OTH2021-01387

HYDROLOGY ANALYSIS
FOR A-TOWN TRACT 17703
LOT 5 – AREA F

City of Anaheim
County of Orange

PREPARED FOR:
LMC
A LENNAR COMPANY

PREPARED BY:
HUNSAKER & ASSOCIATES IRVINE, INC.
3 HUGHES
IRVINE, CA 92618
(949) 583-1010

OCTOBER 19, 2021

W.O. #3916-66

HYDROLOGY ANALYSIS
FOR A-TOWN TRACT 17703
LOT 5 – AREA F

City of Anaheim
County of Orange

PREPARED UNDER THE SUPERVISION OF:

Tu Trinh 10/19/2021

Tu Trinh, R.C.E. 71555 Date

TABLE OF CONTENTS

SECTION 1

A. PROJECT LOCATION

The project site is A-Town, Tract 17703, Lot 5 – Area F, in the City of Anaheim (City), County of Orange. The site is bounded by Park Street on the north, Union Street on the west, and existing commercial buildings on the southeast (see attached Vicinity Map for more information).

B. STUDY PURPOSE

This study is to accompany the submittal of a Water Quality report. This study is preliminary, determines the 100-year, 25-year, 10-year, and 2-year peak storm runoff produced from the project site as approved by the City in 2016 for the overall Tract 17703 .

C. METHODOLOGY

The methodology used to determine the peak discharges for A-Town is based upon the criteria set forth in the *Orange County Flood Control District Hydrology Manual* dated 1986 as incorporated in the Advanced Engineering Software (AES) "RATSC" Program. The hydrologic soil types were determined from the Hydrologic Classification of Soils map contained in the *Orange County Hydrology Manual.*

D. DISCUSSION

Per the current Site Plan, storm runoff produced from Lot 5 - Area F will drain in the same pattern as shown on the proposed hydrology map of the approved drainage report dated April 27, 2016.

Below is the summary table of flow rates of Proposed Area "F" in 2021 compared to the Approved Report in 2016. Note: at node #2, there will be a confluence with flows produced from the south portion of Proposed Area "E" (approved by the City on 7/16/2021).

	Proposed 2021	**Approved 2016**	
Node #	Flow/Area (10-yr)	**Flow/Area (10-yr)**	**Node #**
2	3.7 / 1.39 = 2.66 cfs/ac	**8.0 / 3.35 = 2.39 cfs/ac**	**63**
4	2.3 / 0.83 = 2.77 cfs/ac	**2.4 / 0.90 = 2.67 cfs/ac**	**67**
7	5.0 / 2.00 = 2.50 cfs/ac	**5.2 / 1.99 = 2.61 cfs/ac**	**69**

Flooding exhibit is also provided. All habitable structure is at least 1 foot above the ponding water surface elevation.

Per the Street Flow Calculations

4" rolled curb at 0.5% street slope can carry up to 3.0cfs. CBs in areas F1 & F2 are at sump locations, the flow rates shown should be divided by 2.
Flows on each side of node #2 is 2.85cfs < 3.0cfs
Flows on each side of node #4 is 1.75cfs < 3.0cfs

6" vertical curb at 1% can carry up to 11.46cfs. Flow at node #7 is only 7.8cfs.

Notes: The numbers shown above are Q100 to check flooding. Proposed Strom Drain lines will be designed for Q10 only.

First flush flows produced from the site are collected in the proposed inlets throughout the site, then being diverted in the proposed diversion structures to drain to proposed Modular Wetland Systems. Treated flows are then stored in the proposed Storage Vaults. Water quality structures are shown on the hydrology map with dimensions and are addressed in details in a separate WQMP report.

Street Flow Calculation

Designer: T.T.
Date: 8/3/2021

******* INPUT DATA *******

PKWY CROSS SLOPE = 0.02080
STREET CROSS SLOPE = 0.0200
HALF STREET WIDTH = 25.00'
HALF R/W WIDTH = 25.00'
GUTTER WIDTH = 1.50'
GUTTER HIKE = 0.083'
GUTTER LIP = 0.03125'
CURB BATTER = 1.000'
GUTTER HEIGHT = 0.33'
N (ROAD) = 0.015 N (PKWY) = 0.030

******* OUTPUT DATA *******

SLOPE OF CURB FACE = 3.030
SLOPE OF GUTTER = 0.055
Curb Height Less Than Crown

Depth	Flood Width	Area	Perim	Q/So^1/2
0.10	1.80	0.10	1.59	1.6
0.11	1.83	0.12	1.61	2.1
0.12	2.15	0.14	1.94	2.4
0.13	2.68	0.16	2.45	2.7
0.14	3.21	0.19	2.96	3.1
0.15	3.74	0.23	3.47	3.7
0.16	4.27	0.27	3.98	4.4
0.17	4.80	0.31	4.49	5.3
0.18	5.33	0.36	5.00	6.3
0.19	5.86	0.42	5.51	7.5
0.20	6.39	0.48	6.02	8.9
0.21	6.92	0.55	6.53	10.4
0.22	7.45	0.62	7.04	12.2
0.23	7.98	0.70	7.55	14.1
0.24	8.51	0.78	8.06	16.3
0.25	9.05	0.87	8.57	18.7
0.26	9.58	0.96	9.08	21.3
0.27	10.11	1.06	9.59	24.2
0.28	10.64	1.16	10.10	27.3
0.29	11.17	1.27	10.61	30.6
0.30	11.70	1.39	11.12	34.3
0.31	12.23	1.51	11.63	38.2
0.32	12.76	1.63	12.14	42.4

FLOW EXCEEDS RIGHT OF WAY

Q = So^1/2 x 42.4
= 0.005^1/2 x 42.4
= 3.0 cfs

GUTTER WIDTH	=	1.5 ft	CURB HEIGHT	=	6 in
GUTTER HIKE	=	1.5 in	PAVEMENT X-SLOPE	=	2%
LIP	=	0.03 ft	PARKWAY X-SLOPE	=	2%
			Manning's 'n'	=	0.015

Y	A	FW	Q/S^.5
0.16	0.14	1.5	2.7
0.17	0.16	2.0	2.8
0.18	0.18	2.5	3.0
0.19	0.21	3.0	3.4
0.20	0.24	3.5	3.9
0.21	0.28	4.0	4.5
0.22	0.32	4.5	5.3
0.23	0.37	5.0	6.3
0.24	0.42	5.5	7.3
0.25	0.48	6.0	8.6
0.26	0.54	6.5	10.0
0.27	0.61	7.0	11.6
0.28	0.68	7.5	13.3
0.29	0.76	8.0	15.3
0.30	0.84	8.5	17.5
0.31	0.93	9.0	19.8
0.32	1.02	9.5	22.4
0.33	1.12	10.0	25.2
0.34	1.22	10.5	28.3
0.35	1.33	11.0	31.5
0.36	1.44	11.5	35.1
0.37	1.56	12.0	38.9
0.38	1.68	12.5	42.9
0.39	1.81	**13.0**	47.2
FLASH C.L. 13 FT HALF STREET			
0.40	1.94	13.5	51.8
0.41	2.08	**14.0**	56.7
FLASH C.L. 14 FT HALF STREET			
0.42	2.22	14.5	61.9
0.43	2.37	15.0	67.4
0.44	2.52	15.5	73.1
0.45	2.68	16.0	79.2
0.46	2.84	16.5	85.7
0.47	3.01	17.0	92.4
0.48	3.18	17.5	99.5
0.49	3.36	**18.0**	106.9
FLASH C.L. 18 FT HALF STREET			
0.50	3.54	18.5	114.6
FLASH TOP OF CURB			
0.51	3.73	19.0	120.8
0.52	3.93	19.5	127.6
0.53	4.14	**20.0**	134.9
FLASH C.L. 20 FT HALF STREET			
0.54	4.36	20.5	142.8
0.55	4.59	21.0	151.2
0.56	4.83	21.5	160.2
0.57	5.08	**22.0**	169.8
FLASH C.L. 22 FT HALF STREET			
0.58	5.34	22.5	179.9
0.59	5.61	23.0	190.6
0.60	5.89	23.5	202.0
0.61	6.18	24.0	213.9
0.62	6.48	24.5	226.5
0.63	6.79	25.0	239.7
0.64	7.11	25.5	253.6
0.65	7.44	**26.0**	268.1
FLASH C.L. 26 FT HALF STREET			

Q = S^.5 x 114.6
= 0.01^.5 x 114.6
= 11.46 cfs

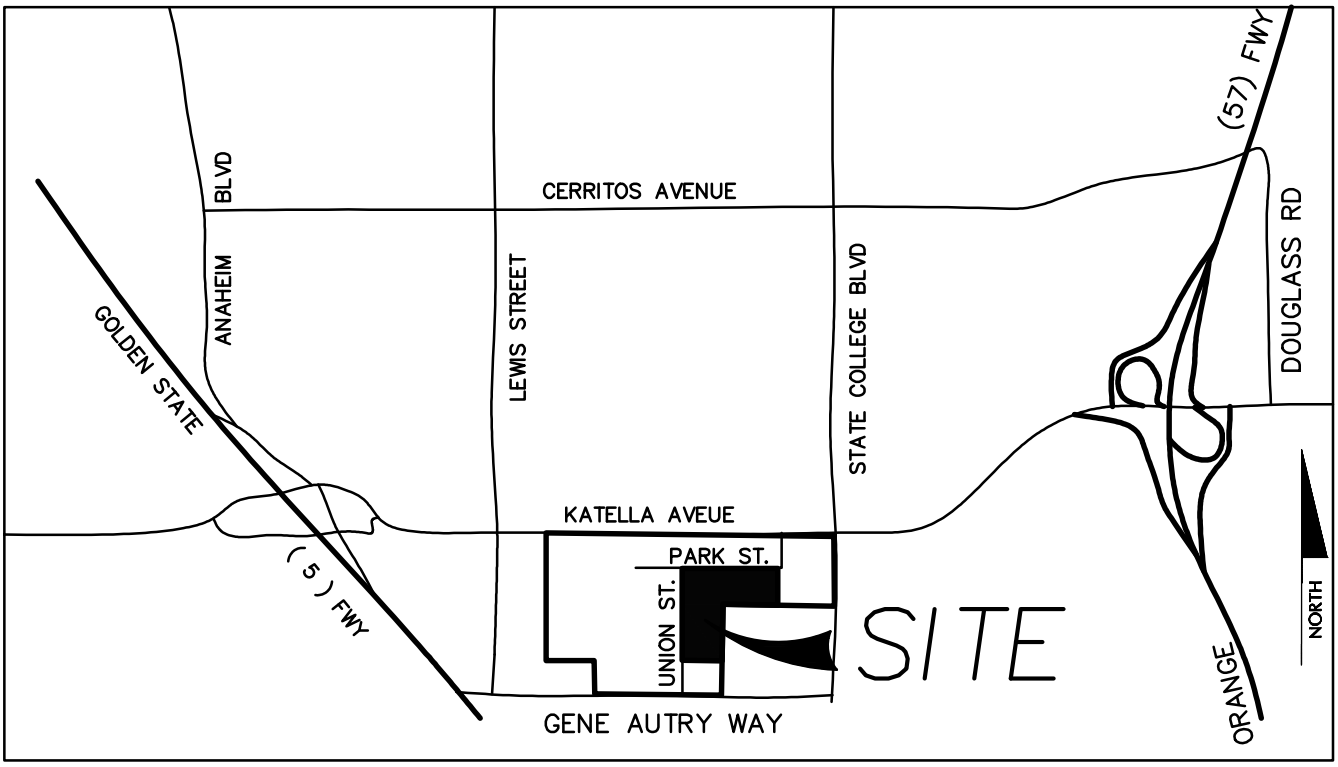
AFTER FLASHING, VALUES ARE APPLICABLE FOR THE NEXT SECTIONS

CITY OF ANAHEIM

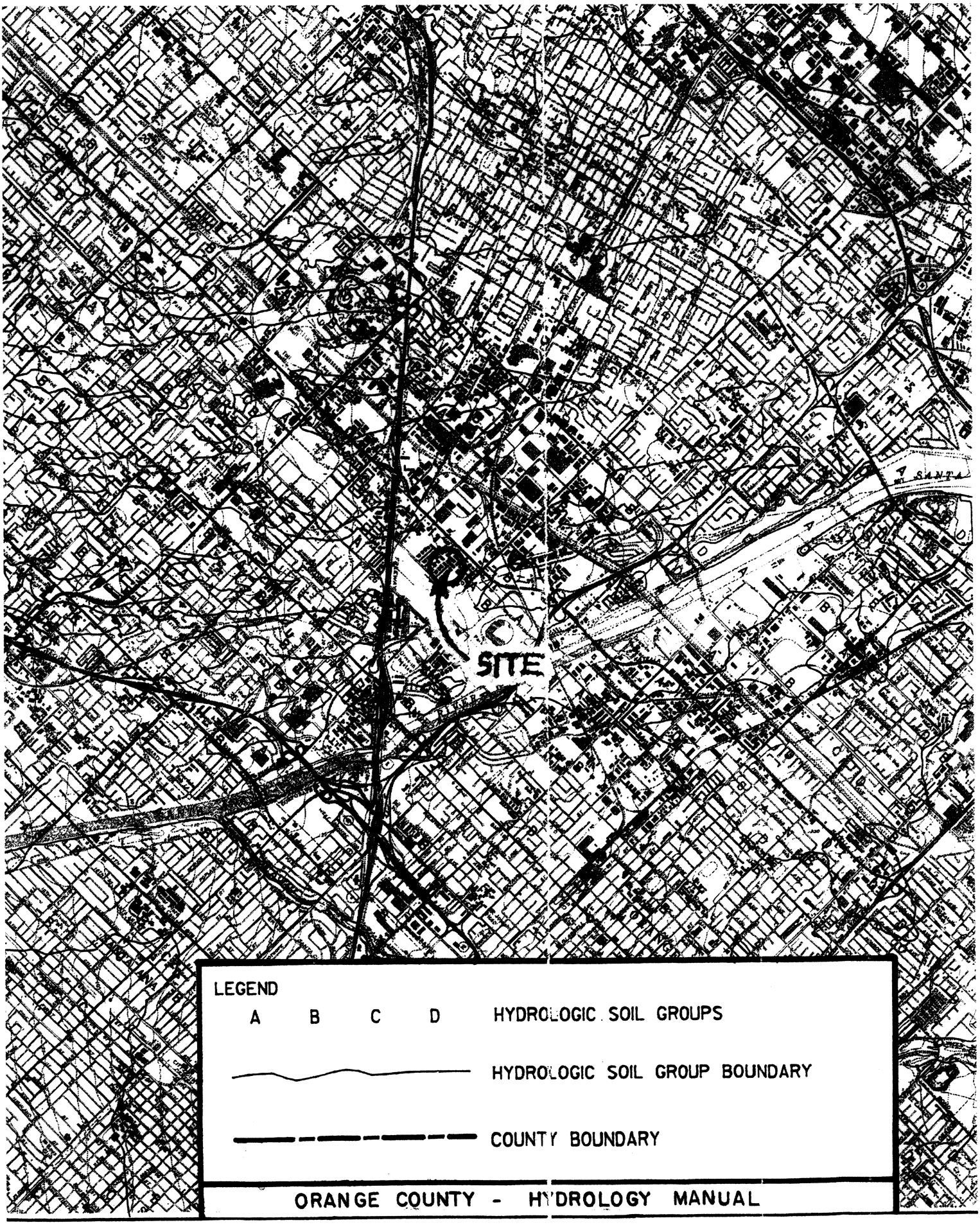
STREET FLOW CHART

TABLE

II-2-3



VICINITY MAP



ORANGE COUNTY - HYDROLOGY MANUAL

SECTION 2

HYDROLOGY CALCULATIONS

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******************************************************************************
             RATIONAL METHOD HYDROLOGY COMPUTER PROGRAM PACKAGE
            (Reference: 1986 ORANGE COUNTY HYDROLOGY CRITERION)
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                          Analysis prepared by:

                          HUNSAKER & ASSOCIATES
                                Irvine,Inc
                       Planning * Engineering * Surveying
          Three Hughes * Irvine, California 92618 * (949)583-1010

 ************************* DESCRIPTION OF STUDY *************************
* W.O. #3916-66, TRACT 17703, AREA 'F'                                  *
* 100-YR STUDY                                                          *
* PROPOSED CONDITION                                                    *
 ************************************************************************

  FILE NAME: 17703F.DAT
  TIME/DATE OF STUDY: 16:15 08/09/2021
==============================================================================
  USER SPECIFIED HYDROLOGY AND HYDRAULIC MODEL INFORMATION:
==============================================================================
                     --*TIME-OF-CONCENTRATION MODEL*--

  USER SPECIFIED STORM EVENT(YEAR) =  100.00
  SPECIFIED MINIMUM PIPE SIZE(INCH) =  18.00
  SPECIFIED PERCENT OF GRADIENTS(DECIMAL) TO USE FOR FRICTION SLOPE = 0.90
  *DATA BANK RAINFALL USED*
  *ANTECEDENT MOISTURE CONDITION (AMC) III ASSUMED FOR RATIONAL METHOD*

  *USER-DEFINED STREET-SECTIONS FOR COUPLED PIPEFLOW AND STREETFLOW MODEL*
     HALF-  CROWN TO   STREET-CROSSFALL:   CURB  GUTTER-GEOMETRIES:  MANNING
     WIDTH  CROSSFALL  IN-  / OUT-/PARK-  HEIGHT  WIDTH  LIP   HIKE  FACTOR
NO.   (FT)     (FT)    SIDE / SIDE/ WAY    (FT)    (FT)  (FT)  (FT)    (n)
===  =====  =========  ==================  ======  ===== ====== ===== =======
  1   30.0     20.0    0.018/0.018/0.020   0.67    2.00 0.0313 0.167 0.0150

  GLOBAL STREET FLOW-DEPTH CONSTRAINTS:
    1. Relative Flow-Depth =  0.00 FEET
       as (Maximum Allowable Street Flow Depth) - (Top-of-Curb)
    2. (Depth)*(Velocity) Constraint =  6.0 (FT*FT/S)
  *SIZE PIPE WITH A FLOW CAPACITY GREATER THAN
   OR EQUAL TO THE UPSTREAM TRIBUTARY PIPE.*
  *USER-SPECIFIED MINIMUM TOPOGRAPHIC SLOPE ADJUSTMENT NOT SELECTED

******************************************************************************
  FLOW PROCESS FROM NODE      1.00 TO NODE      2.00 IS CODE =  21
------------------------------------------------------------------------------
  >>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
  >>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<
==============================================================================
  INITIAL SUBAREA FLOW-LENGTH(FEET) =   280.00
  ELEVATION DATA: UPSTREAM(FEET) =    154.30  DOWNSTREAM(FEET) =    152.20

  Tc = K*[(LENGTH** 3.00)/(ELEVATION CHANGE)]**0.20
  SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) =    8.211
  * 100 YEAR RAINFALL INTENSITY(INCH/HR) =  4.657
  SUBAREA Tc AND LOSS RATE DATA(AMC III):
   DEVELOPMENT TYPE/      SCS SOIL   AREA      Fp         Ap     SCS   Tc
       LAND USE            GROUP   (ACRES)  (INCH/HR)  (DECIMAL)  CN  (MIN.)
  RESIDENTIAL
  "11+ DWELLINGS/ACRE"       A        1.39      0.40     0.200    52    8.21
  SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) =  0.40
  SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap =  0.200
  SUBAREA RUNOFF(CFS) =      5.73
  TOTAL AREA(ACRES) =      1.39   PEAK FLOW RATE(CFS) =      5.73

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 FLOW PROCESS FROM NODE      3.00 TO NODE      4.00 IS CODE =  21
----------------------------------------------------------------------------
 >>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
 >>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<
============================================================================
 INITIAL SUBAREA FLOW-LENGTH(FEET) =   255.00
 ELEVATION DATA: UPSTREAM(FEET) =    154.30  DOWNSTREAM(FEET) =    152.40

 Tc = K*[(LENGTH** 3.00)/(ELEVATION CHANGE)]**0.20
 SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) =    7.920
 * 100 YEAR RAINFALL INTENSITY(INCH/HR) =  4.754
 SUBAREA Tc AND LOSS RATE DATA(AMC III):
  DEVELOPMENT TYPE/      SCS SOIL   AREA      Fp         Ap     SCS   Tc
      LAND USE            GROUP   (ACRES)  (INCH/HR)  (DECIMAL)  CN  (MIN.)
 RESIDENTIAL
 "11+ DWELLINGS/ACRE"       A        0.83      0.40     0.200    52    7.92
 SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) =  0.40
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap =  0.200
 SUBAREA RUNOFF(CFS) =      3.49
 TOTAL AREA(ACRES) =      0.83   PEAK FLOW RATE(CFS) =      3.49

****************************************************************************
 FLOW PROCESS FROM NODE      5.00 TO NODE      6.00 IS CODE =  21
----------------------------------------------------------------------------
 >>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
 >>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<
============================================================================
 INITIAL SUBAREA FLOW-LENGTH(FEET) =   250.00
 ELEVATION DATA: UPSTREAM(FEET) =    154.30  DOWNSTREAM(FEET) =    152.30

 Tc = K*[(LENGTH** 3.00)/(ELEVATION CHANGE)]**0.20
 SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) =    7.746
 * 100 YEAR RAINFALL INTENSITY(INCH/HR) =  4.815
 SUBAREA Tc AND LOSS RATE DATA(AMC III):
  DEVELOPMENT TYPE/      SCS SOIL   AREA      Fp         Ap     SCS   Tc
      LAND USE            GROUP   (ACRES)  (INCH/HR)  (DECIMAL)  CN  (MIN.)
 RESIDENTIAL
 "11+ DWELLINGS/ACRE"       A        0.68      0.40     0.200    52    7.75
 SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) =  0.40
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap =  0.200
 SUBAREA RUNOFF(CFS) =      2.90
 TOTAL AREA(ACRES) =      0.68   PEAK FLOW RATE(CFS) =      2.90

****************************************************************************
 FLOW PROCESS FROM NODE      6.00 TO NODE      7.00 IS CODE =  51
----------------------------------------------------------------------------
 >>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<<
 >>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<<
============================================================================
 ELEVATION DATA: UPSTREAM(FEET) =    152.30  DOWNSTREAM(FEET) =    149.30
 CHANNEL LENGTH THRU SUBAREA(FEET) =   320.00   CHANNEL SLOPE =  0.0094
 CHANNEL BASE(FEET) =    0.00   "Z" FACTOR =   4.000
 MANNING'S FACTOR = 0.015   MAXIMUM DEPTH(FEET) =   1.00
 * 100 YEAR RAINFALL INTENSITY(INCH/HR) =  4.406
 SUBAREA LOSS RATE DATA(AMC III):
  DEVELOPMENT TYPE/      SCS SOIL   AREA      Fp         Ap     SCS
      LAND USE            GROUP   (ACRES)  (INCH/HR)  (DECIMAL)  CN
 RESIDENTIAL
 "11+ DWELLINGS/ACRE"       A        1.32      0.40     0.200    52
 SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) =  0.40
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap =  0.200
 TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) =       5.47
 TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) =   4.11
 AVERAGE FLOW DEPTH(FEET) =   0.58   TRAVEL TIME(MIN.) =   1.30
 Tc(MIN.) =    9.04
 SUBAREA AREA(ACRES) =     1.32       SUBAREA RUNOFF(CFS) =    5.14
 EFFECTIVE AREA(ACRES) =      2.00     AREA-AVERAGED Fm(INCH/HR) =   0.08
 AREA-AVERAGED Fp(INCH/HR) =   0.40  AREA-AVERAGED Ap =   0.20
 TOTAL AREA(ACRES) =        2.0         PEAK FLOW RATE(CFS) =       7.79

 END OF SUBAREA CHANNEL FLOW HYDRAULICS:
```

```
  DEPTH(FEET) =  0.66   FLOW VELOCITY(FEET/SEC.) =   4.48
  LONGEST FLOWPATH FROM NODE      5.00 TO NODE      7.00 =     570.00 FEET.
============================================================================
  END OF STUDY SUMMARY:
  TOTAL AREA(ACRES)     =        2.0  TC(MIN.) =      9.04
  EFFECTIVE AREA(ACRES) =      2.00  AREA-AVERAGED Fm(INCH/HR)=  0.08
  AREA-AVERAGED Fp(INCH/HR) =  0.40  AREA-AVERAGED Ap = 0.200
  PEAK FLOW RATE(CFS)   =       7.79
============================================================================
============================================================================
  END OF RATIONAL METHOD ANALYSIS
```

```
************************************************************************
            RATIONAL METHOD HYDROLOGY COMPUTER PROGRAM PACKAGE
           (Reference: 1986 ORANGE COUNTY HYDROLOGY CRITERION)
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             Ver. 23.0  Release Date: 07/01/2016  License ID 1239

                          Analysis prepared by:

                          HUNSAKER & ASSOCIATES
                                Irvine,Inc
                      Planning * Engineering * Surveying
          Three Hughes * Irvine, California 92618 * (949)583-1010

 ************************* DESCRIPTION OF STUDY *************************
* W.O. #3916-66, TRACT 17703, AREA 'F'                                  *
* 25-YR STUDY                                                           *
* PROPOSED CONDITION                                                    *
 ************************************************************************

  FILE NAME: 17703F.DAT
  TIME/DATE OF STUDY: 16:17 08/09/2021
========================================================================
  USER SPECIFIED HYDROLOGY AND HYDRAULIC MODEL INFORMATION:
========================================================================
                    --*TIME-OF-CONCENTRATION MODEL*--

  USER SPECIFIED STORM EVENT(YEAR) =   25.00
  SPECIFIED MINIMUM PIPE SIZE(INCH) =  18.00
  SPECIFIED PERCENT OF GRADIENTS(DECIMAL) TO USE FOR FRICTION SLOPE = 0.90
  *DATA BANK RAINFALL USED*
  *ANTECEDENT MOISTURE CONDITION (AMC) II ASSUMED FOR RATIONAL METHOD*

  *USER-DEFINED STREET-SECTIONS FOR COUPLED PIPEFLOW AND STREETFLOW MODEL*
     HALF-  CROWN TO   STREET-CROSSFALL:   CURB  GUTTER-GEOMETRIES:  MANNING
     WIDTH  CROSSFALL  IN-  / OUT-/PARK-  HEIGHT  WIDTH  LIP   HIKE  FACTOR
NO.   (FT)     (FT)    SIDE / SIDE/ WAY    (FT)    (FT)  (FT)  (FT)    (n)
===  =====  =========  ==================  ======  ===== ====== ===== =======
  1   30.0     20.0    0.018/0.018/0.020   0.67    2.00 0.0312 0.167 0.0150

  GLOBAL STREET FLOW-DEPTH CONSTRAINTS:
    1. Relative Flow-Depth =  0.00 FEET
       as (Maximum Allowable Street Flow Depth) - (Top-of-Curb)
    2. (Depth)*(Velocity) Constraint =  6.0 (FT*FT/S)
  *SIZE PIPE WITH A FLOW CAPACITY GREATER THAN
   OR EQUAL TO THE UPSTREAM TRIBUTARY PIPE.*
  *USER-SPECIFIED MINIMUM TOPOGRAPHIC SLOPE ADJUSTMENT NOT SELECTED

************************************************************************
  FLOW PROCESS FROM NODE      1.00 TO NODE      2.00 IS CODE =  21
------------------------------------------------------------------------
  >>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
  >>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<
========================================================================
  INITIAL SUBAREA FLOW-LENGTH(FEET) =   280.00
  ELEVATION DATA: UPSTREAM(FEET) =    154.30  DOWNSTREAM(FEET) =    152.20

  Tc = K*[(LENGTH** 3.00)/(ELEVATION CHANGE)]**0.20
  SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) =    8.211
  *  25 YEAR RAINFALL INTENSITY(INCH/HR) =  3.643
  SUBAREA Tc AND LOSS RATE DATA(AMC  II):
   DEVELOPMENT TYPE/      SCS SOIL   AREA      Fp         Ap     SCS   Tc
       LAND USE            GROUP   (ACRES)  (INCH/HR)  (DECIMAL)  CN  (MIN.)
  RESIDENTIAL
  "11+ DWELLINGS/ACRE"       A        1.39      0.40     0.200    32    8.21
  SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) =  0.40
  SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap =  0.200
  SUBAREA RUNOFF(CFS) =      4.46
  TOTAL AREA(ACRES) =      1.39   PEAK FLOW RATE(CFS) =      4.46

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```
 FLOW PROCESS FROM NODE      3.00 TO NODE      4.00 IS CODE =  21
----------------------------------------------------------------------------
 >>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
 >>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<
============================================================================
 INITIAL SUBAREA FLOW-LENGTH(FEET) =   255.00
 ELEVATION DATA: UPSTREAM(FEET) =    154.30  DOWNSTREAM(FEET) =    152.40

 Tc = K*[(LENGTH** 3.00)/(ELEVATION CHANGE)]**0.20
 SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) =    7.920
 *  25 YEAR RAINFALL INTENSITY(INCH/HR) =  3.718
 SUBAREA Tc AND LOSS RATE DATA(AMC  II):
  DEVELOPMENT TYPE/      SCS SOIL   AREA      Fp         Ap     SCS   Tc
      LAND USE            GROUP   (ACRES)  (INCH/HR)  (DECIMAL)  CN  (MIN.)
 RESIDENTIAL
 "11+ DWELLINGS/ACRE"       A        0.83      0.40     0.200    32    7.92
 SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) =  0.40
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap =  0.200
 SUBAREA RUNOFF(CFS) =      2.72
 TOTAL AREA(ACRES) =      0.83   PEAK FLOW RATE(CFS) =      2.72

****************************************************************************
 FLOW PROCESS FROM NODE      5.00 TO NODE      6.00 IS CODE =  21
----------------------------------------------------------------------------
 >>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
 >>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<
============================================================================
 INITIAL SUBAREA FLOW-LENGTH(FEET) =   250.00
 ELEVATION DATA: UPSTREAM(FEET) =    154.30  DOWNSTREAM(FEET) =    152.30

 Tc = K*[(LENGTH** 3.00)/(ELEVATION CHANGE)]**0.20
 SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) =    7.746
 *  25 YEAR RAINFALL INTENSITY(INCH/HR) =  3.765
 SUBAREA Tc AND LOSS RATE DATA(AMC  II):
  DEVELOPMENT TYPE/      SCS SOIL   AREA      Fp         Ap     SCS   Tc
      LAND USE            GROUP   (ACRES)  (INCH/HR)  (DECIMAL)  CN  (MIN.)
 RESIDENTIAL
 "11+ DWELLINGS/ACRE"       A        0.68      0.40     0.200    32    7.75
 SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) =  0.40
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap =  0.200
 SUBAREA RUNOFF(CFS) =      2.26
 TOTAL AREA(ACRES) =      0.68   PEAK FLOW RATE(CFS) =      2.26

****************************************************************************
 FLOW PROCESS FROM NODE      6.00 TO NODE      7.00 IS CODE =  51
----------------------------------------------------------------------------
 >>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<<
 >>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<<
============================================================================
 ELEVATION DATA: UPSTREAM(FEET) =    152.30  DOWNSTREAM(FEET) =    149.30
 CHANNEL LENGTH THRU SUBAREA(FEET) =   320.00   CHANNEL SLOPE =  0.0094
 CHANNEL BASE(FEET) =    0.00   "Z" FACTOR =   4.000
 MANNING'S FACTOR = 0.015   MAXIMUM DEPTH(FEET) =   1.00
 *  25 YEAR RAINFALL INTENSITY(INCH/HR) =  3.432
 SUBAREA LOSS RATE DATA(AMC  II):
  DEVELOPMENT TYPE/      SCS SOIL   AREA      Fp         Ap     SCS
      LAND USE            GROUP   (ACRES)  (INCH/HR)  (DECIMAL)  CN
 RESIDENTIAL
 "11+ DWELLINGS/ACRE"       A        1.32      0.40     0.200    32
 SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) =  0.40
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap =  0.200
 TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) =       4.25
 TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) =   3.88
 AVERAGE FLOW DEPTH(FEET) =   0.52   TRAVEL TIME(MIN.) =   1.38
 Tc(MIN.) =    9.12
 SUBAREA AREA(ACRES) =     1.32       SUBAREA RUNOFF(CFS) =    3.98
 EFFECTIVE AREA(ACRES) =      2.00     AREA-AVERAGED Fm(INCH/HR) =   0.08
 AREA-AVERAGED Fp(INCH/HR) =   0.40  AREA-AVERAGED Ap =   0.20
 TOTAL AREA(ACRES) =        2.0         PEAK FLOW RATE(CFS) =       6.03

 END OF SUBAREA CHANNEL FLOW HYDRAULICS:
```

```
  DEPTH(FEET) =  0.60   FLOW VELOCITY(FEET/SEC.) =   4.22
  LONGEST FLOWPATH FROM NODE      5.00 TO NODE      7.00 =     570.00 FEET.
============================================================================
  END OF STUDY SUMMARY:
  TOTAL AREA(ACRES)     =        2.0  TC(MIN.) =      9.12
  EFFECTIVE AREA(ACRES) =      2.00  AREA-AVERAGED Fm(INCH/HR)=  0.08
  AREA-AVERAGED Fp(INCH/HR) =  0.40  AREA-AVERAGED Ap = 0.200
  PEAK FLOW RATE(CFS)   =       6.03
============================================================================
============================================================================
  END OF RATIONAL METHOD ANALYSIS
```

```
******************************************************************************
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                          Analysis prepared by:

                          HUNSAKER & ASSOCIATES
                                Irvine,Inc
                       Planning * Engineering * Surveying
          Three Hughes * Irvine, California 92618 * (949)583-1010

 ************************* DESCRIPTION OF STUDY *************************
* W.O. #3916-66, TRACT 17703, AREA 'F'                                  *
* 10-YR STUDY                                                           *
* PROPOSED CONDITION                                                    *
 ************************************************************************

  FILE NAME: 17703F.DAT
  TIME/DATE OF STUDY: 16:18 08/09/2021
==============================================================================
  USER SPECIFIED HYDROLOGY AND HYDRAULIC MODEL INFORMATION:
==============================================================================
                     --*TIME-OF-CONCENTRATION MODEL*--

  USER SPECIFIED STORM EVENT(YEAR) =   10.00
  SPECIFIED MINIMUM PIPE SIZE(INCH) =  18.00
  SPECIFIED PERCENT OF GRADIENTS(DECIMAL) TO USE FOR FRICTION SLOPE = 0.90
  *DATA BANK RAINFALL USED*
  *ANTECEDENT MOISTURE CONDITION (AMC) II ASSUMED FOR RATIONAL METHOD*

