tc (MIN.) = 8.205
* 100 YEAR RAINFALL INTENSITY (INCH/HR) = 4.658
SUBAREA Tc AND LOSS RATE DATA (AMC III):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN	Tc (MIN.)
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CONDOMINIUMS B 0.19 0.30 0.350 76 8.21
SUBAREA AVERAGE PERVIOUS LOSS RATE, F_p (INCH/HR) = 0.30
SUBAREA AVERAGE PERVIOUS AREA FRACTION, A_p = 0.350
SUBAREA RUNOFF(CFS) = 0.78
TOTAL AREA(ACRES) = 0.19 PEAK FLOW RATE(CFS) = 0.78

FLOW PROCESS FROM NODE 66.00 TO NODE 57.00 IS CODE = 31

>>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 156.00 DOWNSTREAM(FEET) = 149.10
FLOW LENGTH(FEET) = 45.00 MANNING'S N = 0.013
ESTIMATED PIPE DIAMETER(INCH) INCREASED TO 12.000
DEPTH OF FLOW IN 12.0 INCH PIPE IS 1.9 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 9.40
ESTIMATED PIPE DIAMETER(INCH) = 12.000 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 0.78
PIPE TRAVEL TIME(MIN.) = 0.08 T_c (MIN.) = 8.29
LONGEST FLOWPATH FROM NODE 65.00 TO NODE 57.00 = 180.00 FEET.

FLOW PROCESS FROM NODE 57.00 TO NODE 57.00 IS CODE = 1

>>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
>>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<

=====

TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
TIME OF CONCENTRATION(MIN.) = 8.29
RAINFALL INTENSITY(INCH/HR) = 4.63
AREA-AVERAGED F_m (INCH/HR) = 0.11
AREA-AVERAGED F_p (INCH/HR) = 0.30
AREA-AVERAGED A_p = 0.35
EFFECTIVE STREAM AREA(ACRES) = 0.19
TOTAL STREAM AREA(ACRES) = 0.19
PEAK FLOW RATE(CFS) AT CONFLUENCE = 0.78

** CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	T_c (MIN.)	Intensity (INCH/HR)	$F_p(F_m)$ (INCH/HR)	A_p	A_e (ACRES)	HEADWATER NODE
1	77.18	9.57	4.264	0.30(0.08)	0.25	17.8	51.00
1	78.14	9.85	4.195	0.30(0.07)	0.25	18.4	58.00
1	78.30	9.90	4.183	0.30(0.07)	0.25	18.5	43.00
1	78.75	10.04	4.149	0.30(0.07)	0.25	18.7	45.00
1	78.77	10.05	4.147	0.30(0.07)	0.25	18.8	55.00
1	79.30	10.24	4.103	0.30(0.07)	0.25	19.1	59.00
1	79.77	10.42	4.062	0.30(0.07)	0.25	19.5	30.00
1	79.92	10.49	4.046	0.30(0.07)	0.25	19.6	63.00
1	80.48	10.79	3.983	0.30(0.07)	0.24	20.1	60.00
1	80.70	10.90	3.958	0.30(0.07)	0.24	20.3	38.00
1	81.40	11.29	3.880	0.30(0.07)	0.24	21.0	30.00
1	81.57	11.41	3.857	0.30(0.07)	0.24	21.2	36.00

1	82.32	11.96	3.754	0.30 (0.07)	0.24	22.0	14.00
1	82.76	12.25	3.702	0.30 (0.07)	0.24	22.5	26.00
1	82.84	12.32	3.691	0.30 (0.07)	0.24	22.6	27.00
1	83.16	12.59	3.644	0.30 (0.07)	0.24	23.1	38.00
1	83.54	12.98	3.581	0.31 (0.07)	0.24	23.7	20.00
1	83.79	13.45	3.509	0.30 (0.07)	0.24	24.3	17.00
1	83.83	13.53	3.498	0.30 (0.07)	0.24	24.4	23.00
1	83.84	13.55	3.494	0.30 (0.07)	0.24	24.5	29.00
1	83.96	14.62	3.346	0.30 (0.07)	0.23	25.8	50.00
1	83.64	14.89	3.311	0.30 (0.07)	0.23	26.0	39.00
1	83.45	15.00	3.297	0.30 (0.07)	0.23	26.0	34.00
1	83.02	15.24	3.267	0.30 (0.07)	0.23	26.1	24.00
1	78.37	17.95	2.975	0.30 (0.07)	0.23	27.4	12.00
1	73.49	21.20	2.704	0.30 (0.07)	0.22	28.8	64.00
1	73.29	21.33	2.695	0.30 (0.07)	0.22	28.8	10.00
1	73.00	21.56	2.678	0.30 (0.07)	0.22	28.9	32.00
2	0.78	8.29	4.633	0.30 (0.11)	0.35	0.2	65.00

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	73.44	8.29	4.633	0.30 (0.08)	0.25	15.6	65.00
2	77.90	9.57	4.264	0.30 (0.08)	0.25	18.0	51.00
3	78.85	9.85	4.195	0.30 (0.08)	0.25	18.6	58.00
4	79.00	9.90	4.183	0.30 (0.08)	0.25	18.7	43.00
5	79.44	10.04	4.149	0.30 (0.08)	0.25	18.9	45.00
6	79.47	10.05	4.147	0.30 (0.08)	0.25	19.0	55.00
7	79.98	10.24	4.103	0.30 (0.08)	0.25	19.3	59.00
8	80.45	10.42	4.062	0.30 (0.08)	0.25	19.7	30.00
9	80.60	10.49	4.046	0.30 (0.08)	0.25	19.8	63.00
10	81.14	10.79	3.983	0.30 (0.07)	0.25	20.3	60.00
11	81.36	10.90	3.958	0.30 (0.07)	0.25	20.5	38.00
12	82.05	11.29	3.880	0.30 (0.07)	0.24	21.2	30.00
13	82.22	11.41	3.857	0.30 (0.07)	0.24	21.3	36.00
14	82.95	11.96	3.754	0.30 (0.07)	0.24	22.2	14.00
15	83.38	12.25	3.702	0.30 (0.07)	0.24	22.7	26.00
16	83.46	12.32	3.691	0.30 (0.07)	0.24	22.8	27.00
17	83.77	12.59	3.644	0.30 (0.07)	0.24	23.2	38.00
18	84.14	12.98	3.581	0.30 (0.07)	0.24	23.9	20.00
19	84.38	13.45	3.509	0.30 (0.07)	0.24	24.5	17.00
20	84.41	13.53	3.498	0.30 (0.07)	0.24	24.6	23.00
21	84.42	13.55	3.494	0.30 (0.07)	0.24	24.6	29.00
22	84.51	14.62	3.346	0.30 (0.07)	0.24	25.9	50.00
23	84.19	14.89	3.311	0.30 (0.07)	0.23	26.1	39.00
24	83.99	15.00	3.297	0.30 (0.07)	0.23	26.2	34.00
25	83.56	15.24	3.267	0.30 (0.07)	0.23	26.3	24.00
26	78.86	17.95	2.975	0.30 (0.07)	0.23	27.6	12.00
27	73.94	21.20	2.704	0.30 (0.07)	0.22	29.0	64.00
28	73.74	21.33	2.695	0.30 (0.07)	0.22	29.0	10.00
29	73.44	21.56	2.678	0.30 (0.07)	0.22	29.1	32.00

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE (CFS) = 84.51 Tc (MIN.) = 14.62
EFFECTIVE AREA (ACRES) = 25.94 AREA-AVERAGED Fm (INCH/HR) = 0.07
AREA-AVERAGED Fp (INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.24
TOTAL AREA (ACRES) = 29.1
LONGEST FLOWPATH FROM NODE 10.00 TO NODE 57.00 = 2385.00 FEET.

FLOW PROCESS FROM NODE 57.00 TO NODE 67.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<

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ELEVATION DATA: UPSTREAM (FEET) = 149.10 DOWNSTREAM (FEET) = 148.50
FLOW LENGTH (FEET) = 275.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 54.0 INCH PIPE IS 41.8 INCHES
PIPE-FLOW VELOCITY (FEET/SEC.) = 6.40
ESTIMATED PIPE DIAMETER (INCH) = 54.00 NUMBER OF PIPES = 1
PIPE-FLOW (CFS) = 84.51
PIPE TRAVEL TIME (MIN.) = 0.72 Tc (MIN.) = 15.33
LONGEST FLOWPATH FROM NODE 10.00 TO NODE 67.00 = 2660.00 FEET.

FLOW PROCESS FROM NODE 67.00 TO NODE 67.00 IS CODE = 10

>>>>MAIN-STREAM MEMORY COPIED ONTO MEMORY BANK # 1 <<<<<

FLOW PROCESS FROM NODE 68.00 TO NODE 69.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

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INITIAL SUBAREA FLOW-LENGTH (FEET) = 365.00
ELEVATION DATA: UPSTREAM (FEET) = 168.20 DOWNSTREAM (FEET) = 167.30

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$
SUBAREA ANALYSIS USED MINIMUM Tc (MIN.) = 11.404
* 100 YEAR RAINFALL INTENSITY (INCH/HR) = 3.858

SUBAREA Tc AND LOSS RATE DATA (AMC III):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN	Tc (MIN.)
APARTMENTS	B	1.00	0.30	0.200	76	11.40

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp (INCH/HR) = 0.30

SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.200

SUBAREA RUNOFF (CFS) = 3.42

TOTAL AREA (ACRES) = 1.00 PEAK FLOW RATE (CFS) = 3.42

FLOW PROCESS FROM NODE 69.00 TO NODE 70.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<


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ELEVATION DATA: UPSTREAM(FEET) = 162.00 DOWNSTREAM(FEET) = 159.00
FLOW LENGTH(FEET) = 185.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 12.0 INCH PIPE IS 7.9 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 6.21
ESTIMATED PIPE DIAMETER(INCH) = 12.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 3.42
PIPE TRAVEL TIME(MIN.) = 0.50 Tc(MIN.) = 11.90
LONGEST FLOWPATH FROM NODE 68.00 TO NODE 70.00 = 550.00 FEET.

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FLOW PROCESS FROM NODE 70.00 TO NODE 71.00 IS CODE = 1

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>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
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TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION(MIN.) = 11.90
RAINFALL INTENSITY(INCH/HR) = 3.76
AREA-AVERAGED Fm(INCH/HR) = 0.06
AREA-AVERAGED Fp(INCH/HR) = 0.30
AREA-AVERAGED Ap = 0.20
EFFECTIVE STREAM AREA(ACRES) = 1.00
TOTAL STREAM AREA(ACRES) = 1.00
PEAK FLOW RATE(CFS) AT CONFLUENCE = 3.42

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FLOW PROCESS FROM NODE 71.00 TO NODE 70.00 IS CODE = 21

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>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<
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INITIAL SUBAREA FLOW-LENGTH(FEET) = 250.00
ELEVATION DATA: UPSTREAM(FEET) = 167.20 DOWNSTREAM(FEET) = 166.80

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Tc = K*[(LENGTH** 3.00)/(ELEVATION CHANGE)]**0.20
SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) = 10.688
* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 4.004
SUBAREA Tc AND LOSS RATE DATA(AMC III):
DEVELOPMENT TYPE/          SCS SOIL  AREA      Fp          Ap          SCS    Tc
LAND USE                   GROUP   (ACRES) (INCH/HR) (DECIMAL) CN (MIN.)
APARTMENTS                  B       0.66     0.30     0.200     76    10.69
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.30
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.200
SUBAREA RUNOFF(CFS) = 2.34
TOTAL AREA(ACRES) = 0.66 PEAK FLOW RATE(CFS) = 2.34

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FLOW PROCESS FROM NODE 70.00 TO NODE 70.00 IS CODE = 1

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>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<
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TOTAL NUMBER OF STREAMS = 2

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CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:

TIME OF CONCENTRATION(MIN.) = 10.69
RAINFALL INTENSITY(INCH/HR) = 4.00
AREA-AVERAGED Fm(INCH/HR) = 0.06
AREA-AVERAGED Fp(INCH/HR) = 0.30
AREA-AVERAGED Ap = 0.20
EFFECTIVE STREAM AREA(ACRES) = 0.66
TOTAL STREAM AREA(ACRES) = 0.66
PEAK FLOW RATE(CFS) AT CONFLUENCE = 2.34

** CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	3.42	11.90	3.764	0.30(0.06)	0.20	1.0	68.00
2	2.34	10.69	4.004	0.30(0.06)	0.20	0.7	71.00

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	5.61	10.69	4.004	0.30(0.06)	0.20	1.6	71.00
2	5.62	11.90	3.764	0.30(0.06)	0.20	1.7	68.00

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE(CFS) = 5.62 Tc(MIN.) = 11.90
EFFECTIVE AREA(ACRES) = 1.66 AREA-AVERAGED Fm(INCH/HR) = 0.06
AREA-AVERAGED Fp(INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.20
TOTAL AREA(ACRES) = 1.7
LONGEST FLOWPATH FROM NODE 68.00 TO NODE 70.00 = 550.00 FEET.

FLOW PROCESS FROM NODE 33.00 TO NODE 33.00 IS CODE = 12

>>>>CLEAR MEMORY BANK # 3 <<<<<

FLOW PROCESS FROM NODE 70.00 TO NODE 70.00 IS CODE = 10

>>>>MAIN-STREAM MEMORY COPIED ONTO MEMORY BANK # 3 <<<<<

FLOW PROCESS FROM NODE 72.00 TO NODE 73.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<

>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

INITIAL SUBAREA FLOW-LENGTH(FEET) = 230.00

ELEVATION DATA: UPSTREAM(FEET) = 168.00 DOWNSTREAM(FEET) = 166.80

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$

SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) = 8.161

* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 4.673

SUBAREA Tc AND LOSS RATE DATA(AMC III):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN	Tc (MIN.)
APARTMENTS	B	0.28	0.30	0.200	76	8.16

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.30

SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.200

SUBAREA RUNOFF(CFS) = 1.16

TOTAL AREA(ACRES) = 0.28 PEAK FLOW RATE(CFS) = 1.16

FLOW PROCESS FROM NODE 73.00 TO NODE 70.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<

>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<

ELEVATION DATA: UPSTREAM(FEET) = 160.00 DOWNSTREAM(FEET) = 159.00

FLOW LENGTH(FEET) = 270.00 MANNING'S N = 0.013

DEPTH OF FLOW IN 12.0 INCH PIPE IS 6.3 INCHES

PIPE-FLOW VELOCITY(FEET/SEC.) = 2.76

ESTIMATED PIPE DIAMETER(INCH) = 12.00 NUMBER OF PIPES = 1

PIPE-FLOW(CFS) = 1.16

PIPE TRAVEL TIME(MIN.) = 1.63 Tc(MIN.) = 9.79

LONGEST FLOWPATH FROM NODE 72.00 TO NODE 70.00 = 500.00 FEET.

FLOW PROCESS FROM NODE 70.00 TO NODE 70.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

TOTAL NUMBER OF STREAMS = 2

CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:

TIME OF CONCENTRATION(MIN.) = 9.79

RAINFALL INTENSITY(INCH/HR) = 4.21

AREA-AVERAGED Fm(INCH/HR) = 0.06

AREA-AVERAGED Fp(INCH/HR) = 0.30

AREA-AVERAGED Ap = 0.20

EFFECTIVE STREAM AREA(ACRES) = 0.28

TOTAL STREAM AREA(ACRES) = 0.28

PEAK FLOW RATE(CFS) AT CONFLUENCE = 1.16

FLOW PROCESS FROM NODE 72.00 TO NODE 70.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<

>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

INITIAL SUBAREA FLOW-LENGTH(FEET) = 390.00

ELEVATION DATA: UPSTREAM(FEET) = 167.20 DOWNSTREAM(FEET) = 166.80

Tc = K*[(LENGTH** 3.00)/(ELEVATION CHANGE)]**0.20

SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) = 13.956

* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 3.436

SUBAREA Tc AND LOSS RATE DATA (AMC III):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN	Tc (MIN.)
APARTMENTS	B	1.49	0.30	0.200	76	13.96

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp (INCH/HR) = 0.30
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.200
 SUBAREA RUNOFF (CFS) = 4.53
 TOTAL AREA (ACRES) = 1.49 PEAK FLOW RATE (CFS) = 4.53

 FLOW PROCESS FROM NODE 70.00 TO NODE 70.00 IS CODE = 1

 >>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
 >>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<

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TOTAL NUMBER OF STREAMS = 2
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
 TIME OF CONCENTRATION (MIN.) = 13.96
 RAINFALL INTENSITY (INCH/HR) = 3.44
 AREA-AVERAGED Fm (INCH/HR) = 0.06
 AREA-AVERAGED Fp (INCH/HR) = 0.30
 AREA-AVERAGED Ap = 0.20
 EFFECTIVE STREAM AREA (ACRES) = 1.49
 TOTAL STREAM AREA (ACRES) = 1.49
 PEAK FLOW RATE (CFS) AT CONFLUENCE = 4.53

** CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	1.16	9.79	4.210	0.30 (0.06)	0.20	0.3	72.00
2	4.53	13.96	3.436	0.30 (0.06)	0.20	1.5	72.00

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
 CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	5.07	9.79	4.210	0.30 (0.06)	0.20	1.3	72.00
2	5.47	13.96	3.436	0.30 (0.06)	0.20	1.8	72.00

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE (CFS) = 5.47 Tc (MIN.) = 13.96
 EFFECTIVE AREA (ACRES) = 1.77 AREA-AVERAGED Fm (INCH/HR) = 0.06
 AREA-AVERAGED Fp (INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.20
 TOTAL AREA (ACRES) = 1.8
 LONGEST FLOWPATH FROM NODE 72.00 TO NODE 70.00 = 500.00 FEET.

