

Lincoln Colony Apartments Project

Appendix M

Infiltration Memorandum, August 2021

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soil PACIFIC INC.

Geotechnical and Environmental Services

Project No. A-7075up-20 August 3, 2021

Mr. Jerry Zomoredian 914 West Lincoln Avenue Anaheim, California

Log# OTH2021-01363

## Subject: On-Site Infiltration at Deeper Elevation Proposed Apartment Building 914 West Lincoln Avenue, Anaheim, California

Dear Sir;

Pursuant to the City of Anaheim Public Works Division inquiry date July 8, 2021, we are pleased to submit this clarification letter concerning the questioned items:

Item 3: This office was commissioned to perform on-site infiltration testing at the designated spot in order to verify the feasibility of the proposed dry well at depth of 10 to 35 feet below grade. This evaluation is based on the review of the primary WQMP design, as suggested by the project Civil Engineer. The location of the tested area is depicted on the attached plot plan.

On August 2, 2021, a bucket auger drill rig was used to perform a 6 inches boring to a depth of -35 feet as planned. Encountered soils at the proposed infiltration depth and were mainly granular sandy soil, interbedded with more silty matrix at depth of 19 to 26 feet. The encountered materials are not subject to have expansion potential. Subsurface soils within the proposed infiltration basin are mostly damp and consolidated.

The proposed infiltration basin will be placed and capped at a minimum of 10 feet away horizontally and vertically from the adjacent foundation (proposed or existing building or foundation if there is any) in order to not have an adverse effect on the proposed foundation and or public property.

On-site percolation testing was performed between -10 feet below the existing grade and -35 feet the depth of the proposed drywell for on-site infiltration purposes. As shown on the attached calculation sheet 372 inches of water column percolated in 65 minutes intervals. Used auger outward diameter is about 6 inches, however, the vibration auger drilling within sandy soils causes enlarging of the borehole diameter. Therefore, the borehole diameter of 7 inches has been assumed in the design calculation.

The calculated design rate of on-site percolation on a drywell at the suggested depth is 4.45 inches per hour. The Civil engineer in charge of WQMP will consider a factor safety into the design.

The proposed dry well at the proposed depth ranges that should be capped at -10 feet below the existing grade shall be filled by 3/4 single-size gravel. Prior to backfill, a 12 inches heavy schedule PCV perforated pipe will be placed in the middle of dry well. Gravel pack will be placed around the pipe the will receive the filtered sheet flow water subject to discharge through on-site infiltration.

Placing of the gravel must be observed by this office representative, any contaminated gravel (mixed by smaller size particles or soils) will be refused and or required to remove from the dry well. This action might have a heavy financial burden on the owner/developer, which the contractor must avoid. The upper 10 feet of the well will be capped and sealed to avoid infiltration above the 10 feet depth setback. The dry well shall be located a minimum of 10 feet away horizontally and vertically from any foundation and or public property. This distance shall be measured from the edge and top of the dry well.

The cap portion (upper 10 feet) will be backfilled using bentonite mixed slurry to prevent surficial infiltration during overflow. Two automatic submersible sump pumps with housing will be attached to the top of the infiltration basin /dry well at about -10 feet depth to flush out the overflow. The project civil engineer will connect the overflow pipe to a discharging pipe per his justification

If a prefabricated concrete box is used to shore the upper portion of the dry well during the construction of the well, then a surrounding void area of the box between the box and excavated wall must be sealed by 3 sacs slurry mix injection grouting.

The opportunity to be of service is appreciated. Should any question arise concerning this clarification letter please contact this office for further clarification.

Respectfully submitted,



Log of Sub-surface Explor			JSCS Letter			at Tama: D C	B-1(infiltration)			
	Wt:						Equipment Type: D-7700		Boring # B-1	
Bulk/Bag	Drop		Graphic	٦		Diameter	7"	Logged by:YK	Date:7/1/21	
Ring	SPT	PT Laboratory		4		Depth:	35 feet	G.water: - feet	Backfilled:Y	
Elev. (feet)	N	M%	D.D.			Desc	ription of	Earth Materials		
-  - 5-					SM SM/ SP	Damp, To	psoil. vn, brown fi	ne to medium grain	with some gravel. ned silty sand/ sand ,	
- - - - - - - - - - - - - - - - - - -					SM	Gray, to b	to Damp, N rown, fine to Dense and d	o medium grained s	sand, with some silt,	
20-								to medium garined ayers of sandy silt.		
- - 30 - - - 35						Borwn light with some s		to medium garine	d silty sand and sand	
- - - 40-					]	End of Bori	ng 35 feet. (	Ground water not e	ncountered.	
Log	depicts	conditi	ions at the	time	and lo	cation drill	ed.			
oil Pacific Inc.					Project Name:914 W. Lincoln, Anaheim, CA					
eotechnical and Environmental Services					Project Number: A-7075-inf-21					
				Re	port E	)ate:		Figure:		

## Porchet Method, Aka Inverse Borehole Method

∆T := 65	ne Interval 10 Minutes					
D0 := 144	itial Depth to Water, (inch)					
Df := 372	Final Depth to Water, (inch)					
Dr := 420	Total Depth of the Test Hole					
r := 7	Test Hole Redius, Inch					
H0 := Dr – D0	Initial height of water at the selected time interval					
$\begin{array}{l} H0=276\\ Hf:=Dr-Df \end{array}$	Final height of water at the selected time interval					
Hf = 48						
$\Delta H := H0 - Hf$ $\Delta H = 228$	$\Delta H = \Delta DC$ hange in height over the time interval					

$$Havg := \frac{(H0 + Hf)}{2}$$
$$Havg = 162$$

The Conversion Equation is used:

$$IR := \frac{\Delta H \cdot (60 \cdot r)}{\Delta T \cdot (r + 2Havg)}$$

IR = 4.451inch /Hour

914 Lincoln, Anaheim Project No. A-7075-20 B – 1(infiltration)