  *USER-DEFINED STREET-SECTIONS FOR COUPLED PIPEFLOW AND STREETFLOW MODEL*
     HALF-  CROWN TO   STREET-CROSSFALL:   CURB  GUTTER-GEOMETRIES:  MANNING
     WIDTH  CROSSFALL  IN-  / OUT-/PARK-  HEIGHT  WIDTH  LIP   HIKE  FACTOR
NO.   (FT)     (FT)    SIDE / SIDE/ WAY    (FT)    (FT)  (FT)  (FT)    (n)
===  =====  =========  ==================  ======  ===== ====== ===== =======
  1   30.0     20.0    0.018/0.018/0.020   0.67    2.00 0.0312 0.167 0.0150

  GLOBAL STREET FLOW-DEPTH CONSTRAINTS:
    1. Relative Flow-Depth =  0.00 FEET
       as (Maximum Allowable Street Flow Depth) - (Top-of-Curb)
    2. (Depth)*(Velocity) Constraint =  6.0 (FT*FT/S)
  *SIZE PIPE WITH A FLOW CAPACITY GREATER THAN
   OR EQUAL TO THE UPSTREAM TRIBUTARY PIPE.*
  *USER-SPECIFIED MINIMUM TOPOGRAPHIC SLOPE ADJUSTMENT NOT SELECTED

******************************************************************************
  FLOW PROCESS FROM NODE      1.00 TO NODE      2.00 IS CODE =  21
------------------------------------------------------------------------------
  >>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
  >>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<
==============================================================================
  INITIAL SUBAREA FLOW-LENGTH(FEET) =   280.00
  ELEVATION DATA: UPSTREAM(FEET) =    154.30  DOWNSTREAM(FEET) =    152.20

  Tc = K*[(LENGTH** 3.00)/(ELEVATION CHANGE)]**0.20
  SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) =    8.211
  *  10 YEAR RAINFALL INTENSITY(INCH/HR) =  3.055
  SUBAREA Tc AND LOSS RATE DATA(AMC  II):
   DEVELOPMENT TYPE/      SCS SOIL   AREA      Fp         Ap     SCS   Tc
       LAND USE            GROUP   (ACRES)  (INCH/HR)  (DECIMAL)  CN  (MIN.)
  RESIDENTIAL
  "11+ DWELLINGS/ACRE"       A        1.39      0.40     0.200    32    8.21
  SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) =  0.40
  SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap =  0.200
  SUBAREA RUNOFF(CFS) =      3.72
  TOTAL AREA(ACRES) =      1.39   PEAK FLOW RATE(CFS) =      3.72

******************************************************************************
```

```
 FLOW PROCESS FROM NODE      3.00 TO NODE      4.00 IS CODE =  21
----------------------------------------------------------------------------
 >>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
 >>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<
============================================================================
 INITIAL SUBAREA FLOW-LENGTH(FEET) =   255.00
 ELEVATION DATA: UPSTREAM(FEET) =    154.30  DOWNSTREAM(FEET) =    152.40

 Tc = K*[(LENGTH** 3.00)/(ELEVATION CHANGE)]**0.20
 SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) =    7.920
 *  10 YEAR RAINFALL INTENSITY(INCH/HR) =  3.119
 SUBAREA Tc AND LOSS RATE DATA(AMC  II):
  DEVELOPMENT TYPE/      SCS SOIL   AREA      Fp         Ap     SCS   Tc
      LAND USE            GROUP   (ACRES)  (INCH/HR)  (DECIMAL)  CN  (MIN.)
 RESIDENTIAL
 "11+ DWELLINGS/ACRE"       A        0.83      0.40     0.200    32    7.92
 SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) =  0.40
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap =  0.200
 SUBAREA RUNOFF(CFS) =      2.27
 TOTAL AREA(ACRES) =      0.83   PEAK FLOW RATE(CFS) =      2.27

****************************************************************************
 FLOW PROCESS FROM NODE      5.00 TO NODE      6.00 IS CODE =  21
----------------------------------------------------------------------------
 >>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
 >>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<
============================================================================
 INITIAL SUBAREA FLOW-LENGTH(FEET) =   250.00
 ELEVATION DATA: UPSTREAM(FEET) =    154.30  DOWNSTREAM(FEET) =    152.30

 Tc = K*[(LENGTH** 3.00)/(ELEVATION CHANGE)]**0.20
 SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) =    7.746
 *  10 YEAR RAINFALL INTENSITY(INCH/HR) =  3.159
 SUBAREA Tc AND LOSS RATE DATA(AMC  II):
  DEVELOPMENT TYPE/      SCS SOIL   AREA      Fp         Ap     SCS   Tc
      LAND USE            GROUP   (ACRES)  (INCH/HR)  (DECIMAL)  CN  (MIN.)
 RESIDENTIAL
 "11+ DWELLINGS/ACRE"       A        0.68      0.40     0.200    32    7.75
 SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) =  0.40
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap =  0.200
 SUBAREA RUNOFF(CFS) =      1.88
 TOTAL AREA(ACRES) =      0.68   PEAK FLOW RATE(CFS) =      1.88

****************************************************************************
 FLOW PROCESS FROM NODE      6.00 TO NODE      7.00 IS CODE =  51
----------------------------------------------------------------------------
 >>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<<
 >>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<<
============================================================================
 ELEVATION DATA: UPSTREAM(FEET) =    152.30  DOWNSTREAM(FEET) =    149.30
 CHANNEL LENGTH THRU SUBAREA(FEET) =   320.00   CHANNEL SLOPE =  0.0094
 CHANNEL BASE(FEET) =    0.00   "Z" FACTOR =   4.000
 MANNING'S FACTOR = 0.015   MAXIMUM DEPTH(FEET) =   1.00
 *  10 YEAR RAINFALL INTENSITY(INCH/HR) =  2.862
 SUBAREA LOSS RATE DATA(AMC  II):
  DEVELOPMENT TYPE/      SCS SOIL   AREA      Fp         Ap     SCS
      LAND USE            GROUP   (ACRES)  (INCH/HR)  (DECIMAL)  CN
 RESIDENTIAL
 "11+ DWELLINGS/ACRE"       A        1.32      0.40     0.200    32
 SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) =  0.40
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap =  0.200
 TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) =       3.54
 TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) =   3.67
 AVERAGE FLOW DEPTH(FEET) =   0.49   TRAVEL TIME(MIN.) =   1.45
 Tc(MIN.) =    9.20
 SUBAREA AREA(ACRES) =     1.32       SUBAREA RUNOFF(CFS) =    3.31
 EFFECTIVE AREA(ACRES) =      2.00     AREA-AVERAGED Fm(INCH/HR) =   0.08
 AREA-AVERAGED Fp(INCH/HR) =   0.40  AREA-AVERAGED Ap =   0.20
 TOTAL AREA(ACRES) =        2.0         PEAK FLOW RATE(CFS) =       5.01

 END OF SUBAREA CHANNEL FLOW HYDRAULICS:
```

```
  DEPTH(FEET) =  0.56   FLOW VELOCITY(FEET/SEC.) =   4.00
  LONGEST FLOWPATH FROM NODE      5.00 TO NODE      7.00 =     570.00 FEET.
============================================================================
  END OF STUDY SUMMARY:
  TOTAL AREA(ACRES)     =        2.0  TC(MIN.) =      9.20
  EFFECTIVE AREA(ACRES) =      2.00  AREA-AVERAGED Fm(INCH/HR)=  0.08
  AREA-AVERAGED Fp(INCH/HR) =  0.40  AREA-AVERAGED Ap = 0.200
  PEAK FLOW RATE(CFS)   =       5.01
============================================================================
============================================================================
  END OF RATIONAL METHOD ANALYSIS
```

```
******************************************************************************
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                          Analysis prepared by:

                          HUNSAKER & ASSOCIATES
                                Irvine,Inc
                      Planning * Engineering * Surveying
          Three Hughes * Irvine, California 92618 * (949)583-1010

 ************************* DESCRIPTION OF STUDY *************************
* W.O. #3916-66, TRACT 17703, AREA 'F'                                  *
* 2-YR STUDY                                                            *
* PROPOSED CONDITION                                                    *
 ************************************************************************

  FILE NAME: 17703F.DAT
  TIME/DATE OF STUDY: 16:20 08/09/2021
==============================================================================
  USER SPECIFIED HYDROLOGY AND HYDRAULIC MODEL INFORMATION:
==============================================================================
                     --*TIME-OF-CONCENTRATION MODEL*--

  USER SPECIFIED STORM EVENT(YEAR) =    2.00
  SPECIFIED MINIMUM PIPE SIZE(INCH) =  18.00
  SPECIFIED PERCENT OF GRADIENTS(DECIMAL) TO USE FOR FRICTION SLOPE = 0.90
  *DATA BANK RAINFALL USED*
  *ANTECEDENT MOISTURE CONDITION (AMC) I ASSUMED FOR RATIONAL METHOD*

  *USER-DEFINED STREET-SECTIONS FOR COUPLED PIPEFLOW AND STREETFLOW MODEL*
     HALF-  CROWN TO   STREET-CROSSFALL:   CURB  GUTTER-GEOMETRIES:  MANNING
     WIDTH  CROSSFALL  IN-  / OUT-/PARK-  HEIGHT  WIDTH  LIP   HIKE  FACTOR
NO.   (FT)     (FT)    SIDE / SIDE/ WAY    (FT)    (FT)  (FT)  (FT)    (n)
===  =====  =========  ==================  ======  ===== ====== ===== =======
  1   30.0     20.0    0.018/0.018/0.020   0.67    2.00 0.0312 0.167 0.0150

  GLOBAL STREET FLOW-DEPTH CONSTRAINTS:
    1. Relative Flow-Depth =  0.00 FEET
       as (Maximum Allowable Street Flow Depth) - (Top-of-Curb)
    2. (Depth)*(Velocity) Constraint =  6.0 (FT*FT/S)
  *SIZE PIPE WITH A FLOW CAPACITY GREATER THAN
   OR EQUAL TO THE UPSTREAM TRIBUTARY PIPE.*
  *USER-SPECIFIED MINIMUM TOPOGRAPHIC SLOPE ADJUSTMENT NOT SELECTED

******************************************************************************
  FLOW PROCESS FROM NODE      1.00 TO NODE      2.00 IS CODE =  21
------------------------------------------------------------------------------
  >>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
  >>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<
==============================================================================
  INITIAL SUBAREA FLOW-LENGTH(FEET) =   280.00
  ELEVATION DATA: UPSTREAM(FEET) =    154.30  DOWNSTREAM(FEET) =    152.20

  Tc = K*[(LENGTH** 3.00)/(ELEVATION CHANGE)]**0.20
  SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) =    8.211
  *   2 YEAR RAINFALL INTENSITY(INCH/HR) =  1.703
  SUBAREA Tc AND LOSS RATE DATA(AMC  I ):
   DEVELOPMENT TYPE/      SCS SOIL   AREA      Fp         Ap     SCS   Tc
       LAND USE            GROUP   (ACRES)  (INCH/HR)  (DECIMAL)  CN  (MIN.)
  RESIDENTIAL
  "11+ DWELLINGS/ACRE"       A        1.39      0.40     0.200    17    8.21
  SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) =  0.40
  SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap =  0.200
  SUBAREA RUNOFF(CFS) =      2.03
  TOTAL AREA(ACRES) =      1.39   PEAK FLOW RATE(CFS) =      2.03

******************************************************************************
```

```
 FLOW PROCESS FROM NODE      3.00 TO NODE      4.00 IS CODE =  21
----------------------------------------------------------------------------
 >>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
 >>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<
============================================================================
 INITIAL SUBAREA FLOW-LENGTH(FEET) =   255.00
 ELEVATION DATA: UPSTREAM(FEET) =    154.30  DOWNSTREAM(FEET) =    152.40

 Tc = K*[(LENGTH** 3.00)/(ELEVATION CHANGE)]**0.20
 SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) =    7.920
 *   2 YEAR RAINFALL INTENSITY(INCH/HR) =  1.738
 SUBAREA Tc AND LOSS RATE DATA(AMC  I ):
  DEVELOPMENT TYPE/      SCS SOIL   AREA      Fp         Ap     SCS   Tc
      LAND USE            GROUP   (ACRES)  (INCH/HR)  (DECIMAL)  CN  (MIN.)
 RESIDENTIAL
 "11+ DWELLINGS/ACRE"       A        0.83      0.40     0.200    17    7.92
 SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) =  0.40
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap =  0.200
 SUBAREA RUNOFF(CFS) =      1.24
 TOTAL AREA(ACRES) =      0.83   PEAK FLOW RATE(CFS) =      1.24

****************************************************************************
 FLOW PROCESS FROM NODE      5.00 TO NODE      6.00 IS CODE =  21
----------------------------------------------------------------------------
 >>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
 >>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<
============================================================================
 INITIAL SUBAREA FLOW-LENGTH(FEET) =   250.00
 ELEVATION DATA: UPSTREAM(FEET) =    154.30  DOWNSTREAM(FEET) =    152.30

 Tc = K*[(LENGTH** 3.00)/(ELEVATION CHANGE)]**0.20
 SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) =    7.746
 *   2 YEAR RAINFALL INTENSITY(INCH/HR) =  1.761
 SUBAREA Tc AND LOSS RATE DATA(AMC  I ):
  DEVELOPMENT TYPE/      SCS SOIL   AREA      Fp         Ap     SCS   Tc
      LAND USE            GROUP   (ACRES)  (INCH/HR)  (DECIMAL)  CN  (MIN.)
 RESIDENTIAL
 "11+ DWELLINGS/ACRE"       A        0.68      0.40     0.200    17    7.75
 SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) =  0.40
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap =  0.200
 SUBAREA RUNOFF(CFS) =      1.03
 TOTAL AREA(ACRES) =      0.68   PEAK FLOW RATE(CFS) =      1.03

****************************************************************************
 FLOW PROCESS FROM NODE      6.00 TO NODE      7.00 IS CODE =  51
----------------------------------------------------------------------------
 >>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<<
 >>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<<
============================================================================
 ELEVATION DATA: UPSTREAM(FEET) =    152.30  DOWNSTREAM(FEET) =    149.30
 CHANNEL LENGTH THRU SUBAREA(FEET) =   320.00   CHANNEL SLOPE =  0.0094
 CHANNEL BASE(FEET) =    0.00   "Z" FACTOR =   4.000
 MANNING'S FACTOR = 0.015   MAXIMUM DEPTH(FEET) =   1.00
 *   2 YEAR RAINFALL INTENSITY(INCH/HR) =  1.570
 SUBAREA LOSS RATE DATA(AMC  I ):
  DEVELOPMENT TYPE/      SCS SOIL   AREA      Fp         Ap     SCS
      LAND USE            GROUP   (ACRES)  (INCH/HR)  (DECIMAL)  CN
 RESIDENTIAL
 "11+ DWELLINGS/ACRE"       A        1.32      0.40     0.200    17
 SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) =  0.40
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap =  0.200
 TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) =       1.92
 TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) =   3.12
 AVERAGE FLOW DEPTH(FEET) =   0.39   TRAVEL TIME(MIN.) =   1.71
 Tc(MIN.) =    9.46
 SUBAREA AREA(ACRES) =     1.32       SUBAREA RUNOFF(CFS) =    1.77
 EFFECTIVE AREA(ACRES) =      2.00     AREA-AVERAGED Fm(INCH/HR) =   0.08
 AREA-AVERAGED Fp(INCH/HR) =   0.40  AREA-AVERAGED Ap =   0.20
 TOTAL AREA(ACRES) =        2.0         PEAK FLOW RATE(CFS) =       2.68