 FLOW PROCESS FROM NODE 70.00 TO NODE 70.00 IS CODE = 11

 >>>>CONFLUENCE MEMORY BANK # 3 WITH THE MAIN-STREAM MEMORY<<<<<

** MAIN STREAM CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	5.07	9.79	4.210	0.30 (0.06)	0.20	1.3	72.00
2	5.47	13.96	3.436	0.30 (0.06)	0.20	1.8	72.00
LONGEST FLOWPATH FROM NODE			72.00	TO NODE	70.00 =	500.00	FEET.

** MEMORY BANK # 3 CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	5.61	10.69	4.004	0.30 (0.06)	0.20	1.6	71.00
2	5.62	11.90	3.764	0.30 (0.06)	0.20	1.7	68.00
LONGEST FLOWPATH FROM NODE			68.00	TO NODE	70.00 =	550.00	FEET.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	10.47	9.79	4.210	0.30 (0.06)	0.20	2.8	72.00
2	10.76	10.69	4.004	0.30 (0.06)	0.20	3.0	71.00
3	10.89	11.90	3.764	0.30 (0.06)	0.20	3.2	68.00
4	10.59	13.96	3.436	0.30 (0.06)	0.20	3.4	72.00
TOTAL AREA (ACRES) =			3.4				

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE (CFS) = 10.89 Tc (MIN.) = 11.901
EFFECTIVE AREA (ACRES) = 3.21 AREA-AVERAGED Fm (INCH/HR) = 0.06
AREA-AVERAGED Fp (INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.20
TOTAL AREA (ACRES) = 3.4
LONGEST FLOWPATH FROM NODE 68.00 TO NODE 70.00 = 550.00 FEET.

FLOW PROCESS FROM NODE 70.00 TO NODE 70.00 IS CODE = 12

>>>>CLEAR MEMORY BANK # 3 <<<<<

FLOW PROCESS FROM NODE 70.00 TO NODE 67.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<

>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<

ELEVATION DATA: UPSTREAM (FEET) = 155.00 DOWNSTREAM (FEET) = 148.50
FLOW LENGTH (FEET) = 30.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 12.0 INCH PIPE IS 7.2 INCHES
PIPE-FLOW VELOCITY (FEET/SEC.) = 22.08
ESTIMATED PIPE DIAMETER (INCH) = 12.00 NUMBER OF PIPES = 1
PIPE-FLOW (CFS) = 10.89
PIPE TRAVEL TIME (MIN.) = 0.02 Tc (MIN.) = 11.92
LONGEST FLOWPATH FROM NODE 68.00 TO NODE 67.00 = 580.00 FEET.

FLOW PROCESS FROM NODE 67.00 TO NODE 67.00 IS CODE = 11

>>>>>CONFLUENCE MEMORY BANK # 1 WITH THE MAIN-STREAM MEMORY<<<<<

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 ** MAIN STREAM CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	10.47	9.81	4.204	0.30 (0.06)	0.20	2.8	72.00
2	10.76	10.71	3.999	0.30 (0.06)	0.20	3.0	71.00
3	10.89	11.92	3.760	0.30 (0.06)	0.20	3.2	68.00
4	10.59	13.98	3.433	0.30 (0.06)	0.20	3.4	72.00
LONGEST FLOWPATH FROM NODE			68.00	TO NODE	67.00 =	580.00	FEET.

** MEMORY BANK # 1 CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	73.44	9.03	4.410	0.30 (0.08)	0.25	15.6	65.00
2	77.90	10.30	4.090	0.30 (0.08)	0.25	18.0	51.00
3	78.85	10.57	4.028	0.30 (0.08)	0.25	18.6	58.00
4	79.00	10.62	4.018	0.30 (0.08)	0.25	18.7	43.00
5	79.44	10.76	3.988	0.30 (0.08)	0.25	18.9	45.00
6	79.47	10.77	3.986	0.30 (0.08)	0.25	19.0	55.00
7	79.98	10.96	3.947	0.30 (0.08)	0.25	19.3	59.00
8	80.45	11.14	3.909	0.30 (0.08)	0.25	19.7	30.00
9	80.60	11.21	3.895	0.30 (0.08)	0.25	19.8	63.00
10	81.14	11.50	3.838	0.30 (0.07)	0.25	20.3	60.00
11	81.36	11.62	3.816	0.30 (0.07)	0.25	20.5	38.00
12	82.05	12.01	3.745	0.30 (0.07)	0.24	21.2	30.00
13	82.22	12.13	3.724	0.30 (0.07)	0.24	21.3	36.00
14	82.95	12.68	3.631	0.30 (0.07)	0.24	22.2	14.00
15	83.38	12.97	3.583	0.30 (0.07)	0.24	22.7	26.00
16	83.46	13.04	3.573	0.30 (0.07)	0.24	22.8	27.00
17	83.77	13.31	3.530	0.30 (0.07)	0.24	23.2	38.00
18	84.14	13.70	3.473	0.30 (0.07)	0.24	23.9	20.00
19	84.38	14.17	3.406	0.30 (0.07)	0.24	24.5	17.00
20	84.41	14.24	3.396	0.30 (0.07)	0.24	24.6	23.00
21	84.42	14.27	3.392	0.30 (0.07)	0.24	24.6	29.00
22	84.51	15.33	3.256	0.30 (0.07)	0.24	25.9	50.00
23	84.19	15.60	3.223	0.30 (0.07)	0.23	26.1	39.00
24	83.99	15.72	3.210	0.30 (0.07)	0.23	26.2	34.00
25	83.56	15.96	3.182	0.30 (0.07)	0.23	26.3	24.00
26	78.86	18.67	2.908	0.30 (0.07)	0.23	27.6	12.00
27	73.94	21.94	2.651	0.30 (0.07)	0.22	29.0	64.00
28	73.74	22.07	2.642	0.30 (0.07)	0.22	29.0	10.00
29	73.44	22.30	2.627	0.30 (0.07)	0.22	29.1	32.00
LONGEST FLOWPATH FROM NODE			10.00	TO NODE	67.00 =	2660.00	FEET.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	83.56	9.03	4.410	0.30 (0.07)	0.24	18.1	65.00
2	86.68	9.81	4.204	0.30 (0.07)	0.24	19.8	72.00
3	88.53	10.30	4.090	0.30 (0.07)	0.24	20.9	51.00
4	89.57	10.57	4.028	0.30 (0.07)	0.24	21.5	58.00
5	89.74	10.62	4.018	0.30 (0.07)	0.24	21.6	43.00

6	90.04	10.71	3.999	0.30	(0.07)	0.24	21.8	71.00
7	90.21	10.76	3.988	0.30	(0.07)	0.24	21.9	45.00
8	90.24	10.77	3.986	0.30	(0.07)	0.24	21.9	55.00
9	90.77	10.96	3.947	0.30	(0.07)	0.24	22.3	59.00
10	91.26	11.14	3.909	0.30	(0.07)	0.24	22.7	30.00
11	91.42	11.21	3.895	0.30	(0.07)	0.24	22.9	63.00
12	91.99	11.50	3.838	0.30	(0.07)	0.24	23.4	60.00
13	92.22	11.62	3.816	0.30	(0.07)	0.24	23.6	38.00
14	92.79	11.92	3.760	0.30	(0.07)	0.24	24.2	68.00
15	92.93	12.01	3.745	0.30	(0.07)	0.24	24.4	30.00
16	93.08	12.13	3.724	0.30	(0.07)	0.24	24.6	36.00
17	93.73	12.68	3.631	0.30	(0.07)	0.24	25.5	14.00
18	94.11	12.97	3.583	0.30	(0.07)	0.24	26.0	26.00
19	94.19	13.04	3.573	0.30	(0.07)	0.24	26.1	27.00
20	94.45	13.31	3.530	0.30	(0.07)	0.24	26.6	38.00
21	94.77	13.70	3.473	0.30	(0.07)	0.24	27.3	20.00
22	94.88	13.98	3.433	0.30	(0.07)	0.23	27.7	72.00
23	94.89	14.17	3.406	0.30	(0.07)	0.23	27.9	17.00
24	94.89	14.24	3.396	0.30	(0.07)	0.23	28.0	23.00
25	94.89	14.27	3.392	0.30	(0.07)	0.23	28.1	29.00
26	94.55	15.33	3.256	0.30	(0.07)	0.23	29.4	50.00
27	94.13	15.60	3.223	0.30	(0.07)	0.23	29.6	39.00
28	93.89	15.72	3.210	0.30	(0.07)	0.23	29.6	34.00
29	93.37	15.96	3.182	0.30	(0.07)	0.23	29.8	24.00
30	87.81	18.67	2.908	0.30	(0.07)	0.23	31.1	12.00
31	82.07	21.94	2.651	0.30	(0.07)	0.22	32.4	64.00
32	81.85	22.07	2.642	0.30	(0.07)	0.22	32.4	10.00
33	81.51	22.30	2.627	0.30	(0.07)	0.22	32.5	32.00
TOTAL AREA (ACRES) =			32.5					

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE (CFS) = 94.89 Tc (MIN.) = 14.242
EFFECTIVE AREA (ACRES) = 28.04 AREA-AVERAGED Fm (INCH/HR) = 0.07
AREA-AVERAGED Fp (INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.23
TOTAL AREA (ACRES) = 32.5
LONGEST FLOWPATH FROM NODE 10.00 TO NODE 67.00 = 2660.00 FEET.

FLOW PROCESS FROM NODE 67.00 TO NODE 74.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<

=====

ELEVATION DATA: UPSTREAM (FEET) = 148.50 DOWNSTREAM (FEET) = 148.00
FLOW LENGTH (FEET) = 280.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 60.0 INCH PIPE IS 43.9 INCHES
PIPE-FLOW VELOCITY (FEET/SEC.) = 6.17
ESTIMATED PIPE DIAMETER (INCH) = 60.00 NUMBER OF PIPES = 1
PIPE-FLOW (CFS) = 94.89
PIPE TRAVEL TIME (MIN.) = 0.76 Tc (MIN.) = 15.00
LONGEST FLOWPATH FROM NODE 10.00 TO NODE 74.00 = 2940.00 FEET.

FLOW PROCESS FROM NODE 74.00 TO NODE 74.00 IS CODE = 10

=====
>>>>MAIN-STREAM MEMORY COPIED ONTO MEMORY BANK # 3 <<<<<
=====

FLOW PROCESS FROM NODE 75.00 TO NODE 76.00 IS CODE = 21
=====

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<
=====

INITIAL SUBAREA FLOW-LENGTH(FEET) = 275.00
ELEVATION DATA: UPSTREAM(FEET) = 167.00 DOWNSTREAM(FEET) = 165.30

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$

SUBAREA ANALYSIS USED MINIMUM T_c (MIN.) = 9.415

* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 4.305

SUBAREA T_c AND LOSS RATE DATA(AMC III):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	F_p (INCH/HR)	A_p (DECIMAL)	SCS CN	T_c (MIN.)
CONDOMINIUMS	B	0.70	0.30	0.350	76	9.41

SUBAREA AVERAGE PERVIOUS LOSS RATE, F_p (INCH/HR) = 0.30

SUBAREA AVERAGE PERVIOUS AREA FRACTION, A_p = 0.350

SUBAREA RUNOFF(CFS) = 2.65

TOTAL AREA(ACRES) = 0.70 PEAK FLOW RATE(CFS) = 2.65

FLOW PROCESS FROM NODE 76.00 TO NODE 76.00 IS CODE = 1
=====

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
=====

TOTAL NUMBER OF STREAMS = 2

CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:

TIME OF CONCENTRATION(MIN.) = 9.41

RAINFALL INTENSITY(INCH/HR) = 4.31

AREA-AVERAGED F_m (INCH/HR) = 0.11

AREA-AVERAGED F_p (INCH/HR) = 0.30

AREA-AVERAGED A_p = 0.35

EFFECTIVE STREAM AREA(ACRES) = 0.70

TOTAL STREAM AREA(ACRES) = 0.70

PEAK FLOW RATE(CFS) AT CONFLUENCE = 2.65

FLOW PROCESS FROM NODE 77.00 TO NODE 76.00 IS CODE = 21
=====

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<
=====

INITIAL SUBAREA FLOW-LENGTH(FEET) = 255.00
ELEVATION DATA: UPSTREAM(FEET) = 167.00 DOWNSTREAM(FEET) = 166.30

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$

SUBAREA ANALYSIS USED MINIMUM T_c (MIN.) = 10.745

* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 3.991

SUBAREA T_c AND LOSS RATE DATA(AMC III):

DEVELOPMENT TYPE/ LAND USE SCS SOIL GROUP AREA (ACRES) Fp (INCH/HR) Ap (DECIMAL) SCS CN Tc (MIN.)

CONDOMINIUMS B 0.51 0.30 0.350 76 10.75

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp (INCH/HR) = 0.30

SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.350

SUBAREA RUNOFF (CFS) = 1.78

TOTAL AREA (ACRES) = 0.51 PEAK FLOW RATE (CFS) = 1.78

FLOW PROCESS FROM NODE 76.00 TO NODE 76.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<

=====

TOTAL NUMBER OF STREAMS = 2

CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:

TIME OF CONCENTRATION (MIN.) = 10.75

RAINFALL INTENSITY (INCH/HR) = 3.99

AREA-AVERAGED Fm (INCH/HR) = 0.11

AREA-AVERAGED Fp (INCH/HR) = 0.30

AREA-AVERAGED Ap = 0.35

EFFECTIVE STREAM AREA (ACRES) = 0.51

TOTAL STREAM AREA (ACRES) = 0.51

PEAK FLOW RATE (CFS) AT CONFLUENCE = 1.78

** CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp (Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	2.65	9.41	4.305	0.30 (0.11)	0.35	0.7	75.00
2	1.78	10.75	3.991	0.30 (0.11)	0.35	0.5	77.00

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO

CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp (Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	4.34	9.41	4.305	0.30 (0.11)	0.35	1.1	75.00
2	4.23	10.75	3.991	0.30 (0.10)	0.35	1.2	77.00

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE (CFS) = 4.34 Tc (MIN.) = 9.41

EFFECTIVE AREA (ACRES) = 1.15 AREA-AVERAGED Fm (INCH/HR) = 0.10

AREA-AVERAGED Fp (INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.35

TOTAL AREA (ACRES) = 1.2

LONGEST FLOWPATH FROM NODE 75.00 TO NODE 76.00 = 275.00 FEET.

FLOW PROCESS FROM NODE 76.00 TO NODE 79.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<

>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<

=====

ELEVATION DATA: UPSTREAM (FEET) = 154.00 DOWNSTREAM (FEET) = 152.00

FLOW LENGTH(FEET) = 300.00 MANNING'S N = 0.013
 DEPTH OF FLOW IN 15.0 INCH PIPE IS 10.6 INCHES
 PIPE-FLOW VELOCITY(FEET/SEC.) = 4.70
 ESTIMATED PIPE DIAMETER(INCH) = 15.00 NUMBER OF PIPES = 1
 PIPE-FLOW(CFS) = 4.34
 PIPE TRAVEL TIME(MIN.) = 1.06 Tc(MIN.) = 10.48
 LONGEST FLOWPATH FROM NODE 75.00 TO NODE 79.00 = 575.00 FEET.