 END OF SUBAREA CHANNEL FLOW HYDRAULICS:
```

```
  DEPTH(FEET) =  0.44   FLOW VELOCITY(FEET/SEC.) =   3.47
  LONGEST FLOWPATH FROM NODE      5.00 TO NODE      7.00 =     570.00 FEET.
============================================================================
  END OF STUDY SUMMARY:
  TOTAL AREA(ACRES)     =        2.0  TC(MIN.) =      9.46
  EFFECTIVE AREA(ACRES) =      2.00  AREA-AVERAGED Fm(INCH/HR)=  0.08
  AREA-AVERAGED Fp(INCH/HR) =  0.40  AREA-AVERAGED Ap = 0.200
  PEAK FLOW RATE(CFS)   =       2.68
============================================================================
============================================================================
  END OF RATIONAL METHOD ANALYSIS
```

SECTION 3

HYDROLOGY MAP

PARK ST.

MERIDIAN ST.

UNION ST.

EXIST 18" RCP
LINE 'E-2e'
PLAN NO. 36208

EXIST 24" RCP
LINE 'E-2'
PLAN NO. 36208

EXIST 18" RCP
LINE 'E-2'
PLAN NO. 36208

EXIST 18" RCP
LINE 'E-2c'
PLAN NO. 26196
& 36207

FUTURE FIRE HYDRANT PER DEVELOPMENT AREA E

FUTURE WATER LINE PER DEVELOPMENT AREA E

EX. WATER LINE

EX. STORM DRAIN

EX. SEWER LINE

EX. IRRIGATION METER FOR RECYCLE WATER

EX. CURB

EX. SIDEWALK

EX. RIGHT OF WAY

TO BE RELOCATED

PROPOSED LOCATION FIRE HYDRANT

PROPOSED LOCATION EX. WATER LINE

PROPOSED LOCATION EX. SIDEWALK

PROPOSED LOCATION EX. CURB

PROPOSED LOCATION EX. RIGHT OF WAY

DRIVEWAY PER STD 115-B

8'X40' LOADING & DELIVERIES

TREE TO BE REMOVED

"A" DR.

"B" DRIVE

"C" DRIVE

"D" DR.

ALLEY "E"

ALLEY "F"

ALLEY "G"

ALLEY "H"

ALLEY "I"

BLDG-1 FF=151.6

BLDG-2 FF=151.8

BLDG-3 FF=152.8

BLDG-4 FF=153.5

BLDG-5 FF=154.0

BLDG-6 FF=154.3

BLDG-7 FF=154.1

BLDG-8 FF=153.8

BLDG-9 FF=154.1

BLDG-10 FF=154.3

BLDG-12 FF=153.5

BLDG-13 FF=153.5

BLDG-14 FF=154.4

BLDG-15 FF=154.5

BLDG-16 FF=153.2

BLDG-17 FF=151.9

Q_{10} =2.3cfs
Q_{25} =2.7cfs
Q_{100}=3.5cfs
t=7.8
Q2 =1.2cfs
EL=152.4

Q_{10} =3.7cfs
Q_{25} =4.5cfs
Q_{100}=5.7cfs
t=8.2
Q2 =2.0cfs
EL=152.2

Q_{10} =1.9cfs
Q_{25} =2.3cfs
Q_{100}=2.9cfs
t=7.8
Q2 =1.0cfs
EL=152.3

Q_{10} =5.0cfs
Q_{25} =6.0cfs
Q_{100}=7.8cfs
t=9.2
Q2 =2.7cfs
EL=149.3

F1 1.39

F2 0.83

F3 0.68

F4 1.32

L=255'

L=280'

L=250'

L=320'

8'x20' VAULT

4'x13' MWS

10'x24' VAULT

8'x12' MWS

16'x25' VAULT

8'x16' MWS

EL=154.3

Nuisance Flow

APN: 0232-121-20

APN: 0232-121-21

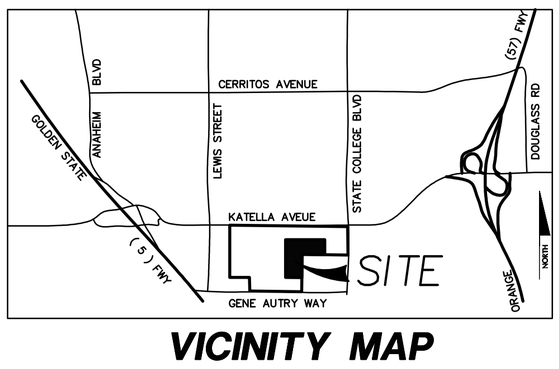
EXISTING RETAINING WALL

ASPH.

TRASH ENCLOSURE & BULK TRASH

STRIPED FOR TURNAROUND

DDC



VICINITY MAP

LEGEND

- MAJOR DRAINAGE BOUNDARY
- MINOR DRAINAGE BOUNDARY
- 1 NODE NUMBER
- A / 1.0 AREA DESIGNATION / AREA ACREAGE (IN ACRES)
- Q_{10} =6.0cfs, Q_{100}=8.0cfs, t=15.0, C.B. #1 PEAK FLOW RATE / TIME OF CONCENTRATION
- PROPOSED STORM DRAIN
- A SOIL GROUP

SCALE: 1" = 30'

30 15 0 30 90

GRAPHIC SCALE 1"=30'

PREPARED FOR:

LMC

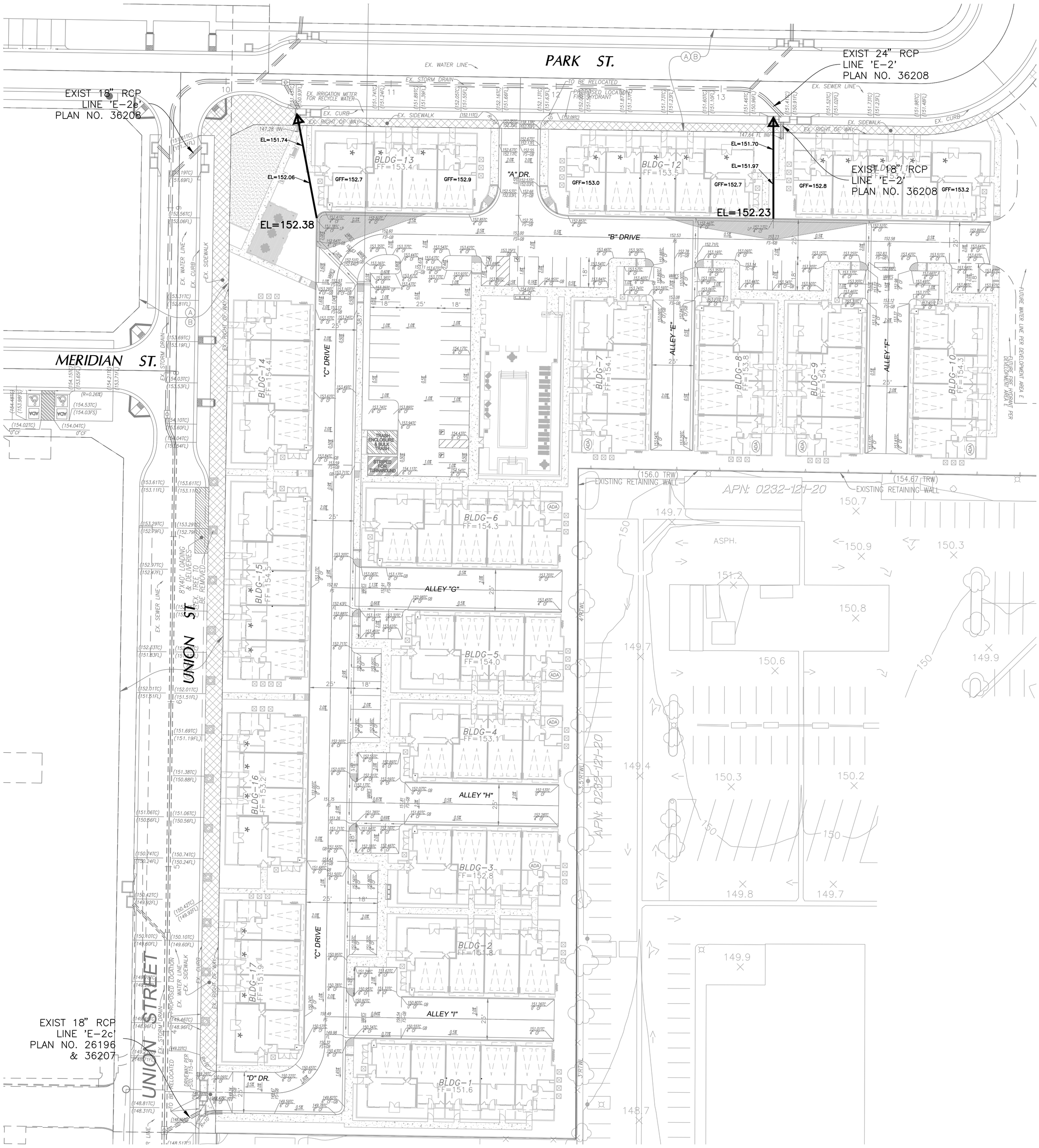
A LENNAR COMPANY

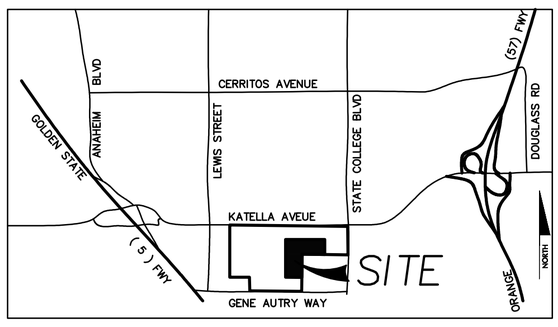
PREPARED BY:

HUNSAKER & ASSOCIATES
IRVINE, INC.
PLANNING ▪ ENGINEERING ▪ SURVEYING
Three Hughes ▪ Irvine, CA 92618 ▪ PH: (949) 583-1010 ▪ FX: (949) 583-0759

HYDROLOGY MAP FOR A-TOWN TRACT NO. 17703 LOT 5 - AREA F

W.O. #3916-66





VICINITY MAP

LEGEND

- FLOODING AREA
- PROPOSED STORM DRAIN
- EL=100.00 OVERFLOW ELEVATION
- FLOW PATH

SCALE: 1" = 30'

30 15 0 30 90

GRAPHIC SCALE 1"=30'

PREPARED FOR:

LMC

A LENNAR COMPANY

PREPARED BY:

HUNSAKER & ASSOCIATES
IRVINE, INC.
PLANNING ■ ENGINEERING ■ SURVEYING
Three Hughes ■ Irvine, CA 92618 ■ PH: (949) 583-1010 ■ FX: (949) 583-0759

FLOODING EXHIBIT FOR A-TOWN TRACT NO. 17703 LOT 5 - AREA F

W.O. #3916-66

PLOTTED BY: Tu Trinh DATE: Oct. 19, 2021 10:39:34 AM FILE: F:\0108\Engineering\SY_HYDROLOGY\Tr 17703 Area F\Flooding Exhibit for 17703 Area F.dwg

SECTION 4

REFERENCES

APPROVED

SD IMPROVEMENT PLANS

FOR A-TOWN TRACT 17703

CITY OF ANAHEIM
STORM DRAIN IMPROVEMENT PLANS
FOR
TRACT 17703

GENERAL NOTES

1. ALL WORK WITHIN THE RIGHT-OF WAY WILL REQUIRE A RIGHT-OF-WAY EXCAVATION & CONSTRUCTION PERMIT.

2. CONTACT THE FIELD ENGINEER AT (714) 765-5126 48 HOURS PRIOR TO ANY OF THIS WORK.

3. ALL WORK SHALL BE IN ACCORDANCE WITH THE APPLICABLE SECTIONS OF STANDARD SPECIFICATIONS FOR PUBLIC WORKS CONSTRUCTION, CITY OF ANAHEIM STANDARD PLANS, CONTRACT DOCUMENTS AND STANDARD SPECIFICATION SUPPLEMENT AND THE LATEST REVISIONS THEREOF."

4. THE CONTRACTOR SHALL NOTIFY UNDERGROUND SERVICE ALERT AT 1-800-422-4133 A MINIMUM OF 48 HOURS PRIOR TO START OF CONSTRUCTION WITHIN THE RIGHT-OF-WAY.

5. ALL PAVING REMOVAL SHALL BE SAWCUT AT THE FIELD ENGINEER'S DIRECTION. MINIMUM DEPTH OF CUT: 0.12'.

6. PAVING SHALL BE SAWCUT AND REMOVED A MINIMUM OF 1' FROM CURB AND GUTTER CONSTRUCTION OR RECONSTRUCTION.

7. THE PROTECTION AND ADJUSTMENT OF ALL UTILITIES SHALL BE THE RESPONSIBILITY OF THE DEVELOPER.

8. IT SHALL BE THE CONTRACTOR'S RESPONSIBILITY TO PROTECT AND/OR RELOCATE ALL TRAFFIC SIGNS AND TRAFFIC SIGNALS AS SHOWN ON THE PLANS OR AS REQUIRED BY THE FIELD ENGINEER.

9. IRRIGATION LINES SHALL BE REMOVED, RELOCATED OUT OF THE STREET RIGHT-OF-WAY, ENCASED IN A STEEL SLEEVE, OR REPLACED WITH CAST IRON AS DIRECTED BY THE PARKS DIVISION. NOTIFY THE PARKS DIVISION AT (714) 765-5155 48 HOURS PRIOR TO WORK.

10. REPORTS OF COMPACTION WITHIN THE RIGHT-OF-WAY SHALL BE SUBMITTED TO AND ACCEPTED BY THE FIELD ENGINEER PRIOR TO PLACING ANY PAVING.

11. IN THE EVENT THAT EXISTING STRIPING IS OBLITERATED BY CONSTRUCTION, IT WILL BE THE RESPONSIBILITY OF THE DEVELOPER TO REPLACE SAID STRIPING.

12. PRIOR TO CONSTRUCTION, ALL SURVEY POINTS THAT MAY BE DISTURBED SHALL BE TIED OUT AND A CORNER RECORD OF EACH POINT SHALL BE FILED WITH THE COUNTY SUPERVISOR. A COPY OF THE RECORDED CORNER RECORD SHALL BE SUBMITTED TO THE CITY'S FIELD SERVICES DIVISION. FOLLOWING THE COMPLETION OF THE CONSTRUCTION, A CORNER RECORD OF EACH POINT THAT WAS DISTURBED SHALL BE FILED WITH THE COUNTY SURVEYOR. A COPY OF THE RECORDED CORNER RECORD SHALL BE SUBMITTED TO THE CITY'S FIELD SERVICES DIVISION PRIOR TO THE RECORDING OF A CERTIFICATE OF COMPLETION OR RELEASE OF BONDS.

GENERAL NOTES FOR STORM DRAIN AND SEWER IMPROVEMENT PLANS

1. PRIOR TO DELIVERY OF STORM DRAIN PIPES, CONTACT FIELD ENGINEERING AT (714) 765-5126 FOR D-LOAD TEST APPROVAL.

2. THE CONTRACTOR SHALL CHECK THE FLOW LINE ELEVATION OF EXISTING JUNCTURE AGAINST CONTROL PROVIDED PRIOR TO CONSTRUCTION. IF THE EXISTING FLOW LINE VARIES FROM THE PLAN ELEVATION BY MORE THAN ±0.10' THE ENGINEER SHALL ADJUST THE GRADES BETWEEN THE JUNCTURE AND THE NEXT UPSTREAM MANHOLE, WITH THE APPROVAL OF THE FIELD ENGINEER.

3. ADJUST ALL MANHOLES TO GRADE AFTER PLACING FINAL LIFT OF ASPHALT.

4. 2-SACK SAND CEMENT SLURRY TRENCH BACKFILL SHALL BE REQUIRED TO 1' ABOVE THE FINISH TOP OF PIPE WHERE LINE IS CONSTRUCTED AT A SLOPE OF 5:1 OR MORE.

5. CERTIFICATION BY A LICENSED SURVEYOR OR REGISTERED CIVIL ENGINEER SHALL BE SUBMITTED PRIOR TO FINAL ACCEPTANCE CERTIFYING THAT THE STORM DRAIN SYSTEM WAS INSTALLED ON GRADE PER APPROVED PLAN.

BASIS OF BEARINGS:

BEARINGS SHOWN HEREON ARE BASED UPON THE BEARINGS BETWEEN O.C.S. HORIZONTAL CONTROL STATION GPS NO. 3094 AND STATION GPS NO. 3086 BEING NORTH 89°07'46" WEST PER RECORDS ON FILE IN THE OFFICE OF THE ORANGE COUNTY SURVEYOR.

LEGAL DESCRIPTION:

IN THE CITY OF ANAHEIM, COUNTY OF ORANGE, STATE OF CALIFORNIA, BEING A SUBDIVISION OF LOTS 2 THROUGH 10, INCLUSIVE, LOTS 12 THROUGH 14, INCLUSIVE, LOTS A, B, MARKET STREET, BRYANT STREET, GRANVILLE DRIVE, TRIAD STREET, UNION STREET, MERIDIAN LANE, METRO DRIVE, PARK LANE, KATELLA AVENUE, GENE AUTRY WAY, STATE COLLEGE BOULEVARD AND WESTSIDE DRIVE OF TRACT NO. 16859 AS SHOWN ON A MAP RECORDED IN BOOK 892, PAGES 1 THROUGH 10, INCLUSIVE, OF MISCELLANEOUS MAPS, IN THE OFFICE OF THE COUNTY RECORDER OF SAID COUNTY.

NOTICE TO CONTRACTOR PRIOR TO CONSTRUCTION

BEFORE PROCEEDING WITH THIS WORK, THE CONTRACTOR SHALL CAREFULLY CHECK AND VERIFY ALL CONDITIONS, QUANTITIES, DIMENSIONS AND SURFACE GRADE ELEVATIONS, UTILITY CONNECTION ELEVATIONS AND LOCATIONS AND SHALL REPORT ALL DISCREPANCIES TO THE ENGINEER PRIOR TO THE COMMENCEMENT OF WORK.

UNAUTHORIZED CHANGES & USES: THE ENGINEER PREPARING THESE PLANS WILL NOT BE RESPONSIBLE FOR, OR LIABLE FOR, UNAUTHORIZED CHANGES TO, DELETIONS FORM OR USES OF THESE PLANS. ALL CHANGES TO THE PLANS MUST BE PRESENTED IN WRITING AND MUST BE APPROVED BY THE PREPARER OF THESE PLANS AND THE ENGINEER OF RECORD IN WRITING.

CONTRACTOR SHALL IMPLEMENT AND MAINATAIN EROSION CONTROL MEASURES IN ACCORDANCE WITH EROSION CONTROL PLAN GRA.

UNDERGROUND SERVICE ALERT

Call:Toll FREE
1-800-422-4133
TWO WORKING DAYS BEFORE YOU DIG

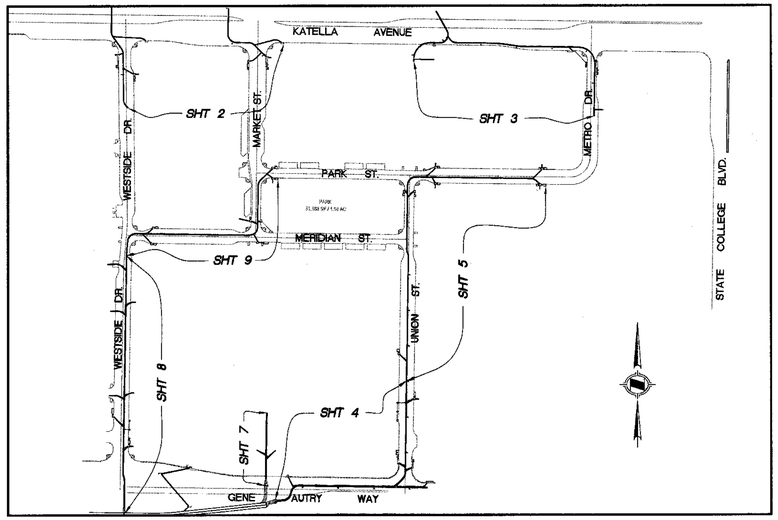
"CAUTION": Remember that the USA Center notifies only those utilities belonging to the center. There could be other utilities present at the work site. The center will inform you of whom they will notify.

WATER NOTES
(SEWER/STORM DRAIN IMPROVEMENT)

1. ALL EXISTING PUBLIC WATER FACILITIES SHALL BE PROTECTED IN PLACE. THE CONTRACTOR PERFORMING THIS WORK SHALL BE RESPONSIBLE FOR ALL COSTS AS A RESULT OF ANY DAMAGE OR REPAIRS TO EXISTING WATER FACILITIES.
2. IN CASE OF AN EMERGENCY, THE CONTRACTOR SHALL IMMEDIATELY CONTACT WATER FIELD/OPERATIONS AT (714)765-4580 AND WATER ENGINEERING (714)765-5196.
3. SMALL WATER SERVICE LATERALS (I.E. 2" AND SMALLER) ARE TYPICALLY NOT SHOWN ON PLANS, BUT MUST BE PROTECTED IN PLACE. THE NORMAL DEPTH OF COVER FOR WATER SERVICE LATERALS IS 30".
4. UNLESS OTHERWISE APPROVED, THE FOLLOWING CLEARANCES SHALL BE MAINTAINED BETWEEN EXISTING WATER FACILITIES AND ANY OTHER PROPOSED IMPROVEMENTS:
   A. A MINIMUM SEPARATION OF 12 INCHES FOR VERTICAL CROSSINGS.
   B. A MINIMUM SEPARATION OF 5 FEET FOR HORIZONTAL/PARALLEL INSTALLATIONS, EXCEPT FOR SANITARY SEWERS WHICH REQUIRE A MINIMUM HORIZONTAL SEPARATION OF 10-FEET.
   C. *A MINIMUM OF 2-FEET AROUND ANY ABOVE-GROUND WATER FACILITY/APPURTENANCE.*
5. ALL UNUSED WATER FACILITIES SHALL BE ABANDONED AT THE MAIN AS DIRECTED BY THE ENGINEER. PRIOR TO ABANDONMENT OF SERVICES 4" AND LARGER, THE CONTRACTOR SHALL ENSURE SERVICE LINE IS RESTRAINED TO THE MAINS.
6. ADJUST EXISTING WATER VALVE BOX TO GRADE PER WATER STD. W-152.
7. PROVIDE LATERAL SUPPORT EXISTING WATER MAIN DURING SEWER CONSTRUCTION. INSTALL ADEQUATE SHORING AND BRACING FOR THE FULL DEPTH OF SEWER TRENCH EXCAVATION.
8. CONTRACTOR SHALL CONTACT CITY OF ANAHEIM WATER INSPECTION (714)765-4591 PRIOR TO BEGINNING WORK. ALL EXCAVATION AROUND EXISTING PUBLIC WATER FACILITIES SHALL BE OBSERVED BY CITY WATER INSPECTOR.

ABBREVIATIONS

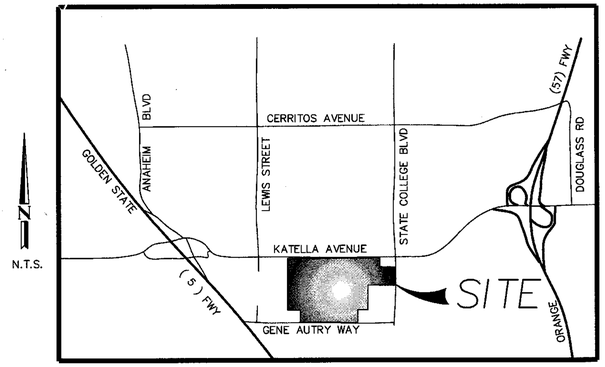
FS	FINISH SURFACE
TC	TOP OF CURB
FL	FLOW LINE
CL	CENTER LINE
BC	BEGIN CURVE
EC	END CURVE
W	WATER
S	SEWER
CB	CATCH BASIN
LF	LINEAR FEET
HGL	HYDRAULIC GRADE LINE
EA	EACH
MH	MANHOLE
JS	JUNCTION STRUCTURE
RCP	REINFORCED CONCRETE PIPE
PRC	POINT OF REVERSE CURVE



INDEX MAP
SCALE: 1" = 200'

INDEX OF SHEETS:

	PLAN NO.	
SHEET 1	– 36204 –	TITLE SHEET
SHEET 2	– 36205 –	EX. LINE 'A-1' EXTENSION, LINE 'A-3' & EX. LINE 'B-2' EXTENSION
SHEET 3	– 36206 –	AS-BUILT LINES 'C' & 'D'
SHEET 4	– 36207 –	AS-BUILT LINES 'E', 'E-1', 'E-2', 'E-3', 'E-2a' & 'E-2b'
SHEET 5	– 36208 –	LINE 'E-2' EXTENSION
SHEET 6	– 36209 –	LINES 'E-2c' – 'E-2i'
SHEET 7	– 36210 –	AS-BUILT LINE 'F' PLAN NO. 26198
SHEET 8	– 36211 –	LINES 'G' FR. STA. 10+00.00 TO 17+22.26, 'G-1' & 'G-2'
SHEET 9	– 36212 –	LINES 'G' FROM STA. 17+22.26 TO 23+33.71, 'G-8' & 'G-9'
SHEET 10	– 36213 –	LINES 'G-3' – 'G-7' & 'G-10' – 'G-13'
SHEET 11	– 36214 –	DETAILS
SHEET 12	– 36215 –	STRUCTURE DETAILS
SHEET 13	– 36216 –	STRUCTURE DETAILS



VICINITY MAP

CONSTRUCTION NOTES & QUANTITY ESTIMATE

NO.	DESCRIPTION	AMOUNT	UNIT
47	CONST MODIFIED J.S. PER SPPWC STD PLAN NO 333-2, DETAIL ON SHEET 8	1	EA.
48	INST. 1'H x 6'W R.C.B.	8	L.F.
49	REMOVE PLUG AND JOIN EXISTING	4	EA.
50	TRANS. FROM 1'H x 6'W RCB TO 30" RCP PER SPPWC STD. PLAN NO. 342-2	17.5	L.F.
51	CONST. CONCRETE COLLAR PER SPPWC STD. PLAN NO. 380-4	6	EA.
52	CONST. J.S. PER SPPWC STD. PLAN NO. 331-3	15	EA.
53	INST. 18" RCP (SEE PROFILE FOR D-LOAD) BEDDING PER OCPW STD. NO. 1319, DETAIL ON SHEET 13	552	L.F.
55	INST. 30" RCP (SEE PROFILE FOR D-LOAD) BEDDING PER OCPW STD. NO. 1319, DETAIL ON SHEET 13	551	L.F.
56	CONST THE MAXWELL IV AUXILIARY PER TORRENT RESOURCES, DETAIL ON SHT. 12	3	EA.
57	INST. 22.5° ELBOW	3	EA.
58	CONST. C.B. PER CITY OF ANAHEIM STD. DETAIL NO. 300-2, CASE A WITH AUTOMATIC RETRACTABLE SCREEN & CONNECTOR PIPE SCREEN ★★	18	EA.
59	CONST. DOUBLE BRICK AND MORTAR PLUG	5	EA.
60	INST. 24" RCP (SEE PROFILE FOR D-LOAD) BEDDING PER OCPW STD. NO. 1319, DETAIL ON SHEET 13	1323	L.F.
61	CONST. M.H. PER SPPWC STD. PLAN NO. 321-2	6	EA.
62	CONST. BLANKET PROTECTION PER SPPWC STD. PLAN NO. 225-2 WITH 2 SACK CEMENT SAND SLURRY (NO CONCRETE)	36	L.F.
63	INST. 4" SCH-80 PVC OUTLET PIPE	150	L.F.
64	CONST. THE MAXWELL IV PER TORRENT RESOURCES, DETAIL ON SHEET 13	9	EA.
65	CONST. 4'x10' FILTERRA BOX WITH 4" SCH-80 PVC OUTLET PIPE	2	EA.
66	CONST. 4'x12' FILTERRA BOX WITH 4" SCH-80 PVC OUTLET PIPE	2	EA.
67	CONST. 4'x16' FILTERRA BOX WITH 6" SCH-80 PVC OUTLET PIPE	1	EA.
68	CONST. MODIFIED MH PER SPPWC STD PLAN NO 321-2, DETAIL ON SHEET 12	1	EA.
69	CONST. MODIFIED MH PER SPPWC STD PLAN NO 321-2, DETAIL ON SHEET 12	1	EA.
70	CONST. 4'x4' FILTERRA BOX WITH 4" SCH-80 PVC OUTLET PIPE	4	EA.
71	CONST. 4'x6' FILTERRA BOX WITH 4" SCH-80 PVC OUTLET PIPE	7	EA.
72	CONST. 4'x8' FILTERRA BOX WITH 4" SCH-80 PVC OUTLET PIPE	4	EA.
76	CONST. 6'x6' FILTERRA BOX WITH 6" SCH-80 PVC OUTLET PIPE	2	EA.
77	CONST. AUTOMATIC RETRACTABLE SCREEN AND CONNECTOR PIPE SCREEN ON EX. CATCH BASIN ★★	5	EA.
80	INST. 6" SCH-80 PVC OUTLET PIPE	–	L.F.

★★ INSTALL AUTOMATIC RETRACTABLE SCREEN AND CONNECTOR PIPE SCREEN, RETRACTABLE SCREEN AND CONNECTOR PIPE SCREEN SHALL BE CONSTRUCTED OF CORROSION RESISTANT MATERIALS AND SHALL BE "FULL CAPTURE" DESIGN AND OF MAINTAINABLE DESIGN. CONTRACTOR SHALL MAKE PRODUCT SUBMITTAL TO ENGINEER FOR APPROVAL PRIOR TO CONSTRUCTION.

NOTE:
- SUPPORT FOR CONDUITS ACROSS TRENCHES PER SPPWC STD. PLAN NO. 224-2
- TRENCH REPLACEMENT PER CITY OF ANAHEIM STD. DETAIL NO. 132-C

I HEREBY CERTIFY THAT THE IMPROVEMENTS HAVE BEEN COMPLETED IN CONFORMANCE WITH THIS APPROVAL PLAN.

EDWARD A. MANDICH
RCE 59089
EXP.: 06/30/17

HUNSAKER & ASSOCIATES
3 HUGHES, IRVINE CA 92618
(949) 583-1010

DATE

SERVICE AGENCIES :

CITY OF ANAHEIM, ENGINEERING DIV.	(714) 765-5176
CITY OF ANAHEIM, ELECTRICAL DEPT.	(714) 765-5156
CITY OF ANAHEIM, FIELD ENGR.	(714) 765-5126
CITY OF ANAHEIM, WATER ENGR. DIV.	(714) 765-5196
PACIFIC TELEPHONE	(714) 633-0811
SO. CALIFORNIA GAS COMPANY	(714) 634-3258

THESE PLANS HAVE BEEN EXAMINED AND IS APPROVED ONLY AS TO COMPATIBILITY WITH ADJOINING EXISTING OR FUTURE IMPROVEMENTS AND CONFORMANCE WITH CITY OF ANAHEIM STANDARD DETAILS AND THE REQUIREMENTS FOR THE FOLLOWING APPLICABLE FACTORS: RIGHT-OF-WAY, ROADWAY MATERIALS, ALIGNMENTS AND GRADES, HYDROLOGY AND HYDRAULIC DESIGN OF STORMDRAIN OR SANITARY SEWER SYSTEMS AND UNDER GROUND CONDUIT OR OPEN CHANNEL CHANNEL ALIGMENTS, GRADES, SIZES AND MATERIALS.

ENGINEER'S NOTE TO CONTRACTOR :
THE EXISTENCE LOCATION OF ANY UNDERGROUND UTILITIES, PIPES, AND/OR STRUCTURES SHOWN ON THESE PLANS WERE OBTAINED BY A SEARCH OF AVAILABLE RECORDS. TO THE BEST OF OUR KNOWLEDGE, THERE ARE NO EXISTING UTILITIES EXCEPT AS SHOWN ON THESE PLANS. THE CONTRACTOR SHALL AS CERTAIN THE TRUE VERTICAL AND HORIZONTAL LOCATION AND SIZE OF THOSE TO BE USED OF ANY UNDERGROUND UTILITIES AND SHALL BE RESPONSIBLE FOR DAMAGE TO ANY PUBLIC OR PRIVATE UTILITIES AS SHOWN HEREON.

ENGINEER:
H&A
HUNSAKER & ASSOCIATES
IRVINE, INC.
PLANNING ▪ ENGINEERING ▪ SURVEYING
Three Hughes ▪ Irvine, CA 92618
FX: (949) 583-0759 ▪ PH: (949) 583-1010
EDWARD A. MANDICH, RCE NO. 59089 — DATE 5-6-16

REGISTERED PROFESSIONAL ENGINEER — EDWARD A. MANDICH — No. 59089 — CIVIL — STATE OF CALIFORNIA

RCP 2015-11013

OWNER:
PT METRO, LLC
25 ENTERPRISE, 3rd FLOOR
ALISO VIEJO, CA 92656
(949) 349-8000

SOILS ENGINEER:
GROUP DELTA CONSULTANTS
32 MAUCHLY, SUITE B
IRVINE, CA 92618
PH: (949) 450-2100

FOR CITY USE ONLY — W.O. 3916-14 STORM DRAIN IMPROVEMENT PLANS — ROOM: — BIN: — DOC: 740 26

D-108

REVISIONS

NO.	INIT.	DATE	DESCRIPTION	DATE	APP'D
1	T.T.	11/1/16	REVISED LINES 'G-2' & 'G-3' ON SHEETS 8 & 10, PLAN NO'S 36211 & 36213	11/1/16	BZG

REFERENCES

BENCH MARK: 9A-22 ELEVATION: 151.99 (NAVD88)
BRASS CAP MKD C OF A BM, IN THE TOP OF CURB AT THE SE ECR. KATELLA AVENUE/STATE COLLEGE BLVD.
ELEVATION: 151.99 (NAVD88)

PLANS FOR THESE IMPROVEMENTS: STREET:
SEWER: 36168-36170 WATER: W-3175
STORM DRAIN: 36204-36216
LANDSCAPE:

APPROVAL

ELECTRICAL ENGINEERING DIVISION — DATE 5-24-16
WATER ENGINEERING DIVISION — DATE 5/23/16
RECOMMEND APPROVAL — DEVELOPMENT SERVICES MANAGER — DATE 5/25/16
PLANNING DIVISION — DATE
CITY ENGINEER — DATE 5/26/16

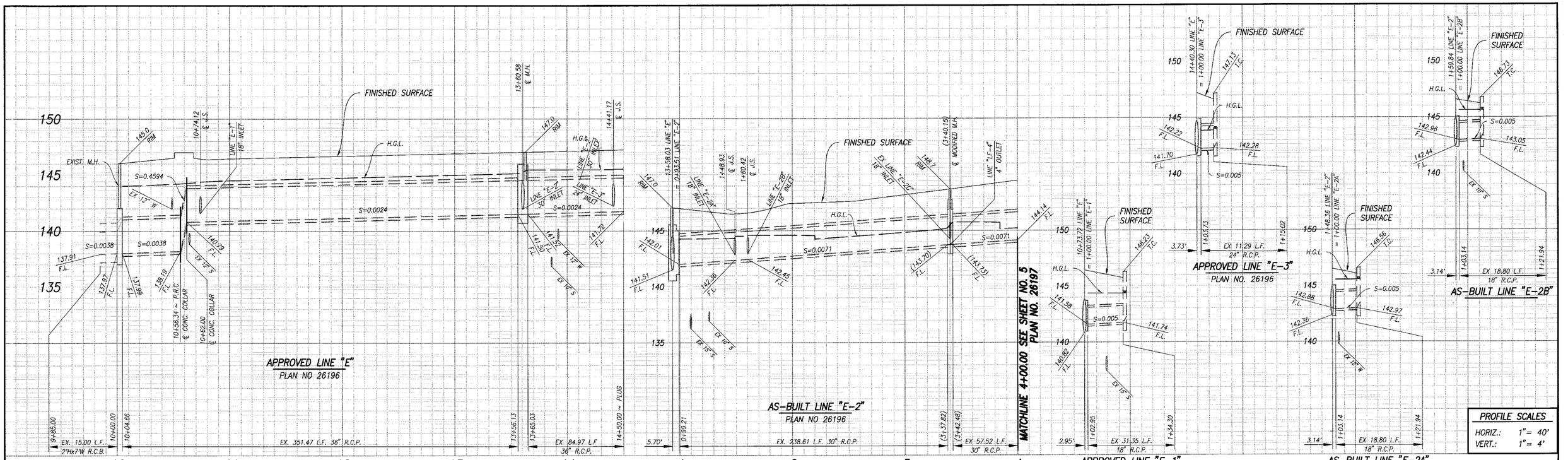
STORM DRAIN PLANS
TRACT 17703

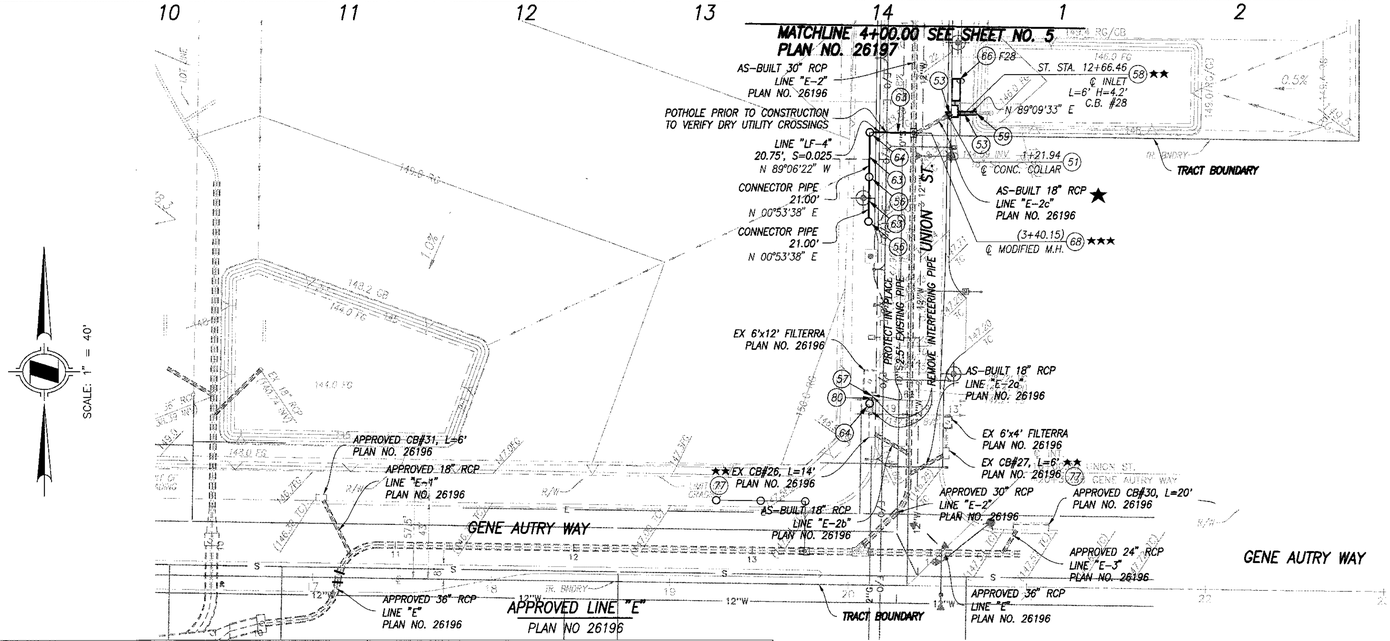
CITY OF ANAHEIM

SHEET 1 OF 13

PLAN NO. 36204

362-04





CONSTRUCTION NOTES

(51) CONST. CONCRETE COLLAR PER SPPWC STD. PLAN NO. 380-4
(53) INST. 18" R.C.P. (SEE PROFILE FOR D-LOAD) BEDDING PER OCPW STD. NO. 1319, DETAIL ON SHEET 13
(56) CONST. THE MAXWELL IV AUXILIARY PER TORRENT RESOURCES, DETAIL ON SHEET 12
(57) INST. 22.5° ELBOW
(58) CONST. CATCH BASIN PER CITY OF ANAHEIM STD. DETAIL NO. 300-2, CASE A WITH AUTOMATIC RETRACTABLE SCREEN AND CONNECTOR PIPE SCREEN ★★
(63) INST. 4" SCH-80 PVC OUTLET PIPE
(64) CONST. THE MAXWELL IV PER TORRENT RESOURCES, DETAIL ON SHEET 13
(66) CONST. 4'x12' FILTERRA BOX WITH 4" SCH-80 PVC OUTLET PIPE
★★★ (68) CONST. MODIFIED M.H. PER SPPWC STD. PLAN NO. 321-2 DETAIL ON SHEET 12
(77) CONSTRUCT AUTOMATIC RETRACTABLE SREEN AND CONNECTOR PIPE SCREEN ON EX. CATCH BASIN ★★
(80) INST. 6" SCH-80 PVC OUTLET PIPE
★ PROFILE ON SHEET NO. 6

★★★ NOTE: TEMPORARY BLOCK 4" OUTLET TO DRY WELLS UNTIL ALL THE UPSTREAM PARCELS ARE STABLIZED OR COMPLETED

HYDRAULIC DATA

LINE	STA to STA	Q_{10}	DIA	So	n	Dc	Vc	Sc	Dn	Vn	Vf	Sf
"E"	10+04.66 10+56.34	34.2	36"	0.0038	0.013	1.96	7.42	0.0051	2.19	6.56	5.12	0.0029
"E"	10+56.34 10+62.00	34.2	36"	0.4594	0.013	1.96	7.42	0.0051	0.57	38.33	5.12	0.0029
"E"	10+62.00 10+74.12	34.2	36"	0.0024	0.013	1.96	7.42	0.0051	FULL	---	5.12	0.0029
"E"	10+74.12 13+56.13	33.3	36"	0.0024	0.013	1.96	7.42	0.0051	FULL	---	5.12	0.0029
"E"	13+65.03 14+41.17	11.2	36"	0.0024	0.013	1.51	6.19	0.0043	1.81	4.96	3.13	0.0011
"E"	14+41.17 14+50.00	0.00	36"	0.0024	0.013	0.00	0.00	0.0000	0.00	0.00	0.00	0.0000
"E-2"	0+99.21 1+48.93	24.6	30"	0.0071	0.013	1.74	7.15	0.0059	1.62	7.74	5.32	0.0040
"E-2"	1+48.93 1+60.42	22.8	30"	0.0071	0.013	1.68	6.92	0.0056	1.54	7.62	4.93	0.0035
"E-2"	1+60.42 3+37.82	22.0	30"	0.0071	0.013	1.57	6.58	0.0052	1.42	7.41	4.36	0.0027
"E-2"	3+42.48 4+00.00	16.6	30"	0.0071	0.013	1.28	5.70	0.0046	1.13	6.72	2.93	0.0012
"E-1"	1+02.95 1+34.30	1.1	18"	0.0050	0.013	0.44	3.21	0.0049	0.44	3.23	0.79	0.0002
"E-2A"	1+03.14 1+21.94	2.4	18"	0.0050	0.013	0.55	3.60	0.0050	0.55	3.61	1.19	0.0004
"E-2B"	1+03.14 1+21.94	1.1	18"	0.0050	0.013	0.66	4.02	0.0051	0.66	3.98	1.70	0.0008
"E-2C"	1+02.02 1+21.94	6.1	18"	0.0150	0.013	1.03	5.48	0.0068	1.17	4.78	4.02	0.0046
"E-3"	1+03.73 1+15.02	11.2	24"	0.0050	0.013	1.14	5.49	0.0052	1.16	5.40	3.25	0.0020

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REVISIONS

NO.	INIT.	DATE	DESCRIPTION	DATE	APP'D

REFERENCES

BENCH MARK: 9A-22 ELEVATION: 151.99 (NAVD88)
KATELLA AVENUE/STATE COLLEGE BLVD.
BRASS CAP MKD C OF A BM, IN THE TOP OF CURB AT THE SE ECR.
ELEVATION: 151.99 (NAVD88)

PLANS FOR THESE IMPROVEMENTS: STREET:
SEWER: 36168-36170 WATER: W-3175
STORM DRAIN: 36204-36216
LANDSCAPE:

APPROVAL

FOR APPROVAL, SEE PLAN NO. 36204

ENGINEER:
HUNSAKER & ASSOCIATES IRVINE, INC.
PLANNING ▪ ENGINEERING ▪ SURVEYING
Three Hughes ▪ Irvine, CA 92618
FX: (949) 583-0759 ▪ PH: (949) 583-1010
EDWARD A. MANDICH, RCE NO. 59089 — DATE 5-6-16
REGISTERED PROFESSIONAL ENGINEER — EDWARD A. MANDICH — No. 59089 — CIVIL — STATE OF CALIFORNIA

RCP 2015-11013

OWNER:
PT METRO, LLC
25 ENTERPRISE, 3rd FLOOR
ALISO VIEJO, CA 92656
(949) 349-8000

SOILS ENGINEER:
GROUP DELTA CONSULTANTS
32 MAUCHLY, SUITE B
IRVINE, CA 92618
PH: (949) 450-2100

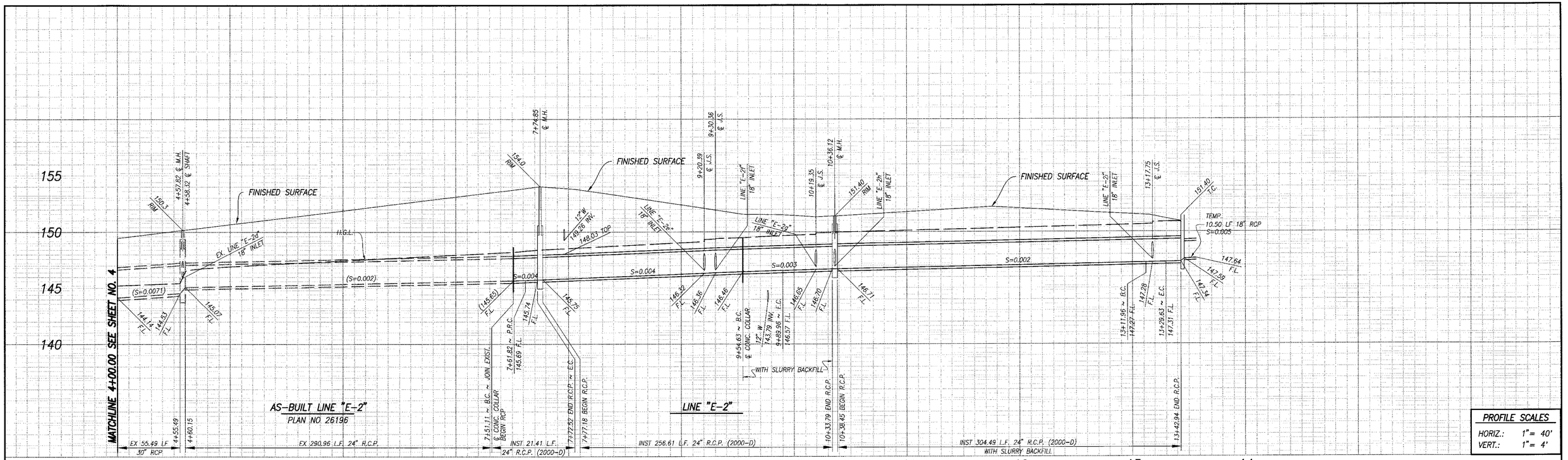
STORM DRAIN PLANS
TRACT 17703
APPROVED LINES "E", "E-1" & "E-3"
AS-BUILT LINES "E-2", "E-2a", "E-2b" & "E-2c"

CITY OF ANAHEIM

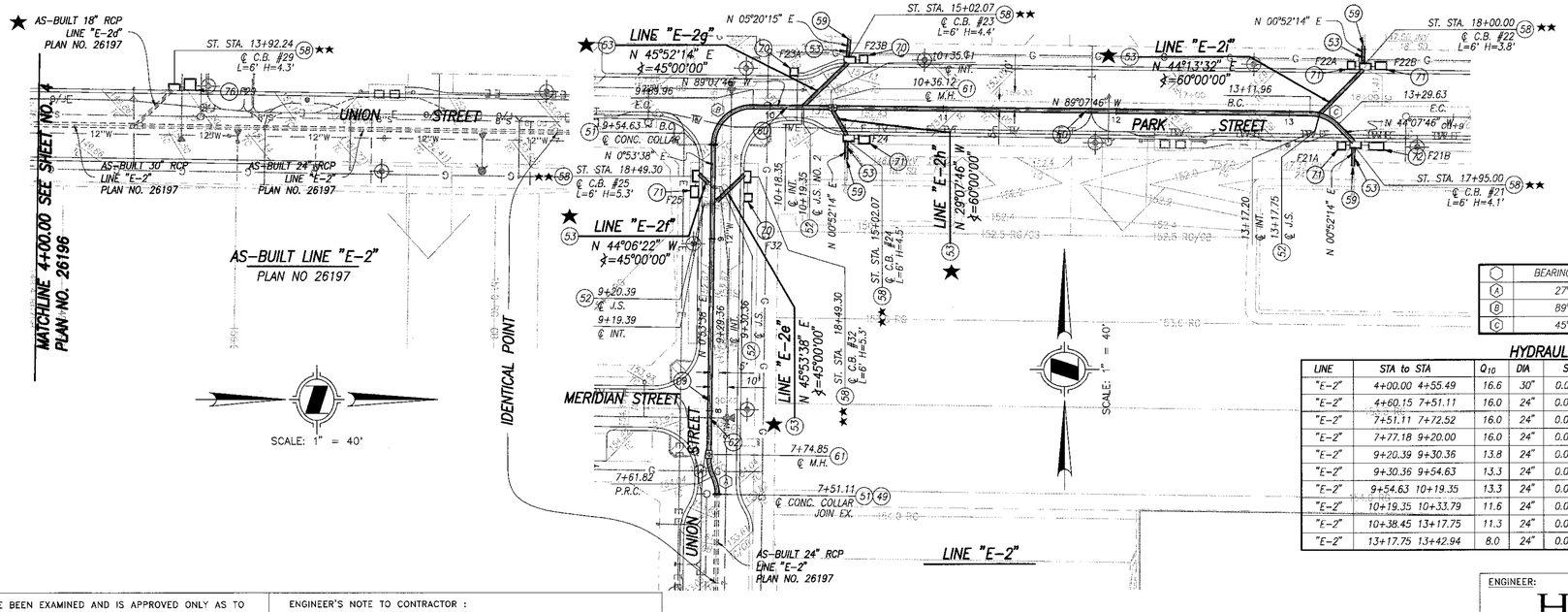
SHEET 4 OF 13
PLAN NO. 36207

BID SET
0-108
FOR CITY USE ONLY
W.O. 3916-14 STORM DRAIN IMPROVEMENT PLANS
DOC: 74426
BIN:
ROOM:
36207

PLOTTED BY: Tiffany Nguyen DATE: Apr. 27, 2016 03:11:16 PM FILE: F:\0108\Engineering\TR_17703\Imp_Storm Drain\SHT-04.dwg



PROFILE SCALES	
HORIZ.:	1"= 40'
VERT.:	1"= 4'



★★ INSTALL AUTOMATIC RETRACTABLE SCREEN AND CONNECTOR PIPE SCREEN, RETRACTABLE SCREEN AND CONNECTOR PIPE SCREEN SHALL BE CONSTRUCTED OF CORROSION RESISTANT MATERIALS AND SHALL BE "FULL CAPTURE" DESIGN AND OF MAINTAINABLE DESIGN. CONTRACTOR SHALL MAKE PRODUCT SUBMITTAL TO ENGINEER FOR APPROVAL PRIOR TO CONSTRUCTION.

CONSTRUCTION NOTES

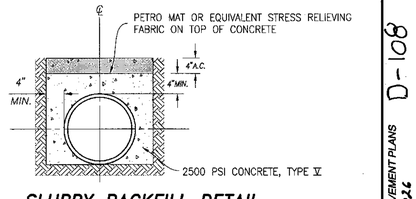
(49) REMOVE PLUG AND JOIN EXISTING
(51) CONST. CONCRETE COLLAR PER SPPWC STD. PLAN NO. 380-4
(52) CONST. J.S. PER SPPWC STD. PLAN NO. 331-3
(53) INST. 18" R.C.P. (SEE PROFILE FOR D-LOAD) BEDDING PER OCPW STD. NO. 1319, DETAIL ON SHEET 13
(58) CONST. C.B. PER CITY OF ANAHEIM STD. DETAIL NO. 300-2, CASE A WITH AUTOMATIC RETRACTABLE SCREEN AND CONNECTOR PIPE SCREEN★★
(59) CONST. DOUBLE BRICK AND MORTAR PLUG
(60) INST. 24" R.C.P. (SEE PROFILE FOR D-LOAD) BEDDING PER OCPW STD. NO. 1319, DETAIL ON SHEET 13
(61) CONST. M.H. PER SPPWC STD. PLAN NO. 321-2
(62) CONST. BLANKET PROTECTION PER SPPWC STD. PLAN NO. 225-2 WITH 2 SACK CEMENT SAND SLURRY (NO CONCRETE)
(70) CONST. 4'x4' FILTERRA BOX WITH 4" SCH-80 PVC OUTLET PIPE
(71) CONST. 4'x6' FILTERRA BOX WITH 4" SCH-80 PVC OUTLET PIPE
(72) CONST. 4'x8' FILTERRA BOX WITH 4" SCH-80 PVC OUTLET PIPE
(76) CONST. 6'x6' FILTERRA BOX WITH 6" SCH-80 PVC OUTLET PIPE
★ PROFILE ON SHEET NO. 6

CURVE DATA

	BEARING/DELTA	RADIUS	LENGTH	TANGENT
(A)	27°15'58"	22.50'	10.71'	5.46'
(B)	89°58'36"	22.50'	35.33'	22.49'
(C)	45°00'00"	22.50'	17.67'	9.32'

HYDRAULIC DATA

LINE	STA to STA	Q_{10}	DIA	So	n	Dc	Vc	Sc	Dn	Vn	Vf	Sf
"E-2"	4+00.00 4+55.49	16.6	30"	0.0071	0.013	1.38	5.99	0.0048	1.22	6.97	3.38	0.0016
"E-2"	4+60.15 7+51.11	16.0	24"	0.0020	0.013	1.44	6.60	0.0066	FULL	---	5.09	0.0050
"E-2"	7+51.11 7+72.52	16.0	24"	0.0040	0.013	1.44	6.60	0.0066	FULL	---	5.09	0.0050
"E-2"	7+77.18 9+20.00	16.0	24"	0.0040	0.013	1.44	6.60	0.0066	FULL	---	5.09	0.0050
"E-2"	9+20.39 9+30.36	13.8	24"	0.0040	0.013	1.34	6.18	0.0060	1.58	5.19	4.39	0.0037
"E-2"	9+30.36 9+54.63	13.3	24"	0.0040	0.013	1.31	6.09	0.0059	1.53	5.17	4.23	0.0035
"E-2"	9+54.63 10+19.35	13.3	24"	0.0030	0.013	1.31	6.09	0.0059	FULL	---	4.23	0.0035
"E-2"	10+19.35 10+33.79	11.6	24"	0.0030	0.013	1.22	5.77	0.0055	1.54	4.48	3.69	0.0026
"E-2"	10+38.45 13+17.75	11.3	24"	0.0020	0.013	1.21	5.71	0.0054	FULL	---	3.60	0.0025
"E-2"	13+17.75 13+42.94	8.0	24"	0.0020	0.013	1.01	5.05	0.0049	1.34	3.57	2.55	0.0013



SLURRY BACKFILL DETAIL

N.T.S.

THESE PLANS HAVE BEEN EXAMINED AND IS APPROVED ONLY AS TO COMPATIBILITY WITH ADJOINING EXISTING OR FUTURE IMPROVEMENTS AND CONFORMANCE WITH CITY OF ANAHEIM STANDARD DETAILS AND THE REQUIREMENTS FOR THE FOLLOWING APPLICABLE FACTORS: RIGHT-OF-WAY, ROADWAY MATERIALS, ALIGNMENTS AND GRADES, HYDROLOGY AND HYDRAULIC DESIGN OF STORMDRAIN OR SANITARY SEWER SYSTEMS AND UNDER GROUND CONDUIT OR OPEN CHANNEL CHANNEL ALIGMENTS, GRADES, SIZES AND MATERIALS.

ENGINEER'S NOTE TO CONTRACTOR :
THE EXISTENCE LOCATION OF ANY UNDERGROUND UTILITIES, PIPES, AND/OR STRUCTURES SHOWN ON THESE PLANS WERE OBTAINED BY A SEARCH OF AVAILABLE RECORDS. TO THE BEST OF OUR KNOWLEDGE, THERE ARE NO EXISTING UTILITIES EXCEPT AS SHOWN ON THESE PLANS. THE CONTRACTOR SHALL AS CERTAIN THE TRUE VERTICAL AND HORIZONTAL LOCATION AND SIZE OF THOSE TO BE USED OF ANY UNDERGROUND UTILITIES AND SHALL BE RESPONSIBLE FOR DAMAGE TO ANY PUBLIC OR PRIVATE UTILITIES AS SHOWN HEREON.

ENGINEER:
H&A HUNSAKER & ASSOCIATES IRVINE, INC.
PLANNING ▪ ENGINEERING ▪ SURVEYING
Three Hughes ▪ Irvine, CA 92618
FX: (949) 583-0759 ▪ PH: (949) 583-1010
EDWARD A. MANDICH, RCE NO. 59089 — DATE 5-6-16
REGISTERED PROFESSIONAL ENGINEER — EDWARD A. MANDICH — No. 59089 — CIVIL — STATE OF CALIFORNIA

RCP 2015-11013
OWNER: PT METRO, LLC, 25 ENTERPRISE, 3rd FLOOR, ALISO VIEJO, CA 92656, (949) 349-8000
SOILS ENGINEER: GROUP DELTA CONSULTANTS, 32 MAUCHLY, SUITE B, IRVINE, CA 92618, PH: (949) 450-2100

FOR CITY USE ONLY — W.O. 3916-14 STORM DRAIN IMPROVEMENT PLANS — BIN: — DOC: 740216

REVISIONS

NO.	INIT.	DATE	DESCRIPTION	DATE	APP'D

REFERENCES

BENCH MARK: 9A-22 ELEVATION: 151.99 (NAVD88)
KATELLA AVENUE/STATE COLLEGE BLVD.
BRASS CAP MKD C OF A BM, IN THE TOP OF CURB AT THE SE ECR.
ELEVATION: 151.99 (NAVD88)

PLANS FOR THESE IMPROVEMENTS: STREET:
SEWER: 36188-36170 WATER: W-3175
STORM DRAIN: 36204-36216
LANDSCAPE:

APPROVAL

FOR APPROVAL, SEE PLAN NO. 36204

STORM DRAIN PLANS TRACT 17703

LINE "E-2" EXTENSION

CITY OF ANAHEIM

SHEET 5 OF 13
PLAN NO. 36208

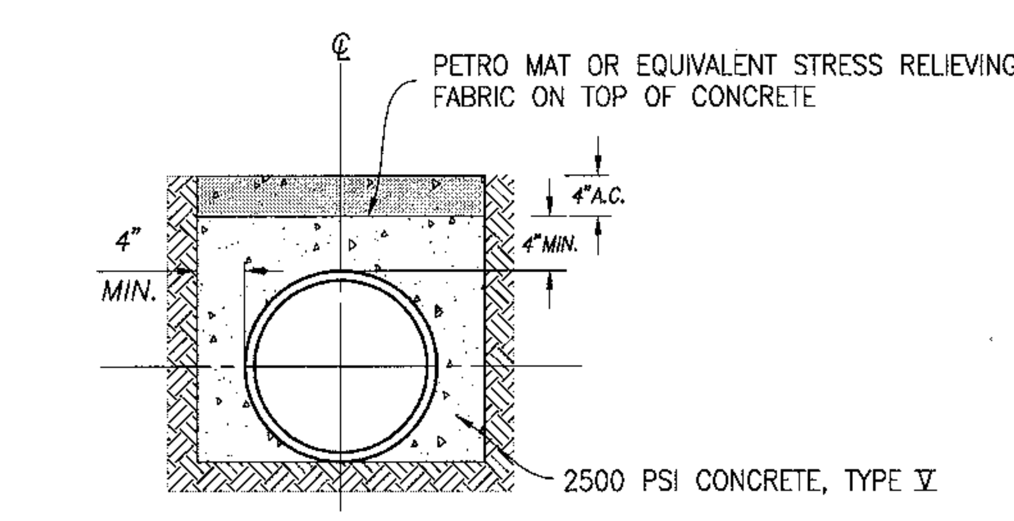
D-108
36208

PLOTTED BY: Tiffany Nguyen DATE: Apr. 27, 2016 03:21:03 PM FILE: F:\0108\Engineering\TR_17703\Imp_Storm Drain\SHT-05.dwg

HYDRAULIC DATA

PLAN ON SHEETS NO 4 & 5

LINE	STA to STA	Q_{10}	DIA	So	n	Dc	Vc	Sc	Dn	Vn	Vf	Sf
"E2c"	---	6.1	18"	0.0150	0.013	0.95	5.14	0.0063	0.73	7.18	3.45	0.0034
"E2d"	---	0.8	18"	0.0040	0.013	0.33	2.74	0.0050	0.35	2.54	0.45	0.0001
"E2e"	---	2.4	18"	0.0020	0.013	0.59	3.75	0.0050	0.76	2.67	1.36	0.0005
"E2f"	---	0.6	18"	0.0020	0.013	0.29	2.54	0.0051	0.36	1.82	0.34	0.0000
"E2g"	---	2.1	18"	0.0020	0.013	0.55	3.60	0.0050	0.70	2.58	1.19	0.0004
"E2h"	---	0.5	18"	0.0020	0.013	0.26	2.42	0.0052	0.33	1.73	0.28	0.0000
"E2i"	---	3.9	18"	0.0020	0.013	0.76	4.37	0.0054	1.04	2.97	2.21	0.0014



SLURRY BACKFILL DETAIL

N.T.S.

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RCP 2015-11013

ENGINEER:
HUNSAKER & ASSOCIATES
IRVINE, INC.
PLANNING • ENGINEERING • SURVEYING
Three Hughes • Irvine, CA 92618
FX: (949) 583-0759 • PH: (949) 583-1010
EDWARD A. MANDICH, RCE NO. 59089 — 5-6-16 DATE
REGISTERED PROFESSIONAL ENGINEER EDWARD A. MANDICH No. 59089 CIVIL STATE OF CALIFORNIA

OWNER:
PT METRO, LLC
25 ENTERPRISE, 3rd FLOOR
ALISO VIEJO, CA 92656
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SOILS ENGINEER:
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32 MAUCHLY, SUITE B
IRVINE, CA 92618
PH: (949) 450-2100

REVISIONS

NO.	INIT.	DATE	DESCRIPTION	DATE	APP'D

REFERENCES

BENCH MARK: 9A-22 ELEVATION: 151.99 (NAVD88)
KATELLA AVENUE/STATE COLLEGE BLVD.
BRASS CAP MKD C OF A BM, IN THE TOP OF CURB AT THE SE ECR.
ELEVATION: 151.99 (NAVD88)

PLANS FOR THESE IMPROVEMENTS: STREET:
SEWER: 36168-36170 WATER: W-3175
STORM DRAIN: 36204-36216
LANDSCAPE:

APPROVAL

FOR APPROVAL, SEE PLAN NO. 36204

STORM DRAIN PLANS
TRACT 17703

LINES "E-2c" - "E-2i"

CITY OF ANAHEIM

SHEET 6 OF 13

PLAN NO. 36209

FOR CITY USE ONLY W.O. 3916-14 STORM DRAIN IMPROVEMENT PLANS ROOM: BIN: DOC: 74026

D-108

36209

APPROVED

HYDROLOGY CALCS AND MAP

FOR A-TOWN TRACT 17703

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**************************************************************************
          RATIONAL METHOD HYDROLOGY COMPUTER PROGRAM PACKAGE
         (Reference: 1986 ORANGE COUNTY HYDROLOGY CRITERION)
       (c) Copyright 1983-2013 Advanced Engineering Software (aes)
           Ver. 20.0  Release Date: 06/01/2013  License ID 1239

                        Analysis prepared by:

                        HUNSAKER & ASSOCIATES
                             Irvine,Inc
                     Planning * Engineering * Surveying
              Three Hughes * Irvine, California 92618 * (949)583-1010

 ************************* DESCRIPTION OF STUDY *************************
* W.O. #1855-150, A-TOWN METRO                                          *
* 10-YR STUDY                                                           *
* PROPOSED CONDITION                                                    *
 **************************************************************************

  FILE NAME: 17703.DAT
  TIME/DATE OF STUDY: 20:01 03/23/2015
==========================================================================
  USER SPECIFIED HYDROLOGY AND HYDRAULIC MODEL INFORMATION:
==========================================================================
                     --*TIME-OF-CONCENTRATION MODEL*--

  USER SPECIFIED STORM EVENT(YEAR) =   10.00
  SPECIFIED MINIMUM PIPE SIZE(INCH) =  18.00
  SPECIFIED PERCENT OF GRADIENTS(DECIMAL) TO USE FOR FRICTION SLOPE = 0.90
  *DATA BANK RAINFALL USED*
  *ANTECEDENT MOISTURE CONDITION (AMC) II ASSUMED FOR RATIONAL METHOD*

  *USER-DEFINED STREET-SECTIONS FOR COUPLED PIPEFLOW AND STREETFLOW MODEL*
     HALF-  CROWN TO   STREET-CROSSFALL:   CURB  GUTTER-GEOMETRIES:  MANNING
     WIDTH  CROSSFALL  IN-  / OUT-/PARK-  HEIGHT  WIDTH  LIP   HIKE  FACTOR
NO.   (FT)     (FT)     SIDE / SIDE/ WAY    (FT)    (FT)  (FT)  (FT)    (n)
===  =====  ========== ================== ======  ===== ====== ===== =======
  1   30.0     20.0    0.018/0.018/0.020   0.67    2.00 0.0312 0.167 0.0150

  GLOBAL STREET FLOW-DEPTH CONSTRAINTS:
    1. Relative Flow-Depth =  0.00 FEET
       as (Maximum Allowable Street Flow Depth) - (Top-of-Curb)
    2. (Depth)*(Velocity) Constraint =  6.0 (FT*FT/S)
  *SIZE PIPE WITH A FLOW CAPACITY GREATER THAN
   OR EQUAL TO THE UPSTREAM TRIBUTARY PIPE.*
  *USER-SPECIFIED MINIMUM TOPOGRAPHIC SLOPE ADJUSTMENT NOT SELECTED

**************************************************************************
  FLOW PROCESS FROM NODE       1.00 TO NODE       2.00 IS CODE =  21
--------------------------------------------------------------------------
  >>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
  >>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<
==========================================================================
  INITIAL SUBAREA FLOW-LENGTH(FEET) =   230.00
  ELEVATION DATA: UPSTREAM(FEET) =    151.75  DOWNSTREAM(FEET) =    149.08
  Tc = K*[(LENGTH** 3.00)/(ELEVATION CHANGE)]**0.20
  SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) =    6.525
  *  10 YEAR RAINFALL INTENSITY(INCH/HR) =  3.485
  SUBAREA Tc AND LOSS RATE DATA(AMC  II):
   DEVELOPMENT TYPE/       SCS SOIL   AREA      Fp         Ap      SCS   Tc
       LAND USE            GROUP   (ACRES)  (INCH/HR)  (DECIMAL)  CN  (MIN.)
  COMMERCIAL                 A        0.20      0.40       0.100    32    6.53
  SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) =  0.40
  SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap =  0.100
  SUBAREA RUNOFF(CFS) =      0.62
  TOTAL AREA(ACRES) =      0.20   PEAK FLOW RATE(CFS) =      0.62

**************************************************************************
  FLOW PROCESS FROM NODE       2.00 TO NODE       3.00 IS CODE =  61
--------------------------------------------------------------------------
  >>>>>COMPUTE STREET FLOW TRAVEL TIME THRU SUBAREA<<<<<
  >>>>>(STANDARD CURB SECTION USED)<<<<<
==========================================================================
  UPSTREAM ELEVATION(FEET) =  149.08  DOWNSTREAM ELEVATION(FEET) =  148.38
  STREET LENGTH(FEET) =   133.00   CURB HEIGHT(INCHES) =  6.0
  STREET HALFWIDTH(FEET) = 21.00

  DISTANCE FROM CROWN TO CROSSFALL GRADEBREAK(FEET) =  16.00
  INSIDE STREET CROSSFALL(DECIMAL) =  0.020
  OUTSIDE STREET CROSSFALL(DECIMAL) =  0.020

  SPECIFIED NUMBER OF HALFSTREETS CARRYING RUNOFF =  1
  STREET PARKWAY CROSSFALL(DECIMAL) =  0.020
  Manning's FRICTION FACTOR for Streetflow Section(curb-to-curb) =   0.0150
  Manning's FRICTION FACTOR for Back-of-Walk Flow Section =   0.0200

    **TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) =       3.21
    STREETFLOW MODEL RESULTS USING ESTIMATED FLOW:
    STREET FLOW DEPTH(FEET) =  0.38
    HALFSTREET FLOOD WIDTH(FEET) =   12.66
    AVERAGE FLOW VELOCITY(FEET/SEC.) =    1.86
    PRODUCT OF DEPTH&VELOCITY(FT*FT/SEC.) =    0.71
  STREET FLOW TRAVEL TIME(MIN.) =   1.19   Tc(MIN.) =    7.71
  *  10 YEAR RAINFALL INTENSITY(INCH/HR) =  3.166
  SUBAREA LOSS RATE DATA(AMC  II):
   DEVELOPMENT TYPE/       SCS SOIL   AREA      Fp         Ap      SCS
       LAND USE            GROUP   (ACRES)  (INCH/HR)  (DECIMAL)  CN
  APARTMENTS                 A        1.86      0.40       0.200    32
  SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) =  0.40
  SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap =  0.200
  SUBAREA AREA(ACRES) =    1.86      SUBAREA RUNOFF(CFS) =    5.17
  EFFECTIVE AREA(ACRES) =      2.06    AREA-AVERAGED Fm(INCH/HR) =  0.08
  AREA-AVERAGED Fp(INCH/HR) =  0.40  AREA-AVERAGED Ap =  0.19
  TOTAL AREA(ACRES) =       2.1       PEAK FLOW RATE(CFS) =       5.73

  END OF SUBAREA STREET FLOW HYDRAULICS:
  DEPTH(FEET) = 0.45   HALFSTREET FLOOD WIDTH(FEET) =  15.97
  FLOW VELOCITY(FEET/SEC.) =  2.15   DEPTH*VELOCITY(FT*FT/SEC.) =   0.96
  LONGEST FLOWPATH FROM NODE       1.00 TO NODE       3.00 =     363.00 FEET.
```

```
****************************************************************************
 FLOW PROCESS FROM NODE       3.00 TO NODE       7.00 IS CODE =  31
----------------------------------------------------------------------------
 >>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
 >>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<
============================================================================
 ELEVATION DATA: UPSTREAM(FEET) =   143.24  DOWNSTREAM(FEET) =   143.15
 FLOW LENGTH(FEET) =    41.00   MANNING'S N =  0.013
 DEPTH OF FLOW IN  21.0 INCH PIPE IS  14.4 INCHES
 PIPE-FLOW VELOCITY(FEET/SEC.) =   3.26
 ESTIMATED PIPE DIAMETER(INCH) =  21.00    NUMBER OF PIPES =   1
 PIPE-FLOW(CFS) =        5.73
 PIPE TRAVEL TIME(MIN.) =   0.21    Tc(MIN.) =    7.92
 LONGEST FLOWPATH FROM NODE       1.00 TO NODE       7.00 =     404.00 FEET.

****************************************************************************
 FLOW PROCESS FROM NODE       7.00 TO NODE       7.00 IS CODE =   1
----------------------------------------------------------------------------
 >>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
============================================================================
 TOTAL NUMBER OF STREAMS =  2
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM  1 ARE:
 TIME OF CONCENTRATION(MIN.) =    7.92
 RAINFALL INTENSITY(INCH/HR) =   3.12
 AREA-AVERAGED Fm(INCH/HR) =  0.08
 AREA-AVERAGED Fp(INCH/HR) =  0.40
 AREA-AVERAGED Ap =  0.19
 EFFECTIVE STREAM AREA(ACRES) =        2.06
 TOTAL STREAM AREA(ACRES) =        2.06
 PEAK FLOW RATE(CFS) AT CONFLUENCE =        5.73

****************************************************************************
 FLOW PROCESS FROM NODE       4.00 TO NODE       5.00 IS CODE =  21
----------------------------------------------------------------------------
 >>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
 >>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<
============================================================================
 INITIAL SUBAREA FLOW-LENGTH(FEET) =   235.00
 ELEVATION DATA: UPSTREAM(FEET) =    151.75  DOWNSTREAM(FEET) =    149.08

 Tc = K*[(LENGTH** 3.00)/(ELEVATION CHANGE)]**0.20
 SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) =    6.610
 *  10 YEAR RAINFALL INTENSITY(INCH/HR) =  3.459
 SUBAREA Tc AND LOSS RATE DATA(AMC  II):
  DEVELOPMENT TYPE/       SCS SOIL   AREA      Fp         Ap      SCS   Tc
     LAND USE             GROUP   (ACRES)  (INCH/HR)  (DECIMAL)  CN  (MIN.)
 COMMERCIAL                 A        0.21      0.40      0.100     32    6.61
 SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) =  0.40
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap =  0.100
 SUBAREA RUNOFF(CFS) =      0.65
 TOTAL AREA(ACRES) =      0.21   PEAK FLOW RATE(CFS) =      0.65

****************************************************************************
 FLOW PROCESS FROM NODE       5.00 TO NODE       6.00 IS CODE =  61
----------------------------------------------------------------------------
 >>>>>COMPUTE STREET FLOW TRAVEL TIME THRU SUBAREA<<<<<
 >>>>>(STANDARD CURB SECTION USED)<<<<<
============================================================================
 UPSTREAM ELEVATION(FEET) =  149.08  DOWNSTREAM ELEVATION(FEET) =  147.65
 STREET LENGTH(FEET) =   185.00   CURB HEIGHT(INCHES) =  6.0
 STREET HALFWIDTH(FEET) = 21.00

 DISTANCE FROM CROWN TO CROSSFALL GRADEBREAK(FEET) =  16.00
 INSIDE STREET CROSSFALL(DECIMAL) =  0.020
 OUTSIDE STREET CROSSFALL(DECIMAL) =  0.020

 SPECIFIED NUMBER OF HALFSTREETS CARRYING RUNOFF =  1
 STREET PARKWAY CROSSFALL(DECIMAL) =  0.020
 Manning's FRICTION FACTOR for Streetflow Section(curb-to-curb) =   0.0150
 Manning's FRICTION FACTOR for Back-of-Walk Flow Section =   0.0200

   **TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) =       0.87
   STREETFLOW MODEL RESULTS USING ESTIMATED FLOW:
   STREET FLOW DEPTH(FEET) =  0.26
   HALFSTREET FLOOD WIDTH(FEET) =    6.53
   AVERAGE FLOW VELOCITY(FEET/SEC.) =    1.60
   PRODUCT OF DEPTH&VELOCITY(FT*FT/SEC.) =    0.41
 STREET FLOW TRAVEL TIME(MIN.) =   1.93   Tc(MIN.) =    8.54
 *  10 YEAR RAINFALL INTENSITY(INCH/HR) =  2.988
 SUBAREA LOSS RATE DATA(AMC  II):
  DEVELOPMENT TYPE/       SCS SOIL   AREA      Fp         Ap      SCS
     LAND USE             GROUP   (ACRES)  (INCH/HR)  (DECIMAL)  CN
 COMMERCIAL                 A        0.17      0.40      0.100     32
 SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) =  0.40
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap =  0.100
 SUBAREA AREA(ACRES) =    0.17      SUBAREA RUNOFF(CFS) =    0.45
 EFFECTIVE AREA(ACRES) =      0.38    AREA-AVERAGED Fm(INCH/HR) =  0.04
 AREA-AVERAGED Fp(INCH/HR) =  0.40  AREA-AVERAGED Ap =  0.10
 TOTAL AREA(ACRES) =        0.4       PEAK FLOW RATE(CFS) =        1.01

 END OF SUBAREA STREET FLOW HYDRAULICS:
 DEPTH(FEET) = 0.27   HALFSTREET FLOOD WIDTH(FEET) =   6.97
 FLOW VELOCITY(FEET/SEC.) =  1.67   DEPTH*VELOCITY(FT*FT/SEC.) =   0.44
 LONGEST FLOWPATH FROM NODE       4.00 TO NODE       6.00 =     420.00 FEET.

****************************************************************************
 FLOW PROCESS FROM NODE       6.00 TO NODE       6.00 IS CODE =  81
----------------------------------------------------------------------------
 >>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<
============================================================================
 MAINLINE Tc(MIN.) =    8.54
 *  10 YEAR RAINFALL INTENSITY(INCH/HR) =  2.988
 SUBAREA LOSS RATE DATA(AMC  II):
  DEVELOPMENT TYPE/       SCS SOIL   AREA      Fp         Ap      SCS
     LAND USE             GROUP   (ACRES)  (INCH/HR)  (DECIMAL)  CN
 APARTMENTS                 A        1.76      0.40      0.200     32
 SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) =  0.40
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap =  0.200
 SUBAREA AREA(ACRES) =    1.76      SUBAREA RUNOFF(CFS) =    4.61
 EFFECTIVE AREA(ACRES) =      2.14    AREA-AVERAGED Fm(INCH/HR) =  0.07
 AREA-AVERAGED Fp(INCH/HR) =  0.40  AREA-AVERAGED Ap =  0.18
 TOTAL AREA(ACRES) =        2.1       PEAK FLOW RATE(CFS) =        5.61
```