FLOW PROCESS FROM NODE 79.00 TO NODE 79.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

TOTAL NUMBER OF STREAMS = 2
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
 TIME OF CONCENTRATION(MIN.) = 10.48
 RAINFALL INTENSITY(INCH/HR) = 4.05
 AREA-AVERAGED Fm(INCH/HR) = 0.10
 AREA-AVERAGED Fp(INCH/HR) = 0.30
 AREA-AVERAGED Ap = 0.35
 EFFECTIVE STREAM AREA(ACRES) = 1.15
 TOTAL STREAM AREA(ACRES) = 1.21
 PEAK FLOW RATE(CFS) AT CONFLUENCE = 4.34

FLOW PROCESS FROM NODE 78.00 TO NODE 79.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
 >>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

INITIAL SUBAREA FLOW-LENGTH(FEET) = 200.00
 ELEVATION DATA: UPSTREAM(FEET) = 165.90 DOWNSTREAM(FEET) = 165.30

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$
 SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) = 9.578
 * 100 YEAR RAINFALL INTENSITY(INCH/HR) = 4.263
 SUBAREA Tc AND LOSS RATE DATA(AMC III):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN	Tc (MIN.)
CONDOMINIUMS	B	0.40	0.30	0.350	76	9.58

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.30
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.350
 SUBAREA RUNOFF(CFS) = 1.50
 TOTAL AREA(ACRES) = 0.40 PEAK FLOW RATE(CFS) = 1.50

FLOW PROCESS FROM NODE 79.00 TO NODE 79.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
 >>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<

TOTAL NUMBER OF STREAMS = 2
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
 TIME OF CONCENTRATION(MIN.) = 9.58

RAINFALL INTENSITY (INCH/HR) = 4.26
 AREA-AVERAGED Fm (INCH/HR) = 0.11
 AREA-AVERAGED Fp (INCH/HR) = 0.30
 AREA-AVERAGED Ap = 0.35
 EFFECTIVE STREAM AREA (ACRES) = 0.40
 TOTAL STREAM AREA (ACRES) = 0.40
 PEAK FLOW RATE (CFS) AT CONFLUENCE = 1.50

** CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp (Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	4.34	10.48	4.049	0.30 (0.11)	0.35	1.1	75.00
1	4.23	11.81	3.780	0.30 (0.10)	0.35	1.2	77.00
2	1.50	9.58	4.263	0.30 (0.11)	0.35	0.4	78.00

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
 CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp (Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	5.67	9.58	4.263	0.30 (0.10)	0.35	1.4	78.00
2	5.76	10.48	4.049	0.30 (0.11)	0.35	1.5	75.00
3	5.56	11.81	3.780	0.30 (0.10)	0.35	1.6	77.00

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE (CFS) = 5.76 Tc (MIN.) = 10.48
 EFFECTIVE AREA (ACRES) = 1.55 AREA-AVERAGED Fm (INCH/HR) = 0.11
 AREA-AVERAGED Fp (INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.35
 TOTAL AREA (ACRES) = 1.6
 LONGEST FLOWPATH FROM NODE 75.00 TO NODE 79.00 = 575.00 FEET.

FLOW PROCESS FROM NODE 79.00 TO NODE 74.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
 >>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<

=====
 ELEVATION DATA: UPSTREAM (FEET) = 152.00 DOWNSTREAM (FEET) = 148.00
 FLOW LENGTH (FEET) = 50.00 MANNING'S N = 0.013
 DEPTH OF FLOW IN 12.0 INCH PIPE IS 6.6 INCHES
 PIPE-FLOW VELOCITY (FEET/SEC.) = 12.99
 ESTIMATED PIPE DIAMETER (INCH) = 12.00 NUMBER OF PIPES = 1
 PIPE-FLOW (CFS) = 5.76
 PIPE TRAVEL TIME (MIN.) = 0.06 Tc (MIN.) = 10.54
 LONGEST FLOWPATH FROM NODE 75.00 TO NODE 74.00 = 625.00 FEET.

FLOW PROCESS FROM NODE 74.00 TO NODE 74.00 IS CODE = 11

>>>>CONFLUENCE MEMORY BANK # 3 WITH THE MAIN-STREAM MEMORY<<<<<

** MAIN STREAM CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	5.67	9.64	4.247	0.30 (0.10)	0.35	1.4	78.00
2	5.76	10.54	4.035	0.30 (0.11)	0.35	1.5	75.00
3	5.56	11.88	3.768	0.30 (0.10)	0.35	1.6	77.00
LONGEST FLOWPATH FROM NODE				75.00	TO NODE	74.00 =	625.00 FEET.

** MEMORY BANK # 3 CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	83.56	9.81	4.205	0.30 (0.07)	0.24	18.1	65.00
2	86.68	10.59	4.024	0.30 (0.07)	0.24	19.8	72.00
3	88.53	11.07	3.923	0.30 (0.07)	0.24	20.9	51.00
4	89.57	11.35	3.868	0.30 (0.07)	0.24	21.5	58.00
5	89.74	11.40	3.859	0.30 (0.07)	0.24	21.6	43.00
6	90.04	11.49	3.842	0.30 (0.07)	0.24	21.8	71.00
7	90.21	11.54	3.832	0.30 (0.07)	0.24	21.9	45.00
8	90.24	11.55	3.830	0.30 (0.07)	0.24	21.9	55.00
9	90.77	11.73	3.795	0.30 (0.07)	0.24	22.3	59.00
10	91.26	11.92	3.761	0.30 (0.07)	0.24	22.7	30.00
11	91.42	11.99	3.749	0.30 (0.07)	0.24	22.9	63.00
12	91.99	12.28	3.697	0.30 (0.07)	0.24	23.4	60.00
13	92.22	12.40	3.677	0.30 (0.07)	0.24	23.6	38.00
14	92.79	12.70	3.627	0.30 (0.07)	0.24	24.2	68.00
15	92.93	12.78	3.613	0.30 (0.07)	0.24	24.4	30.00
16	93.08	12.90	3.594	0.30 (0.07)	0.24	24.6	36.00
17	93.73	13.44	3.512	0.30 (0.07)	0.24	25.5	14.00
18	94.11	13.73	3.469	0.30 (0.07)	0.24	26.0	26.00
19	94.19	13.79	3.459	0.30 (0.07)	0.24	26.1	27.00
20	94.45	14.07	3.420	0.30 (0.07)	0.24	26.6	38.00
21	94.77	14.46	3.368	0.30 (0.07)	0.24	27.3	20.00
22	94.88	14.74	3.331	0.30 (0.07)	0.23	27.7	72.00
23	94.89	14.93	3.306	0.30 (0.07)	0.23	27.9	17.00
24	94.89	15.00	3.297	0.30 (0.07)	0.23	28.0	23.00
25	94.89	15.03	3.294	0.30 (0.07)	0.23	28.1	29.00
26	94.55	16.09	3.167	0.30 (0.07)	0.23	29.4	50.00
27	94.13	16.36	3.137	0.30 (0.07)	0.23	29.6	39.00
28	93.89	16.47	3.125	0.30 (0.07)	0.23	29.6	34.00
29	93.37	16.74	3.096	0.30 (0.07)	0.23	29.8	24.00
30	87.81	19.45	2.841	0.30 (0.07)	0.23	31.1	12.00
31	82.07	22.72	2.599	0.30 (0.07)	0.22	32.4	64.00
32	81.85	22.86	2.590	0.30 (0.07)	0.22	32.4	10.00
33	81.51	23.08	2.575	0.30 (0.07)	0.22	32.5	32.00
LONGEST FLOWPATH FROM NODE				10.00	TO NODE	74.00 =	2940.00 FEET.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	88.63	9.64	4.247	0.30 (0.08)	0.25	19.3	78.00
2	89.25	9.81	4.205	0.30 (0.08)	0.25	19.6	65.00
3	92.23	10.54	4.035	0.30 (0.08)	0.25	21.3	75.00
4	92.42	10.59	4.024	0.30 (0.08)	0.25	21.4	72.00
5	94.20	11.07	3.923	0.30 (0.08)	0.25	22.4	51.00
6	95.20	11.35	3.868	0.30 (0.08)	0.25	23.1	58.00

7	95.36	11.40	3.859	0.30	(0.08)	0.25	23.2	43.00
8	95.66	11.49	3.842	0.30	(0.08)	0.25	23.4	71.00
9	95.82	11.54	3.832	0.30	(0.08)	0.25	23.5	45.00
10	95.84	11.55	3.830	0.30	(0.08)	0.25	23.5	55.00
11	96.35	11.73	3.795	0.30	(0.08)	0.25	23.9	59.00
12	96.71	11.88	3.768	0.30	(0.08)	0.25	24.2	77.00
13	96.80	11.92	3.761	0.30	(0.08)	0.25	24.3	30.00
14	96.94	11.99	3.749	0.30	(0.08)	0.25	24.5	63.00
15	97.44	12.28	3.697	0.30	(0.07)	0.25	25.0	60.00
16	97.64	12.40	3.677	0.30	(0.07)	0.25	25.3	38.00
17	98.13	12.70	3.627	0.30	(0.07)	0.25	25.8	68.00
18	98.25	12.78	3.613	0.30	(0.07)	0.25	26.0	30.00
19	98.37	12.90	3.594	0.30	(0.07)	0.25	26.2	36.00
20	98.89	13.44	3.512	0.30	(0.07)	0.24	27.1	14.00
21	99.22	13.73	3.469	0.30	(0.07)	0.24	27.6	26.00
22	99.27	13.79	3.459	0.30	(0.07)	0.24	27.7	27.00
23	99.48	14.07	3.420	0.30	(0.07)	0.24	28.2	38.00
24	99.72	14.46	3.368	0.30	(0.07)	0.24	28.9	20.00
25	99.77	14.74	3.331	0.30	(0.07)	0.24	29.3	72.00
26	99.74	14.93	3.306	0.30	(0.07)	0.24	29.6	17.00
27	99.73	15.00	3.297	0.30	(0.07)	0.24	29.7	23.00
28	99.72	15.03	3.294	0.30	(0.07)	0.24	29.7	29.00
29	99.20	16.09	3.167	0.30	(0.07)	0.24	31.0	50.00
30	98.72	16.36	3.137	0.30	(0.07)	0.24	31.2	39.00
31	98.47	16.47	3.125	0.30	(0.07)	0.24	31.3	34.00
32	97.90	16.74	3.096	0.30	(0.07)	0.24	31.4	24.00
33	91.96	19.45	2.841	0.30	(0.07)	0.23	32.7	12.00
34	85.86	22.72	2.599	0.30	(0.07)	0.23	34.0	64.00
35	85.62	22.86	2.590	0.30	(0.07)	0.23	34.0	10.00
36	85.25	23.08	2.575	0.30	(0.07)	0.23	34.1	32.00
TOTAL AREA (ACRES) =			34.1					

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE (CFS) = 99.77 Tc (MIN.) = 14.736
EFFECTIVE AREA (ACRES) = 29.29 AREA-AVERAGED Fm (INCH/HR) = 0.07
AREA-AVERAGED Fp (INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.24
TOTAL AREA (ACRES) = 34.1
LONGEST FLOWPATH FROM NODE 10.00 TO NODE 74.00 = 2940.00 FEET.

FLOW PROCESS FROM NODE 74.00 TO NODE 74.00 IS CODE = 12

>>>>CLEAR MEMORY BANK # 3 <<<<<

FLOW PROCESS FROM NODE 74.00 TO NODE 74.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

TOTAL NUMBER OF STREAMS = 2

CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:

TIME OF CONCENTRATION (MIN.) = 14.74

RAINFALL INTENSITY (INCH/HR) = 3.33

AREA-AVERAGED Fm (INCH/HR) = 0.07
 AREA-AVERAGED Fp (INCH/HR) = 0.30
 AREA-AVERAGED Ap = 0.24
 EFFECTIVE STREAM AREA (ACRES) = 29.29
 TOTAL STREAM AREA (ACRES) = 34.13
 PEAK FLOW RATE (CFS) AT CONFLUENCE = 99.77

FLOW PROCESS FROM NODE 59.00 TO NODE 74.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
 >>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

INITIAL SUBAREA FLOW-LENGTH (FEET) = 300.00
 ELEVATION DATA: UPSTREAM (FEET) = 166.80 DOWNSTREAM (FEET) = 165.00

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$
 SUBAREA ANALYSIS USED MINIMUM Tc (MIN.) = 9.807
 * 100 YEAR RAINFALL INTENSITY (INCH/HR) = 4.206

SUBAREA Tc AND LOSS RATE DATA (AMC III):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN	Tc (MIN.)
CONDOMINIUMS	B	0.41	0.30	0.350	76	9.81

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp (INCH/HR) = 0.30

SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.350

SUBAREA RUNOFF (CFS) = 1.51

TOTAL AREA (ACRES) = 0.41 PEAK FLOW RATE (CFS) = 1.51

FLOW PROCESS FROM NODE 74.00 TO NODE 74.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
 >>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<

TOTAL NUMBER OF STREAMS = 2
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
 TIME OF CONCENTRATION (MIN.) = 9.81
 RAINFALL INTENSITY (INCH/HR) = 4.21
 AREA-AVERAGED Fm (INCH/HR) = 0.11
 AREA-AVERAGED Fp (INCH/HR) = 0.30
 AREA-AVERAGED Ap = 0.35
 EFFECTIVE STREAM AREA (ACRES) = 0.41
 TOTAL STREAM AREA (ACRES) = 0.41
 PEAK FLOW RATE (CFS) AT CONFLUENCE = 1.51

** CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp (Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	88.63	9.64	4.247	0.30 (0.08)	0.25	19.3	78.00
1	89.25	9.81	4.205	0.30 (0.08)	0.25	19.6	65.00
1	92.23	10.54	4.035	0.30 (0.08)	0.25	21.3	75.00
1	92.42	10.59	4.024	0.30 (0.08)	0.25	21.4	72.00
1	94.20	11.07	3.923	0.30 (0.08)	0.25	22.4	51.00
1	95.20	11.35	3.868	0.30 (0.08)	0.25	23.1	58.00

1	95.36	11.40	3.859	0.30 (0.08)	0.25	23.2	43.00
1	95.66	11.49	3.842	0.30 (0.08)	0.25	23.4	71.00
1	95.82	11.54	3.832	0.30 (0.08)	0.25	23.5	45.00
1	95.84	11.55	3.830	0.30 (0.08)	0.25	23.5	55.00
1	96.35	11.73	3.795	0.30 (0.08)	0.25	23.9	59.00
1	96.71	11.88	3.768	0.30 (0.08)	0.25	24.2	77.00
1	96.80	11.92	3.761	0.30 (0.08)	0.25	24.3	30.00
1	96.94	11.99	3.749	0.30 (0.08)	0.25	24.5	63.00
1	97.44	12.28	3.697	0.30 (0.07)	0.25	25.0	60.00
1	97.64	12.40	3.677	0.30 (0.07)	0.25	25.3	38.00
1	98.13	12.70	3.627	0.30 (0.07)	0.25	25.8	68.00
1	98.25	12.78	3.613	0.30 (0.07)	0.25	26.0	30.00
1	98.37	12.90	3.594	0.30 (0.07)	0.25	26.2	36.00
1	98.89	13.44	3.512	0.30 (0.07)	0.24	27.1	14.00
1	99.22	13.73	3.469	0.30 (0.07)	0.24	27.6	26.00
1	99.27	13.79	3.459	0.30 (0.07)	0.24	27.7	27.00
1	99.48	14.07	3.420	0.30 (0.07)	0.24	28.2	38.00
1	99.72	14.46	3.368	0.30 (0.07)	0.24	28.9	20.00
1	99.77	14.74	3.331	0.30 (0.07)	0.24	29.3	72.00
1	99.74	14.93	3.306	0.30 (0.07)	0.24	29.6	17.00
1	99.73	15.00	3.297	0.30 (0.07)	0.24	29.7	23.00
1	99.72	15.03	3.294	0.30 (0.07)	0.24	29.7	29.00
1	99.20	16.09	3.167	0.30 (0.07)	0.24	31.0	50.00
1	98.72	16.36	3.137	0.30 (0.07)	0.24	31.2	39.00
1	98.47	16.47	3.125	0.30 (0.07)	0.24	31.3	34.00
1	97.90	16.74	3.096	0.30 (0.07)	0.24	31.4	24.00
1	91.96	19.45	2.841	0.30 (0.07)	0.23	32.7	12.00
1	85.86	22.72	2.599	0.30 (0.07)	0.23	34.0	64.00
1	85.62	22.86	2.590	0.30 (0.07)	0.23	34.0	10.00
1	85.25	23.08	2.575	0.30 (0.07)	0.23	34.1	32.00
2	1.51	9.81	4.206	0.30 (0.11)	0.35	0.4	59.00

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	90.14	9.64	4.247	0.30 (0.08)	0.25	19.7	78.00
2	90.75	9.81	4.206	0.30 (0.08)	0.25	20.0	59.00
3	90.76	9.81	4.205	0.30 (0.08)	0.25	20.0	65.00
4	93.68	10.54	4.035	0.30 (0.08)	0.25	21.7	75.00
5	93.87	10.59	4.024	0.30 (0.08)	0.25	21.8	72.00
6	95.61	11.07	3.923	0.30 (0.08)	0.25	22.9	51.00
7	96.59	11.35	3.868	0.30 (0.08)	0.25	23.5	58.00
8	96.75	11.40	3.859	0.30 (0.08)	0.25	23.6	43.00
9	97.04	11.49	3.842	0.30 (0.08)	0.25	23.8	71.00
10	97.19	11.54	3.832	0.30 (0.08)	0.25	23.9	45.00
11	97.22	11.55	3.830	0.30 (0.08)	0.25	24.0	55.00
12	97.71	11.73	3.795	0.30 (0.08)	0.25	24.4	59.00
13	98.06	11.88	3.768	0.30 (0.08)	0.25	24.7	77.00
14	98.15	11.92	3.761	0.30 (0.08)	0.25	24.7	30.00
15	98.29	11.99	3.749	0.30 (0.08)	0.25	24.9	63.00
16	98.76	12.28	3.697	0.30 (0.08)	0.25	25.4	60.00

17	98.95	12.40	3.677	0.30	(0.08)	0.25	25.7	38.00
18	99.43	12.70	3.627	0.30	(0.08)	0.25	26.2	68.00
19	99.54	12.78	3.613	0.30	(0.08)	0.25	26.4	30.00
20	99.66	12.90	3.594	0.30	(0.08)	0.25	26.6	36.00
21	100.15	13.44	3.512	0.30	(0.07)	0.25	27.5	14.00
22	100.46	13.73	3.469	0.30	(0.07)	0.25	28.0	26.00
23	100.51	13.79	3.459	0.30	(0.07)	0.24	28.2	27.00
24	100.71	14.07	3.420	0.30	(0.07)	0.24	28.6	38.00
25	100.92	14.46	3.368	0.30	(0.07)	0.24	29.3	20.00
26	100.96	14.74	3.331	0.30	(0.07)	0.24	29.7	72.00
27	100.93	14.93	3.306	0.30	(0.07)	0.24	30.0	17.00
28	100.91	15.00	3.297	0.30	(0.07)	0.24	30.1	23.00
29	100.90	15.03	3.294	0.30	(0.07)	0.24	30.1	29.00
30	100.33	16.09	3.167	0.30	(0.07)	0.24	31.4	50.00
31	99.84	16.36	3.137	0.30	(0.07)	0.24	31.6	39.00
32	99.58	16.47	3.125	0.30	(0.07)	0.24	31.7	34.00
33	99.01	16.74	3.096	0.30	(0.07)	0.24	31.8	24.00
34	92.97	19.45	2.841	0.30	(0.07)	0.23	33.1	12.00
35	86.78	22.72	2.599	0.30	(0.07)	0.23	34.4	64.00
36	86.53	22.86	2.590	0.30	(0.07)	0.23	34.5	10.00
37	86.16	23.08	2.575	0.30	(0.07)	0.23	34.5	32.00

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE (CFS) = 100.96 Tc (MIN.) = 14.74
EFFECTIVE AREA (ACRES) = 29.70 AREA-AVERAGED Fm (INCH/HR) = 0.07
AREA-AVERAGED Fp (INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.24
TOTAL AREA (ACRES) = 34.5
LONGEST FLOWPATH FROM NODE 10.00 TO NODE 74.00 = 2940.00 FEET.

FLOW PROCESS FROM NODE 80.00 TO NODE 81.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

INITIAL SUBAREA FLOW-LENGTH (FEET) = 290.00
ELEVATION DATA: UPSTREAM (FEET) = 166.40 DOWNSTREAM (FEET) = 165.90

Tc = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20
SUBAREA ANALYSIS USED MINIMUM Tc (MIN.) = 10.484
* 100 YEAR RAINFALL INTENSITY (INCH/HR) = 4.048
SUBAREA Tc AND LOSS RATE DATA (AMC III):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN	Tc (MIN.)
COMMERCIAL	B	0.76	0.30	0.100	76	10.48

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp (INCH/HR) = 0.30
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.100
SUBAREA RUNOFF (CFS) = 2.75
TOTAL AREA (ACRES) = 0.76 PEAK FLOW RATE (CFS) = 2.75

FLOW PROCESS FROM NODE 81.00 TO NODE 82.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<

>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 153.00 DOWNSTREAM(FEET) = 147.60
FLOW LENGTH(FEET) = 320.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 12.0 INCH PIPE IS 6.8 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 6.03
ESTIMATED PIPE DIAMETER(INCH) = 12.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 2.75
PIPE TRAVEL TIME(MIN.) = 0.89 Tc(MIN.) = 11.37
LONGEST FLOWPATH FROM NODE 80.00 TO NODE 82.00 = 610.00 FEET.

FLOW PROCESS FROM NODE 82.00 TO NODE 82.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<

=====

TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION(MIN.) = 11.37
RAINFALL INTENSITY(INCH/HR) = 3.86
AREA-AVERAGED Fm(INCH/HR) = 0.03
AREA-AVERAGED Fp(INCH/HR) = 0.30
AREA-AVERAGED Ap = 0.10
EFFECTIVE STREAM AREA(ACRES) = 0.76
TOTAL STREAM AREA(ACRES) = 0.76
PEAK FLOW RATE(CFS) AT CONFLUENCE = 2.75

FLOW PROCESS FROM NODE 81.00 TO NODE 82.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<
>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

=====

INITIAL SUBAREA FLOW-LENGTH(FEET) = 320.00
ELEVATION DATA: UPSTREAM(FEET) = 165.90 DOWNSTREAM(FEET) = 165.40

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$
SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) = 11.122
* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 3.913

SUBAREA Tc AND LOSS RATE DATA(AMC III):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN	Tc (MIN.)
COMMERCIAL	B	0.59	0.30	0.100	76	11.12

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.30

SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.100

SUBAREA RUNOFF(CFS) = 2.06

TOTAL AREA(ACRES) = 0.59 PEAK FLOW RATE(CFS) = 2.06

FLOW PROCESS FROM NODE 82.00 TO NODE 82.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<
>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<

TOTAL NUMBER OF STREAMS = 2
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
 TIME OF CONCENTRATION(MIN.) = 11.12
 RAINFALL INTENSITY(INCH/HR) = 3.91
 AREA-AVERAGED Fm(INCH/HR) = 0.03
 AREA-AVERAGED Fp(INCH/HR) = 0.30
 AREA-AVERAGED Ap = 0.10
 EFFECTIVE STREAM AREA(ACRES) = 0.59
 TOTAL STREAM AREA(ACRES) = 0.59
 PEAK FLOW RATE(CFS) AT CONFLUENCE = 2.06

** CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	2.75	11.37	3.864	0.30(0.03)	0.10	0.8	80.00
2	2.06	11.12	3.913	0.30(0.03)	0.10	0.6	81.00

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
 CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	4.79	11.12	3.913	0.30(0.03)	0.10	1.3	81.00
2	4.78	11.37	3.864	0.30(0.03)	0.10	1.3	80.00

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE(CFS) = 4.79 Tc(MIN.) = 11.12
 EFFECTIVE AREA(ACRES) = 1.33 AREA-AVERAGED Fm(INCH/HR) = 0.03
 AREA-AVERAGED Fp(INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.10
 TOTAL AREA(ACRES) = 1.3
 LONGEST FLOWPATH FROM NODE 80.00 TO NODE 82.00 = 610.00 FEET.

=====
 END OF STUDY SUMMARY:

TOTAL AREA(ACRES) = 1.3 TC(MIN.) = 11.12
 EFFECTIVE AREA(ACRES) = 1.33 AREA-AVERAGED Fm(INCH/HR) = 0.03
 AREA-AVERAGED Fp(INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.100
 PEAK FLOW RATE(CFS) = 4.79

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	4.79	11.12	3.913	0.30(0.03)	0.10	1.3	81.00
2	4.78	11.37	3.864	0.30(0.03)	0.10	1.3	80.00

=====
 END OF RATIONAL METHOD ANALYSIS

RATIONAL METHOD HYDROLOGY COMPUTER PROGRAM PACKAGE
(Reference: 1986 ORANGE COUNTY HYDROLOGY CRITERION)
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Ver. 20.0 Release Date: 06/01/2013 License ID 1654

Analysis prepared by:

FUSCOE ENGINEERING, Inc
16795 Von Karman Ave., #100
Irvine, CA 92606
949-474-1960

***** DESCRIPTION OF STUDY *****
* SOUTH STREET ANALYSIS *
* 100 YEAR *
* *

FILE NAME: FRS100.DAT
TIME/DATE OF STUDY: 13:14 07/23/2017

=====

USER SPECIFIED HYDROLOGY AND HYDRAULIC MODEL INFORMATION:

=====

--*TIME-OF-CONCENTRATION MODEL*--

USER SPECIFIED STORM EVENT (YEAR) = 100.00
SPECIFIED MINIMUM PIPE SIZE (INCH) = 12.00
SPECIFIED PERCENT OF GRADIENTS (DECIMAL) TO USE FOR FRICTION SLOPE = 0.95
DATA BANK RAINFALL USED
ANTECEDENT MOISTURE CONDITION (AMC) III ASSUMED FOR RATIONAL METHOD

USER-DEFINED STREET-SECTIONS FOR COUPLED PIPEFLOW AND STREETFLOW MODEL

NO.	HALF- WIDTH (FT)	CROWN TO CROSSFALL (FT)	STREET-CROSSFALL: IN- / OUT- / PARK- SIDE / SIDE / WAY	CURB HEIGHT (FT)	GUTTER-GEOMETRIES: WIDTH LIP HIKE (FT) (FT) (FT)	MANNING FACTOR (n)
1	30.0	20.0	0.018/0.018/0.020	0.67	2.00 0.0312 0.167	0.0150

GLOBAL STREET FLOW-DEPTH CONSTRAINTS:

1. Relative Flow-Depth = 0.67 FEET
as (Maximum Allowable Street Flow Depth) - (Top-of-Curb)
 2. (Depth)*(Velocity) Constraint = 6.0 (FT*FT/S)
- *SIZE PIPE WITH A FLOW CAPACITY GREATER THAN
OR EQUAL TO THE UPSTREAM TRIBUTARY PIPE.*
*USER-SPECIFIED MINIMUM TOPOGRAPHIC SLOPE ADJUSTMENT NOT SELECTED

FLOW PROCESS FROM NODE 10.00 TO NODE 11.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

=====

INITIAL SUBAREA FLOW-LENGTH (FEET) = 310.00
ELEVATION DATA: UPSTREAM (FEET) = 168.00 DOWNSTREAM (FEET) = 167.00

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$
SUBAREA ANALYSIS USED MINIMUM T_c (MIN.) = 9.499
* 100 YEAR RAINFALL INTENSITY (INCH/HR) = 4.283

SUBAREA T_c AND LOSS RATE DATA (AMC III):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	F_p (INCH/HR)	A_p (DECIMAL)	SCS CN	T_c (MIN.)
COMMERCIAL	B	1.43	0.30	0.100	76	9.50

SUBAREA AVERAGE PERVIOUS LOSS RATE, F_p (INCH/HR) = 0.30

SUBAREA AVERAGE PERVIOUS AREA FRACTION, A_p = 0.100

SUBAREA RUNOFF (CFS) = 5.47

TOTAL AREA (ACRES) = 1.43 PEAK FLOW RATE (CFS) = 5.47

FLOW PROCESS FROM NODE 11.00 TO NODE 12.00 IS CODE = 62

>>>>COMPUTE STREET FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>(STREET TABLE SECTION # 1 USED)<<<<<

=====

UPSTREAM ELEVATION (FEET) = 167.00 DOWNSTREAM ELEVATION (FEET) = 165.80
STREET LENGTH (FEET) = 360.00 CURB HEIGHT (INCHES) = 8.0
STREET HALFWIDTH (FEET) = 30.00

DISTANCE FROM CROWN TO CROSSFALL GRADEBREAK (FEET) = 20.00

INSIDE STREET CROSSFALL (DECIMAL) = 0.018

OUTSIDE STREET CROSSFALL (DECIMAL) = 0.018

SPECIFIED NUMBER OF HALFSTREETS CARRYING RUNOFF = 1

STREET PARKWAY CROSSFALL (DECIMAL) = 0.020

Manning's FRICTION FACTOR for Streetflow Section (curb-to-curb) = 0.0150

Manning's FRICTION FACTOR for Back-of-Walk Flow Section = 0.0200

**TRAVEL TIME COMPUTED USING ESTIMATED FLOW (CFS) = 10.65

STREETFLOW MODEL RESULTS USING ESTIMATED FLOW:

STREET FLOW DEPTH (FEET) = 0.59

HALFSTREET FLOOD WIDTH (FEET) = 23.71

AVERAGE FLOW VELOCITY (FEET/SEC.) = 2.04

PRODUCT OF DEPTH&VELOCITY (FT*FT/SEC.) = 1.20

STREET FLOW TRAVEL TIME (MIN.) = 2.94 T_c (MIN.) = 12.44

* 100 YEAR RAINFALL INTENSITY (INCH/HR) = 3.671

SUBAREA LOSS RATE DATA (AMC III):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	F_p (INCH/HR)	A_p (DECIMAL)	SCS CN
COMMERCIAL	B	3.15	0.30	0.100	76

SUBAREA AVERAGE PERVIOUS LOSS RATE, F_p (INCH/HR) = 0.30

SUBAREA AVERAGE PERVIOUS AREA FRACTION, A_p = 0.100

SUBAREA AREA (ACRES) = 3.15 SUBAREA RUNOFF (CFS) = 10.32

EFFECTIVE AREA (ACRES) = 4.58 AREA-AVERAGED F_m (INCH/HR) = 0.03

AREA-AVERAGED F_p (INCH/HR) = 0.30 AREA-AVERAGED A_p = 0.10

TOTAL AREA (ACRES) = 4.6 PEAK FLOW RATE (CFS) = 15.01

END OF SUBAREA STREET FLOW HYDRAULICS:

DEPTH (FEET) = 0.65 HALFSTREET FLOOD WIDTH (FEET) = 27.07
FLOW VELOCITY (FEET/SEC.) = 2.23 DEPTH*VELOCITY (FT*FT/SEC.) = 1.44
LONGEST FLOWPATH FROM NODE 10.00 TO NODE 12.00 = 670.00 FEET.

FLOW PROCESS FROM NODE 12.00 TO NODE 13.00 IS CODE = 62

>>>>COMPUTE STREET FLOW TRAVEL TIME THRU SUBAREA<<<<<

>>>>(STREET TABLE SECTION # 1 USED)<<<<<

=====

UPSTREAM ELEVATION (FEET) = 165.80 DOWNSTREAM ELEVATION (FEET) = 164.00
STREET LENGTH (FEET) = 265.00 CURB HEIGHT (INCHES) = 8.0
STREET HALFWIDTH (FEET) = 30.00

DISTANCE FROM CROWN TO CROSSFALL GRADEBREAK (FEET) = 20.00
INSIDE STREET CROSSFALL (DECIMAL) = 0.018
OUTSIDE STREET CROSSFALL (DECIMAL) = 0.018

SPECIFIED NUMBER OF HALFSTREETS CARRYING RUNOFF = 1
STREET PARKWAY CROSSFALL (DECIMAL) = 0.020
Manning's FRICTION FACTOR for Streetflow Section (curb-to-curb) = 0.0150
Manning's FRICTION FACTOR for Back-of-Walk Flow Section = 0.0200

**TRAVEL TIME COMPUTED USING ESTIMATED FLOW (CFS) = 15.30
STREETFLOW MODEL RESULTS USING ESTIMATED FLOW:
STREET FLOW DEPTH (FEET) = 0.59
HALFSTREET FLOOD WIDTH (FEET) = 23.71
AVERAGE FLOW VELOCITY (FEET/SEC.) = 2.93
PRODUCT OF DEPTH&VELOCITY (FT*FT/SEC.) = 1.72
STREET FLOW TRAVEL TIME (MIN.) = 1.51 Tc (MIN.) = 13.94
* 100 YEAR RAINFALL INTENSITY (INCH/HR) = 3.438
SUBAREA LOSS RATE DATA (AMC III):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
COMMERCIAL	B	0.19	0.30	0.100	76

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp (INCH/HR) = 0.30
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.100
SUBAREA AREA (ACRES) = 0.19 SUBAREA RUNOFF (CFS) = 0.58
EFFECTIVE AREA (ACRES) = 4.77 AREA-AVERAGED Fm (INCH/HR) = 0.03
AREA-AVERAGED Fp (INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.10
TOTAL AREA (ACRES) = 4.8 PEAK FLOW RATE (CFS) = 15.01

END OF SUBAREA STREET FLOW HYDRAULICS:

DEPTH (FEET) = 0.58 HALFSTREET FLOOD WIDTH (FEET) = 23.55
FLOW VELOCITY (FEET/SEC.) = 2.91 DEPTH*VELOCITY (FT*FT/SEC.) = 1.70
LONGEST FLOWPATH FROM NODE 10.00 TO NODE 13.00 = 935.00 FEET.