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****************************************************************************
 FLOW PROCESS FROM NODE       6.00 TO NODE       6.00 IS CODE =  81
----------------------------------------------------------------------------
 >>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<
============================================================================
 MAINLINE Tc(MIN.) =     8.54
 *  10 YEAR RAINFALL INTENSITY(INCH/HR) =  2.988
 SUBAREA LOSS RATE DATA(AMC  II):
  DEVELOPMENT TYPE/       SCS SOIL   AREA      Fp         Ap      SCS
      LAND USE             GROUP   (ACRES)  (INCH/HR)  (DECIMAL)  CN
 COMMERCIAL                  A        0.38      0.40      0.100    32
 SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) =  0.40
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap =  0.100
 SUBAREA AREA(ACRES) =     0.38      SUBAREA RUNOFF(CFS) =     1.01
 EFFECTIVE AREA(ACRES) =      2.52   AREA-AVERAGED Fm(INCH/HR) =  0.07
 AREA-AVERAGED Fp(INCH/HR) =  0.40  AREA-AVERAGED Ap =  0.17
 TOTAL AREA(ACRES) =        2.5       PEAK FLOW RATE(CFS) =       6.62

****************************************************************************
 FLOW PROCESS FROM NODE       6.00 TO NODE       7.00 IS CODE =  31
----------------------------------------------------------------------------
 >>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
 >>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<
============================================================================
 ELEVATION DATA: UPSTREAM(FEET) =    143.49  DOWNSTREAM(FEET) =    143.15
 FLOW LENGTH(FEET) =     13.50   MANNING'S N =  0.013
 ESTIMATED PIPE DIAMETER(INCH) INCREASED TO 18.000
 DEPTH OF FLOW IN  18.0 INCH PIPE IS    8.1 INCHES
 PIPE-FLOW VELOCITY(FEET/SEC.) =    8.55
 ESTIMATED PIPE DIAMETER(INCH) =  18.00    NUMBER OF PIPES =    1
 PIPE-FLOW(CFS) =        6.62
 PIPE TRAVEL TIME(MIN.) =    0.03    Tc(MIN.) =     8.56
 LONGEST FLOWPATH FROM NODE       4.00 TO NODE       7.00 =      433.50 FEET.

****************************************************************************
 FLOW PROCESS FROM NODE       7.00 TO NODE       7.00 IS CODE =   1
----------------------------------------------------------------------------
 >>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
 >>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<
============================================================================
 TOTAL NUMBER OF STREAMS =  2
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM  2 ARE:
 TIME OF CONCENTRATION(MIN.) =     8.56
 RAINFALL INTENSITY(INCH/HR) =    2.98
 AREA-AVERAGED Fm(INCH/HR) =  0.07
 AREA-AVERAGED Fp(INCH/HR) =  0.40
 AREA-AVERAGED Ap =  0.17
 EFFECTIVE STREAM AREA(ACRES) =       2.52
 TOTAL STREAM AREA(ACRES) =       2.52
 PEAK FLOW RATE(CFS) AT CONFLUENCE =       6.62

 ** CONFLUENCE DATA **
  STREAM       Q       Tc    Intensity   Fp(Fm)      Ap      Ae      HEADWATER
  NUMBER     (CFS)   (MIN.) (INCH/HR) (INCH/HR)           (ACRES)     NODE
     1        5.73     7.92     3.118  0.40( 0.08) 0.19        2.1        1.00
     2        6.62     8.56     2.983  0.40( 0.07) 0.17        2.5        4.00

 RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
 CONFLUENCE FORMULA USED FOR  2 STREAMS.

 ** PEAK FLOW RATE TABLE **
  STREAM       Q       Tc    Intensity   Fp(Fm)      Ap      Ae      HEADWATER
  NUMBER     (CFS)   (MIN.) (INCH/HR) (INCH/HR)           (ACRES)     NODE
     1       12.14     7.92     3.118  0.40( 0.07) 0.18        4.4        1.00
     2       12.10     8.56     2.983  0.40( 0.07) 0.18        4.6        4.00

 COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
 PEAK FLOW RATE(CFS) =      12.14    Tc(MIN.) =     7.92
 EFFECTIVE AREA(ACRES) =       4.39   AREA-AVERAGED Fm(INCH/HR) =  0.07
 AREA-AVERAGED Fp(INCH/HR) =  0.40  AREA-AVERAGED Ap =  0.18
 TOTAL AREA(ACRES) =        4.6
 LONGEST FLOWPATH FROM NODE       4.00 TO NODE       7.00 =      433.50 FEET.

****************************************************************************
 FLOW PROCESS FROM NODE       7.00 TO NODE       8.00 IS CODE =  31
----------------------------------------------------------------------------
 >>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
 >>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<
============================================================================
 ELEVATION DATA: UPSTREAM(FEET) =    143.15  DOWNSTREAM(FEET) =    143.04
 FLOW LENGTH(FEET) =     55.80   MANNING'S N =  0.013
 DEPTH OF FLOW IN  27.0 INCH PIPE IS   20.6 INCHES
 PIPE-FLOW VELOCITY(FEET/SEC.) =    3.73
 ESTIMATED PIPE DIAMETER(INCH) =  27.00    NUMBER OF PIPES =    1
 PIPE-FLOW(CFS) =       12.14
 PIPE TRAVEL TIME(MIN.) =    0.25    Tc(MIN.) =     8.17
 LONGEST FLOWPATH FROM NODE       4.00 TO NODE       8.00 =      489.30 FEET.

****************************************************************************
 FLOW PROCESS FROM NODE       8.00 TO NODE       8.00 IS CODE =  81
----------------------------------------------------------------------------
 >>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<
============================================================================
 MAINLINE Tc(MIN.) =     8.17
 *  10 YEAR RAINFALL INTENSITY(INCH/HR) =  3.063
 SUBAREA LOSS RATE DATA(AMC  II):
  DEVELOPMENT TYPE/       SCS SOIL   AREA      Fp         Ap      SCS
      LAND USE             GROUP   (ACRES)  (INCH/HR)  (DECIMAL)  CN
 COMMERCIAL                  A        0.66      0.40      0.100    32
 SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) =  0.40
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap =  0.100
 SUBAREA AREA(ACRES) =     0.66      SUBAREA RUNOFF(CFS) =     1.80
 EFFECTIVE AREA(ACRES) =      5.05   AREA-AVERAGED Fm(INCH/HR) =  0.07
 AREA-AVERAGED Fp(INCH/HR) =  0.40  AREA-AVERAGED Ap =  0.17
 TOTAL AREA(ACRES) =        5.2       PEAK FLOW RATE(CFS) =       13.62