FLOW PROCESS FROM NODE 14.00 TO NODE 15.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<

>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

=====

INITIAL SUBAREA FLOW-LENGTH (FEET) = 270.00
ELEVATION DATA: UPSTREAM (FEET) = 166.60 DOWNSTREAM (FEET) = 166.00

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$

SUBAREA ANALYSIS USED MINIMUM T_c (MIN.) = 9.684

* 100 YEAR RAINFALL INTENSITY (INCH/HR) = 4.236

SUBAREA T_c AND LOSS RATE DATA (AMC III):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	F_p (INCH/HR)	A_p (DECIMAL)	SCS CN	T_c (MIN.)
COMMERCIAL	B	0.92	0.30	0.100	76	9.68

SUBAREA AVERAGE PERVIOUS LOSS RATE, F_p (INCH/HR) = 0.30

SUBAREA AVERAGE PERVIOUS AREA FRACTION, A_p = 0.100

SUBAREA RUNOFF (CFS) = 3.48

TOTAL AREA (ACRES) = 0.92 PEAK FLOW RATE (CFS) = 3.48

FLOW PROCESS FROM NODE 15.00 TO NODE 16.00 IS CODE = 62

>>>>COMPUTE STREET FLOW TRAVEL TIME THRU SUBAREA<<<<<

>>>>(STREET TABLE SECTION # 1 USED)<<<<<

=====

UPSTREAM ELEVATION (FEET) = 166.00 DOWNSTREAM ELEVATION (FEET) = 164.00
STREET LENGTH (FEET) = 670.00 CURB HEIGHT (INCHES) = 8.0
STREET HALFWIDTH (FEET) = 30.00

DISTANCE FROM CROWN TO CROSSFALL GRADEBREAK (FEET) = 20.00

INSIDE STREET CROSSFALL (DECIMAL) = 0.018

OUTSIDE STREET CROSSFALL (DECIMAL) = 0.018

SPECIFIED NUMBER OF HALFSTREETS CARRYING RUNOFF = 1

STREET PARKWAY CROSSFALL (DECIMAL) = 0.020

Manning's FRICTION FACTOR for Streetflow Section (curb-to-curb) = 0.0150

Manning's FRICTION FACTOR for Back-of-Walk Flow Section = 0.0200

**TRAVEL TIME COMPUTED USING ESTIMATED FLOW (CFS) = 7.29

STREETFLOW MODEL RESULTS USING ESTIMATED FLOW:

STREET FLOW DEPTH (FEET) = 0.53

HALFSTREET FLOOD WIDTH (FEET) = 20.82

AVERAGE FLOW VELOCITY (FEET/SEC.) = 1.79

PRODUCT OF DEPTH&VELOCITY (FT*FT/SEC.) = 0.96

STREET FLOW TRAVEL TIME (MIN.) = 6.22 T_c (MIN.) = 15.91

* 100 YEAR RAINFALL INTENSITY (INCH/HR) = 3.188

SUBAREA LOSS RATE DATA (AMC III):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	F_p (INCH/HR)	A_p (DECIMAL)	SCS CN
COMMERCIAL	B	2.66	0.30	0.100	76

SUBAREA AVERAGE PERVIOUS LOSS RATE, F_p (INCH/HR) = 0.30

SUBAREA AVERAGE PERVIOUS AREA FRACTION, A_p = 0.100

SUBAREA AREA (ACRES) = 2.66 SUBAREA RUNOFF (CFS) = 7.56

EFFECTIVE AREA (ACRES) = 3.58 AREA-AVERAGED F_m (INCH/HR) = 0.03

AREA-AVERAGED F_p (INCH/HR) = 0.30 AREA-AVERAGED A_p = 0.10

TOTAL AREA (ACRES) = 3.6 PEAK FLOW RATE (CFS) = 10.17

END OF SUBAREA STREET FLOW HYDRAULICS:

DEPTH(FEET) = 0.59 HALFSTREET FLOOD WIDTH(FEET) = 23.79
FLOW VELOCITY(FEET/SEC.) = 1.94 DEPTH*VELOCITY(FT*FT/SEC.) = 1.14
LONGEST FLOWPATH FROM NODE 14.00 TO NODE 16.00 = 940.00 FEET.

=====

END OF STUDY SUMMARY:

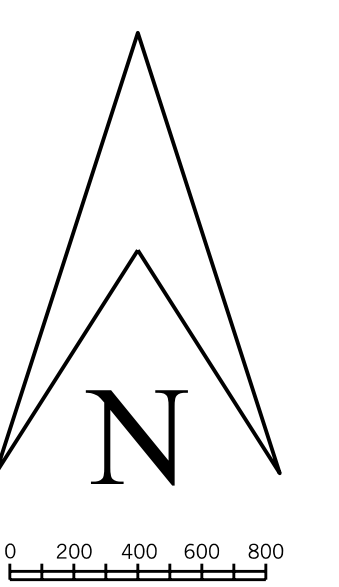
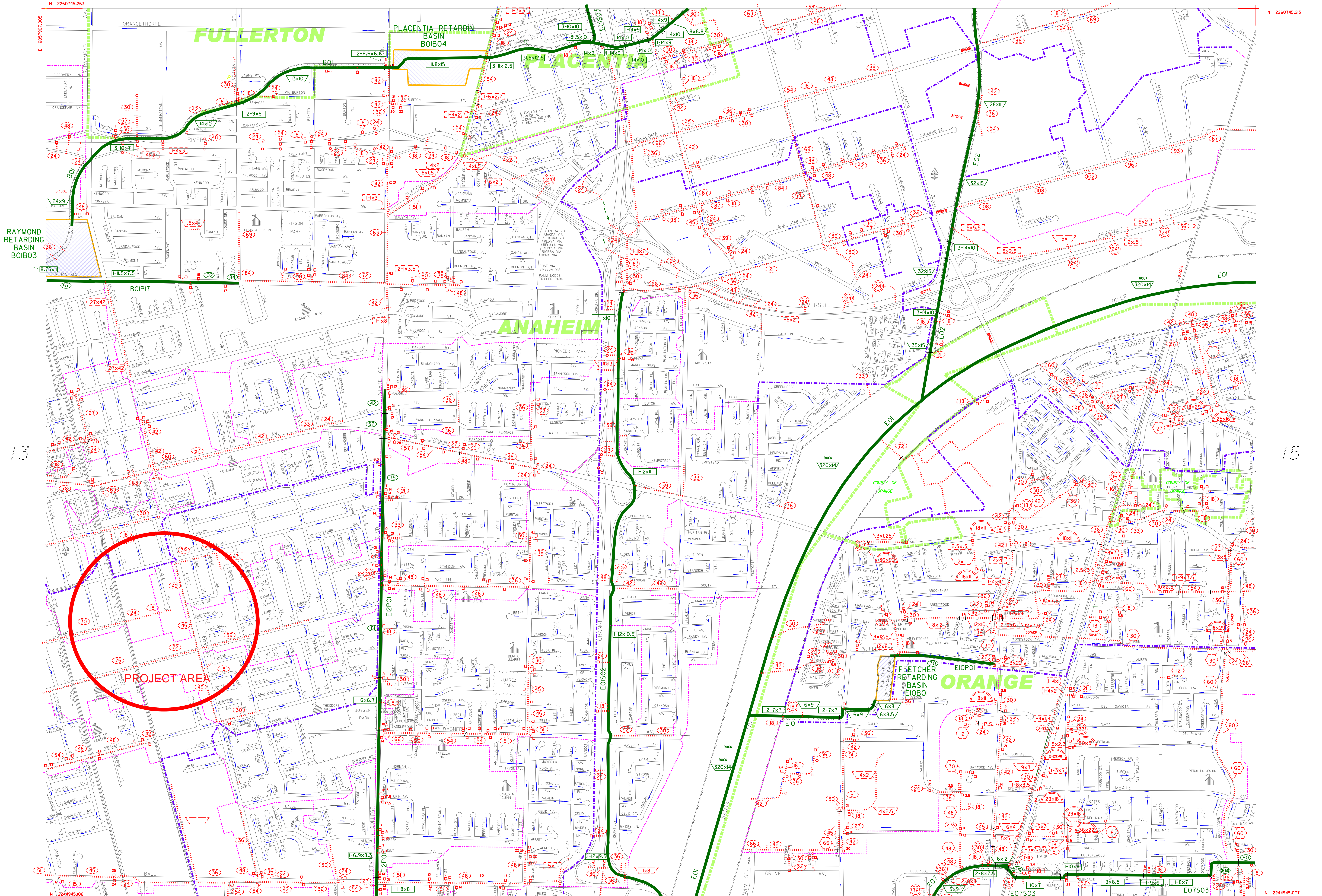
TOTAL AREA(ACRES) = 3.6 TC(MIN.) = 15.91
EFFECTIVE AREA(ACRES) = 3.58 AREA-AVERAGED Fm(INCH/HR) = 0.03
AREA-AVERAGED Fp(INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.100
PEAK FLOW RATE(CFS) = 10.17

=====

END OF RATIONAL METHOD ANALYSIS

APPENDIX 4

**OFF SITE DRAINAGE AREA EXHIBIT
(IN POCKET)**



NOTICE

The drainage information has been prepared for information purposes only. The location, ownership, facility information and limits have been determined from available information provided by public agencies, but may not be exact, accurate, or up-to-date. The user of this information is responsible for verifying exact location, ownership, accuracy, and the regional versus local character of drainage facilities.

Additional information may be obtained from public plans and recorded deeds. Facility designations included with this information are for convenience only and are not controlling or intended to imply ownership by the County or the Orange County Flood Control District (OCFCD). The information is being provided as a courtesy and neither the County of Orange nor OCFCD assume any liabilities for inaccuracy of the information.

To notify OC Public Works Flood Control Section of additions or corrections, please contact Sal Gutierrez at (714) 834-5396 or by email at sal.gutierrez@ocpwc.gov.com

ORANGE COUNTY FLOOD CONTROL DISTRICT			
BASE MAP OF DRAINAGE FACILITIES IN ORANGE COUNTY			
REVISION	DATE	SHEET NO.	DWG. NO.
S. GUTIERREZ	APR. 27, 2010	14	MAPS-113-3

- Channel Drainage Area Boundary
 - Major Sub-Area Drainage Boundary
 - Minor Sub-Area Drainage Boundary
 - Existing O.C.F.C.D. Facility
 - Existing Local Facility
 - Existing Retarding Basin or Reservoir
 - Natural Watercourse
 - City Limits
 - Greenbelt
 - Pump Station
 - Catch Basin (length in feet)
 - Drop Inlet or Other Entry
 - OCFCD Basins or Reservoirs
- Ownership: (If other than City or County): Private = P State = S Federal = F

- EXISTING FACILITIES**
- LOCAL**
- Earth Trapezoidal Channel (base width by height in feet)
 - Reinforced Concrete Trapezoidal Channel (base width by height in feet)
 - Reinforced Concrete Rectangular Channel (base width by height in feet)
 - Reinforced Concrete Box (RCB) (number of barrels-span by height in feet)
 - Reinforced Concrete Pipe (RCP) (diameter in inches)
 - Metal Sheet Channel (MSC) (base width by pile height in feet, Sheet pile total length)
 - Corrugated Metal Pipe (CMP) (diameter in inches)
 - Concrete Pipe (diameter in inches)
 - Concrete Oval Pipe (width by height in inches)
 - Steel Pipe (diameter in inches)
 - Reinforced Concrete Arch (base span by height in inches)
 - Corrugated Metal Arch (base span by height in inches)

APPENDIX 5

MASTER PLAN OF DRAINAGE



CITY OF ANAHEIM

**Master Plan of Storm Drainage for
Anaheim Barber City Channel
Tributary Area**

JUNE 2009

6. Drainage Area 19

Drainage Area 19 drains approximately 643 acres, and is generally bounded by East Steet on the east, Ball Road and Vermont Avenue on the south, Union Pacific Railroad and Walnut Street on the west, with the northerly boundary of the Drainage Area meandering along Santa Ana Street, South Street, Lincoln Avenue, and Broadway. Generally, water flows over land and then through pipes from east to west and ties into the South Street storm drain flowing westerly across the I-5 Freeway which then joins the Walnut Street storm drain flowing south then Ball Road storm drain flowing west to the ABC Channel.

7.1 Hydrologic Analysis

The hydrologic analysis was performed for Drainage Area 19 in accordance with the criteria outlined in Chapter 3 and can be found in Appendix C. The hydrology map for Drainage Area 19 can be found in Appendix D. The following table shows the flow rates at the outlet of the Drainage Area for 10-, 25-, and 100-year storm events.

Table 7 – Drainage Area 19 Summary of Hydrology

Node	Location	Cumulative Drainage Area (ac)	10-Year Flow (cfs)	25-Year Flow (cfs)	100-Year Flow (cfs)
1970	West Bellevue Drive & I-5 Fwy	484	537	667	859
1978	Ball Road at ABC Channel	643	615	786	1067

7.2 Analysis of Existing Improvements

Currently the major storm drain system Drainage Area 19 begins in South St. east of S. East St. and continues west along South St. and confluences with several laterals. The line continues to Harbor Blvd, where it confluences with a major storm drain system at the intersection of South Street and Harbor Blvd. The Harbor Blvd storm drain line begins as a 24" RCP at a flow bifurcation structure on a 93" RCP at the intersection of Harbor Blvd and Lincoln Blvd. It continues south and crosses under a 42" RCP at Broadway. Excess flow from the lateral is then forced up an existing junction structure box to the 42" RCP in Broadway. The lateral continues south and increases to a 72" RCP before it confluences with the 78" mainline RCP in South St and transitions to a 96" RCP in South St.

The storm drain continues west to Bellevue Drive, where it turns southerly and transitions to a 12' x 6' RCB. The RCB then turns westerly and crosses under the I-5 Freeway to Hampshire Drive, then south on Walnut St. , west on Ball Road and discharges into the ABC Channel.

As shown in Appendix B, the estimated flow diverted from the 93" RCP in Lincoln Blvd to the 24" lateral in Harbor Blvd is estimated to be 24 CFS. This analysis assumes flows in excess of this amount in the lateral, will be diverted to the 42" RCP in Broadway.

The following street segments do not meet the City's flooded width criteria:

1. Harbor Blvd from Santa Ana St to South St.
2. South St. from Harbor Blvd to Bellevue Dr.
3. Bellevue Dr. from South St. to I-5
4. Hampshire Ave. from Disneyland Dr. to Walnut St.
5. Walnut St. from Hampshire Ave. to Ball Road
6. Ball Road from Walnut St. to ABC Channel

The existing street flow hydraulic calculations for Drainage Area 19 can be found in Appendices A and B.

7.3 Proposed Improvements

As discussed under Section 7.2, several streets in Drainage Area 19 do not meet the City's allowable flood width requirement. A few of the street segments currently do not have underground storm drain improvements.

This report recommends construction of new storm drain improvements to convey the full flow of the 10 year storm along the following street segments:

- Install 700 Ft of 39" RCP in West St from Santa Ana St. to Water St.
- Install 810 Ft of 39" RCP in Janss St from Water St. to South St.
- Install 270 Ft of 39" RCP in Dickel St. from Hampshire. to Clementine St.
- Install 530 Ft of 39" RCP in Hampshire Ave. from Harbor Blvd to Dickel St.

The remaining street segments that do not meet the flood width requirement currently have underground storm drains but do not have sufficient flow capacity. Appendix B provides the specific improvement options for the street segments listed in Section 7.2 in order to meet the flood width criteria.

Alternatives 1 and 2 below provide the approximate sizes of conduits to replace or augment each existing storm drain capacity, respectively. Alternative 3 is a combination of Alternatives 1 and 2 where the smaller sized conduits are removed and replaced and the larger conduits are protected in place and augmented with new parallel conduits.

Drainage Area 19 Alternative Improvements:

Alternative 1-

- **Harbor Blvd from Santa Ana St to South St.-** Remove and replace existing 60" and 72" RCP from Santa Ana to South St with a new 84" RCP.
- **South St. from Harbor Blvd to Bellevue Dr.-** Appendix B shows the proposed new conduits as RCP. However, because of the large sizes of the required pipes, it is recommended to replace the existing storm drains with reinforced concrete box conduits, as follows:
 - **South St. from Harbor to CitronSt. -**Remove and replace existing 96" RCP with a new 8.5' x 8' RCB
 - **South St. from Citron St. to Bellevue Dr.-** Remove and replace existing 96" RCP with a new 12' x 9.5' RCB
- **Bellevue Dr. from South St. to I-5-** Remove and replace existing 12' x 6' RCB with a new 11.5' x 9'-7" RCB.
- **CalTrans I-5 Crossing at Hampshire from W. Bellvue to Disneyland Dr.-** Remove and replace the existing 12' x 6' RCB with a 13.5' x 10' RCB.
- **Hampshire Ave. from Disneyland Dr. to Walnut St.-** Remove and replace existing 2-8' x 5' RCB with a new 2- 9.5' x 8' RCB.
- **Walnut St. from Hampshire Ave. to Ball Road-** Remove and replace existing 6.75' x 10.5' RCB with a new 13' x 11.5' RCB.
- **Ball Road from Walnut St. to ABC Channel-** Remove and replace existing 6.5' x 11' RCB with a new 13' x 11.5' RCB.

Alternative 2-

- **Harbor Blvd from Santa Ana St to South St.-** Construct a new 72" RCP parallel to the existing 60" and 72" RCP between Santa Ana and South St.
- **South St. from Harbor Blvd to Bellevue Dr.-** It is also possible to increase the underground storm drain capacity by constructing new storm drains as shown below. However, this option would require extensive utility relocation and may create conflicts with existing sanitary sewer laterals.
 - **South St. from Harbor to Citron St.-** Construct a new 51" RCP parallel to the existing 96" RCP.
 - **South St. from Citron St. to Bellevue Dr.-** Construct a 114" RCP parallel to the existing 96" RCP.
- **Bellevue Dr. from South St. to I-5-** Construct a new 10.5' x6' RCB parallel to the existing 12' x 6' RCB.
- **CalTrans I-5 Crossing at Hampshire from W. Bellvue to Disneyland Dr.-** Construct a new 8.5' x 6' DRCB parallel to the existing 12' x 6' RCB
- **Hampshire Ave. from Disneyland Dr. to Walnut St.-** Construct a new 10.5' x 5' DRCB parallel to the existing 2-8' x 5' RCB.

- **Walnut St. from Hampshire Ave. to Ball Road-** Construct a new 9' x 6.5' DRCB parallel to the existing 6.75' x 10.5' RCB.
- **Ball Road from Walnut St. to ABC Channel-** Construct a new 9' x 6.5' DRCB parallel to the existing 6.5' x 11' RCB.

Alternative 3- (Recommended)

- **Harbor Blvd from Santa Ana St to South St.-** Remove and replace existing 60" and 72" RCP from Santa Ana to South St with a new 84" RCP.
- **South St. from Harbor Blvd to Bellevue Dr.-** Appendix B shows the proposed new conduits as RCP. However, because of the large sizes of the required pipes, it is recommended to replace the existing storm drains with reinforced concrete box conduits, as follows:
 - **South St. from Harbor to CitronSt. -**Remove and replace existing 96" RCP with a new 8.5' x 8' RCB
 - **South St. from Citron St. to Bellevue Dr.-** Remove and replace existing 96" RCP with a new 12' x 9.5' RCB
- **Bellevue Dr. from South St. to I-5-** Construct a new 10.5' x 6' RCB parallel to the existing 12' x 6' RCB.
- **CalTrans I-5 Crossing at Hampshire from W. Bellvue to Disneyland Dr.-** Construct a new 8.5' x 6' DRCB parallel to the existing 12' x 6' RCB
- **Hampshire Ave. from Disneyland Dr. to Walnut St.-** Construct a new 10.5' x 5' DRCB parallel to the existing 2-8' x 5' RCB.
- **Walnut St. from Hampshire Ave. to Ball Road-** Construct a new 9' x 6.5' DRCB parallel to the existing 6.75' x 10.5' RCB.
- **Ball Road from Walnut St. to ABC Channel-** Construct a new 9' x 6.5' DRCB parallel to the existing 6.5' x 11' RCB.

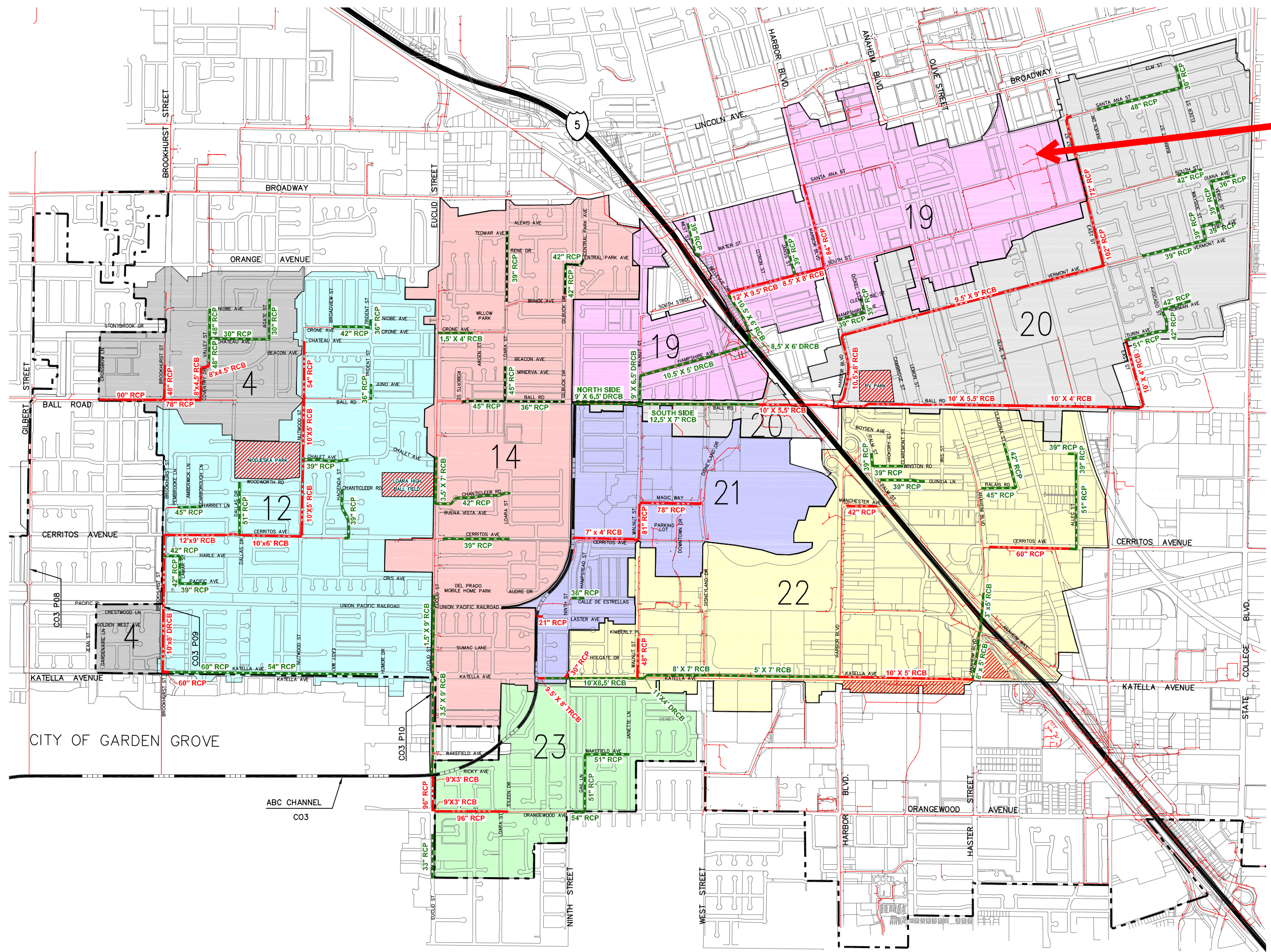
7.4 Cost Estimates

Summarized in Table 8 are the construction cost estimates by project location for Drainage Area 19. The cost estimates were prepared as discussed in Section 4.4. The detailed cost estimates for Drainage Area 19 can be found in Appendix E.

Table 8 – Drainage Area 19 Cost Estimate (2008 Dollars)

Drainage Area	Street	Type of Facility	Size	Estimated Cost
19	Harbor Blvd. (from Santa Ana to South)	Replacement per Alternative 3	84" RCP	\$1,383,000
19	South St. (from Harbor to Citron)	Replacement per Alternative 3	8.5'X8 RCB	\$2,691,000
	(from Citron to Bellvue)	Replacement per Alternative 3	12'X9.5' RCB	
19	Bellvue Dr. (from South to I-5)	New Parallel per Alternative 3	10.5'X6' RCB	\$790,000
19	CalTrans I-5 crossing (from Belvue to Disneyland Dr.)	New Parallel per Alternative 3	8.5' x 6' DRCB	\$1,702,000
19	Hampshire Ave. (from Disneyland Dr to Walnut)	New Parallel per Alternative 3	10.5' x 5' DRCB	\$3,380,000
19	Walnut St. (from Ball to Hampshire)	New Parallel per Alternative 3	9' x 6.5' DRCB	\$894,000
19	Ball Rd. (from Walnut to ABC Channel)	New Parallel per Alternative 3	9' x 6.5' DRCB	\$2,067,000
19	West St. (from Water to Santa Ana)	New	39" RCP	\$302,000
19	Janss St. (from Water to South)	New	39" RCP	\$330,000
19	Dickel/Hampshire (from Clementine to Harbor)	New	39" RCP	\$328,000
TOTAL				\$13,867,000

PROJECT AREA



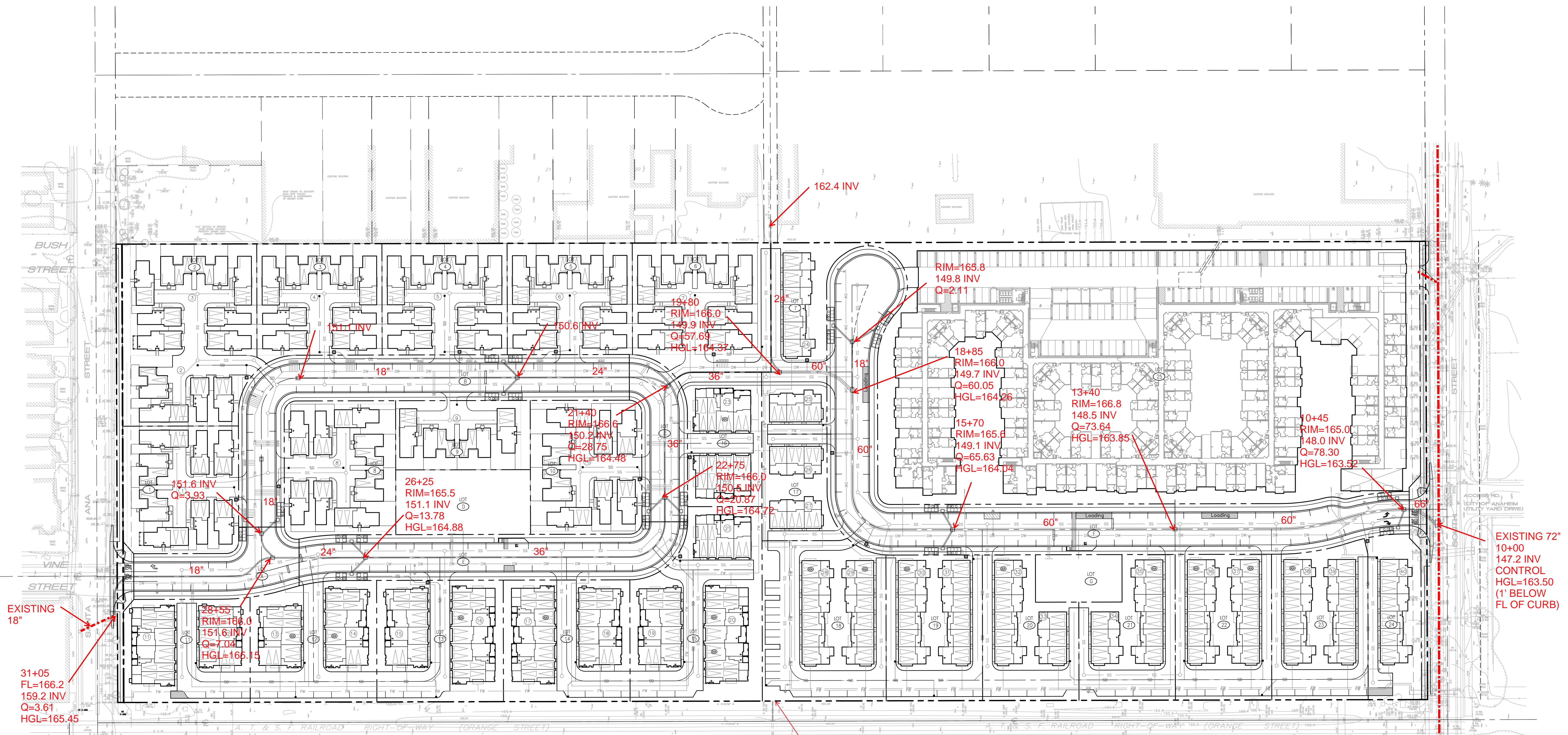
LEGEND	
[Grey Box]	Drainage Area 4
[Cyan Box]	Drainage Area 12
[Pink Box]	Drainage Area 14
[Purple Box]	Drainage Area 19
[Light Grey Box]	Drainage Area 20
[Blue Box]	Drainage Area 21
[Yellow Box]	Drainage Area 22
[Green Box]	Drainage Area 23
[Dashed Line]	Anaheim City Limits
[Thick Black Line]	Anaheim-Barber City Channel
[Red Line]	Existing Storm Drain
[Green Dashed Line]	Proposed Storm Drain (New)
[Red Dashed Line]	Proposed Storm Drain (R&R)
[Red Hatched Box]	Potential Retarding Basin Location



Figure 2
Proposed Storm Drain Improvements
City of Anaheim Master Plan of Storm Drainage for Anaheim-Barber City Channel Tributary Area

APPENDIX 6

PROPOSED STORM DRAIN EXHIBIT (IN POCKET)



EXISTING 18"

31+05
FL=166.2
159.2 INV
Q=3.61
HGL=165.45

28+55
RIM=166.0
151.6 INV
Q=7.04
HGL=165.15

26+25
RIM=165.5
151.1 INV
Q=13.78
HGL=164.88

21+40
RIM=166.6
150.2 INV
Q=28.75
HGL=164.48

22+75
RIM=166.0
150.5 INV
Q=20.87
HGL=164.72

19+80
RIM=165.0
149.9 INV
Q=57.69
HGL=164.37

162.4 INV

RIM=165.8
149.8 INV
Q=2.11

18+85
RIM=166.0
149.7 INV
Q=60.05
HGL=164.26

15+70
RIM=165.8
149.1 INV
Q=65.63
HGL=164.04

13+40
RIM=166.8
148.5 INV
Q=73.64
HGL=163.85

10+45
RIM=165.0
148.0 INV
Q=78.30
HGL=163.52

EXISTING 72"
10+00
147.2 INV
CONTROL
HGL=163.50
(1' BELOW
FL OF CURB)

EXISTING 24" STORM DRAIN TO BE
REMOVED AND PLUGGED AT PROPERTY
LINE AS PART OF ON-SITE GRADING PLAN
AND PERMIT.