****************************************************************************
 FLOW PROCESS FROM NODE      11.00 TO NODE      12.00 IS CODE =  21
----------------------------------------------------------------------------
 >>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
 >>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<
```

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==========================================================================
 INITIAL SUBAREA FLOW-LENGTH(FEET) =   300.00
 ELEVATION DATA: UPSTREAM(FEET) =    152.08  DOWNSTREAM(FEET) =    148.28

 Tc = K*[(LENGTH** 3.00)/(ELEVATION CHANGE)]**0.20
 SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) =    7.132
 *  10 YEAR RAINFALL INTENSITY(INCH/HR) =  3.312
 SUBAREA Tc AND LOSS RATE DATA(AMC  II):
  DEVELOPMENT TYPE/       SCS SOIL   AREA      Fp         Ap      SCS   Tc
      LAND USE            GROUP   (ACRES)  (INCH/HR)  (DECIMAL)  CN  (MIN.)
 COMMERCIAL                 A        0.35      0.40      0.100    32    7.13
 SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) =  0.40
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap =  0.100
 SUBAREA RUNOFF(CFS) =      1.03
 TOTAL AREA(ACRES) =      0.35   PEAK FLOW RATE(CFS) =      1.03

**************************************************************************
 FLOW PROCESS FROM NODE     12.00 TO NODE     13.00 IS CODE =  61
--------------------------------------------------------------------------
 >>>>>COMPUTE STREET FLOW TRAVEL TIME THRU SUBAREA<<<<<
 >>>>>(STANDARD CURB SECTION USED)<<<<<
==========================================================================
 UPSTREAM ELEVATION(FEET) =  148.28  DOWNSTREAM ELEVATION(FEET) =  147.40
 STREET LENGTH(FEET) =   113.00   CURB HEIGHT(INCHES) =  6.0
 STREET HALFWIDTH(FEET) = 24.00

 DISTANCE FROM CROWN TO CROSSFALL GRADEBREAK(FEET) =  19.00
 INSIDE STREET CROSSFALL(DECIMAL) =  0.020
 OUTSIDE STREET CROSSFALL(DECIMAL) =  0.020

 SPECIFIED NUMBER OF HALFSTREETS CARRYING RUNOFF =  1
 STREET PARKWAY CROSSFALL(DECIMAL) =  0.020
 Manning's FRICTION FACTOR for Streetflow Section(curb-to-curb) =   0.0150
 Manning's FRICTION FACTOR for Back-of-Walk Flow Section =   0.0200

   **TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) =      1.38
   STREETFLOW MODEL RESULTS USING ESTIMATED FLOW:
   STREET FLOW DEPTH(FEET) =  0.29
   HALFSTREET FLOOD WIDTH(FEET) =    8.08
   AVERAGE FLOW VELOCITY(FEET/SEC.) =    1.80
   PRODUCT OF DEPTH&VELOCITY(FT*FT/SEC.) =    0.52
 STREET FLOW TRAVEL TIME(MIN.) =   1.05   Tc(MIN.) =    8.18
 *  10 YEAR RAINFALL INTENSITY(INCH/HR) =  3.062
 SUBAREA LOSS RATE DATA(AMC  II):
  DEVELOPMENT TYPE/       SCS SOIL   AREA      Fp         Ap      SCS
      LAND USE            GROUP   (ACRES)  (INCH/HR)  (DECIMAL)  CN
 COMMERCIAL                 A        0.26      0.40      0.100    32
 SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) =  0.40
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap =  0.100
 SUBAREA AREA(ACRES) =    0.26      SUBAREA RUNOFF(CFS) =    0.71
 EFFECTIVE AREA(ACRES) =      0.61    AREA-AVERAGED Fm(INCH/HR) =  0.04
 AREA-AVERAGED Fp(INCH/HR) =  0.40  AREA-AVERAGED Ap =  0.10
 TOTAL AREA(ACRES) =       0.6       PEAK FLOW RATE(CFS) =      1.66

 END OF SUBAREA STREET FLOW HYDRAULICS:
 DEPTH(FEET) = 0.30   HALFSTREET FLOOD WIDTH(FEET) =   8.82
 FLOW VELOCITY(FEET/SEC.) =  1.85   DEPTH*VELOCITY(FT*FT/SEC.) =   0.56
 LONGEST FLOWPATH FROM NODE     11.00 TO NODE     13.00 =     413.00 FEET.

**************************************************************************
 FLOW PROCESS FROM NODE     13.00 TO NODE     13.00 IS CODE =  81
--------------------------------------------------------------------------
 >>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<
==========================================================================
 MAINLINE Tc(MIN.) =    8.18
 *  10 YEAR RAINFALL INTENSITY(INCH/HR) =  3.062
 SUBAREA LOSS RATE DATA(AMC  II):
  DEVELOPMENT TYPE/       SCS SOIL   AREA      Fp         Ap      SCS
      LAND USE            GROUP   (ACRES)  (INCH/HR)  (DECIMAL)  CN
 APARTMENTS                 A        1.96      0.40      0.200    32
 SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) =  0.40
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap =  0.200
 SUBAREA AREA(ACRES) =    1.96      SUBAREA RUNOFF(CFS) =    5.26
 EFFECTIVE AREA(ACRES) =      2.57   AREA-AVERAGED Fm(INCH/HR) =  0.07
 AREA-AVERAGED Fp(INCH/HR) =  0.40  AREA-AVERAGED Ap =  0.18
 TOTAL AREA(ACRES) =       2.6       PEAK FLOW RATE(CFS) =       6.92

**************************************************************************
 FLOW PROCESS FROM NODE     13.00 TO NODE     14.00 IS CODE =  31
--------------------------------------------------------------------------
 >>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
 >>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<
==========================================================================
 ELEVATION DATA: UPSTREAM(FEET) =   143.21  DOWNSTREAM(FEET) =   143.15
 FLOW LENGTH(FEET) =    28.60   MANNING'S N =  0.013
 DEPTH OF FLOW IN  24.0 INCH PIPE IS  14.9 INCHES
 PIPE-FLOW VELOCITY(FEET/SEC.) =   3.39
 ESTIMATED PIPE DIAMETER(INCH) =  24.00    NUMBER OF PIPES =   1
 PIPE-FLOW(CFS) =       6.92
 PIPE TRAVEL TIME(MIN.) =   0.14    Tc(MIN.) =    8.32
 LONGEST FLOWPATH FROM NODE     11.00 TO NODE     14.00 =     441.60 FEET.

**************************************************************************
 FLOW PROCESS FROM NODE     14.00 TO NODE     14.00 IS CODE =  81
--------------------------------------------------------------------------
 >>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<
==========================================================================
 MAINLINE Tc(MIN.) =    8.32
 *  10 YEAR RAINFALL INTENSITY(INCH/HR) =  3.032
 SUBAREA LOSS RATE DATA(AMC  II):
  DEVELOPMENT TYPE/       SCS SOIL   AREA      Fp         Ap      SCS
      LAND USE            GROUP   (ACRES)  (INCH/HR)  (DECIMAL)  CN
 COMMERCIAL                 A        0.77      0.40      0.100    32
 SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) =  0.40
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap =  0.100
 SUBAREA AREA(ACRES) =    0.77      SUBAREA RUNOFF(CFS) =    2.07
 EFFECTIVE AREA(ACRES) =      3.34   AREA-AVERAGED Fm(INCH/HR) =  0.06
 AREA-AVERAGED Fp(INCH/HR) =  0.40  AREA-AVERAGED Ap =  0.16
 TOTAL AREA(ACRES) =       3.3       PEAK FLOW RATE(CFS) =       8.92

**************************************************************************
 FLOW PROCESS FROM NODE     21.00 TO NODE     22.00 IS CODE =  21
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------------------------------------------------------------------------
 >>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
 >>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<
========================================================================
 INITIAL SUBAREA FLOW-LENGTH(FEET) =   245.00
 ELEVATION DATA: UPSTREAM(FEET) =    154.00  DOWNSTREAM(FEET) =    151.70

 Tc = K*[(LENGTH** 3.00)/(ELEVATION CHANGE)]**0.20
 SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) =    7.442
 *  10 YEAR RAINFALL INTENSITY(INCH/HR) =  3.232
 SUBAREA Tc AND LOSS RATE DATA(AMC  II):
  DEVELOPMENT TYPE/       SCS SOIL   AREA      Fp         Ap      SCS   Tc
      LAND USE            GROUP   (ACRES)  (INCH/HR)  (DECIMAL)  CN  (MIN.)
 APARTMENTS                 A        0.66      0.40      0.200    32    7.44
 SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) =  0.40
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap =  0.200
 SUBAREA RUNOFF(CFS) =       1.87
 TOTAL AREA(ACRES) =       0.66   PEAK FLOW RATE(CFS) =       1.87

************************************************************************
 FLOW PROCESS FROM NODE      22.00 TO NODE      23.00 IS CODE =  61
------------------------------------------------------------------------
 >>>>>COMPUTE STREET FLOW TRAVEL TIME THRU SUBAREA<<<<<
 >>>>>(STANDARD CURB SECTION USED)<<<<<
========================================================================
 UPSTREAM ELEVATION(FEET) =  151.70  DOWNSTREAM ELEVATION(FEET) =  150.90
 STREET LENGTH(FEET) =   325.00   CURB HEIGHT(INCHES) =  6.0
 STREET HALFWIDTH(FEET) = 15.00

 DISTANCE FROM CROWN TO CROSSFALL GRADEBREAK(FEET) =  10.00
 INSIDE STREET CROSSFALL(DECIMAL) =  0.020
 OUTSIDE STREET CROSSFALL(DECIMAL) =  0.020

 SPECIFIED NUMBER OF HALFSTREETS CARRYING RUNOFF =  1
 STREET PARKWAY CROSSFALL(DECIMAL) =  0.020
 Manning's FRICTION FACTOR for Streetflow Section(curb-to-curb) =   0.0150
 Manning's FRICTION FACTOR for Back-of-Walk Flow Section =   0.0200

   **TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) =       4.37
   ***STREET FLOW SPLITS OVER STREET-CROWN***
   FULL DEPTH(FEET) =    0.43   FLOOD WIDTH(FEET) =   15.00
   FULL HALF-STREET VELOCITY(FEET/SEC.) =    1.41
   SPLIT DEPTH(FEET) =    0.31   SPLIT FLOOD WIDTH(FEET) =    9.26
   SPLIT FLOW(CFS) =    1.04   SPLIT VELOCITY(FEET/SEC.) =    1.06
   STREETFLOW MODEL RESULTS USING ESTIMATED FLOW:
   STREET FLOW DEPTH(FEET) =  0.43
   HALFSTREET FLOOD WIDTH(FEET) =   15.00
   AVERAGE FLOW VELOCITY(FEET/SEC.) =    1.41
   PRODUCT OF DEPTH&VELOCITY(FT*FT/SEC.) =    0.60
 STREET FLOW TRAVEL TIME(MIN.) =   3.85   Tc(MIN.) =   11.29
 *  10 YEAR RAINFALL INTENSITY(INCH/HR) =  2.545
 SUBAREA LOSS RATE DATA(AMC  II):
  DEVELOPMENT TYPE/       SCS SOIL   AREA      Fp         Ap      SCS
      LAND USE            GROUP   (ACRES)  (INCH/HR)  (DECIMAL)  CN
 APARTMENTS                 A        2.25      0.40      0.200    32
 SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) =  0.40
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap =  0.200
 SUBAREA AREA(ACRES) =     2.25      SUBAREA RUNOFF(CFS) =     4.99
 EFFECTIVE AREA(ACRES) =      2.91    AREA-AVERAGED Fm(INCH/HR) =  0.08
 AREA-AVERAGED Fp(INCH/HR) =  0.40  AREA-AVERAGED Ap =  0.20
 TOTAL AREA(ACRES) =       2.9       PEAK FLOW RATE(CFS) =       6.46

 END OF SUBAREA STREET FLOW HYDRAULICS:
 DEPTH(FEET) = 0.43   HALFSTREET FLOOD WIDTH(FEET) =  15.00
 FLOW VELOCITY(FEET/SEC.) =  1.41   DEPTH*VELOCITY(FT*FT/SEC.) =   0.60
 LONGEST FLOWPATH FROM NODE     21.00 TO NODE     23.00 =     570.00 FEET.

************************************************************************
 FLOW PROCESS FROM NODE      23.00 TO NODE      24.00 IS CODE =  31
------------------------------------------------------------------------
 >>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
 >>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<
========================================================================
 ELEVATION DATA: UPSTREAM(FEET) =   145.49  DOWNSTREAM(FEET) =   145.28
 FLOW LENGTH(FEET) =    89.00   MANNING'S N =  0.013
 DEPTH OF FLOW IN  21.0 INCH PIPE IS  15.3 INCHES
 PIPE-FLOW VELOCITY(FEET/SEC.) =   3.43
 ESTIMATED PIPE DIAMETER(INCH) =  21.00    NUMBER OF PIPES =   1
 PIPE-FLOW(CFS) =       6.46
 PIPE TRAVEL TIME(MIN.) =   0.43    Tc(MIN.) =   11.73
 LONGEST FLOWPATH FROM NODE     21.00 TO NODE     24.00 =     659.00 FEET.

************************************************************************
 FLOW PROCESS FROM NODE      24.00 TO NODE      24.00 IS CODE =  81
------------------------------------------------------------------------
 >>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<
========================================================================
 MAINLINE Tc(MIN.) =   11.73
 *  10 YEAR RAINFALL INTENSITY(INCH/HR) =  2.491
 SUBAREA LOSS RATE DATA(AMC  II):
  DEVELOPMENT TYPE/       SCS SOIL   AREA      Fp         Ap      SCS
      LAND USE            GROUP   (ACRES)  (INCH/HR)  (DECIMAL)  CN
 COMMERCIAL                 A        0.48      0.40      0.100    32
 SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) =  0.40
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap =  0.100
 SUBAREA AREA(ACRES) =     0.48      SUBAREA RUNOFF(CFS) =     1.06
 EFFECTIVE AREA(ACRES) =      3.39   AREA-AVERAGED Fm(INCH/HR) =  0.07
 AREA-AVERAGED Fp(INCH/HR) =  0.40  AREA-AVERAGED Ap =  0.19
 TOTAL AREA(ACRES) =       3.4       PEAK FLOW RATE(CFS) =       7.37

************************************************************************
 FLOW PROCESS FROM NODE      24.00 TO NODE      30.00 IS CODE =  31
------------------------------------------------------------------------
 >>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
 >>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<
========================================================================
 ELEVATION DATA: UPSTREAM(FEET) =   145.28  DOWNSTREAM(FEET) =   143.40
 FLOW LENGTH(FEET) =   348.60   MANNING'S N =  0.013
 DEPTH OF FLOW IN  21.0 INCH PIPE IS  12.6 INCHES
 PIPE-FLOW VELOCITY(FEET/SEC.) =   4.91
 ESTIMATED PIPE DIAMETER(INCH) =  21.00    NUMBER OF PIPES =   1
 PIPE-FLOW(CFS) =       7.37
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 PIPE TRAVEL TIME(MIN.) =   1.18    Tc(MIN.) =   12.91
 LONGEST FLOWPATH FROM NODE     21.00 TO NODE     30.00 =    1007.60 FEET.

****************************************************************************
 FLOW PROCESS FROM NODE     30.00 TO NODE     30.00 IS CODE =   1
----------------------------------------------------------------------------
 >>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
============================================================================
 TOTAL NUMBER OF STREAMS =  2
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM  1 ARE:
 TIME OF CONCENTRATION(MIN.) =   12.91
 RAINFALL INTENSITY(INCH/HR) =   2.36
 AREA-AVERAGED Fm(INCH/HR) =  0.07
 AREA-AVERAGED Fp(INCH/HR) =  0.40
 AREA-AVERAGED Ap =  0.19
 EFFECTIVE STREAM AREA(ACRES) =       3.39
 TOTAL STREAM AREA(ACRES) =       3.39
 PEAK FLOW RATE(CFS) AT CONFLUENCE =       7.37

****************************************************************************
 FLOW PROCESS FROM NODE     25.00 TO NODE     26.00 IS CODE =  21
----------------------------------------------------------------------------
 >>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
 >>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<
============================================================================
 INITIAL SUBAREA FLOW-LENGTH(FEET) =   250.00
 ELEVATION DATA: UPSTREAM(FEET) =    153.50  DOWNSTREAM(FEET) =    151.00

 Tc = K*[(LENGTH** 3.00)/(ELEVATION CHANGE)]**0.20
 SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) =    7.408
 *  10 YEAR RAINFALL INTENSITY(INCH/HR) =  3.241
 SUBAREA Tc AND LOSS RATE DATA(AMC  II):
  DEVELOPMENT TYPE/       SCS SOIL   AREA      Fp         Ap      SCS   Tc
      LAND USE             GROUP   (ACRES)  (INCH/HR)  (DECIMAL)  CN  (MIN.)
 APARTMENTS                  A        0.60      0.40      0.200    32    7.41
 SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) =  0.40
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap =  0.200
 SUBAREA RUNOFF(CFS) =       1.71
 TOTAL AREA(ACRES) =       0.60    PEAK FLOW RATE(CFS) =       1.71

****************************************************************************
 FLOW PROCESS FROM NODE     26.00 TO NODE     27.00 IS CODE =  51
----------------------------------------------------------------------------
 >>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<<
 >>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<<
============================================================================
 ELEVATION DATA: UPSTREAM(FEET) =    151.00  DOWNSTREAM(FEET) =    150.00
 CHANNEL LENGTH THRU SUBAREA(FEET) =   110.00   CHANNEL SLOPE =  0.0091
 CHANNEL BASE(FEET) =    5.00   "Z" FACTOR =   5.000
 MANNING'S FACTOR = 0.015   MAXIMUM DEPTH(FEET) =   1.00
 *  10 YEAR RAINFALL INTENSITY(INCH/HR) =  3.076
 SUBAREA LOSS RATE DATA(AMC  II):
  DEVELOPMENT TYPE/       SCS SOIL   AREA      Fp         Ap      SCS
      LAND USE             GROUP   (ACRES)  (INCH/HR)  (DECIMAL)  CN
 APARTMENTS                  A        0.56      0.40      0.200    32
 SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) =  0.40
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap =  0.200
 TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) =       2.46
 TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) =   2.60
 AVERAGE FLOW DEPTH(FEET) =   0.16   TRAVEL TIME(MIN.) =   0.71
 Tc(MIN.) =    8.11
 SUBAREA AREA(ACRES) =      0.56      SUBAREA RUNOFF(CFS) =    1.51
 EFFECTIVE AREA(ACRES) =      1.16     AREA-AVERAGED Fm(INCH/HR) =   0.08
 AREA-AVERAGED Fp(INCH/HR) =   0.40  AREA-AVERAGED Ap =   0.20
 TOTAL AREA(ACRES) =       1.2       PEAK FLOW RATE(CFS) =       3.13

 END OF SUBAREA CHANNEL FLOW HYDRAULICS:
 DEPTH(FEET) =  0.19   FLOW VELOCITY(FEET/SEC.) =   2.80
 LONGEST FLOWPATH FROM NODE     25.00 TO NODE     27.00 =     360.00 FEET.

****************************************************************************
 FLOW PROCESS FROM NODE     27.00 TO NODE     28.00 IS CODE =  31
----------------------------------------------------------------------------
 >>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
 >>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<
============================================================================
 ELEVATION DATA: UPSTREAM(FEET) =   144.50  DOWNSTREAM(FEET) =   144.30
 FLOW LENGTH(FEET) =    70.00   MANNING'S N =  0.013
 ESTIMATED PIPE DIAMETER(INCH) INCREASED TO 18.000
 DEPTH OF FLOW IN  18.0 INCH PIPE IS   9.9 INCHES
 PIPE-FLOW VELOCITY(FEET/SEC.) =   3.14
 ESTIMATED PIPE DIAMETER(INCH) =  18.00    NUMBER OF PIPES =   1
 PIPE-FLOW(CFS) =       3.13
 PIPE TRAVEL TIME(MIN.) =   0.37    Tc(MIN.) =    8.49
 LONGEST FLOWPATH FROM NODE     25.00 TO NODE     28.00 =     430.00 FEET.

****************************************************************************
 FLOW PROCESS FROM NODE     28.00 TO NODE     28.00 IS CODE =  81
----------------------------------------------------------------------------
 >>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<
============================================================================
 MAINLINE Tc(MIN.) =    8.49
 *  10 YEAR RAINFALL INTENSITY(INCH/HR) =  2.998
 SUBAREA LOSS RATE DATA(AMC  II):
  DEVELOPMENT TYPE/       SCS SOIL   AREA      Fp         Ap      SCS
      LAND USE             GROUP   (ACRES)  (INCH/HR)  (DECIMAL)  CN
 APARTMENTS                  A        1.54      0.40      0.200    32
 SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) =  0.40
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap =  0.200
 SUBAREA AREA(ACRES) =    1.54      SUBAREA RUNOFF(CFS) =    4.04
 EFFECTIVE AREA(ACRES) =      2.70   AREA-AVERAGED Fm(INCH/HR) =  0.08
 AREA-AVERAGED Fp(INCH/HR) =  0.40  AREA-AVERAGED Ap =  0.20
 TOTAL AREA(ACRES) =       2.7       PEAK FLOW RATE(CFS) =       7.09

****************************************************************************
 FLOW PROCESS FROM NODE     28.00 TO NODE     29.00 IS CODE =  31
----------------------------------------------------------------------------
 >>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
 >>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<
============================================================================
 ELEVATION DATA: UPSTREAM(FEET) =   144.30  DOWNSTREAM(FEET) =   144.00
 FLOW LENGTH(FEET) =    85.40   MANNING'S N =  0.013
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```
 DEPTH OF FLOW IN  21.0 INCH PIPE IS  14.2 INCHES
 PIPE-FLOW VELOCITY(FEET/SEC.) =  4.11
 ESTIMATED PIPE DIAMETER(INCH) =  21.00    NUMBER OF PIPES =  1
 PIPE-FLOW(CFS) =       7.09
 PIPE TRAVEL TIME(MIN.) =  0.35    Tc(MIN.) =    8.83
 LONGEST FLOWPATH FROM NODE     25.00 TO NODE     29.00 =     515.40 FEET.

****************************************************************************
 FLOW PROCESS FROM NODE     29.00 TO NODE     29.00 IS CODE =  81
----------------------------------------------------------------------------
 >>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<
============================================================================
 MAINLINE Tc(MIN.) =    8.83
 *  10 YEAR RAINFALL INTENSITY(INCH/HR) =  2.930
 SUBAREA LOSS RATE DATA(AMC  II):
  DEVELOPMENT TYPE/       SCS SOIL   AREA      Fp         Ap      SCS
      LAND USE             GROUP   (ACRES)  (INCH/HR)  (DECIMAL)  CN
 COMMERCIAL                 A        0.71      0.40      0.100    32
 SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) =  0.40
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap =  0.100
 SUBAREA AREA(ACRES) =    0.71     SUBAREA RUNOFF(CFS) =    1.85
 EFFECTIVE AREA(ACRES) =     3.41   AREA-AVERAGED Fm(INCH/HR) =  0.07
 AREA-AVERAGED Fp(INCH/HR) =  0.40  AREA-AVERAGED Ap =  0.18
 TOTAL AREA(ACRES) =      3.4      PEAK FLOW RATE(CFS) =      8.77

****************************************************************************
 FLOW PROCESS FROM NODE     29.00 TO NODE     30.00 IS CODE =  31
----------------------------------------------------------------------------
 >>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
 >>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<
============================================================================
 ELEVATION DATA: UPSTREAM(FEET) =   144.00  DOWNSTREAM(FEET) =   143.40
 FLOW LENGTH(FEET) =    43.20   MANNING'S N =  0.013
 DEPTH OF FLOW IN  18.0 INCH PIPE IS  11.6 INCHES
 PIPE-FLOW VELOCITY(FEET/SEC.) =  7.28
 ESTIMATED PIPE DIAMETER(INCH) =  18.00    NUMBER OF PIPES =  1
 PIPE-FLOW(CFS) =       8.77
 PIPE TRAVEL TIME(MIN.) =  0.10    Tc(MIN.) =    8.93
 LONGEST FLOWPATH FROM NODE     25.00 TO NODE     30.00 =     558.60 FEET.

****************************************************************************
 FLOW PROCESS FROM NODE     30.00 TO NODE     30.00 IS CODE =  1
----------------------------------------------------------------------------
 >>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
 >>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<
============================================================================
 TOTAL NUMBER OF STREAMS =  2
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM  2 ARE:
 TIME OF CONCENTRATION(MIN.) =    8.93
 RAINFALL INTENSITY(INCH/HR) =  2.91
 AREA-AVERAGED Fm(INCH/HR) =  0.07
 AREA-AVERAGED Fp(INCH/HR) =  0.40
 AREA-AVERAGED Ap =  0.18
 EFFECTIVE STREAM AREA(ACRES) =      3.41
 TOTAL STREAM AREA(ACRES) =      3.41
 PEAK FLOW RATE(CFS) AT CONFLUENCE =      8.77

 ** CONFLUENCE DATA **
  STREAM       Q      Tc   Intensity   Fp(Fm)      Ap      Ae      HEADWATER
  NUMBER     (CFS)  (MIN.) (INCH/HR) (INCH/HR)            (ACRES)    NODE
     1        7.37   12.91    2.358  0.40( 0.07) 0.19       3.4       21.00
     2        8.77    8.93    2.911  0.40( 0.07) 0.18       3.4       25.00

 RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
 CONFLUENCE FORMULA USED FOR  2 STREAMS.

 ** PEAK FLOW RATE TABLE **
  STREAM       Q      Tc   Intensity   Fp(Fm)      Ap      Ae      HEADWATER
  NUMBER     (CFS)  (MIN.) (INCH/HR) (INCH/HR)            (ACRES)    NODE
     1       15.11    8.93    2.911  0.40( 0.07) 0.18       5.8       25.00
     2       14.43   12.91    2.358  0.40( 0.07) 0.18       6.8       21.00

 COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
 PEAK FLOW RATE(CFS) =      15.11    Tc(MIN.) =    8.93
 EFFECTIVE AREA(ACRES) =      5.76   AREA-AVERAGED Fm(INCH/HR) =  0.07
 AREA-AVERAGED Fp(INCH/HR) =  0.40  AREA-AVERAGED Ap =  0.18
 TOTAL AREA(ACRES) =      6.8
 LONGEST FLOWPATH FROM NODE     21.00 TO NODE     30.00 =    1007.60 FEET.

****************************************************************************
 FLOW PROCESS FROM NODE     31.00 TO NODE     32.00 IS CODE =  21
----------------------------------------------------------------------------
 >>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
 >>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<
============================================================================
 INITIAL SUBAREA FLOW-LENGTH(FEET) =   300.00
 ELEVATION DATA: UPSTREAM(FEET) =    152.00  DOWNSTREAM(FEET) =    150.05

 Tc = K*[(LENGTH** 3.00)/(ELEVATION CHANGE)]**0.20
 SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) =   12.948
 *  10 YEAR RAINFALL INTENSITY(INCH/HR) =  2.353
 SUBAREA Tc AND LOSS RATE DATA(AMC  II):
  DEVELOPMENT TYPE/       SCS SOIL   AREA      Fp         Ap      SCS   Tc
      LAND USE             GROUP   (ACRES)  (INCH/HR)  (DECIMAL)  CN  (MIN.)
 PUBLIC PARK                A        1.38      0.40      0.850    32   12.95
 SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) =  0.40
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap =  0.850
 SUBAREA RUNOFF(CFS) =      2.50
 TOTAL AREA(ACRES) =      1.38   PEAK FLOW RATE(CFS) =      2.50

****************************************************************************
 FLOW PROCESS FROM NODE     32.00 TO NODE     32.00 IS CODE =  81
----------------------------------------------------------------------------
 >>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<
============================================================================
 MAINLINE Tc(MIN.) =   12.95
 *  10 YEAR RAINFALL INTENSITY(INCH/HR) =  2.353
 SUBAREA LOSS RATE DATA(AMC  II):
  DEVELOPMENT TYPE/       SCS SOIL   AREA      Fp         Ap      SCS
      LAND USE             GROUP   (ACRES)  (INCH/HR)  (DECIMAL)  CN
 COMMERCIAL                 A        0.33      0.40      0.100    32
 SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) =  0.40
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 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap =  0.100
 SUBAREA AREA(ACRES) =     0.33      SUBAREA RUNOFF(CFS) =     0.69
 EFFECTIVE AREA(ACRES) =      1.71   AREA-AVERAGED Fm(INCH/HR) =  0.28
 AREA-AVERAGED Fp(INCH/HR) =  0.40  AREA-AVERAGED Ap =  0.71
 TOTAL AREA(ACRES) =        1.7       PEAK FLOW RATE(CFS) =       3.19

****************************************************************************
 FLOW PROCESS FROM NODE      32.00 TO NODE      33.00 IS CODE =  31
----------------------------------------------------------------------------
 >>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
 >>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<
============================================================================
 ELEVATION DATA: UPSTREAM(FEET) =   142.10  DOWNSTREAM(FEET) =   141.80
 FLOW LENGTH(FEET) =   172.00   MANNING'S N =  0.013
 DEPTH OF FLOW IN  18.0 INCH PIPE IS  11.8 INCHES
 PIPE-FLOW VELOCITY(FEET/SEC.) =   2.59
 ESTIMATED PIPE DIAMETER(INCH) =  18.00    NUMBER OF PIPES =   1
 PIPE-FLOW(CFS) =       3.19
 PIPE TRAVEL TIME(MIN.) =   1.11    Tc(MIN.) =   14.05
 LONGEST FLOWPATH FROM NODE      31.00 TO NODE      33.00 =      472.00 FEET.

****************************************************************************
 FLOW PROCESS FROM NODE      33.00 TO NODE      33.00 IS CODE =  81
----------------------------------------------------------------------------
 >>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<
============================================================================
 MAINLINE Tc(MIN.) =   14.05
 *  10 YEAR RAINFALL INTENSITY(INCH/HR) =  2.245
 SUBAREA LOSS RATE DATA(AMC  II):
  DEVELOPMENT TYPE/       SCS SOIL   AREA      Fp         Ap      SCS
      LAND USE            GROUP   (ACRES)  (INCH/HR)  (DECIMAL)  CN
 COMMERCIAL                 A        0.24      0.40      0.100    32
 SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) =  0.40
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap =  0.100
 SUBAREA AREA(ACRES) =     0.24      SUBAREA RUNOFF(CFS) =     0.48
 EFFECTIVE AREA(ACRES) =      1.95   AREA-AVERAGED Fm(INCH/HR) =  0.25
 AREA-AVERAGED Fp(INCH/HR) =  0.40  AREA-AVERAGED Ap =  0.63
 TOTAL AREA(ACRES) =        2.0       PEAK FLOW RATE(CFS) =       3.50

****************************************************************************
 FLOW PROCESS FROM NODE      33.00 TO NODE      34.00 IS CODE =  31
----------------------------------------------------------------------------
 >>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
 >>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<
============================================================================
 ELEVATION DATA: UPSTREAM(FEET) =   141.80  DOWNSTREAM(FEET) =   141.77
 FLOW LENGTH(FEET) =     6.00   MANNING'S N =  0.013
 ESTIMATED PIPE DIAMETER(INCH) INCREASED TO 18.000
 DEPTH OF FLOW IN  18.0 INCH PIPE IS   9.0 INCHES
 PIPE-FLOW VELOCITY(FEET/SEC.) =   3.98
 ESTIMATED PIPE DIAMETER(INCH) =  18.00    NUMBER OF PIPES =   1
 PIPE-FLOW(CFS) =       3.50
 PIPE TRAVEL TIME(MIN.) =   0.03    Tc(MIN.) =   14.08
 LONGEST FLOWPATH FROM NODE      31.00 TO NODE      34.00 =      478.00 FEET.

****************************************************************************
 FLOW PROCESS FROM NODE      34.00 TO NODE      34.00 IS CODE =  81
----------------------------------------------------------------------------
 >>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<
============================================================================
 MAINLINE Tc(MIN.) =   14.08
 *  10 YEAR RAINFALL INTENSITY(INCH/HR) =  2.243
 SUBAREA LOSS RATE DATA(AMC  II):
  DEVELOPMENT TYPE/       SCS SOIL   AREA      Fp         Ap      SCS
      LAND USE            GROUP   (ACRES)  (INCH/HR)  (DECIMAL)  CN
 APARTMENTS                 A        1.44      0.40      0.200    32
 SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) =  0.40
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap =  0.200
 SUBAREA AREA(ACRES) =     1.44      SUBAREA RUNOFF(CFS) =     2.80
 EFFECTIVE AREA(ACRES) =      3.39   AREA-AVERAGED Fm(INCH/HR) =  0.18
 AREA-AVERAGED Fp(INCH/HR) =  0.40  AREA-AVERAGED Ap =  0.45
 TOTAL AREA(ACRES) =        3.4       PEAK FLOW RATE(CFS) =       6.30

****************************************************************************
 FLOW PROCESS FROM NODE      34.00 TO NODE      35.00 IS CODE =  31
----------------------------------------------------------------------------
 >>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
 >>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<
============================================================================
 ELEVATION DATA: UPSTREAM(FEET) =   141.77  DOWNSTREAM(FEET) =   141.72
 FLOW LENGTH(FEET) =    34.00   MANNING'S N =  0.013
 DEPTH OF FLOW IN  24.0 INCH PIPE IS  15.7 INCHES
 PIPE-FLOW VELOCITY(FEET/SEC.) =   2.89
 ESTIMATED PIPE DIAMETER(INCH) =  24.00    NUMBER OF PIPES =   1
 PIPE-FLOW(CFS) =       6.30
 PIPE TRAVEL TIME(MIN.) =   0.20    Tc(MIN.) =   14.28
 LONGEST FLOWPATH FROM NODE      31.00 TO NODE      35.00 =      512.00 FEET.

****************************************************************************
 FLOW PROCESS FROM NODE      35.00 TO NODE      35.00 IS CODE =  81
----------------------------------------------------------------------------
 >>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<
============================================================================
 MAINLINE Tc(MIN.) =   14.28
 *  10 YEAR RAINFALL INTENSITY(INCH/HR) =  2.225
 SUBAREA LOSS RATE DATA(AMC  II):
  DEVELOPMENT TYPE/       SCS SOIL   AREA      Fp         Ap      SCS
      LAND USE            GROUP   (ACRES)  (INCH/HR)  (DECIMAL)  CN
 COMMERCIAL                 A        0.39      0.40      0.100    32
 SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) =  0.40
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap =  0.100
 SUBAREA AREA(ACRES) =     0.39      SUBAREA RUNOFF(CFS) =     0.77
 EFFECTIVE AREA(ACRES) =      3.78   AREA-AVERAGED Fm(INCH/HR) =  0.16
 AREA-AVERAGED Fp(INCH/HR) =  0.40  AREA-AVERAGED Ap =  0.41
 TOTAL AREA(ACRES) =        3.8       PEAK FLOW RATE(CFS) =       7.01

****************************************************************************
 FLOW PROCESS FROM NODE      35.00 TO NODE      36.00 IS CODE =  31
----------------------------------------------------------------------------
 >>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
 >>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<
============================================================================
```

```
 ELEVATION DATA: UPSTREAM(FEET) =   141.72  DOWNSTREAM(FEET) =   141.12
 FLOW LENGTH(FEET) =   297.00   MANNING'S N =  0.013
 DEPTH OF FLOW IN  24.0 INCH PIPE IS  15.2 INCHES
 PIPE-FLOW VELOCITY(FEET/SEC.) =   3.35
 ESTIMATED PIPE DIAMETER(INCH) =  24.00    NUMBER OF PIPES =   1
 PIPE-FLOW(CFS) =       7.01
 PIPE TRAVEL TIME(MIN.) =   1.48    Tc(MIN.) =   15.75
 LONGEST FLOWPATH FROM NODE     31.00 TO NODE     36.00 =     809.00 FEET.

****************************************************************************
 FLOW PROCESS FROM NODE     36.00 TO NODE     36.00 IS CODE =   1
----------------------------------------------------------------------------
 >>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
============================================================================
 TOTAL NUMBER OF STREAMS =  2
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM  1 ARE:
 TIME OF CONCENTRATION(MIN.) =   15.75
 RAINFALL INTENSITY(INCH/HR) =   2.10
 AREA-AVERAGED Fm(INCH/HR) =  0.16
 AREA-AVERAGED Fp(INCH/HR) =  0.40
 AREA-AVERAGED Ap =  0.41
 EFFECTIVE STREAM AREA(ACRES) =       3.78
 TOTAL STREAM AREA(ACRES) =       3.78
 PEAK FLOW RATE(CFS) AT CONFLUENCE =       7.01

****************************************************************************
 FLOW PROCESS FROM NODE     35.10 TO NODE     36.00 IS CODE =  21
----------------------------------------------------------------------------
 >>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
 >>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<
============================================================================
 INITIAL SUBAREA FLOW-LENGTH(FEET) =   246.00
 ELEVATION DATA: UPSTREAM(FEET) =    152.22  DOWNSTREAM(FEET) =    150.75

 Tc = K*[(LENGTH** 3.00)/(ELEVATION CHANGE)]**0.20
 SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) =    7.655
 *  10 YEAR RAINFALL INTENSITY(INCH/HR) =  3.180
 SUBAREA Tc AND LOSS RATE DATA(AMC  II):
  DEVELOPMENT TYPE/       SCS SOIL   AREA      Fp         Ap      SCS   Tc
      LAND USE             GROUP   (ACRES)  (INCH/HR)  (DECIMAL)  CN  (MIN.)
 COMMERCIAL                  A        0.30      0.40      0.100    32    7.66
 SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) =  0.40
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap =  0.100
 SUBAREA RUNOFF(CFS) =      0.85
 TOTAL AREA(ACRES) =      0.30   PEAK FLOW RATE(CFS) =      0.85

****************************************************************************
 FLOW PROCESS FROM NODE     36.00 TO NODE     36.00 IS CODE =   1
----------------------------------------------------------------------------
 >>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
 >>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<
============================================================================
 TOTAL NUMBER OF STREAMS =  2
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM  2 ARE:
 TIME OF CONCENTRATION(MIN.) =    7.66
 RAINFALL INTENSITY(INCH/HR) =   3.18
 AREA-AVERAGED Fm(INCH/HR) =  0.04
 AREA-AVERAGED Fp(INCH/HR) =  0.40
 AREA-AVERAGED Ap =  0.10
 EFFECTIVE STREAM AREA(ACRES) =       0.30
 TOTAL STREAM AREA(ACRES) =       0.30
 PEAK FLOW RATE(CFS) AT CONFLUENCE =       0.85

 ** CONFLUENCE DATA **
  STREAM       Q      Tc    Intensity   Fp(Fm)      Ap      Ae      HEADWATER
  NUMBER     (CFS)   (MIN.) (INCH/HR) (INCH/HR)           (ACRES)     NODE
     1        7.01   15.75    2.103  0.40( 0.16) 0.41       3.8       31.00
     2        0.85    7.66    3.180  0.40( 0.04) 0.10       0.3       35.10

 RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
 CONFLUENCE FORMULA USED FOR  2 STREAMS.

 ** PEAK FLOW RATE TABLE **
  STREAM       Q      Tc    Intensity   Fp(Fm)      Ap      Ae      HEADWATER
  NUMBER     (CFS)   (MIN.) (INCH/HR) (INCH/HR)           (ACRES)     NODE
     1        6.15    7.66    3.180  0.40( 0.15) 0.37       2.1       35.10
     2        7.57   15.75    2.103  0.40( 0.16) 0.39       4.1       31.00

 COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
 PEAK FLOW RATE(CFS) =       7.57    Tc(MIN.) =    15.75
 EFFECTIVE AREA(ACRES) =       4.08   AREA-AVERAGED Fm(INCH/HR) =  0.16
 AREA-AVERAGED Fp(INCH/HR) =  0.40  AREA-AVERAGED Ap =  0.39
 TOTAL AREA(ACRES) =        4.1
 LONGEST FLOWPATH FROM NODE     31.00 TO NODE     36.00 =     809.00 FEET.

****************************************************************************
 FLOW PROCESS FROM NODE     36.00 TO NODE     37.00 IS CODE =  31
----------------------------------------------------------------------------
 >>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
 >>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<
============================================================================
 ELEVATION DATA: UPSTREAM(FEET) =   141.12  DOWNSTREAM(FEET) =   141.07
 FLOW LENGTH(FEET) =    15.00   MANNING'S N =  0.013
 DEPTH OF FLOW IN  21.0 INCH PIPE IS  15.2 INCHES
 PIPE-FLOW VELOCITY(FEET/SEC.) =   4.07
 ESTIMATED PIPE DIAMETER(INCH) =  21.00    NUMBER OF PIPES =   1
 PIPE-FLOW(CFS) =       7.57
 PIPE TRAVEL TIME(MIN.) =   0.06    Tc(MIN.) =   15.82
 LONGEST FLOWPATH FROM NODE     31.00 TO NODE     37.00 =     824.00 FEET.

****************************************************************************
 FLOW PROCESS FROM NODE     37.00 TO NODE     37.00 IS CODE =   1
----------------------------------------------------------------------------
 >>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
============================================================================
 TOTAL NUMBER OF STREAMS =  2
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM  1 ARE:
 TIME OF CONCENTRATION(MIN.) =   15.82
 RAINFALL INTENSITY(INCH/HR) =   2.10
 AREA-AVERAGED Fm(INCH/HR) =  0.16
 AREA-AVERAGED Fp(INCH/HR) =  0.40
 AREA-AVERAGED Ap =  0.39
```

```
 EFFECTIVE STREAM AREA(ACRES) =        4.08
 TOTAL STREAM AREA(ACRES) =        4.08
 PEAK FLOW RATE(CFS) AT CONFLUENCE =        7.57

****************************************************************************
 FLOW PROCESS FROM NODE      36.10 TO NODE      37.00 IS CODE =  21
----------------------------------------------------------------------------
 >>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
 >>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<
============================================================================
 INITIAL SUBAREA FLOW-LENGTH(FEET) =   250.00
 ELEVATION DATA: UPSTREAM(FEET) =    152.22  DOWNSTREAM(FEET) =    150.75

 Tc = K*[(LENGTH** 3.00)/(ELEVATION CHANGE)]**0.20
 SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) =    7.730
 *  10 YEAR RAINFALL INTENSITY(INCH/HR) =  3.163
 SUBAREA Tc AND LOSS RATE DATA(AMC  II):
  DEVELOPMENT TYPE/       SCS SOIL   AREA      Fp          Ap      SCS   Tc
      LAND USE             GROUP   (ACRES)  (INCH/HR)  (DECIMAL)  CN  (MIN.)
 COMMERCIAL                  A        0.19      0.40      0.100    32    7.73
 SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) =  0.40
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap =  0.100
 SUBAREA RUNOFF(CFS) =       0.53
 TOTAL AREA(ACRES) =       0.19   PEAK FLOW RATE(CFS) =       0.53

****************************************************************************
 FLOW PROCESS FROM NODE      37.00 TO NODE      37.00 IS CODE =   1
----------------------------------------------------------------------------
 >>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
 >>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<
============================================================================
 TOTAL NUMBER OF STREAMS =  2
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM  2 ARE:
 TIME OF CONCENTRATION(MIN.) =    7.73
 RAINFALL INTENSITY(INCH/HR) =   3.16
 AREA-AVERAGED Fm(INCH/HR) =  0.04
 AREA-AVERAGED Fp(INCH/HR) =  0.40
 AREA-AVERAGED Ap =  0.10
 EFFECTIVE STREAM AREA(ACRES) =       0.19
 TOTAL STREAM AREA(ACRES) =       0.19
 PEAK FLOW RATE(CFS) AT CONFLUENCE =       0.53

 ** CONFLUENCE DATA **
  STREAM       Q      Tc   Intensity   Fp(Fm)      Ap      Ae      HEADWATER
  NUMBER     (CFS)  (MIN.) (INCH/HR) (INCH/HR)           (ACRES)     NODE