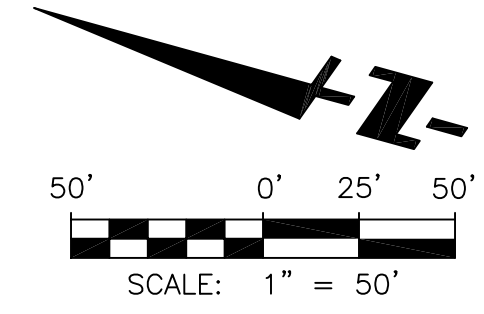
PROPOSED STORM DRAIN EXHIBIT

TECHNICAL SITE PLAN / EAST SOUTH STREET

THE SHOPOFF GROUP
ANAHEIM, CA

LEGEND

- 10+50 STORM DRAIN STATION
- 48" STORM DRAIN LINE SIZE
- 150.0 STORM DRAIN INVERT
- Q=25.00 25 YEAR FLOW
- HGL=166.20 HYDRAULIC GRADE LINE
- RIM=166.0 MANHOLE RIM



MAY 25, 2017

F:\Projects\1330\008\Enb\sh\1330-08-11-Site_Plan.dwg (5/25/2017 3:35 PM) Plotted by: Neil Miles

APPENDIX 7

HYDRAULIC ANALYSIS OF PROPOSED STORM DRAIN

Program Package Serial Number: 7048

WATER SURFACE PROFILE LISTING

Date: 5-29-2017 Time:11: 1:49

FREEMAN

MAIN LINE

25 YEAR ANALYSIS

Station	Invert Elev	Depth (FT)	Water Elev	Q (CFS)	Vel (FPS)	Vel Head	Energy Grd.El.	Super Elev	Critical Depth	Flow Top Width	Height/Dia.-FT	Base Wt/or I.D.	No ZL	Wth Prs/Pip
L/Elem	Ch Slope					SF Ave	HF	SE Dpth	Froude N	Norm Dp	"N"	X-Fall	ZR	Type Ch
1000.000	147.200	16.300	163.500	78.30	3.30	.17	163.67	.00	2.43	.00	5.500	.000	.00	1 .0
45.000	.0178					.0005	.02	.00	.00	1.56	.013	.00	.00	PIPE
1045.000	148.000	15.551	163.551	78.30	3.30	.17	163.72	.00	2.43	.00	5.500	.000	.00	1 .0
JUNCT STR	.0178					.0007	.00	15.55	.00		.013	.00	.00	PIPE
1050.000	148.089	15.434	163.523	73.64	3.75	.22	163.74	.00	2.42	.00	5.000	.000	.00	1 .0
290.000	.0014					.0008	.23	15.43	.00	3.26	.013	.00	.00	PIPE
1340.000	148.490	15.264	163.754	73.64	3.75	.22	163.97	.00	2.42	.00	5.000	.000	.00	1 .0
JUNCT STR	.0020					.0007	.00	15.26	.00		.013	.00	.00	PIPE
1345.000	148.500	15.348	163.848	65.63	3.34	.17	164.02	.00	2.28	.00	5.000	.000	.00	1 .0
225.000	.0025					.0006	.14	15.35	.00	2.52	.013	.00	.00	PIPE
1570.000	149.060	14.931	163.991	65.63	3.34	.17	164.16	.00	2.28	.00	5.000	.000	.00	1 .0
JUNCT STR	.0024					.0006	.00	.00	.00		.013	.00	.00	PIPE
1575.000	149.072	14.968	164.040	60.05	3.06	.15	164.19	.00	2.18	.00	5.000	.000	.00	1 .0
310.000	.0020					.0005	.16	.00	.00	2.56	.013	.00	.00	PIPE
1885.000	149.680	14.554	164.234	60.05	3.06	.15	164.38	.00	2.18	.00	5.000	.000	.00	1 .0
JUNCT STR	.0020					.0005	.00	.00	.00		.013	.00	.00	PIPE
1890.000	149.690	14.566	164.256	57.69	2.94	.13	164.39	.00	2.13	.00	5.000	.000	.00	1 .0
90.000	.0020					.0005	.04	.00	.00	2.49	.013	.00	.00	PIPE

FREEMAN
MAIN LINE
25 YEAR ANALYSIS

Station	Invert Elev	Depth (FT)	Water Elev	Q (CFS)	Vel (FPS)	Vel Head	Energy Grd.El.	Super Elev	Critical Depth	Flow Top Width	Height/Dia.-FT	Base Wt/ I.D.	ZL	No Wth Prs/Pip
L/Elem	Ch Slope					SF Ave	HF	SE Dpth	Froude N	Norm Dp	"N"	X-Fall	ZR	Type Ch
1980.000	149.870	14.449	164.319	57.69	2.94	.13	164.45	.00	2.13	.00	5.000	.000	.00	1 .0
JUNCT STR	.0020					.0004	.00	14.45	.00		.013	.00	.00	PIPE
1985.000	149.880	14.494	164.374	28.75	2.29	.08	164.46	.00	1.59	.00	4.000	.000	.00	1 .0
155.000	.0020					.0004	.06	14.49	.00	1.88	.013	.00	.00	PIPE
2140.000	150.190	14.271	164.461	28.75	2.29	.08	164.54	.00	1.59	.00	4.000	.000	.00	1 .0
JUNCT STR	.0020					.0007	.00	.00	.00		.013	.00	.00	PIPE
2145.000	150.200	14.277	164.477	20.87	2.95	.14	164.61	.00	1.47	.00	3.000	.000	.00	1 .0
130.000	.0020					.0010	.13	14.28	.00	1.85	.013	.00	.00	PIPE
2275.000	150.460	14.144	164.604	20.87	2.95	.14	164.74	.00	1.47	.00	3.000	.000	.00	1 .0
JUNCT STR	.0020					.0007	.00	14.14	.00		.013	.00	.00	PIPE
2280.000	150.470	14.246	164.716	13.78	1.95	.06	164.78	.00	1.18	.00	3.000	.000	.00	1 .0
345.000	.0019					.0004	.15	14.25	.00	1.46	.013	.00	.00	PIPE
2625.000	151.110	13.754	164.864	13.78	1.95	.06	164.92	.00	1.18	.00	3.000	.000	.00	1 .0
JUNCT STR	.0020					.0007	.00	13.75	.00		.013	.00	.00	PIPE
2630.000	151.120	13.759	164.879	7.04	2.24	.08	164.96	.00	.94	.00	2.000	.000	.00	1 .0
225.000	.0019					.0010	.22	13.76	.00	1.25	.013	.00	.00	PIPE
2855.000	151.550	13.547	165.097	7.04	2.24	.08	165.18	.00	.94	.00	2.000	.000	.00	1 .0
JUNCT STR	.0018					.0011	.01	.00	.00		.013	.00	.00	PIPE

FREEMAN
MAIN LINE
25 YEAR ANALYSIS

```

*****
| Invert | Depth | Water | Q | Vel | Vel | Energy | Super | Critical | Flow Top | Height/ | Base Wt | | No Wth
Station | Elev | (FT) | Elev | (CFS) | (FPS) | Head | Grd.El. | Elev | Depth | Width | Dia.-FT | or I.D. | ZL | Prs/Pip
-|- -|- -|- -|- -|- -|- -|- -|- -|- -|- -|- -|- -|- -|- -|-
L/Elem |Ch Slope | | | | | SF Ave | HF | SE Dpth | Froude N | Norm Dp | "N" | X-Fall | ZR | Type Ch
*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|*****
| | | | | | | | | | | | | | | |
2860.000 | 151.559 | 13.590 | 165.149 | 3.61 | 2.04 | .06 | 165.21 | .00 | .73 | .00 | 1.500 | .000 | .00 | 1 | .0
-|- -|- -|- -|- -|- -|- -|- -|- -|- -|- -|- -|- -|- -|- -|-
245.000 | .0312 | | | | | .0012 | .29 | .00 | .00 | .45 | .013 | .00 | .00 | PIPE
WALL ENTRANCE
| | | | | | | | | | | | | | | |
3105.000 | 159.200 | 6.252 | 165.452 | 3.61 | 2.04 | .06 | 165.52 | .00 | .73 | .00 | 1.500 | .000 | .00 | 0 | .0
-|- -|- -|- -|- -|- -|- -|- -|- -|- -|- -|- -|- -|- -|- -|-

```

APPENDIX 8

STREET FLOW ANALYSIS OF SANTA ANA AND SOUTH STREETS

Hydraulic Analysis Report

Project Data

Project Title: Freeman Site
Designer:
Project Date: July 24, 2017
Project Units: U.S. Customary Units

Channel Analysis: Channel Analysis North Side of South Street

Input Parameters

Channel Type: Custom Cross Section

Cross Section Data

Elevation (ft)	Elevation (ft)	Manning's n
0.00	164.50	0.0150
20.00	164.20	0.0150
22.00	164.03	0.0150
22.10	164.70	0.0150
30.00	164.86	-----

Longitudinal Slope: 0.0050 ft/ft

Flow: 15.0100 cfs

Result Parameters

Depth: 0.5474 ft
Area of Flow: 5.4946 ft²
Wetted Perimeter: 22.6403 ft
Hydraulic Radius: 0.2427 ft
Average Velocity: 2.7318 ft/s
Top Width: 22.0817 ft
Froude Number: 0.9651
Critical Depth: 0.5415 ft
Critical Velocity: 2.7973 ft/s
Critical Slope: 0.0054 ft/ft
Critical Top Width: 22.08 ft
Calculated Max Shear Stress: 0.1708 lb/ft²
Calculated Avg Shear Stress: 0.0757 lb/ft²
Composite Manning's n Equation: Lotter method
Manning's n: 0.0150

Results: Depth of 0.55 is below TC

Hydraulic Analysis Report

Project Data

Project Title: Freeman Site

Project Date: July 17, 2017

Project Units: U.S. Customary Units

Channel Analysis: Channel Analysis of Santa Ana Street Flow

Input Parameters

Channel Type: Street Cross Section

Cross Section Data

Station (ft)	Elevation (ft)	Manning's n
0.00	167.00	0.0150
20.00	166.70	0.0150
22.00	166.53	0.0150
22.10	167.13	0.0150
30.00	167.29	-----

Longitudinal Slope: 0.0050 ft/ft

Flow: 4.1900 cfs Q100

Result Parameters

Depth: 0.3937 ft

Area of Flow: 2.2990 ft²

Wetted Perimeter: 17.3238 ft

Hydraulic Radius: 0.1327 ft

Average Velocity: 1.8225 ft/s

Top Width: 16.9814 ft

Froude Number: 0.8729

Critical Depth: 0.3792 ft

Critical Velocity: 2.0350 ft/s

Critical Slope: 0.0067 ft/ft

Critical Top Width: 16.01 ft

Calculated Max Shear Stress: 0.1228 lb/ft²

Calculated Avg Shear Stress: 0.0414 lb/ft²

Composite Manning's n Equation: Lotter method

Manning's n: 0.0150

Result: Depth of 0.3937 ft is below TC.

Hydraulic Analysis Report

Project Data

Project Title: Freeman Site

Designer:

Project Date: July 24, 2017

Project Units: U.S. Customary Units

Channel Analysis: Channel Analysis South Side of South Street

Input Parameters

Channel Type: Custom Cross Section

Cross Section Data

Elevation (ft)	Elevation (ft)	Manning's n
0.00	164.50	0.0150
20.00	164.20	0.0150
22.00	164.03	0.0150
22.10	164.70	0.0150
30.00	164.86	-----

Longitudinal Slope: 0.0050 ft/ft

Flow: 10.1700 cfs

Result Parameters

Depth: 0.4954 ft

Area of Flow: 4.3461 ft²

Wetted Perimeter: 22.5357 ft

Hydraulic Radius: 0.1929 ft

Average Velocity: 2.3400 ft/s

Top Width: 22.0739 ft

Froude Number: 0.9294

Critical Depth: 0.4860 ft

Critical Velocity: 2.4572 ft/s

Critical Slope: 0.0059 ft/ft

Critical Top Width: 22.07 ft

Calculated Max Shear Stress: 0.1546 lb/ft²

Calculated Avg Shear Stress: 0.0602 lb/ft²

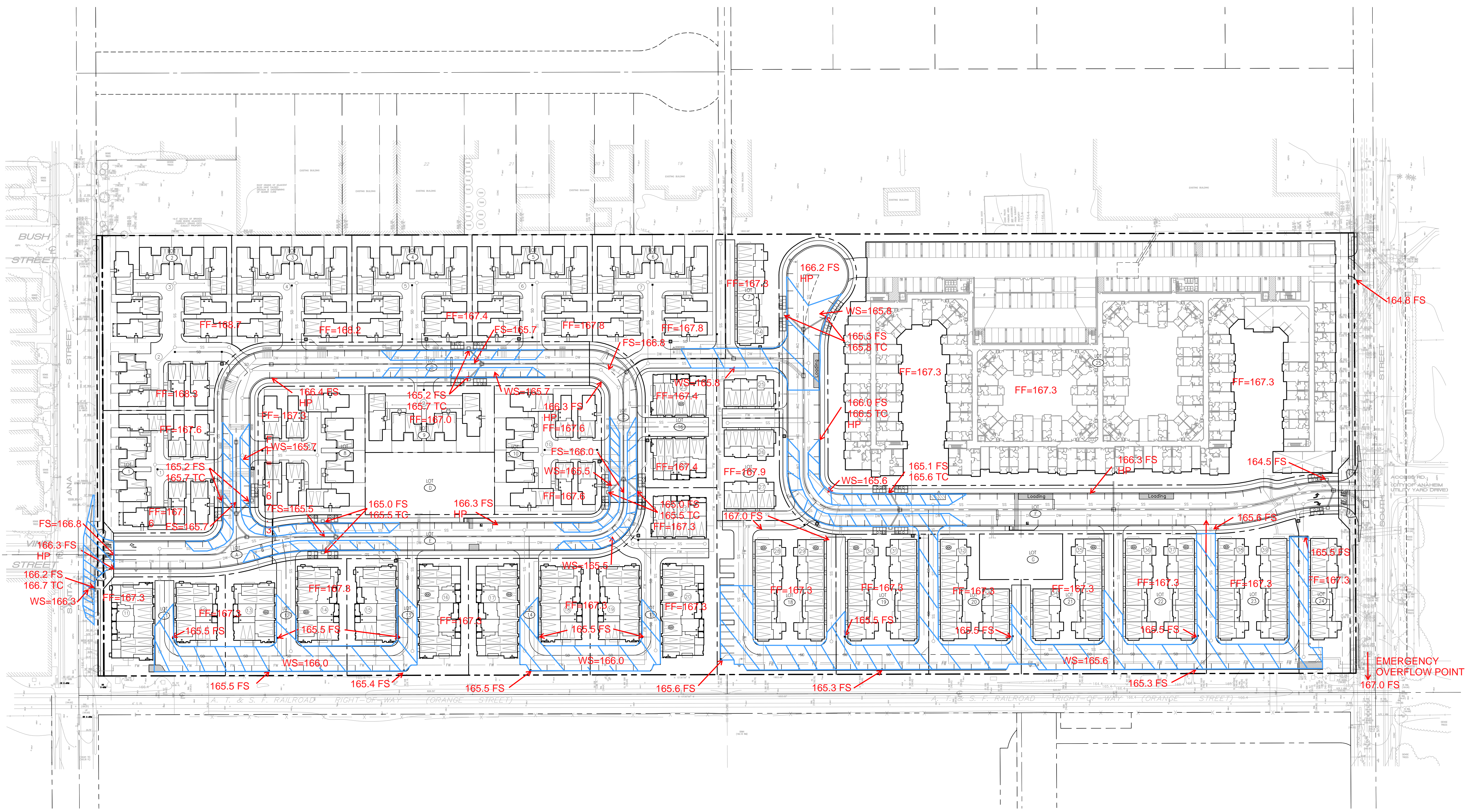
Composite Manning's n Equation: Lotter method

Manning's n: 0.0150

Result: Depth of 0.49 is below TC

APPENDIX 9

**PONDING EXHIBIT
(IN POCKET)**




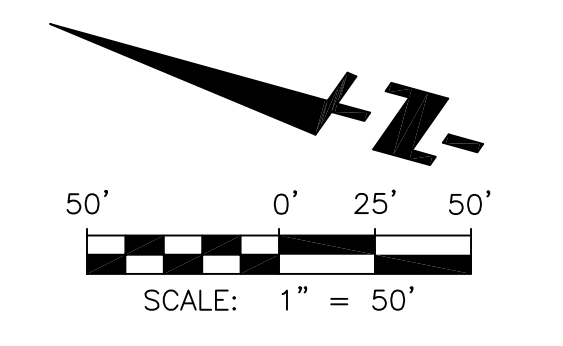
PONDING EXHIBIT

TECHNICAL SITE PLAN / EAST SOUTH STREET

THE SHOPOFF GROUP
ANAHEIM, CA

LEGEND

- PAD=166.0 BUILDING PAD
- WS=165.0 MAXIMUM WATER SURFACE ELEVATION ASSUMING PLUGGED INLET
-  LIMITS OF PONDING



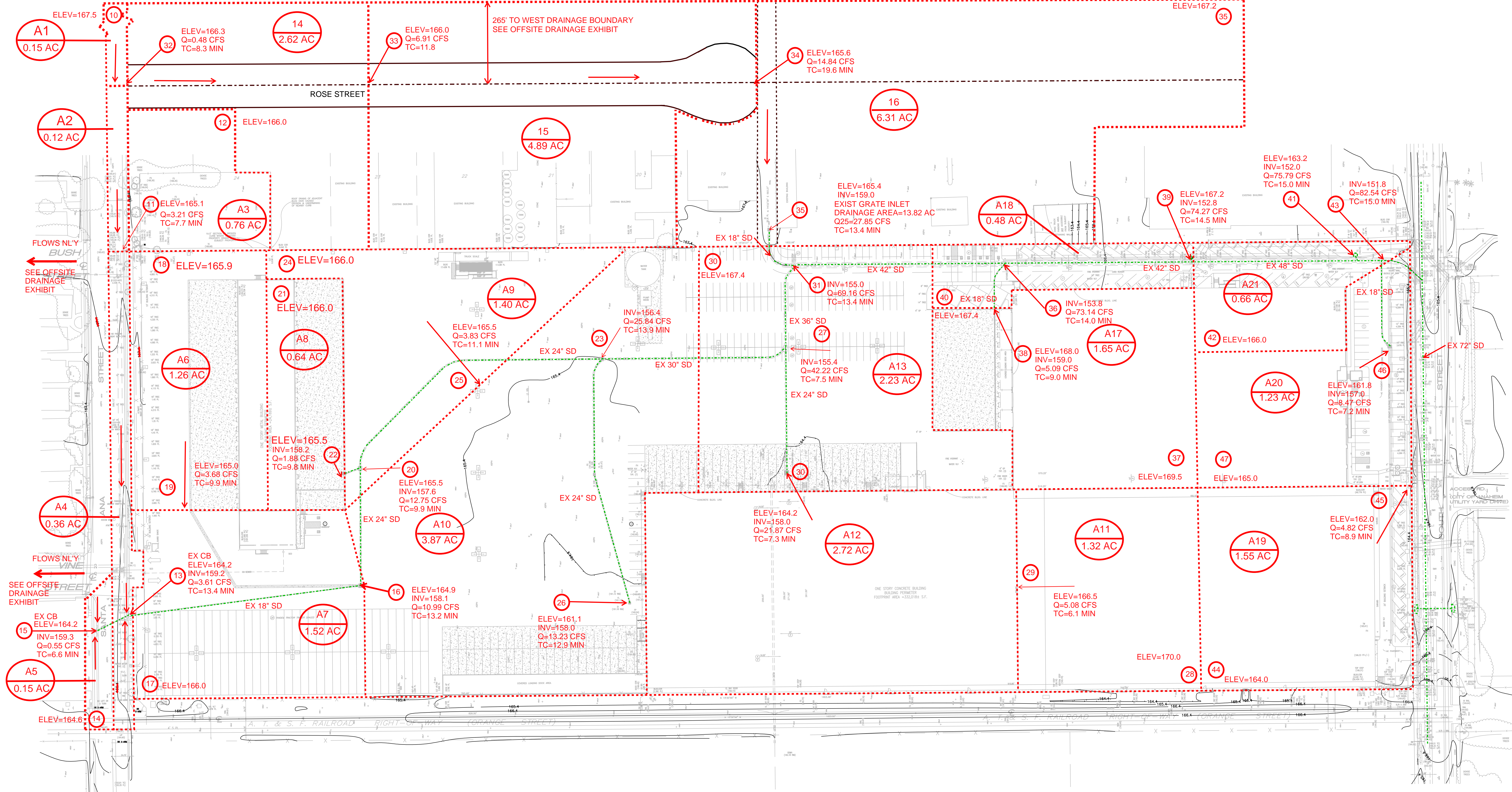
FUSCOE
ENGINEERING
16795 Von Karman, Suite 100, Irvine, California 92606
tel 949.474.1960 • fax 949.474.5315 • www.fuscoe.com

MAY 25, 2017

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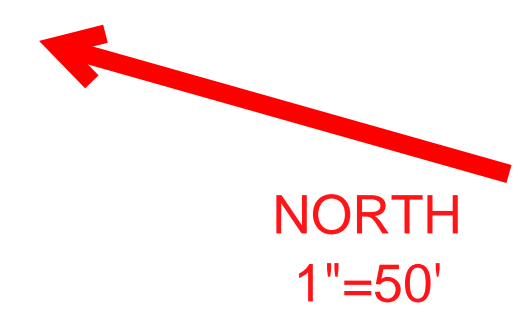
APPENDIX 10

EXISTING HYDROLOGY MAP (IN POCKET)



LEGEND

- DRAINAGE AREA BOUNDARY
- A1
0.22 AC AREA DESIGNATION
- 14 AREA (ACRES)
- 14 HYDROLOGY NODE
- Q=10.00 CFS
- TC=9.0 MIN
- 25 YEAR FLOW RATE
- TIME OF CONCENTRATION

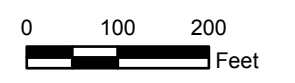
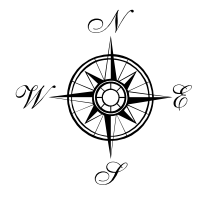


NOTE: ADD 2.4' TO ELEVATIONS SHOWN FOR CURRENT DATUM

EXISTING CONDITION HYDROLOGY MAP

APPENDIX 11

PROPOSED HYDROLOGY MAP SOUTH STREET (IN POCKET)



Legend

- Manholes
- Laterals
- Catch Basin
- Main Line
- Private Line
- Cal Trans Line
- County Line
- On-Site Private Line
- Other

Point Structure

- TYPE
- ANCHOR
 - BULKHEAD
 - CLEAN OUT
 - COLLAR
 - CONTERCEPTOR DRAIN
 - CURB OPENING
 - DESILTING BASIN
 - DETECTION BOX
 - DRAIN BOX
 - DROP INLET
 - ▽ FLOW THUR
 - GRATE INLET
 - HEADWALL
 - INLET
 - JUNCTION
 - OUTLET
 - PLUG
 - PUMP STATION
 - RISER
 - SAND-OIL INTERCEPTOR
 - STILLING WELL
 - ◇ TRANSITION
 - TRASH ENCLOSURE

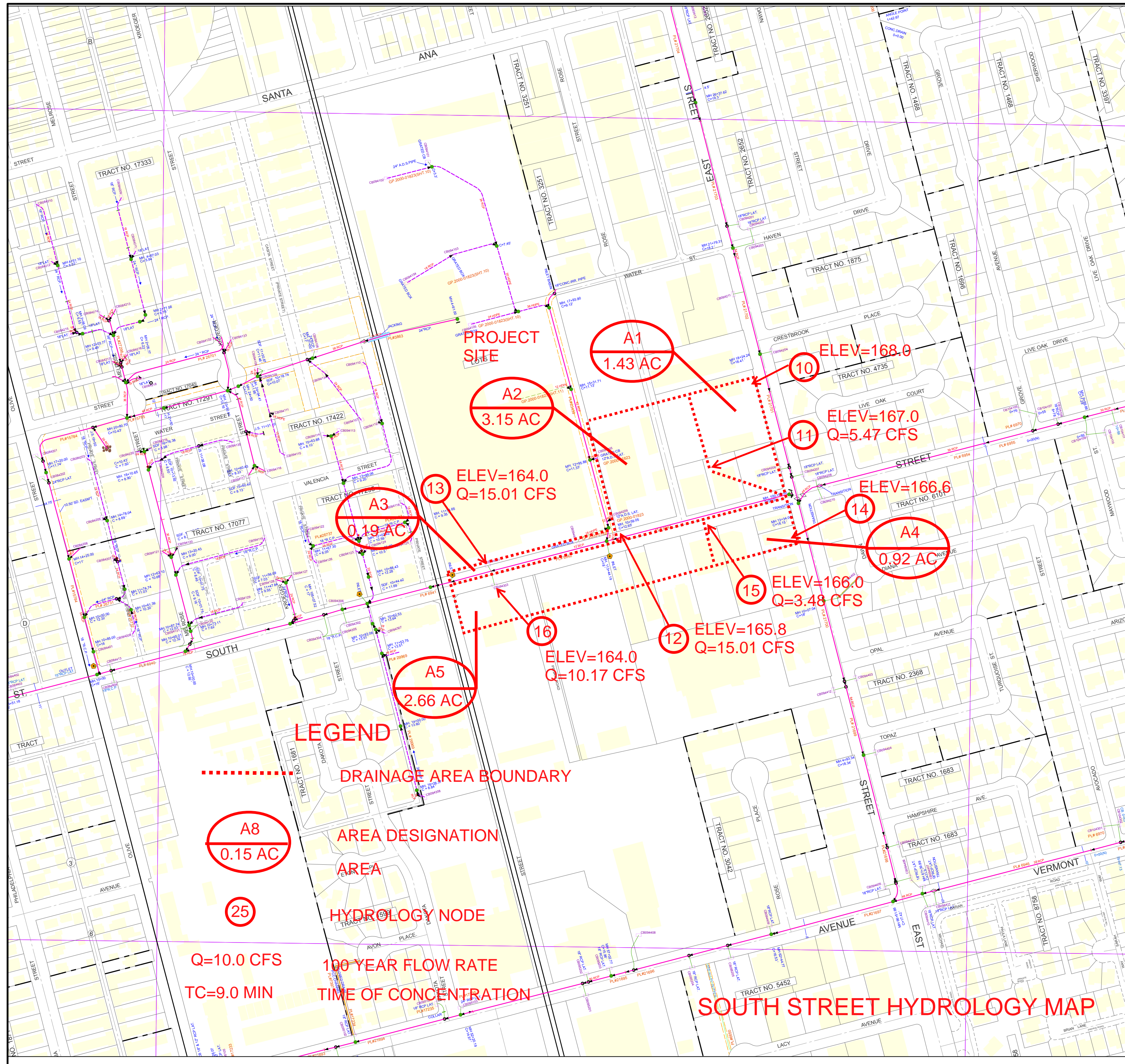
Building Footprints

83	93	103
84	94	104
85	95	105

STORM DRAIN

94

Portions of these plans contain critical infrastructure information. Unauthorized release or reproduction of these documents may result in civil penalty or other action. This map is not an official record of the city but is compiled from data furnished by private contractors and other sources. Location and sizes of geographic features and facilities shown are based on these sources. This information is furnished to any person strictly as a convenience and the city does not assume any responsibility for its accuracy or completeness.



A1
1.43 AC

10
ELEV=168.0

A2
3.15 AC

11
ELEV=167.0
Q=5.47 CFS

13
ELEV=164.0
Q=15.01 CFS

14
ELEV=166.6

A3
0.19 AC

15
ELEV=166.0
Q=3.48 CFS

A4
0.92 AC

16
ELEV=164.0
Q=10.17 CFS

12
ELEV=165.8
Q=15.01 CFS

A5
2.66 AC

A8
0.15 AC

25

Q=10.0 CFS
TC=9.0 MIN

100 YEAR FLOW RATE
TIME OF CONCENTRATION

SOUTH STREET HYDROLOGY MAP

LEGEND

— DRAINAGE AREA BOUNDARY

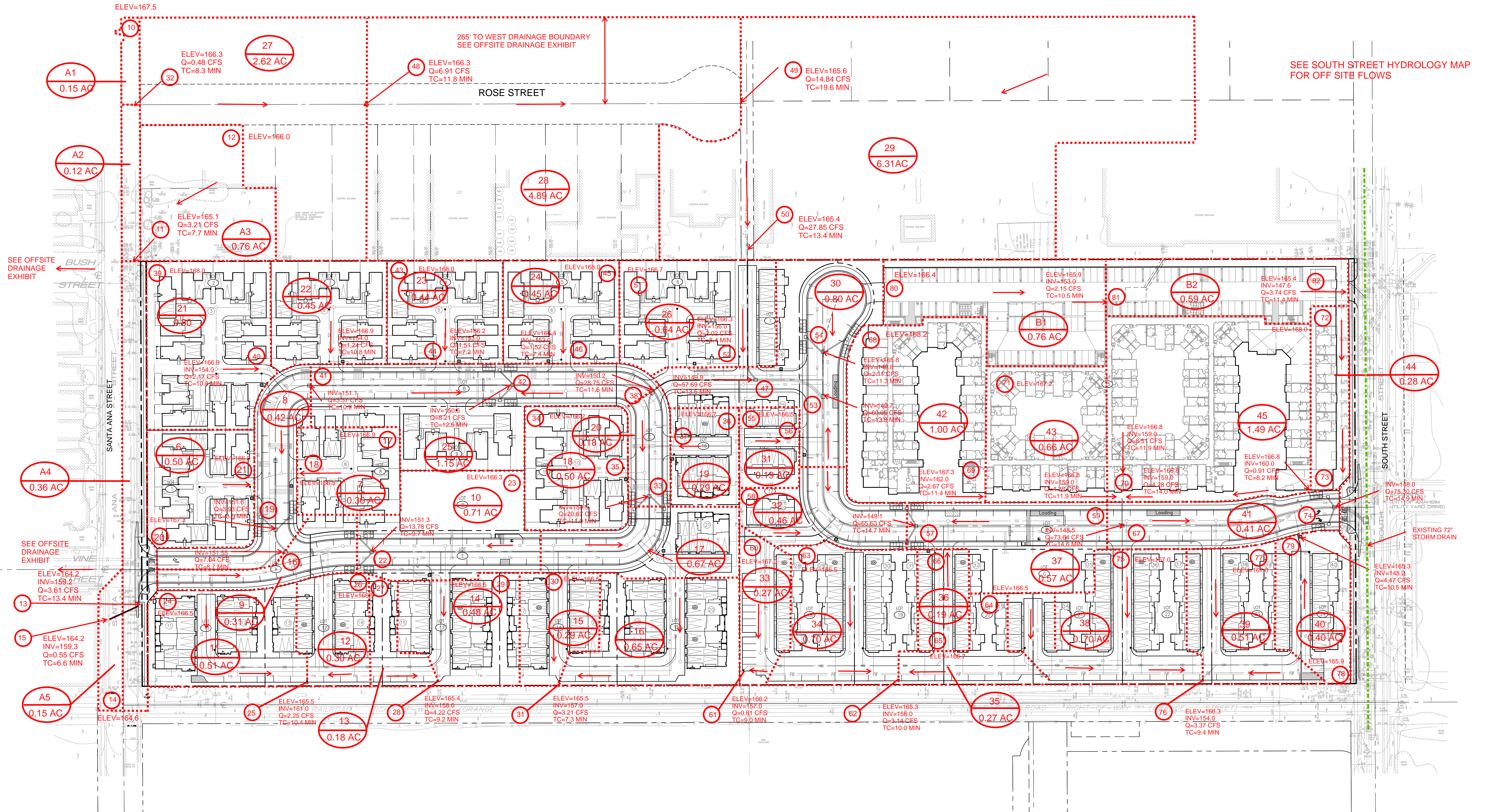
— AREA DESIGNATION

— AREA

— HYDROLOGY NODE

APPENDIX 12

PROPOSED HYDROLOGY MAP (IN POCKET)



SEE SOUTH STREET HYDROLOGY MAP FOR OFF SITE FLOWS

SEE OFFSITE DRAINAGE EXHIBIT

SEE OFFSITE DRAINAGE EXHIBIT

EXISTING 72" STORM DRAIN

- LEGEND**
- DRAINAGE AREA BOUNDARY
 - A8 AREA DESIGNATION
 - 25 HYDROLOGY NODE
 - Q=10.0 CFS 25 YEAR FLOW RATE
 - TC=9.0 MIN TIME OF CONCENTRATION

PROPOSED CONDITION HYDROLOGY MAP

TECHNICAL SITE PLAN / EAST SOUTH STREET

THE SHOPOFF GROUP
ANAHEIM, CA