     1        6.15    7.72    3.165  0.40( 0.15) 0.37       2.1       35.10
     1        7.57   15.82    2.099  0.40( 0.16) 0.39       4.1       31.00
     2        0.53    7.73    3.163  0.40( 0.04) 0.10       0.2       36.10

 RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
 CONFLUENCE FORMULA USED FOR  2 STREAMS.

 ** PEAK FLOW RATE TABLE **
  STREAM       Q      Tc   Intensity   Fp(Fm)      Ap      Ae      HEADWATER
  NUMBER     (CFS)  (MIN.) (INCH/HR) (INCH/HR)           (ACRES)     NODE
     1        6.68    7.72    3.165  0.40( 0.14) 0.35       2.3       35.10
     2        6.68    7.73    3.163  0.40( 0.14) 0.35       2.3       36.10
     3        7.92   15.82    2.099  0.40( 0.15) 0.38       4.3       31.00

 COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
 PEAK FLOW RATE(CFS) =       7.92    Tc(MIN.) =    15.82
 EFFECTIVE AREA(ACRES) =       4.27   AREA-AVERAGED Fm(INCH/HR) =  0.15
 AREA-AVERAGED Fp(INCH/HR) =  0.40  AREA-AVERAGED Ap =  0.38
 TOTAL AREA(ACRES) =       4.3
 LONGEST FLOWPATH FROM NODE      31.00 TO NODE      37.00 =      824.00 FEET.

****************************************************************************
 FLOW PROCESS FROM NODE      37.00 TO NODE      38.00 IS CODE =  31
----------------------------------------------------------------------------
 >>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
 >>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<
============================================================================
 ELEVATION DATA: UPSTREAM(FEET) =    141.07  DOWNSTREAM(FEET) =    140.67
 FLOW LENGTH(FEET) =    136.00   MANNING'S N =  0.013
 DEPTH OF FLOW IN  21.0 INCH PIPE IS  16.7 INCHES
 PIPE-FLOW VELOCITY(FEET/SEC.) =    3.86
 ESTIMATED PIPE DIAMETER(INCH) =  21.00     NUMBER OF PIPES =   1
 PIPE-FLOW(CFS) =       7.92
 PIPE TRAVEL TIME(MIN.) =    0.59    Tc(MIN.) =    16.40
 LONGEST FLOWPATH FROM NODE      31.00 TO NODE      38.00 =      960.00 FEET.

****************************************************************************
 FLOW PROCESS FROM NODE      38.00 TO NODE      38.00 IS CODE =   1
----------------------------------------------------------------------------
 >>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
============================================================================
 TOTAL NUMBER OF STREAMS =  2
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM  1 ARE:
 TIME OF CONCENTRATION(MIN.) =    16.40
 RAINFALL INTENSITY(INCH/HR) =    2.06
 AREA-AVERAGED Fm(INCH/HR) =  0.15
 AREA-AVERAGED Fp(INCH/HR) =  0.40
 AREA-AVERAGED Ap =  0.38
 EFFECTIVE STREAM AREA(ACRES) =       4.27
 TOTAL STREAM AREA(ACRES) =       4.27
 PEAK FLOW RATE(CFS) AT CONFLUENCE =       7.92

****************************************************************************
 FLOW PROCESS FROM NODE      37.10 TO NODE      38.00 IS CODE =  21
----------------------------------------------------------------------------
 >>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
 >>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<
============================================================================
 INITIAL SUBAREA FLOW-LENGTH(FEET) =   300.00
 ELEVATION DATA: UPSTREAM(FEET) =    148.90  DOWNSTREAM(FEET) =    145.59

 Tc = K*[(LENGTH** 3.00)/(ELEVATION CHANGE)]**0.20
 SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) =    7.814
 *  10 YEAR RAINFALL INTENSITY(INCH/HR) =  3.143
 SUBAREA Tc AND LOSS RATE DATA(AMC  II):
  DEVELOPMENT TYPE/       SCS SOIL   AREA      Fp          Ap      SCS   Tc
      LAND USE             GROUP   (ACRES)  (INCH/HR)  (DECIMAL)  CN  (MIN.)
```

```
APARTMENTS                    A        0.73      0.40     0.200    32     7.81
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) =  0.40
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap =  0.200
SUBAREA RUNOFF(CFS) =       2.01
TOTAL AREA(ACRES) =       0.73   PEAK FLOW RATE(CFS) =       2.01

****************************************************************************
FLOW PROCESS FROM NODE      38.00 TO NODE      38.00 IS CODE =   1
----------------------------------------------------------------------------
>>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
>>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<
============================================================================
TOTAL NUMBER OF STREAMS =  2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM  2 ARE:
TIME OF CONCENTRATION(MIN.) =     7.81
RAINFALL INTENSITY(INCH/HR) =   3.14
AREA-AVERAGED Fm(INCH/HR) =  0.08
AREA-AVERAGED Fp(INCH/HR) =  0.40
AREA-AVERAGED Ap =  0.20
EFFECTIVE STREAM AREA(ACRES) =       0.73
TOTAL STREAM AREA(ACRES) =       0.73
PEAK FLOW RATE(CFS) AT CONFLUENCE =       2.01

** CONFLUENCE DATA **
 STREAM       Q      Tc   Intensity   Fp(Fm)      Ap      Ae     HEADWATER
 NUMBER     (CFS)   (MIN.) (INCH/HR) (INCH/HR)          (ACRES)    NODE
    1        6.68    8.32    3.032  0.40( 0.14) 0.35       2.3      35.10
    1        6.68    8.33    3.030  0.40( 0.14) 0.35       2.3      36.10
    1        7.92   16.40    2.055  0.40( 0.15) 0.38       4.3      31.00
    2        2.01    7.81    3.143  0.40( 0.08) 0.20       0.7      37.10

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
CONFLUENCE FORMULA USED FOR  2 STREAMS.

** PEAK FLOW RATE TABLE **
 STREAM       Q      Tc   Intensity   Fp(Fm)      Ap      Ae     HEADWATER
 NUMBER     (CFS)   (MIN.) (INCH/HR) (INCH/HR)          (ACRES)    NODE
    1        8.53    7.81    3.143  0.40( 0.12) 0.31       2.9      37.10
    2        8.62    8.32    3.032  0.40( 0.12) 0.31       3.1      35.10
    3        8.62    8.33    3.030  0.40( 0.12) 0.31       3.1      36.10
    4        9.22   16.40    2.055  0.40( 0.14) 0.35       5.0      31.00

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
PEAK FLOW RATE(CFS) =       9.22    Tc(MIN.) =    16.40
EFFECTIVE AREA(ACRES) =       5.00   AREA-AVERAGED Fm(INCH/HR) =  0.14
AREA-AVERAGED Fp(INCH/HR) =  0.40  AREA-AVERAGED Ap =  0.35
TOTAL AREA(ACRES) =       5.0
LONGEST FLOWPATH FROM NODE     31.00 TO NODE     38.00 =     960.00 FEET.

****************************************************************************
FLOW PROCESS FROM NODE      38.00 TO NODE      39.00 IS CODE =  31
----------------------------------------------------------------------------
>>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<
============================================================================
ELEVATION DATA: UPSTREAM(FEET) =   140.67  DOWNSTREAM(FEET) =   140.12
FLOW LENGTH(FEET) =    99.00   MANNING'S N =  0.013
DEPTH OF FLOW IN  21.0 INCH PIPE IS  14.5 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) =   5.20
ESTIMATED PIPE DIAMETER(INCH) =  21.00    NUMBER OF PIPES =   1
PIPE-FLOW(CFS) =       9.22
PIPE TRAVEL TIME(MIN.) =   0.32    Tc(MIN.) =   16.72
LONGEST FLOWPATH FROM NODE     31.00 TO NODE     39.00 =    1059.00 FEET.

****************************************************************************
FLOW PROCESS FROM NODE      39.00 TO NODE      39.00 IS CODE =   1
----------------------------------------------------------------------------
>>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
============================================================================
TOTAL NUMBER OF STREAMS =  2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM  1 ARE:
TIME OF CONCENTRATION(MIN.) =   16.72
RAINFALL INTENSITY(INCH/HR) =   2.03
AREA-AVERAGED Fm(INCH/HR) =  0.14
AREA-AVERAGED Fp(INCH/HR) =  0.40
AREA-AVERAGED Ap =  0.35
EFFECTIVE STREAM AREA(ACRES) =       5.00
TOTAL STREAM AREA(ACRES) =       5.00
PEAK FLOW RATE(CFS) AT CONFLUENCE =       9.22

****************************************************************************
FLOW PROCESS FROM NODE      38.10 TO NODE      39.00 IS CODE =  21
----------------------------------------------------------------------------
>>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<
============================================================================
INITIAL SUBAREA FLOW-LENGTH(FEET) =   300.00
ELEVATION DATA: UPSTREAM(FEET) =    151.20  DOWNSTREAM(FEET) =    150.00

Tc = K*[(LENGTH** 3.00)/(ELEVATION CHANGE)]**0.20
SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) =    9.572
*  10 YEAR RAINFALL INTENSITY(INCH/HR) =  2.798
SUBAREA Tc AND LOSS RATE DATA(AMC  II):
 DEVELOPMENT TYPE/       SCS SOIL   AREA      Fp          Ap       SCS   Tc
     LAND USE             GROUP   (ACRES)  (INCH/HR)  (DECIMAL)  CN  (MIN.)
APARTMENTS                   A       1.40      0.40      0.200    32     9.57
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) =  0.40
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap =  0.200
SUBAREA RUNOFF(CFS) =       3.42
TOTAL AREA(ACRES) =       1.40   PEAK FLOW RATE(CFS) =       3.42

****************************************************************************
FLOW PROCESS FROM NODE      39.00 TO NODE      39.00 IS CODE =   1
----------------------------------------------------------------------------
>>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
>>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<
============================================================================
TOTAL NUMBER OF STREAMS =  2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM  2 ARE:
TIME OF CONCENTRATION(MIN.) =     9.57
RAINFALL INTENSITY(INCH/HR) =   2.80
AREA-AVERAGED Fm(INCH/HR) =  0.08
```

```
AREA-AVERAGED Fp(INCH/HR) =  0.40
AREA-AVERAGED Ap =  0.20
EFFECTIVE STREAM AREA(ACRES) =       1.40
TOTAL STREAM AREA(ACRES) =       1.40
PEAK FLOW RATE(CFS) AT CONFLUENCE =       3.42

** CONFLUENCE DATA **
 STREAM       Q      Tc   Intensity   Fp(Fm)      Ap      Ae      HEADWATER
 NUMBER     (CFS)  (MIN.) (INCH/HR) (INCH/HR)           (ACRES)     NODE
    1        8.53    8.14    3.071  0.40( 0.12) 0.31       2.9       37.10
    1        8.62    8.64    2.967  0.40( 0.12) 0.31       3.1       35.10
    1        8.62    8.65    2.965  0.40( 0.12) 0.31       3.1       36.10
    1        9.22   16.72    2.033  0.40( 0.14) 0.35       5.0       31.00
    2        3.42    9.57    2.798  0.40( 0.08) 0.20       1.4       38.10

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
CONFLUENCE FORMULA USED FOR  2 STREAMS.

** PEAK FLOW RATE TABLE **
 STREAM       Q      Tc   Intensity   Fp(Fm)      Ap      Ae      HEADWATER
 NUMBER     (CFS)  (MIN.) (INCH/HR) (INCH/HR)           (ACRES)     NODE
    1       11.73    8.14    3.071  0.40( 0.11) 0.28       4.1       37.10
    2       11.90    8.64    2.967  0.40( 0.11) 0.28       4.3       35.10
    3       11.91    8.65    2.965  0.40( 0.11) 0.28       4.3       36.10
    4       12.11    9.57    2.798  0.40( 0.11) 0.28       4.7       38.10
    5       11.68   16.72    2.033  0.40( 0.13) 0.32       6.4       31.00

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
PEAK FLOW RATE(CFS) =      12.11    Tc(MIN.) =      9.57
EFFECTIVE AREA(ACRES) =       4.68   AREA-AVERAGED Fm(INCH/HR) =  0.11
AREA-AVERAGED Fp(INCH/HR) =  0.40  AREA-AVERAGED Ap =  0.28
TOTAL AREA(ACRES) =       6.4
LONGEST FLOWPATH FROM NODE     31.00 TO NODE     39.00 =    1059.00 FEET.

****************************************************************************
 FLOW PROCESS FROM NODE     39.00 TO NODE     40.00 IS CODE =  31
----------------------------------------------------------------------------
 >>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
 >>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<
============================================================================
 ELEVATION DATA: UPSTREAM(FEET) =   140.12  DOWNSTREAM(FEET) =   139.79
 FLOW LENGTH(FEET) =    26.00   MANNING'S N =  0.013
 DEPTH OF FLOW IN  21.0 INCH PIPE IS  13.1 INCHES
 PIPE-FLOW VELOCITY(FEET/SEC.) =   7.65
 ESTIMATED PIPE DIAMETER(INCH) =  21.00    NUMBER OF PIPES =   1
 PIPE-FLOW(CFS) =      12.11
 PIPE TRAVEL TIME(MIN.) =   0.06    Tc(MIN.) =    9.63
 LONGEST FLOWPATH FROM NODE     31.00 TO NODE     40.00 =    1085.00 FEET.

****************************************************************************
 FLOW PROCESS FROM NODE     40.00 TO NODE     40.00 IS CODE =   1
----------------------------------------------------------------------------
 >>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
============================================================================
 TOTAL NUMBER OF STREAMS =  2
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM  1 ARE:
 TIME OF CONCENTRATION(MIN.) =    9.63
 RAINFALL INTENSITY(INCH/HR) =   2.79
 AREA-AVERAGED Fm(INCH/HR) =  0.11
 AREA-AVERAGED Fp(INCH/HR) =  0.40
 AREA-AVERAGED Ap =  0.28
 EFFECTIVE STREAM AREA(ACRES) =       4.68
 TOTAL STREAM AREA(ACRES) =       6.40
 PEAK FLOW RATE(CFS) AT CONFLUENCE =      12.11

****************************************************************************
 FLOW PROCESS FROM NODE     39.10 TO NODE     40.00 IS CODE =  21
----------------------------------------------------------------------------
 >>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
 >>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<
============================================================================
 INITIAL SUBAREA FLOW-LENGTH(FEET) =   300.00
 ELEVATION DATA: UPSTREAM(FEET) =    147.50  DOWNSTREAM(FEET) =    144.52

 Tc = K*[(LENGTH** 3.00)/(ELEVATION CHANGE)]**0.20
 SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) =    7.979
 *  10 YEAR RAINFALL INTENSITY(INCH/HR) =  3.106
 SUBAREA Tc AND LOSS RATE DATA(AMC  II):
  DEVELOPMENT TYPE/       SCS SOIL   AREA      Fp          Ap      SCS   Tc
      LAND USE             GROUP   (ACRES)  (INCH/HR)  (DECIMAL)  CN  (MIN.)
 APARTMENTS                  A       1.01      0.40       0.200     32    7.98
 SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) =  0.40
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap =  0.200
 SUBAREA RUNOFF(CFS) =      2.75
 TOTAL AREA(ACRES) =      1.01   PEAK FLOW RATE(CFS) =      2.75

****************************************************************************
 FLOW PROCESS FROM NODE     40.00 TO NODE     40.00 IS CODE =   1
----------------------------------------------------------------------------
 >>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
 >>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<
============================================================================
 TOTAL NUMBER OF STREAMS =  2
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM  2 ARE:
 TIME OF CONCENTRATION(MIN.) =    7.98
 RAINFALL INTENSITY(INCH/HR) =   3.11
 AREA-AVERAGED Fm(INCH/HR) =  0.08
 AREA-AVERAGED Fp(INCH/HR) =  0.40
 AREA-AVERAGED Ap =  0.20
 EFFECTIVE STREAM AREA(ACRES) =       1.01
 TOTAL STREAM AREA(ACRES) =       1.01
 PEAK FLOW RATE(CFS) AT CONFLUENCE =       2.75

 ** CONFLUENCE DATA **
  STREAM       Q      Tc   Intensity   Fp(Fm)      Ap      Ae      HEADWATER
  NUMBER     (CFS)  (MIN.) (INCH/HR) (INCH/HR)           (ACRES)     NODE
     1       11.73    8.19    3.059  0.40( 0.11) 0.28       4.1       37.10
     1       11.90    8.70    2.956  0.40( 0.11) 0.28       4.3       35.10
     1       11.91    8.71    2.954  0.40( 0.11) 0.28       4.3       36.10
     1       12.11    9.63    2.789  0.40( 0.11) 0.28       4.7       38.10
     1       11.68   16.78    2.029  0.40( 0.13) 0.32       6.4       31.00
     2        2.75    7.98    3.106  0.40( 0.08) 0.20       1.0       39.10
```

```
 RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
 CONFLUENCE FORMULA USED FOR  2 STREAMS.

 ** PEAK FLOW RATE TABLE **
  STREAM       Q      Tc    Intensity   Fp(Fm)      Ap      Ae      HEADWATER
  NUMBER     (CFS)  (MIN.) (INCH/HR) (INCH/HR)            (ACRES)    NODE
     1       14.36    7.98    3.106  0.40( 0.10) 0.26       5.0       39.10
     2       14.44    8.19    3.059  0.40( 0.10) 0.26       5.1       37.10
     3       14.52    8.70    2.956  0.40( 0.11) 0.26       5.3       35.10
     4       14.52    8.71    2.954  0.40( 0.11) 0.26       5.3       36.10
     5       14.58    9.63    2.789  0.40( 0.11) 0.27       5.7       38.10
     6       13.45   16.78    2.029  0.40( 0.12) 0.30       7.4       31.00

 COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
 PEAK FLOW RATE(CFS) =      14.58    Tc(MIN.) =     9.63
 EFFECTIVE AREA(ACRES) =       5.69   AREA-AVERAGED Fm(INCH/HR) =  0.11
 AREA-AVERAGED Fp(INCH/HR) =  0.40  AREA-AVERAGED Ap =  0.27
 TOTAL AREA(ACRES) =        7.4
 LONGEST FLOWPATH FROM NODE     31.00 TO NODE     40.00 =    1085.00 FEET.

****************************************************************************
 FLOW PROCESS FROM NODE     40.00 TO NODE     41.00 IS CODE =  31
----------------------------------------------------------------------------
 >>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
 >>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<
============================================================================
 ELEVATION DATA: UPSTREAM(FEET) =   139.79  DOWNSTREAM(FEET) =   139.10
 FLOW LENGTH(FEET) =   228.00   MANNING'S N =  0.013
 DEPTH OF FLOW IN  27.0 INCH PIPE IS  20.1 INCHES
 PIPE-FLOW VELOCITY(FEET/SEC.) =   4.60
 ESTIMATED PIPE DIAMETER(INCH) =  27.00    NUMBER OF PIPES =   1
 PIPE-FLOW(CFS) =      14.58
 PIPE TRAVEL TIME(MIN.) =   0.83    Tc(MIN.) =   10.45
 LONGEST FLOWPATH FROM NODE     31.00 TO NODE     41.00 =    1313.00 FEET.

****************************************************************************
 FLOW PROCESS FROM NODE     41.00 TO NODE     41.00 IS CODE =   1
----------------------------------------------------------------------------
 >>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
============================================================================
 TOTAL NUMBER OF STREAMS =  2
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM  1 ARE:
 TIME OF CONCENTRATION(MIN.) =   10.45
 RAINFALL INTENSITY(INCH/HR) =   2.66
 AREA-AVERAGED Fm(INCH/HR) =  0.11
 AREA-AVERAGED Fp(INCH/HR) =  0.40
 AREA-AVERAGED Ap =  0.27
 EFFECTIVE STREAM AREA(ACRES) =       5.69
 TOTAL STREAM AREA(ACRES) =       7.41
 PEAK FLOW RATE(CFS) AT CONFLUENCE =      14.58

****************************************************************************
 FLOW PROCESS FROM NODE     40.10 TO NODE     41.00 IS CODE =  21
----------------------------------------------------------------------------
 >>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
 >>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<
============================================================================
 INITIAL SUBAREA FLOW-LENGTH(FEET) =   300.00
 ELEVATION DATA: UPSTREAM(FEET) =    148.80  DOWNSTREAM(FEET) =    147.00

 Tc = K*[(LENGTH** 3.00)/(ELEVATION CHANGE)]**0.20
 SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) =    8.826
 *  10 YEAR RAINFALL INTENSITY(INCH/HR) =  2.931
 SUBAREA Tc AND LOSS RATE DATA(AMC  II):
  DEVELOPMENT TYPE/       SCS SOIL   AREA      Fp         Ap      SCS   Tc
      LAND USE             GROUP   (ACRES)  (INCH/HR)  (DECIMAL)  CN  (MIN.)
 APARTMENTS                  A        2.08      0.40      0.200    32    8.83
 SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) =  0.40
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap =  0.200
 SUBAREA RUNOFF(CFS) =      5.34
 TOTAL AREA(ACRES) =      2.08   PEAK FLOW RATE(CFS) =      5.34

****************************************************************************
 FLOW PROCESS FROM NODE     41.00 TO NODE     41.00 IS CODE =   1
----------------------------------------------------------------------------
 >>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
 >>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<
============================================================================
 TOTAL NUMBER OF STREAMS =  2
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM  2 ARE:
 TIME OF CONCENTRATION(MIN.) =    8.83
 RAINFALL INTENSITY(INCH/HR) =   2.93
 AREA-AVERAGED Fm(INCH/HR) =  0.08
 AREA-AVERAGED Fp(INCH/HR) =  0.40
 AREA-AVERAGED Ap =  0.20
 EFFECTIVE STREAM AREA(ACRES) =       2.08
 TOTAL STREAM AREA(ACRES) =       2.08
 PEAK FLOW RATE(CFS) AT CONFLUENCE =       5.34

 ** CONFLUENCE DATA **
  STREAM       Q      Tc    Intensity   Fp(Fm)      Ap      Ae      HEADWATER
  NUMBER     (CFS)  (MIN.) (INCH/HR) (INCH/HR)            (ACRES)    NODE
     1       14.36    8.81    2.935  0.40( 0.10) 0.26       5.0       39.10
     1       14.44    9.02    2.895  0.40( 0.10) 0.26       5.1       37.10
     1       14.52    9.52    2.806  0.40( 0.11) 0.26       5.3       35.10
     1       14.52    9.53    2.805  0.40( 0.11) 0.26       5.3       36.10
     1       14.58   10.45    2.660  0.40( 0.11) 0.27       5.7       38.10
     1       13.45   17.61    1.973  0.40( 0.12) 0.30       7.4       31.00
     2        5.34    8.83    2.931  0.40( 0.08) 0.20       2.1       40.10

 RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
 CONFLUENCE FORMULA USED FOR  2 STREAMS.

 ** PEAK FLOW RATE TABLE **
  STREAM       Q      Tc    Intensity   Fp(Fm)      Ap      Ae      HEADWATER
  NUMBER     (CFS)  (MIN.) (INCH/HR) (INCH/HR)            (ACRES)    NODE
     1       19.69    8.81    2.935  0.40( 0.10) 0.24       7.1       39.10
     2       19.70    8.83    2.931  0.40( 0.10) 0.24       7.1       40.10
     3       19.71    9.02    2.895  0.40( 0.10) 0.24       7.2       37.10
     4       19.62    9.52    2.806  0.40( 0.10) 0.25       7.4       35.10
     5       19.62    9.53    2.805  0.40( 0.10) 0.25       7.4       36.10
```

```
    6       19.41   10.45    2.660  0.40( 0.10) 0.25       7.8       38.10
    7       16.99   17.61    1.973  0.40( 0.11) 0.28       9.5       31.00

 COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
 PEAK FLOW RATE(CFS) =      19.71    Tc(MIN.) =      9.02
 EFFECTIVE AREA(ACRES) =       7.20   AREA-AVERAGED Fm(INCH/HR) =  0.10
 AREA-AVERAGED Fp(INCH/HR) =  0.40  AREA-AVERAGED Ap =  0.24
 TOTAL AREA(ACRES) =        9.5
 LONGEST FLOWPATH FROM NODE     31.00 TO NODE     41.00 =    1313.00 FEET.

****************************************************************************
 FLOW PROCESS FROM NODE     41.00 TO NODE     42.00 IS CODE =  31
----------------------------------------------------------------------------
 >>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
 >>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<
============================================================================
 ELEVATION DATA: UPSTREAM(FEET) =   139.10  DOWNSTREAM(FEET) =   139.07
 FLOW LENGTH(FEET) =     7.00   MANNING'S N =  0.013
 DEPTH OF FLOW IN  30.0 INCH PIPE IS  19.8 INCHES
 PIPE-FLOW VELOCITY(FEET/SEC.) =   5.73
 ESTIMATED PIPE DIAMETER(INCH) =  30.00    NUMBER OF PIPES =   1
 PIPE-FLOW(CFS) =      19.71
 PIPE TRAVEL TIME(MIN.) =   0.02    Tc(MIN.) =    9.04
 LONGEST FLOWPATH FROM NODE     31.00 TO NODE     42.00 =    1320.00 FEET.

****************************************************************************
 FLOW PROCESS FROM NODE     42.00 TO NODE     42.00 IS CODE =   1
----------------------------------------------------------------------------
 >>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
============================================================================
 TOTAL NUMBER OF STREAMS =  2
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM  1 ARE:
 TIME OF CONCENTRATION(MIN.) =    9.04
 RAINFALL INTENSITY(INCH/HR) =   2.89
 AREA-AVERAGED Fm(INCH/HR) =  0.10
 AREA-AVERAGED Fp(INCH/HR) =  0.40
 AREA-AVERAGED Ap =  0.24
 EFFECTIVE STREAM AREA(ACRES) =       7.20
 TOTAL STREAM AREA(ACRES) =       9.49
 PEAK FLOW RATE(CFS) AT CONFLUENCE =      19.71

****************************************************************************
 FLOW PROCESS FROM NODE     41.10 TO NODE     41.20 IS CODE =  21
----------------------------------------------------------------------------
 >>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
 >>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<
============================================================================
 INITIAL SUBAREA FLOW-LENGTH(FEET) =   264.00
 ELEVATION DATA: UPSTREAM(FEET) =    150.70  DOWNSTREAM(FEET) =    150.10

 Tc = K*[(LENGTH** 3.00)/(ELEVATION CHANGE)]**0.20
 SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) =    9.554
 *  10 YEAR RAINFALL INTENSITY(INCH/HR) =  2.801
 SUBAREA Tc AND LOSS RATE DATA(AMC  II):
  DEVELOPMENT TYPE/       SCS SOIL   AREA      Fp         Ap      SCS   Tc
      LAND USE             GROUP   (ACRES)  (INCH/HR)  (DECIMAL)  CN  (MIN.)
 COMMERCIAL                 A        0.59      0.40      0.100     32     9.55
 SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) =  0.40
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap =  0.100
 SUBAREA RUNOFF(CFS) =      1.47
 TOTAL AREA(ACRES) =      0.59   PEAK FLOW RATE(CFS) =      1.47

****************************************************************************
 FLOW PROCESS FROM NODE     41.20 TO NODE     41.30 IS CODE =  31
----------------------------------------------------------------------------
 >>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
 >>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<
============================================================================
 ELEVATION DATA: UPSTREAM(FEET) =   148.60  DOWNSTREAM(FEET) =   145.34
 FLOW LENGTH(FEET) =   647.00   MANNING'S N =  0.013
 ESTIMATED PIPE DIAMETER(INCH) INCREASED TO 18.000
 DEPTH OF FLOW IN  18.0 INCH PIPE IS   5.6 INCHES
 PIPE-FLOW VELOCITY(FEET/SEC.) =   3.16
 ESTIMATED PIPE DIAMETER(INCH) =  18.00    NUMBER OF PIPES =   1
 PIPE-FLOW(CFS) =      1.47
 PIPE TRAVEL TIME(MIN.) =   3.41    Tc(MIN.) =   12.97
 LONGEST FLOWPATH FROM NODE     41.10 TO NODE     41.30 =     911.00 FEET.

****************************************************************************
 FLOW PROCESS FROM NODE     41.30 TO NODE     41.30 IS CODE =  81
----------------------------------------------------------------------------
 >>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<
============================================================================
 MAINLINE Tc(MIN.) =   12.97
 *  10 YEAR RAINFALL INTENSITY(INCH/HR) =  2.351
 SUBAREA LOSS RATE DATA(AMC  II):
  DEVELOPMENT TYPE/       SCS SOIL   AREA      Fp         Ap      SCS
      LAND USE             GROUP   (ACRES)  (INCH/HR)  (DECIMAL)  CN
 COMMERCIAL                 A        0.63      0.40      0.100     32
 SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) =  0.40
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap =  0.100
 SUBAREA AREA(ACRES) =     0.63     SUBAREA RUNOFF(CFS) =     1.31
 EFFECTIVE AREA(ACRES) =      1.22   AREA-AVERAGED Fm(INCH/HR) =  0.04
 AREA-AVERAGED Fp(INCH/HR) =  0.40  AREA-AVERAGED Ap =  0.10
 TOTAL AREA(ACRES) =       1.2      PEAK FLOW RATE(CFS) =       2.54

****************************************************************************
 FLOW PROCESS FROM NODE     41.30 TO NODE     41.40 IS CODE =  31
----------------------------------------------------------------------------
 >>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
 >>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<
============================================================================
 ELEVATION DATA: UPSTREAM(FEET) =   145.34  DOWNSTREAM(FEET) =   142.67
 FLOW LENGTH(FEET) =   190.00   MANNING'S N =  0.013
 ESTIMATED PIPE DIAMETER(INCH) INCREASED TO 18.000
 DEPTH OF FLOW IN  18.0 INCH PIPE IS   5.7 INCHES
 PIPE-FLOW VELOCITY(FEET/SEC.) =   5.32
 ESTIMATED PIPE DIAMETER(INCH) =  18.00    NUMBER OF PIPES =   1
 PIPE-FLOW(CFS) =      2.54
 PIPE TRAVEL TIME(MIN.) =   0.60    Tc(MIN.) =   13.56
 LONGEST FLOWPATH FROM NODE     41.10 TO NODE     41.40 =    1101.00 FEET.
```

```
****************************************************************************
 FLOW PROCESS FROM NODE     41.40 TO NODE     41.40 IS CODE =  81
----------------------------------------------------------------------------
 >>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<
============================================================================
 MAINLINE Tc(MIN.) =   13.56
 *  10 YEAR RAINFALL INTENSITY(INCH/HR) =  2.291
 SUBAREA LOSS RATE DATA(AMC  II):
  DEVELOPMENT TYPE/       SCS SOIL   AREA      Fp         Ap      SCS
      LAND USE            GROUP   (ACRES)  (INCH/HR)  (DECIMAL)  CN
 COMMERCIAL                 A        0.24      0.40      0.100    32
 SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) =  0.40
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap =  0.100
 SUBAREA AREA(ACRES) =     0.24     SUBAREA RUNOFF(CFS) =     0.49
 EFFECTIVE AREA(ACRES) =      1.46   AREA-AVERAGED Fm(INCH/HR) =  0.04
 AREA-AVERAGED Fp(INCH/HR) =  0.40  AREA-AVERAGED Ap =  0.10
 TOTAL AREA(ACRES) =         1.5      PEAK FLOW RATE(CFS) =       2.96

****************************************************************************
 FLOW PROCESS FROM NODE     41.40 TO NODE     42.00 IS CODE =  31
----------------------------------------------------------------------------
 >>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
 >>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<
============================================================================
 ELEVATION DATA: UPSTREAM(FEET) =   142.67  DOWNSTREAM(FEET) =   139.07
 FLOW LENGTH(FEET) =    82.00   MANNING'S N =  0.013
 ESTIMATED PIPE DIAMETER(INCH) INCREASED TO 18.000
 DEPTH OF FLOW IN  18.0 INCH PIPE IS   4.6 INCHES
 PIPE-FLOW VELOCITY(FEET/SEC.) =   8.36
 ESTIMATED PIPE DIAMETER(INCH) =  18.00    NUMBER OF PIPES =   1
 PIPE-FLOW(CFS) =       2.96
 PIPE TRAVEL TIME(MIN.) =   0.16    Tc(MIN.) =   13.73
 LONGEST FLOWPATH FROM NODE     41.10 TO NODE     42.00 =    1183.00 FEET.

****************************************************************************
 FLOW PROCESS FROM NODE     42.00 TO NODE     42.00 IS CODE =   1
----------------------------------------------------------------------------
 >>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
 >>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<
============================================================================
 TOTAL NUMBER OF STREAMS =  2
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM  2 ARE:
 TIME OF CONCENTRATION(MIN.) =   13.73
 RAINFALL INTENSITY(INCH/HR) =   2.28
 AREA-AVERAGED Fm(INCH/HR) =  0.04
 AREA-AVERAGED Fp(INCH/HR) =  0.40
 AREA-AVERAGED Ap =  0.10
 EFFECTIVE STREAM AREA(ACRES) =       1.46
 TOTAL STREAM AREA(ACRES) =       1.46
 PEAK FLOW RATE(CFS) AT CONFLUENCE =       2.96

 ** CONFLUENCE DATA **
  STREAM       Q      Tc    Intensity   Fp(Fm)      Ap      Ae      HEADWATER
  NUMBER     (CFS)  (MIN.) (INCH/HR) (INCH/HR)          (ACRES)     NODE
     1       19.69    8.83    2.931  0.40( 0.10) 0.24       7.1       39.10
     1       19.70    8.85    2.927  0.40( 0.10) 0.24       7.1       40.10
     1       19.71    9.04    2.891  0.40( 0.10) 0.24       7.2       37.10
     1       19.62    9.54    2.803  0.40( 0.10) 0.25       7.4       35.10
     1       19.62    9.55    2.801  0.40( 0.10) 0.25       7.4       36.10
     1       19.41   10.47    2.657  0.40( 0.10) 0.25       7.8       38.10
     1       16.99   17.63    1.972  0.40( 0.11) 0.28       9.5       31.00
     2        2.96   13.73    2.276  0.40( 0.04) 0.10       1.5       41.10

 RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
 CONFLUENCE FORMULA USED FOR  2 STREAMS.

 ** PEAK FLOW RATE TABLE **
  STREAM       Q      Tc    Intensity   Fp(Fm)      Ap      Ae      HEADWATER
  NUMBER     (CFS)  (MIN.) (INCH/HR) (INCH/HR)          (ACRES)     NODE
     1       22.15    8.83    2.931  0.40( 0.09) 0.23       8.0       39.10
     2       22.16    8.85    2.927  0.40( 0.09) 0.23       8.0       40.10
     3       22.19    9.04    2.891  0.40( 0.09) 0.23       8.2       37.10
     4       22.16    9.54    2.803  0.40( 0.09) 0.23       8.4       35.10
     5       22.16    9.55    2.801  0.40( 0.09) 0.23       8.4       36.10
     6       22.05   10.47    2.657  0.40( 0.09) 0.23       8.9       38.10
     7       21.27   13.73    2.276  0.40( 0.10) 0.24      10.0       41.10
     8       19.55   17.63    1.972  0.40( 0.10) 0.26      11.0       31.00

 COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
 PEAK FLOW RATE(CFS) =      22.19    Tc(MIN.) =     9.04
 EFFECTIVE AREA(ACRES) =       8.16   AREA-AVERAGED Fm(INCH/HR) =  0.09
 AREA-AVERAGED Fp(INCH/HR) =  0.40  AREA-AVERAGED Ap =  0.23
 TOTAL AREA(ACRES) =       11.0
 LONGEST FLOWPATH FROM NODE     31.00 TO NODE     42.00 =    1320.00 FEET.

****************************************************************************
 FLOW PROCESS FROM NODE     42.00 TO NODE     43.00 IS CODE =  31
----------------------------------------------------------------------------
 >>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
 >>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<
============================================================================
 ELEVATION DATA: UPSTREAM(FEET) =   139.07  DOWNSTREAM(FEET) =   138.55
 FLOW LENGTH(FEET) =   105.00   MANNING'S N =  0.013
 DEPTH OF FLOW IN  30.0 INCH PIPE IS  20.5 INCHES
 PIPE-FLOW VELOCITY(FEET/SEC.) =   6.21
 ESTIMATED PIPE DIAMETER(INCH) =  30.00    NUMBER OF PIPES =   1
 PIPE-FLOW(CFS) =      22.19
 PIPE TRAVEL TIME(MIN.) =   0.28    Tc(MIN.) =    9.32
 LONGEST FLOWPATH FROM NODE     31.00 TO NODE     43.00 =    1425.00 FEET.

****************************************************************************
 FLOW PROCESS FROM NODE     43.00 TO NODE     43.00 IS CODE =   1
----------------------------------------------------------------------------
 >>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
============================================================================
 TOTAL NUMBER OF STREAMS =  2
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM  1 ARE:
 TIME OF CONCENTRATION(MIN.) =    9.32
 RAINFALL INTENSITY(INCH/HR) =   2.84
 AREA-AVERAGED Fm(INCH/HR) =  0.09
 AREA-AVERAGED Fp(INCH/HR) =  0.40
 AREA-AVERAGED Ap =  0.23
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```
 EFFECTIVE STREAM AREA(ACRES) =       8.16
 TOTAL STREAM AREA(ACRES) =      10.95
 PEAK FLOW RATE(CFS) AT CONFLUENCE =      22.19

****************************************************************************
 FLOW PROCESS FROM NODE     42.10 TO NODE     39.00 IS CODE =  21
----------------------------------------------------------------------------
 >>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
 >>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<
============================================================================
 INITIAL SUBAREA FLOW-LENGTH(FEET) =   271.00
 ELEVATION DATA: UPSTREAM(FEET) =    151.75  DOWNSTREAM(FEET) =    148.70

 Tc = K*[(LENGTH** 3.00)/(ELEVATION CHANGE)]**0.20
 SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) =    7.011
 *  10 YEAR RAINFALL INTENSITY(INCH/HR) =  3.345
 SUBAREA Tc AND LOSS RATE DATA(AMC  II):
  DEVELOPMENT TYPE/       SCS SOIL   AREA      Fp         Ap      SCS   Tc
      LAND USE             GROUP   (ACRES)  (INCH/HR)  (DECIMAL)  CN  (MIN.)
 COMMERCIAL                  A        0.27      0.40      0.100    32    7.01
 SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) =  0.40
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap =  0.100
 SUBAREA RUNOFF(CFS) =      0.80
 TOTAL AREA(ACRES) =      0.27   PEAK FLOW RATE(CFS) =      0.80

****************************************************************************
 FLOW PROCESS FROM NODE     39.00 TO NODE     43.00 IS CODE =  61
----------------------------------------------------------------------------
 >>>>>COMPUTE STREET FLOW TRAVEL TIME THRU SUBAREA<<<<<
 >>>>>(STANDARD CURB SECTION USED)<<<<<
============================================================================
 UPSTREAM ELEVATION(FEET) =  148.70  DOWNSTREAM ELEVATION(FEET) =  146.25
 STREET LENGTH(FEET) =   356.00   CURB HEIGHT(INCHES) =  6.0
 STREET HALFWIDTH(FEET) = 21.00

 DISTANCE FROM CROWN TO CROSSFALL GRADEBREAK(FEET) =  16.00
 INSIDE STREET CROSSFALL(DECIMAL) =  0.020
 OUTSIDE STREET CROSSFALL(DECIMAL)  =  0.020

 SPECIFIED NUMBER OF HALFSTREETS CARRYING RUNOFF =  1
 STREET PARKWAY CROSSFALL(DECIMAL)  =  0.020
 Manning's FRICTION FACTOR for Streetflow Section(curb-to-curb) =   0.0150
 Manning's FRICTION FACTOR for Back-of-Walk Flow Section =   0.0200

   **TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) =       1.23
   STREETFLOW MODEL RESULTS USING ESTIMATED FLOW:
   STREET FLOW DEPTH(FEET) =  0.28
   HALFSTREET FLOOD WIDTH(FEET) =    7.91
   AVERAGE FLOW VELOCITY(FEET/SEC.) =    1.65
   PRODUCT OF DEPTH&VELOCITY(FT*FT/SEC.) =    0.47
 STREET FLOW TRAVEL TIME(MIN.) =  3.60   Tc(MIN.) =  10.61
 *  10 YEAR RAINFALL INTENSITY(INCH/HR) =  2.638
 SUBAREA LOSS RATE DATA(AMC  II):
  DEVELOPMENT TYPE/       SCS SOIL   AREA      Fp         Ap      SCS
      LAND USE             GROUP   (ACRES)  (INCH/HR)  (DECIMAL)  CN
 COMMERCIAL                  A        0.36      0.40      0.100    32
 SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) =  0.40
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap =  0.100
 SUBAREA AREA(ACRES) =     0.36      SUBAREA RUNOFF(CFS) =     0.84
 EFFECTIVE AREA(ACRES) =      0.63    AREA-AVERAGED Fm(INCH/HR) =  0.04
 AREA-AVERAGED Fp(INCH/HR) =  0.40  AREA-AVERAGED Ap =  0.10
 TOTAL AREA(ACRES) =       0.6        PEAK FLOW RATE(CFS) =       1.47

 END OF SUBAREA STREET FLOW HYDRAULICS:
 DEPTH(FEET) = 0.30   HALFSTREET FLOOD WIDTH(FEET) =   8.59
 FLOW VELOCITY(FEET/SEC.) =  1.72   DEPTH*VELOCITY(FT*FT/SEC.) =   0.51
 LONGEST FLOWPATH FROM NODE     42.10 TO NODE     43.00 =     627.00 FEET.

****************************************************************************
 FLOW PROCESS FROM NODE     43.00 TO NODE     43.00 IS CODE =   1
----------------------------------------------------------------------------
 >>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
 >>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<
============================================================================
 TOTAL NUMBER OF STREAMS =  2
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM  2 ARE:
 TIME OF CONCENTRATION(MIN.) =   10.61
 RAINFALL INTENSITY(INCH/HR) =   2.64
 AREA-AVERAGED Fm(INCH/HR) =  0.04
 AREA-AVERAGED Fp(INCH/HR) =  0.40
 AREA-AVERAGED Ap =  0.10
 EFFECTIVE STREAM AREA(ACRES) =       0.63
 TOTAL STREAM AREA(ACRES) =       0.63
 PEAK FLOW RATE(CFS) AT CONFLUENCE =       1.47

 ** CONFLUENCE DATA **
  STREAM       Q      Tc    Intensity   Fp(Fm)      Ap      Ae      HEADWATER
  NUMBER     (CFS)   (MIN.) (INCH/HR) (INCH/HR)           (ACRES)     NODE
     1       22.15    9.11    2.879  0.40( 0.09) 0.23       8.0       39.10
     1       22.16    9.13    2.875  0.40( 0.09) 0.23       8.0       40.10
     1       22.19    9.32    2.841  0.40( 0.09) 0.23       8.2       37.10
     1       22.16    9.83    2.757  0.40( 0.09) 0.23       8.4       35.10
     1       22.16    9.84    2.755  0.40( 0.09) 0.23       8.4       36.10
     1       22.05   10.76    2.617  0.40( 0.09) 0.23       8.9       38.10
     1       21.27   14.01    2.249  0.40( 0.10) 0.24      10.0       41.10
     1       19.55   17.93    1.953  0.40( 0.10) 0.26      11.0       31.00
     2        1.47   10.61    2.638  0.40( 0.04) 0.10       0.6       42.10

 RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
 CONFLUENCE FORMULA USED FOR  2 STREAMS.

 ** PEAK FLOW RATE TABLE **
  STREAM       Q      Tc    Intensity   Fp(Fm)      Ap      Ae      HEADWATER
  NUMBER     (CFS)   (MIN.) (INCH/HR) (INCH/HR)           (ACRES)     NODE
     1       23.53    9.11    2.879  0.40( 0.09) 0.22       8.6       39.10
     2       23.55    9.13    2.875  0.40( 0.09) 0.22       8.6       40.10
     3       23.59    9.32    2.841  0.40( 0.09) 0.22       8.7       37.10
     4       23.59    9.83    2.757  0.40( 0.09) 0.22       9.0       35.10
     5       23.59    9.84    2.755  0.40( 0.09) 0.22       9.0       36.10
     6       23.54   10.61    2.638  0.40( 0.09) 0.22       9.4       42.10
     7       23.51   10.76    2.617  0.40( 0.09) 0.22       9.5       38.10
     8       22.52   14.01    2.249  0.40( 0.09) 0.23      10.6       41.10
```

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    9       20.63    17.93     1.953  0.40( 0.10) 0.25       11.6       31.00

  COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
  PEAK FLOW RATE(CFS) =       23.59     Tc(MIN.) =      9.83
  EFFECTIVE AREA(ACRES) =       9.01   AREA-AVERAGED Fm(INCH/HR) =  0.09
  AREA-AVERAGED Fp(INCH/HR) =  0.40  AREA-AVERAGED Ap =  0.22
  TOTAL AREA(ACRES) =       11.6
  LONGEST FLOWPATH FROM NODE     31.00 TO NODE     43.00 =    1425.00 FEET.

****************************************************************************
  FLOW PROCESS FROM NODE     43.00 TO NODE     44.00 IS CODE =  31
----------------------------------------------------------------------------
  >>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
  >>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<
============================================================================
  ELEVATION DATA: UPSTREAM(FEET) =   138.55  DOWNSTREAM(FEET) =   138.35
  FLOW LENGTH(FEET) =    40.00   MANNING'S N =  0.013
  DEPTH OF FLOW IN  30.0 INCH PIPE IS  21.4 INCHES
  PIPE-FLOW VELOCITY(FEET/SEC.) =   6.30
  ESTIMATED PIPE DIAMETER(INCH) =  30.00    NUMBER OF PIPES =   1
  PIPE-FLOW(CFS) =      23.59
  PIPE TRAVEL TIME(MIN.) =   0.11    Tc(MIN.) =    9.93
  LONGEST FLOWPATH FROM NODE     31.00 TO NODE     44.00 =    1465.00 FEET.

****************************************************************************
  FLOW PROCESS FROM NODE     44.00 TO NODE     44.00 IS CODE =   1
----------------------------------------------------------------------------
  >>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
============================================================================
  TOTAL NUMBER OF STREAMS =  2
  CONFLUENCE VALUES USED FOR INDEPENDENT STREAM  1 ARE:
  TIME OF CONCENTRATION(MIN.) =    9.93
  RAINFALL INTENSITY(INCH/HR) =   2.74
  AREA-AVERAGED Fm(INCH/HR) =  0.09
  AREA-AVERAGED Fp(INCH/HR) =  0.40
  AREA-AVERAGED Ap =  0.22
  EFFECTIVE STREAM AREA(ACRES) =       9.01
  TOTAL STREAM AREA(ACRES) =      11.58
  PEAK FLOW RATE(CFS) AT CONFLUENCE =      23.59

****************************************************************************
  FLOW PROCESS FROM NODE     43.10 TO NODE     40.00 IS CODE =  21
----------------------------------------------------------------------------
  >>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
  >>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<
============================================================================
  INITIAL SUBAREA FLOW-LENGTH(FEET) =   265.00
  ELEVATION DATA: UPSTREAM(FEET) =    150.73  DOWNSTREAM(FEET) =    148.33

  Tc = K*[(LENGTH** 3.00)/(ELEVATION CHANGE)]**0.20
  SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) =    7.257
  *  10 YEAR RAINFALL INTENSITY(INCH/HR) =  3.279
  SUBAREA Tc AND LOSS RATE DATA(AMC  II):
   DEVELOPMENT TYPE/       SCS SOIL   AREA      Fp         Ap      SCS   Tc
      LAND USE             GROUP   (ACRES)  (INCH/HR)  (DECIMAL)  CN  (MIN.)
  COMMERCIAL                 A        0.22      0.40      0.100    32    7.26
  SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) =  0.40
  SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap =  0.100
  SUBAREA RUNOFF(CFS) =      0.64
  TOTAL AREA(ACRES) =      0.22   PEAK FLOW RATE(CFS) =      0.64

****************************************************************************
  FLOW PROCESS FROM NODE     40.00 TO NODE     44.00 IS CODE =  61
----------------------------------------------------------------------------
  >>>>>COMPUTE STREET FLOW TRAVEL TIME THRU SUBAREA<<<<<
  >>>>>(STANDARD CURB SECTION USED)<<<<<
============================================================================
  UPSTREAM ELEVATION(FEET) =  148.33  DOWNSTREAM ELEVATION(FEET) =  146.08
  STREET LENGTH(FEET) =   364.00   CURB HEIGHT(INCHES) =  6.0
  STREET HALFWIDTH(FEET) = 21.00

  DISTANCE FROM CROWN TO CROSSFALL GRADEBREAK(FEET) =  16.00
  INSIDE STREET CROSSFALL(DECIMAL) =  0.020
  OUTSIDE STREET CROSSFALL(DECIMAL) =  0.020

  SPECIFIED NUMBER OF HALFSTREETS CARRYING RUNOFF =  1
  STREET PARKWAY CROSSFALL(DECIMAL) =  0.020
  Manning's FRICTION FACTOR for Streetflow Section(curb-to-curb) =   0.0150
  Manning's FRICTION FACTOR for Back-of-Walk Flow Section =   0.0200

    **TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) =       0.98
    STREETFLOW MODEL RESULTS USING ESTIMATED FLOW:
    STREET FLOW DEPTH(FEET) =  0.27
    HALFSTREET FLOOD WIDTH(FEET) =    7.28
    AVERAGE FLOW VELOCITY(FEET/SEC.) =    1.51
    PRODUCT OF DEPTH&VELOCITY(FT*FT/SEC.) =    0.41
  STREET FLOW TRAVEL TIME(MIN.) =   4.01   Tc(MIN.) =   11.27
  *  10 YEAR RAINFALL INTENSITY(INCH/HR) =  2.549
  SUBAREA LOSS RATE DATA(AMC  II):
   DEVELOPMENT TYPE/       SCS SOIL   AREA      Fp         Ap      SCS
      LAND USE             GROUP   (ACRES)  (INCH/HR)  (DECIMAL)  CN
  COMMERCIAL                 A        0.30      0.40      0.100    32
  SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) =  0.40
  SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap =  0.100
  SUBAREA AREA(ACRES) =    0.30      SUBAREA RUNOFF(CFS) =    0.68
  EFFECTIVE AREA(ACRES) =      0.52    AREA-AVERAGED Fm(INCH/HR) =  0.04
  AREA-AVERAGED Fp(INCH/HR) =  0.40  AREA-AVERAGED Ap =  0.10
  TOTAL AREA(ACRES) =       0.5        PEAK FLOW RATE(CFS) =       1.17

  END OF SUBAREA STREET FLOW HYDRAULICS:
  DEPTH(FEET) = 0.28   HALFSTREET FLOOD WIDTH(FEET) =   7.91
  FLOW VELOCITY(FEET/SEC.) =  1.58   DEPTH*VELOCITY(FT*FT/SEC.) =   0.45
  LONGEST FLOWPATH FROM NODE     43.10 TO NODE     44.00 =     629.00 FEET.

****************************************************************************
  FLOW PROCESS FROM NODE     44.00 TO NODE     44.00 IS CODE =  81
----------------------------------------------------------------------------
  >>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<
============================================================================
  MAINLINE Tc(MIN.) =   11.27
  *  10 YEAR RAINFALL INTENSITY(INCH/HR) =  2.549
  SUBAREA LOSS RATE DATA(AMC  II):
```

```
 DEVELOPMENT TYPE/       SCS SOIL   AREA      Fp         Ap      SCS
    LAND USE             GROUP   (ACRES)  (INCH/HR)  (DECIMAL)  CN
APARTMENTS                 A       1.46      0.40       0.200    32
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) =  0.40
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap =  0.200
SUBAREA AREA(ACRES) =     1.46     SUBAREA RUNOFF(CFS) =     3.24
EFFECTIVE AREA(ACRES) =      1.98   AREA-AVERAGED Fm(INCH/HR) =  0.07
AREA-AVERAGED Fp(INCH/HR) =  0.40  AREA-AVERAGED Ap =  0.17
TOTAL AREA(ACRES) =        2.0       PEAK FLOW RATE(CFS) =       4.42

****************************************************************************
 FLOW PROCESS FROM NODE      44.00 TO NODE      44.00 IS CODE =   1
----------------------------------------------------------------------------
 >>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
 >>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<
============================================================================
 TOTAL NUMBER OF STREAMS =  2
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM  2 ARE:
 TIME OF CONCENTRATION(MIN.) =   11.27
 RAINFALL INTENSITY(INCH/HR) =   2.55
 AREA-AVERAGED Fm(INCH/HR) =  0.07
 AREA-AVERAGED Fp(INCH/HR) =  0.40
 AREA-AVERAGED Ap =  0.17
 EFFECTIVE STREAM AREA(ACRES) =       1.98
 TOTAL STREAM AREA(ACRES) =       1.98
 PEAK FLOW RATE(CFS) AT CONFLUENCE =       4.42

 ** CONFLUENCE DATA **
 STREAM       Q      Tc   Intensity   Fp(Fm)      Ap      Ae      HEADWATER
 NUMBER     (CFS)  (MIN.) (INCH/HR) (INCH/HR)           (ACRES)    NODE
    1       23.53    9.21    2.860  0.40( 0.09) 0.22       8.6      39.10
    1       23.55    9.23    2.856  0.40( 0.09) 0.22       8.6      40.10
    1       23.59    9.43    2.823  0.40( 0.09) 0.22       8.7      37.10
    1       23.59    9.93    2.740  0.40( 0.09) 0.22       9.0      35.10
    1       23.59    9.94    2.738  0.40( 0.09) 0.22       9.0      36.10
    1       23.54   10.71    2.623  0.40( 0.09) 0.22       9.4      42.10
    1       23.51   10.86    2.603  0.40( 0.09) 0.22       9.5      38.10
    1       22.52   14.12    2.239  0.40( 0.09) 0.23      10.6      41.10
    1       20.63   18.04    1.946  0.40( 0.10) 0.25      11.6      31.00
    2        4.42   11.27    2.549  0.40( 0.07) 0.17       2.0      43.10

 RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
 CONFLUENCE FORMULA USED FOR  2 STREAMS.

 ** PEAK FLOW RATE TABLE **
 STREAM       Q      Tc   Intensity   Fp(Fm)      Ap      Ae      HEADWATER
 NUMBER     (CFS)  (MIN.) (INCH/HR) (INCH/HR)           (ACRES)    NODE
    1       27.60    9.21    2.860  0.40( 0.08) 0.21      10.2      39.10
    2       27.62    9.23    2.856  0.40( 0.08) 0.21      10.2      40.10
    3       27.69    9.43    2.823  0.40( 0.08) 0.21      10.4      37.10
    4       27.79    9.93    2.740  0.40( 0.09) 0.21      10.8      35.10
    5       27.79    9.94    2.738  0.40( 0.09) 0.21      10.8      36.10
    6       27.87   10.71    2.623  0.40( 0.09) 0.21      11.3      42.10
    7       27.86   10.86    2.603  0.40( 0.09) 0.21      11.4      38.10
    8       27.81   11.27    2.549  0.40( 0.09) 0.21      11.6      43.10
    9       26.39   14.12    2.239  0.40( 0.09) 0.22      12.6      41.10
   10       23.98   18.04    1.946  0.40( 0.09) 0.24      13.6      31.00

 COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
 PEAK FLOW RATE(CFS) =      27.87    Tc(MIN.) =    10.71
 EFFECTIVE AREA(ACRES) =      11.33   AREA-AVERAGED Fm(INCH/HR) =  0.09
 AREA-AVERAGED Fp(INCH/HR) =  0.40  AREA-AVERAGED Ap =  0.21
 TOTAL AREA(ACRES) =       13.6
 LONGEST FLOWPATH FROM NODE     31.00 TO NODE     44.00 =    1465.00 FEET.

****************************************************************************
 FLOW PROCESS FROM NODE      61.00 TO NODE      62.00 IS CODE =  21
----------------------------------------------------------------------------
 >>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
 >>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<
============================================================================
 INITIAL SUBAREA FLOW-LENGTH(FEET) =   300.00
 ELEVATION DATA: UPSTREAM(FEET) =    155.00  DOWNSTREAM(FEET) =    152.70

 Tc = K*[(LENGTH** 3.00)/(ELEVATION CHANGE)]**0.20
 SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) =    8.404
 *  10 YEAR RAINFALL INTENSITY(INCH/HR) =  3.015
 SUBAREA Tc AND LOSS RATE DATA(AMC  II):
  DEVELOPMENT TYPE/       SCS SOIL   AREA      Fp         Ap      SCS   Tc
     LAND USE             GROUP   (ACRES)  (INCH/HR)  (DECIMAL)  CN  (MIN.)
 APARTMENTS                 A       1.26      0.40       0.200    32    8.40
 SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) =  0.40
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap =  0.200
 SUBAREA RUNOFF(CFS) =      3.33
 TOTAL AREA(ACRES) =      1.26   PEAK FLOW RATE(CFS) =      3.33

****************************************************************************
 FLOW PROCESS FROM NODE      62.00 TO NODE      63.00 IS CODE =  61
----------------------------------------------------------------------------
 >>>>>COMPUTE STREET FLOW TRAVEL TIME THRU SUBAREA<<<<<
 >>>>>(STANDARD CURB SECTION USED)<<<<<
============================================================================
 UPSTREAM ELEVATION(FEET) =  152.70  DOWNSTREAM ELEVATION(FEET) =  151.40
 STREET LENGTH(FEET) =   210.00   CURB HEIGHT(INCHES) =  6.0
 STREET HALFWIDTH(FEET) = 21.00

 DISTANCE FROM CROWN TO CROSSFALL GRADEBREAK(FEET) =  16.00
 INSIDE STREET CROSSFALL(DECIMAL) =  0.020
 OUTSIDE STREET CROSSFALL(DECIMAL) =  0.020

 SPECIFIED NUMBER OF HALFSTREETS CARRYING RUNOFF =  1
 STREET PARKWAY CROSSFALL(DECIMAL) =  0.020
 Manning's FRICTION FACTOR for Streetflow Section(curb-to-curb) =   0.0150
 Manning's FRICTION FACTOR for Back-of-Walk Flow Section =   0.0200

   **TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) =       5.83
   STREETFLOW MODEL RESULTS USING ESTIMATED FLOW:
   STREET FLOW DEPTH(FEET) =  0.44
   HALFSTREET FLOOD WIDTH(FEET) =   15.59
   AVERAGE FLOW VELOCITY(FEET/SEC.) =    2.29
   PRODUCT OF DEPTH&VELOCITY(FT*FT/SEC.) =    1.00
 STREET FLOW TRAVEL TIME(MIN.) =   1.53   Tc(MIN.) =    9.93
```

```
 *  10 YEAR RAINFALL INTENSITY(INCH/HR) =  2.739
 SUBAREA LOSS RATE DATA(AMC  II):
  DEVELOPMENT TYPE/       SCS SOIL   AREA      Fp         Ap      SCS
      LAND USE             GROUP   (ACRES)  (INCH/HR)  (DECIMAL)  CN
 APARTMENTS                  A        2.09      0.40      0.200    32
 SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) =  0.40
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap =  0.200
 SUBAREA AREA(ACRES) =     2.09      SUBAREA RUNOFF(CFS) =     5.00
 EFFECTIVE AREA(ACRES) =      3.35    AREA-AVERAGED Fm(INCH/HR) =  0.08
 AREA-AVERAGED Fp(INCH/HR) =  0.40  AREA-AVERAGED Ap =  0.20
 TOTAL AREA(ACRES) =        3.3        PEAK FLOW RATE(CFS) =        8.02

 END OF SUBAREA STREET FLOW HYDRAULICS:
 DEPTH(FEET) = 0.48   HALFSTREET FLOOD WIDTH(FEET) =  17.72
 FLOW VELOCITY(FEET/SEC.) =  2.46   DEPTH*VELOCITY(FT*FT/SEC.) =   1.18
 LONGEST FLOWPATH FROM NODE     61.00 TO NODE     63.00 =     510.00 FEET.

****************************************************************************
 FLOW PROCESS FROM NODE     63.00 TO NODE     63.00 IS CODE =   1
----------------------------------------------------------------------------
 >>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
============================================================================
 TOTAL NUMBER OF STREAMS =  2
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM  1 ARE:
 TIME OF CONCENTRATION(MIN.) =    9.93
 RAINFALL INTENSITY(INCH/HR) =   2.74
 AREA-AVERAGED Fm(INCH/HR) =  0.08
 AREA-AVERAGED Fp(INCH/HR) =  0.40
 AREA-AVERAGED Ap =  0.20
 EFFECTIVE STREAM AREA(ACRES) =       3.35
 TOTAL STREAM AREA(ACRES) =       3.35
 PEAK FLOW RATE(CFS) AT CONFLUENCE =       8.02

****************************************************************************
 FLOW PROCESS FROM NODE     62.10 TO NODE     63.00 IS CODE =  21
----------------------------------------------------------------------------
 >>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
 >>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<
============================================================================
 INITIAL SUBAREA FLOW-LENGTH(FEET) =   246.00
 ELEVATION DATA: UPSTREAM(FEET) =    153.90  DOWNSTREAM(FEET) =    151.41

 Tc = K*[(LENGTH** 3.00)/(ELEVATION CHANGE)]**0.20
 SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) =    7.343
 *  10 YEAR RAINFALL INTENSITY(INCH/HR) =  3.257
 SUBAREA Tc AND LOSS RATE DATA(AMC  II):
  DEVELOPMENT TYPE/       SCS SOIL   AREA      Fp         Ap      SCS   Tc
      LAND USE             GROUP   (ACRES)  (INCH/HR)  (DECIMAL)  CN  (MIN.)
 APARTMENTS                  A        1.35      0.40      0.200    32    7.34
 SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) =  0.40
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap =  0.200
 SUBAREA RUNOFF(CFS) =      3.86
 TOTAL AREA(ACRES) =      1.35   PEAK FLOW RATE(CFS) =      3.86

****************************************************************************
 FLOW PROCESS FROM NODE     63.00 TO NODE     63.00 IS CODE =   1
----------------------------------------------------------------------------
 >>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
 >>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<
============================================================================
 TOTAL NUMBER OF STREAMS =  2
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM  2 ARE:
 TIME OF CONCENTRATION(MIN.) =    7.34
 RAINFALL INTENSITY(INCH/HR) =   3.26
 AREA-AVERAGED Fm(INCH/HR) =  0.08
 AREA-AVERAGED Fp(INCH/HR) =  0.40
 AREA-AVERAGED Ap =  0.20
 EFFECTIVE STREAM AREA(ACRES) =       1.35
 TOTAL STREAM AREA(ACRES) =       1.35
 PEAK FLOW RATE(CFS) AT CONFLUENCE =       3.86

 ** CONFLUENCE DATA **
  STREAM       Q      Tc    Intensity   Fp(Fm)      Ap      Ae      HEADWATER
  NUMBER     (CFS)  (MIN.)  (INCH/HR)  (INCH/HR)           (ACRES)    NODE
     1        8.02    9.93     2.739  0.40( 0.08) 0.20        3.3       61.00
     2        3.86    7.34     3.257  0.40( 0.08) 0.20        1.4       62.10

 RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
 CONFLUENCE FORMULA USED FOR  2 STREAMS.

 ** PEAK FLOW RATE TABLE **
  STREAM       Q      Tc    Intensity   Fp(Fm)      Ap      Ae      HEADWATER
  NUMBER     (CFS)  (MIN.)  (INCH/HR)  (INCH/HR)           (ACRES)    NODE
     1       10.94    7.34     3.257  0.40( 0.08) 0.20        3.8       62.10
     2       11.25    9.93     2.739  0.40( 0.08) 0.20        4.7       61.00

 COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
 PEAK FLOW RATE(CFS) =      11.25    Tc(MIN.) =     9.93
 EFFECTIVE AREA(ACRES) =       4.70   AREA-AVERAGED Fm(INCH/HR) =  0.08
 AREA-AVERAGED Fp(INCH/HR) =  0.40  AREA-AVERAGED Ap =  0.20
 TOTAL AREA(ACRES) =        4.7
 LONGEST FLOWPATH FROM NODE     61.00 TO NODE     63.00 =     510.00 FEET.

****************************************************************************
 FLOW PROCESS FROM NODE     63.00 TO NODE     64.00 IS CODE =  31
----------------------------------------------------------------------------
 >>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
 >>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<
============================================================================
 ELEVATION DATA: UPSTREAM(FEET) =   147.30  DOWNSTREAM(FEET) =   146.70
 FLOW LENGTH(FEET) =   305.00   MANNING'S N =  0.013
 DEPTH OF FLOW IN  27.0 INCH PIPE IS  19.3 INCHES
 PIPE-FLOW VELOCITY(FEET/SEC.) =   3.69
 ESTIMATED PIPE DIAMETER(INCH) =  27.00    NUMBER OF PIPES =   1
 PIPE-FLOW(CFS) =      11.25
 PIPE TRAVEL TIME(MIN.) =   1.38    Tc(MIN.) =   11.31
 LONGEST FLOWPATH FROM NODE     61.00 TO NODE     64.00 =     815.00 FEET.

****************************************************************************
 FLOW PROCESS FROM NODE     64.00 TO NODE     64.00 IS CODE =   1
----------------------------------------------------------------------------
 >>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
```

```
================================================================================
 TOTAL NUMBER OF STREAMS =  2
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM  1 ARE:
 TIME OF CONCENTRATION(MIN.) =   11.31
 RAINFALL INTENSITY(INCH/HR) =   2.54
 AREA-AVERAGED Fm(INCH/HR) =  0.08
 AREA-AVERAGED Fp(INCH/HR) =  0.40
 AREA-AVERAGED Ap =  0.20
 EFFECTIVE STREAM AREA(ACRES) =       4.70
 TOTAL STREAM AREA(ACRES) =       4.70
 PEAK FLOW RATE(CFS) AT CONFLUENCE =      11.25

****************************************************************************
 FLOW PROCESS FROM NODE     63.10 TO NODE     64.00 IS CODE =  21
----------------------------------------------------------------------------
 >>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
 >>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<
============================================================================
 INITIAL SUBAREA FLOW-LENGTH(FEET) =   130.00
 ELEVATION DATA: UPSTREAM(FEET) =    152.17  DOWNSTREAM(FEET) =    151.43

 Tc = K*[(LENGTH** 3.00)/(ELEVATION CHANGE)]**0.20
 SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) =    5.990
 *  10 YEAR RAINFALL INTENSITY(INCH/HR) =  3.660
 SUBAREA Tc AND LOSS RATE DATA(AMC  II):
  DEVELOPMENT TYPE/       SCS SOIL   AREA      Fp         Ap      SCS   Tc
      LAND USE             GROUP   (ACRES)  (INCH/HR)  (DECIMAL)  CN  (MIN.)
 COMMERCIAL                  A        0.15      0.40       0.100    32    5.99
 SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) =  0.40
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap =  0.100
 SUBAREA RUNOFF(CFS) =      0.49
 TOTAL AREA(ACRES) =      0.15   PEAK FLOW RATE(CFS) =      0.49

****************************************************************************
 FLOW PROCESS FROM NODE     64.00 TO NODE     64.00 IS CODE =   1
----------------------------------------------------------------------------
 >>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
 >>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<
============================================================================
 TOTAL NUMBER OF STREAMS =  2
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM  2 ARE:
 TIME OF CONCENTRATION(MIN.) =    5.99
 RAINFALL INTENSITY(INCH/HR) =   3.66
 AREA-AVERAGED Fm(INCH/HR) =  0.04
 AREA-AVERAGED Fp(INCH/HR) =  0.40
 AREA-AVERAGED Ap =  0.10
 EFFECTIVE STREAM AREA(ACRES) =       0.15
 TOTAL STREAM AREA(ACRES) =       0.15
 PEAK FLOW RATE(CFS) AT CONFLUENCE =       0.49

 ** CONFLUENCE DATA **
  STREAM      Q      Tc   Intensity   Fp(Fm)      Ap      Ae      HEADWATER
  NUMBER    (CFS)  (MIN.) (INCH/HR) (INCH/HR)           (ACRES)     NODE
     1      10.94   8.73     2.950  0.40( 0.08) 0.20       3.8       62.10
     1      11.25  11.31     2.543  0.40( 0.08) 0.20       4.7       61.00
     2       0.49   5.99     3.660  0.40( 0.04) 0.10       0.2       63.10

 RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
 CONFLUENCE FORMULA USED FOR  2 STREAMS.

 ** PEAK FLOW RATE TABLE **
  STREAM      Q      Tc   Intensity   Fp(Fm)      Ap      Ae      HEADWATER
  NUMBER    (CFS)  (MIN.) (INCH/HR) (INCH/HR)           (ACRES)     NODE
     1       9.86   5.99     3.660  0.40( 0.08) 0.19       2.8       63.10
     2      11.33   8.73     2.950  0.40( 0.08) 0.20       4.0       62.10
     3      11.59  11.31     2.543  0.40( 0.08) 0.20       4.8       61.00

 COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
 PEAK FLOW RATE(CFS) =      11.59    Tc(MIN.) =    11.31
 EFFECTIVE AREA(ACRES) =       4.85   AREA-AVERAGED Fm(INCH/HR) =  0.08
 AREA-AVERAGED Fp(INCH/HR) =  0.40  AREA-AVERAGED Ap =  0.20
 TOTAL AREA(ACRES) =       4.8
 LONGEST FLOWPATH FROM NODE     61.00 TO NODE     64.00 =     815.00 FEET.

****************************************************************************
 FLOW PROCESS FROM NODE     64.00 TO NODE     65.00 IS CODE =  31
----------------------------------------------------------------------------
 >>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
 >>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<
============================================================================
 ELEVATION DATA: UPSTREAM(FEET) =   146.70  DOWNSTREAM(FEET) =   146.65
 FLOW LENGTH(FEET) =    17.00   MANNING'S N =  0.013
 DEPTH OF FLOW IN  24.0 INCH PIPE IS  19.6 INCHES
 PIPE-FLOW VELOCITY(FEET/SEC.) =   4.22
 ESTIMATED PIPE DIAMETER(INCH) =  24.00    NUMBER OF PIPES =   1
 PIPE-FLOW(CFS) =      11.59
 PIPE TRAVEL TIME(MIN.) =   0.07    Tc(MIN.) =   11.38
 LONGEST FLOWPATH FROM NODE     61.00 TO NODE     65.00 =     832.00 FEET.

****************************************************************************
 FLOW PROCESS FROM NODE     65.00 TO NODE     65.00 IS CODE =   1
----------------------------------------------------------------------------
 >>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
============================================================================
 TOTAL NUMBER OF STREAMS =  2
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM  1 ARE:
 TIME OF CONCENTRATION(MIN.) =   11.38
 RAINFALL INTENSITY(INCH/HR) =   2.53
 AREA-AVERAGED Fm(INCH/HR) =  0.08
 AREA-AVERAGED Fp(INCH/HR) =  0.40
 AREA-AVERAGED Ap =  0.20
 EFFECTIVE STREAM AREA(ACRES) =       4.85
 TOTAL STREAM AREA(ACRES) =       4.85
 PEAK FLOW RATE(CFS) AT CONFLUENCE =      11.59

****************************************************************************
 FLOW PROCESS FROM NODE     64.10 TO NODE     65.00 IS CODE =  21
----------------------------------------------------------------------------
 >>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
 >>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<
============================================================================
 INITIAL SUBAREA FLOW-LENGTH(FEET) =   279.00
```

```
 ELEVATION DATA: UPSTREAM(FEET) =    153.50  DOWNSTREAM(FEET) =    151.43

 Tc = K*[(LENGTH** 3.00)/(ELEVATION CHANGE)]**0.20
 SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) =    8.217
 *  10 YEAR RAINFALL INTENSITY(INCH/HR) =  3.054
 SUBAREA Tc AND LOSS RATE DATA(AMC  II):
  DEVELOPMENT TYPE/       SCS SOIL   AREA      Fp         Ap      SCS   Tc
      LAND USE            GROUP   (ACRES)  (INCH/HR)  (DECIMAL)  CN  (MIN.)
 APARTMENTS                 A        0.78      0.40      0.200    32    8.22
 SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) =  0.40
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap =  0.200
 SUBAREA RUNOFF(CFS) =      2.09
 TOTAL AREA(ACRES) =      0.78   PEAK FLOW RATE(CFS) =      2.09

****************************************************************************
 FLOW PROCESS FROM NODE     65.00 TO NODE     65.00 IS CODE =   1
----------------------------------------------------------------------------
 >>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
 >>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<
============================================================================
 TOTAL NUMBER OF STREAMS =  2
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM  2 ARE:
 TIME OF CONCENTRATION(MIN.) =    8.22
 RAINFALL INTENSITY(INCH/HR) =   3.05
 AREA-AVERAGED Fm(INCH/HR) =  0.08
 AREA-AVERAGED Fp(INCH/HR) =  0.40
 AREA-AVERAGED Ap =  0.20
 EFFECTIVE STREAM AREA(ACRES) =       0.78
 TOTAL STREAM AREA(ACRES) =       0.78
 PEAK FLOW RATE(CFS) AT CONFLUENCE =       2.09

 ** CONFLUENCE DATA **
  STREAM       Q      Tc   Intensity   Fp(Fm)      Ap      Ae      HEADWATER
  NUMBER     (CFS)  (MIN.) (INCH/HR) (INCH/HR)          (ACRES)     NODE
     1        9.86    6.06    3.637  0.40( 0.08) 0.19       2.8      63.10
     1       11.33    8.79    2.937  0.40( 0.08) 0.20       4.0      62.10
     1       11.59   11.38    2.534  0.40( 0.08) 0.20       4.8      61.00
     2        2.09    8.22    3.054  0.40( 0.08) 0.20       0.8      64.10

 RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
 CONFLUENCE FORMULA USED FOR  2 STREAMS.

 ** PEAK FLOW RATE TABLE **
  STREAM       Q      Tc   Intensity   Fp(Fm)      Ap      Ae      HEADWATER
  NUMBER     (CFS)  (MIN.) (INCH/HR) (INCH/HR)          (ACRES)     NODE
     1       11.70    6.06    3.637  0.40( 0.08) 0.20       3.4      63.10
     2       13.11    8.22    3.054  0.40( 0.08) 0.20       4.5      64.10
     3       13.34    8.79    2.937  0.40( 0.08) 0.20       4.8      62.10
     4       13.31   11.38    2.534  0.40( 0.08) 0.20       5.6      61.00

 COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
 PEAK FLOW RATE(CFS) =      13.34    Tc(MIN.) =     8.79
 EFFECTIVE AREA(ACRES) =       4.76   AREA-AVERAGED Fm(INCH/HR) =  0.08
 AREA-AVERAGED Fp(INCH/HR) =  0.40  AREA-AVERAGED Ap =  0.20
 TOTAL AREA(ACRES) =        5.6
 LONGEST FLOWPATH FROM NODE     61.00 TO NODE     65.00 =     832.00 FEET.

****************************************************************************
 FLOW PROCESS FROM NODE     65.00 TO NODE     66.00 IS CODE =  31
----------------------------------------------------------------------------
 >>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
 >>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<
============================================================================
 ELEVATION DATA: UPSTREAM(FEET) =   146.65  DOWNSTREAM(FEET) =   146.36
 FLOW LENGTH(FEET) =    89.00   MANNING'S N =  0.013
 DEPTH OF FLOW IN  27.0 INCH PIPE IS  18.2 INCHES
 PIPE-FLOW VELOCITY(FEET/SEC.) =   4.68
 ESTIMATED PIPE DIAMETER(INCH) =  27.00    NUMBER OF PIPES =   1
 PIPE-FLOW(CFS) =      13.34
 PIPE TRAVEL TIME(MIN.) =   0.32    Tc(MIN.) =    9.11
 LONGEST FLOWPATH FROM NODE     61.00 TO NODE     66.00 =     921.00 FEET.

****************************************************************************
 FLOW PROCESS FROM NODE     66.00 TO NODE     66.00 IS CODE =   1
----------------------------------------------------------------------------
 >>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
============================================================================
 TOTAL NUMBER OF STREAMS =  2
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM  1 ARE:
 TIME OF CONCENTRATION(MIN.) =    9.11
 RAINFALL INTENSITY(INCH/HR) =   2.88
 AREA-AVERAGED Fm(INCH/HR) =  0.08
 AREA-AVERAGED Fp(INCH/HR) =  0.40
 AREA-AVERAGED Ap =  0.20
 EFFECTIVE STREAM AREA(ACRES) =       4.76
 TOTAL STREAM AREA(ACRES) =       5.63
 PEAK FLOW RATE(CFS) AT CONFLUENCE =      13.34

****************************************************************************
 FLOW PROCESS FROM NODE     65.10 TO NODE     66.00 IS CODE =  21
----------------------------------------------------------------------------
 >>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
 >>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<
============================================================================
 INITIAL SUBAREA FLOW-LENGTH(FEET) =   163.00
 ELEVATION DATA: UPSTREAM(FEET) =    154.16  DOWNSTREAM(FEET) =    151.82

 Tc = K*[(LENGTH** 3.00)/(ELEVATION CHANGE)]**0.20
 SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) =    5.449
 *  10 YEAR RAINFALL INTENSITY(INCH/HR) =  3.864
 SUBAREA Tc AND LOSS RATE DATA(AMC  II):
  DEVELOPMENT TYPE/       SCS SOIL   AREA      Fp         Ap      SCS   Tc
      LAND USE            GROUP   (ACRES)  (INCH/HR)  (DECIMAL)  CN  (MIN.)
 COMMERCIAL                 A        0.16      0.40      0.100    32    5.45
 SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) =  0.40
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap =  0.100
 SUBAREA RUNOFF(CFS) =      0.55
 TOTAL AREA(ACRES) =      0.16   PEAK FLOW RATE(CFS) =      0.55

****************************************************************************
 FLOW PROCESS FROM NODE     66.00 TO NODE     66.00 IS CODE =   1
----------------------------------------------------------------------------
```

```
>>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
>>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<
============================================================================
 TOTAL NUMBER OF STREAMS =  2
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM  2 ARE:
 TIME OF CONCENTRATION(MIN.) =     5.45
 RAINFALL INTENSITY(INCH/HR) =    3.86
 AREA-AVERAGED Fm(INCH/HR) =  0.04
 AREA-AVERAGED Fp(INCH/HR) =  0.40
 AREA-AVERAGED Ap =  0.10
 EFFECTIVE STREAM AREA(ACRES) =        0.16
 TOTAL STREAM AREA(ACRES) =        0.16
 PEAK FLOW RATE(CFS) AT CONFLUENCE =        0.55

 ** CONFLUENCE DATA **
  STREAM       Q      Tc    Intensity   Fp(Fm)      Ap      Ae      HEADWATER
  NUMBER     (CFS)   (MIN.) (INCH/HR) (INCH/HR)           (ACRES)     NODE
     1       11.70    6.39     3.527  0.40( 0.08) 0.20       3.4       63.10
     1       13.11    8.53     2.988  0.40( 0.08) 0.20       4.5       64.10
     1       13.34    9.11     2.878  0.40( 0.08) 0.20       4.8       62.10
     1       13.31   11.70     2.495  0.40( 0.08) 0.20       5.6       61.00
     2        0.55    5.45     3.864  0.40( 0.04) 0.10       0.2       65.10

 RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
 CONFLUENCE FORMULA USED FOR  2 STREAMS.

 ** PEAK FLOW RATE TABLE **
  STREAM       Q      Tc    Intensity   Fp(Fm)      Ap      Ae      HEADWATER
  NUMBER     (CFS)   (MIN.) (INCH/HR) (INCH/HR)           (ACRES)     NODE
     1       11.50    5.45     3.864  0.40( 0.08) 0.19       3.0       65.10
     2       12.20    6.39     3.527  0.40( 0.08) 0.19       3.5       63.10
     3       13.53    8.53     2.988  0.40( 0.08) 0.19       4.7       64.10
     4       13.75    9.11     2.878  0.40( 0.08) 0.19       4.9       62.10
     5       13.66   11.70     2.495  0.40( 0.08) 0.19       5.8       61.00

 COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
 PEAK FLOW RATE(CFS) =       13.75    Tc(MIN.) =      9.11
 EFFECTIVE AREA(ACRES) =        4.92   AREA-AVERAGED Fm(INCH/HR) =  0.08
 AREA-AVERAGED Fp(INCH/HR) =  0.40  AREA-AVERAGED Ap =  0.19
 TOTAL AREA(ACRES) =         5.8
 LONGEST FLOWPATH FROM NODE      61.00 TO NODE      66.00 =      921.00 FEET.

****************************************************************************
 FLOW PROCESS FROM NODE      66.00 TO NODE      67.00 IS CODE =  31
----------------------------------------------------------------------------
 >>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
 >>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<
============================================================================
 ELEVATION DATA: UPSTREAM(FEET) =   146.36  DOWNSTREAM(FEET) =   146.32
 FLOW LENGTH(FEET) =    10.00   MANNING'S N =  0.013
 DEPTH OF FLOW IN  27.0 INCH PIPE IS  17.3 INCHES
 PIPE-FLOW VELOCITY(FEET/SEC.) =   5.12
 ESTIMATED PIPE DIAMETER(INCH) =  27.00    NUMBER OF PIPES =   1
 PIPE-FLOW(CFS) =       13.75
 PIPE TRAVEL TIME(MIN.) =   0.03    Tc(MIN.) =     9.14
 LONGEST FLOWPATH FROM NODE      61.00 TO NODE      67.00 =      931.00 FEET.

****************************************************************************
 FLOW PROCESS FROM NODE      67.00 TO NODE      67.00 IS CODE =   1
----------------------------------------------------------------------------
 >>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
============================================================================
 TOTAL NUMBER OF STREAMS =  2
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM  1 ARE:
 TIME OF CONCENTRATION(MIN.) =     9.14
 RAINFALL INTENSITY(INCH/HR) =    2.87
 AREA-AVERAGED Fm(INCH/HR) =  0.08
 AREA-AVERAGED Fp(INCH/HR) =  0.40
 AREA-AVERAGED Ap =  0.19
 EFFECTIVE STREAM AREA(ACRES) =        4.92
 TOTAL STREAM AREA(ACRES) =        5.79
 PEAK FLOW RATE(CFS) AT CONFLUENCE =       13.75

****************************************************************************
 FLOW PROCESS FROM NODE      66.10 TO NODE      67.00 IS CODE =  21
----------------------------------------------------------------------------
 >>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
 >>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<
============================================================================
 INITIAL SUBAREA FLOW-LENGTH(FEET) =   300.00
 ELEVATION DATA: UPSTREAM(FEET) =    154.00  DOWNSTREAM(FEET) =    151.92

 Tc = K*[(LENGTH** 3.00)/(ELEVATION CHANGE)]**0.20
 SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) =    8.574
 *  10 YEAR RAINFALL INTENSITY(INCH/HR) =  2.980
 SUBAREA Tc AND LOSS RATE DATA(AMC  II):
  DEVELOPMENT TYPE/       SCS SOIL   AREA      Fp          Ap      SCS   Tc
      LAND USE            GROUP   (ACRES)  (INCH/HR)  (DECIMAL)  CN  (MIN.)
 APARTMENTS                 A        0.90      0.40      0.200     32    8.57
 SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) =  0.40
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap =  0.200
 SUBAREA RUNOFF(CFS) =      2.35
 TOTAL AREA(ACRES) =      0.90   PEAK FLOW RATE(CFS) =      2.35

****************************************************************************
 FLOW PROCESS FROM NODE      67.00 TO NODE      67.00 IS CODE =   1
----------------------------------------------------------------------------
 >>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
 >>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<
============================================================================
 TOTAL NUMBER OF STREAMS =  2
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM  2 ARE:
 TIME OF CONCENTRATION(MIN.) =     8.57
 RAINFALL INTENSITY(INCH/HR) =    2.98
 AREA-AVERAGED Fm(INCH/HR) =  0.08
 AREA-AVERAGED Fp(INCH/HR) =  0.40
 AREA-AVERAGED Ap =  0.20
 EFFECTIVE STREAM AREA(ACRES) =        0.90
 TOTAL STREAM AREA(ACRES) =        0.90
 PEAK FLOW RATE(CFS) AT CONFLUENCE =        2.35

 ** CONFLUENCE DATA **
```

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	11.50	5.48	3.850	0.40(0.08)	0.19	3.0	65.10
1	12.20	6.43	3.516	0.40(0.08)	0.19	3.5	63.10
1	13.53	8.57	2.981	0.40(0.08)	0.19	4.7	64.10
1	13.75	9.14	2.872	0.40(0.08)	0.19	4.9	62.10
1	13.66	11.73	2.491	0.40(0.08)	0.19	5.8	61.00
2	2.35	8.57	2.980	0.40(0.08)	0.20	0.9	66.10

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	13.45	5.48	3.850	0.40(0.08)	0.19	3.6	65.10
2	14.28	6.43	3.516	0.40(0.08)	0.19	4.2	63.10
3	15.88	8.57	2.981	0.40(0.08)	0.19	5.6	64.10
4	15.89	8.57	2.980	0.40(0.08)	0.19	5.6	66.10
5	16.01	9.14	2.872	0.40(0.08)	0.19	5.8	62.10
6	15.62	11.73	2.491	0.40(0.08)	0.20	6.7	61.00

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
PEAK FLOW RATE(CFS) = 16.01 Tc(MIN.) = 9.14
EFFECTIVE AREA(ACRES) = 5.82 AREA-AVERAGED Fm(INCH/HR) = 0.08
AREA-AVERAGED Fp(INCH/HR) = 0.40 AREA-AVERAGED Ap = 0.19
TOTAL AREA(ACRES) = 6.7
LONGEST FLOWPATH FROM NODE 61.00 TO NODE 67.00 = 931.00 FEET.

****
FLOW PROCESS FROM NODE 67.00 TO NODE 68.00 IS CODE = 31

>>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<
==========
ELEVATION DATA: UPSTREAM(FEET) = 146.32 DOWNSTREAM(FEET) = 144.80
FLOW LENGTH(FEET) = 463.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 27.0 INCH PIPE IS 21.0 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 4.82
ESTIMATED PIPE DIAMETER(INCH) = 27.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 16.01
PIPE TRAVEL TIME(MIN.) = 1.60 Tc(MIN.) = 10.75
LONGEST FLOWPATH FROM NODE 61.00 TO NODE 68.00 = 1394.00 FEET.

****
FLOW PROCESS FROM NODE 68.00 TO NODE 68.00 IS CODE = 1

>>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
==========
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION(MIN.) = 10.75
RAINFALL INTENSITY(INCH/HR) = 2.62
AREA-AVERAGED Fm(INCH/HR) = 0.08
AREA-AVERAGED Fp(INCH/HR) = 0.40
AREA-AVERAGED Ap = 0.19
EFFECTIVE STREAM AREA(ACRES) = 5.82
TOTAL STREAM AREA(ACRES) = 6.69
PEAK FLOW RATE(CFS) AT CONFLUENCE = 16.01

****
FLOW PROCESS FROM NODE 67.10 TO NODE 68.00 IS CODE = 21

>>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<
==========
INITIAL SUBAREA FLOW-LENGTH(FEET) = 300.00
ELEVATION DATA: UPSTREAM(FEET) = 154.16 DOWNSTREAM(FEET) = 150.50

Tc = K*[(LENGTH** 3.00)/(ELEVATION CHANGE)]**0.20
SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) = 7.185
* 10 YEAR RAINFALL INTENSITY(INCH/HR) = 3.298
SUBAREA Tc AND LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN	Tc (MIN.)
COMMERCIAL	A	0.27	0.40	0.100	32	7.19

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.40
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.100
SUBAREA RUNOFF(CFS) = 0.79
TOTAL AREA(ACRES) = 0.27 PEAK FLOW RATE(CFS) = 0.79

****
FLOW PROCESS FROM NODE 68.00 TO NODE 68.00 IS CODE = 1

>>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
>>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<
==========
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
TIME OF CONCENTRATION(MIN.) = 7.19
RAINFALL INTENSITY(INCH/HR) = 3.30
AREA-AVERAGED Fm(INCH/HR) = 0.04
AREA-AVERAGED Fp(INCH/HR) = 0.40
AREA-AVERAGED Ap = 0.10
EFFECTIVE STREAM AREA(ACRES) = 0.27
TOTAL STREAM AREA(ACRES) = 0.27
PEAK FLOW RATE(CFS) AT CONFLUENCE = 0.79

** CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	13.45	7.12	3.314	0.40(0.08)	0.19	3.6	65.10
1	14.28	8.05	3.090	0.40(0.08)	0.19	4.2	63.10
1	15.88	10.17	2.702	0.40(0.08)	0.19	5.6	64.10
1	15.89	10.18	2.702	0.40(0.08)	0.19	5.6	66.10
1	16.01	10.75	2.619	0.40(0.08)	0.19	5.8	62.10
1	15.62	13.33	2.314	0.40(0.08)	0.20	6.7	61.00
2	0.79	7.19	3.298	0.40(0.04)	0.10	0.3	67.10

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
CONFLUENCE FORMULA USED FOR 2 STREAMS.

```
** PEAK FLOW RATE TABLE **
 STREAM      Q      Tc   Intensity   Fp(Fm)      Ap      Ae      HEADWATER
 NUMBER    (CFS)  (MIN.) (INCH/HR) (INCH/HR)            (ACRES)     NODE
    1      14.24   7.12    3.314  0.40( 0.07) 0.19       3.9       65.10
    2      14.30   7.19    3.298  0.40( 0.07) 0.19       3.9       67.10
    3      15.03   8.05    3.090  0.40( 0.07) 0.19       4.5       63.10
    4      16.53  10.17    2.702  0.40( 0.08) 0.19       5.8       64.10
    5      16.53  10.18    2.702  0.40( 0.08) 0.19       5.8       66.10
    6      16.64  10.75    2.619  0.40( 0.08) 0.19       6.1       62.10
    7      16.17  13.33    2.314  0.40( 0.08) 0.19       7.0       61.00

 COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
 PEAK FLOW RATE(CFS) =      16.64    Tc(MIN.) =    10.75
 EFFECTIVE AREA(ACRES) =       6.09   AREA-AVERAGED Fm(INCH/HR) =  0.08
 AREA-AVERAGED Fp(INCH/HR) =  0.40  AREA-AVERAGED Ap =  0.19
 TOTAL AREA(ACRES) =        7.0
 LONGEST FLOWPATH FROM NODE     61.00 TO NODE     68.00 =    1394.00 FEET.

****************************************************************************
 FLOW PROCESS FROM NODE     68.00 TO NODE     69.00 IS CODE =  31
----------------------------------------------------------------------------
 >>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
 >>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<
============================================================================
 ELEVATION DATA: UPSTREAM(FEET) =   144.80  DOWNSTREAM(FEET) =   143.71
 FLOW LENGTH(FEET) =   118.00   MANNING'S N =  0.013
 DEPTH OF FLOW IN  24.0 INCH PIPE IS  16.3 INCHES
 PIPE-FLOW VELOCITY(FEET/SEC.) =   7.31
 ESTIMATED PIPE DIAMETER(INCH) =  24.00    NUMBER OF PIPES =   1
 PIPE-FLOW(CFS) =      16.64
 PIPE TRAVEL TIME(MIN.) =   0.27    Tc(MIN.) =   11.02
 LONGEST FLOWPATH FROM NODE     61.00 TO NODE     69.00 =    1512.00 FEET.

****************************************************************************
 FLOW PROCESS FROM NODE     69.00 TO NODE     69.00 IS CODE =   1
----------------------------------------------------------------------------
 >>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
============================================================================
 TOTAL NUMBER OF STREAMS =  2
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM  1 ARE:
 TIME OF CONCENTRATION(MIN.) =   11.02
 RAINFALL INTENSITY(INCH/HR) =   2.58
 AREA-AVERAGED Fm(INCH/HR) =  0.08
 AREA-AVERAGED Fp(INCH/HR) =  0.40
 AREA-AVERAGED Ap =  0.19
 EFFECTIVE STREAM AREA(ACRES) =       6.09
 TOTAL STREAM AREA(ACRES) =       6.96
 PEAK FLOW RATE(CFS) AT CONFLUENCE =      16.64

****************************************************************************
 FLOW PROCESS FROM NODE     68.10 TO NODE     68.20 IS CODE =  21
----------------------------------------------------------------------------
 >>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
 >>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<
============================================================================
 INITIAL SUBAREA FLOW-LENGTH(FEET) =   265.00
 ELEVATION DATA: UPSTREAM(FEET) =    154.10  DOWNSTREAM(FEET) =    151.14

 Tc = K*[(LENGTH** 3.00)/(ELEVATION CHANGE)]**0.20
 SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) =    6.959
 *  10 YEAR RAINFALL INTENSITY(INCH/HR) =  3.359
 SUBAREA Tc AND LOSS RATE DATA(AMC  II):
  DEVELOPMENT TYPE/       SCS SOIL   AREA      Fp         Ap      SCS   Tc
      LAND USE            GROUP   (ACRES)  (INCH/HR)  (DECIMAL)  CN  (MIN.)
 COMMERCIAL                 A        0.20      0.40      0.100     32    6.96
 SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) =  0.40
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap =  0.100
 SUBAREA RUNOFF(CFS) =      0.60
 TOTAL AREA(ACRES) =      0.20   PEAK FLOW RATE(CFS) =      0.60

****************************************************************************
 FLOW PROCESS FROM NODE     68.20 TO NODE     69.00 IS CODE =  61
----------------------------------------------------------------------------
 >>>>>COMPUTE STREET FLOW TRAVEL TIME THRU SUBAREA<<<<<
 >>>>>(STANDARD CURB SECTION USED)<<<<<
============================================================================
 UPSTREAM ELEVATION(FEET) =  151.14  DOWNSTREAM ELEVATION(FEET) =  148.80
 STREET LENGTH(FEET) =   185.00   CURB HEIGHT(INCHES) =  6.0
 STREET HALFWIDTH(FEET) = 21.00

 DISTANCE FROM CROWN TO CROSSFALL GRADEBREAK(FEET) =  16.00
 INSIDE STREET CROSSFALL(DECIMAL) =  0.020
 OUTSIDE STREET CROSSFALL(DECIMAL)  =  0.020

 SPECIFIED NUMBER OF HALFSTREETS CARRYING RUNOFF =  1
 STREET PARKWAY CROSSFALL(DECIMAL)  =  0.020
 Manning's FRICTION FACTOR for Streetflow Section(curb-to-curb) =   0.0150
 Manning's FRICTION FACTOR for Back-of-Walk Flow Section =   0.0200

   **TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) =       0.80
   STREETFLOW MODEL RESULTS USING ESTIMATED FLOW:
   STREET FLOW DEPTH(FEET) =  0.23
   HALFSTREET FLOOD WIDTH(FEET) =    5.41
   AVERAGE FLOW VELOCITY(FEET/SEC.) =    1.94
   PRODUCT OF DEPTH&VELOCITY(FT*FT/SEC.) =    0.45
 STREET FLOW TRAVEL TIME(MIN.) =   1.59   Tc(MIN.) =    8.55
 *  10 YEAR RAINFALL INTENSITY(INCH/HR) =  2.985
 SUBAREA LOSS RATE DATA(AMC  II):
  DEVELOPMENT TYPE/       SCS SOIL   AREA      Fp         Ap      SCS
      LAND USE            GROUP   (ACRES)  (INCH/HR)  (DECIMAL)  CN
 COMMERCIAL                 A        0.15      0.40      0.100     32
 SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) =  0.40
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap =  0.100
 SUBAREA AREA(ACRES) =     0.15     SUBAREA RUNOFF(CFS) =     0.40
 EFFECTIVE AREA(ACRES) =      0.35    AREA-AVERAGED Fm(INCH/HR) =  0.04
 AREA-AVERAGED Fp(INCH/HR) =  0.40  AREA-AVERAGED Ap =  0.10
 TOTAL AREA(ACRES) =        0.4        PEAK FLOW RATE(CFS) =        0.93

 END OF SUBAREA STREET FLOW HYDRAULICS:
 DEPTH(FEET) = 0.24   HALFSTREET FLOOD WIDTH(FEET) =   5.91
 FLOW VELOCITY(FEET/SEC.) =  1.99   DEPTH*VELOCITY(FT*FT/SEC.) =   0.49
 LONGEST FLOWPATH FROM NODE     68.10 TO NODE     69.00 =     450.00 FEET.
```

```
****************************************************************************
 FLOW PROCESS FROM NODE     69.00 TO NODE     69.00 IS CODE =  81
----------------------------------------------------------------------------
 >>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<
============================================================================
 MAINLINE Tc(MIN.) =    8.55
 *  10 YEAR RAINFALL INTENSITY(INCH/HR) =  2.985
 SUBAREA LOSS RATE DATA(AMC  II):
  DEVELOPMENT TYPE/       SCS SOIL   AREA      Fp         Ap      SCS
      LAND USE            GROUP   (ACRES)  (INCH/HR)  (DECIMAL)  CN
 APARTMENTS                 A        1.99      0.40      0.200    32
 SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) =  0.40
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap =  0.200
 SUBAREA AREA(ACRES) =     1.99      SUBAREA RUNOFF(CFS) =     5.20
 EFFECTIVE AREA(ACRES) =      2.34   AREA-AVERAGED Fm(INCH/HR) =  0.07
 AREA-AVERAGED Fp(INCH/HR) =  0.40  AREA-AVERAGED Ap =  0.19
 TOTAL AREA(ACRES) =        2.3       PEAK FLOW RATE(CFS) =       6.13

****************************************************************************
 FLOW PROCESS FROM NODE     69.00 TO NODE     69.00 IS CODE =   1
----------------------------------------------------------------------------
 >>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
 >>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<
============================================================================
 TOTAL NUMBER OF STREAMS =  2
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM  2 ARE:
 TIME OF CONCENTRATION(MIN.) =    8.55
 RAINFALL INTENSITY(INCH/HR) =   2.99
 AREA-AVERAGED Fm(INCH/HR) =  0.07
 AREA-AVERAGED Fp(INCH/HR) =  0.40
 AREA-AVERAGED Ap =  0.19
 EFFECTIVE STREAM AREA(ACRES) =       2.34
 TOTAL STREAM AREA(ACRES) =       2.34
 PEAK FLOW RATE(CFS) AT CONFLUENCE =       6.13

 ** CONFLUENCE DATA **
  STREAM       Q      Tc   Intensity   Fp(Fm)      Ap      Ae     HEADWATER
  NUMBER     (CFS)  (MIN.) (INCH/HR) (INCH/HR)          (ACRES)    NODE
     1       14.24    7.41    3.240  0.40( 0.07) 0.19       3.9      65.10
     1       14.30    7.47    3.225  0.40( 0.07) 0.19       3.9      67.10
     1       15.03    8.32    3.031  0.40( 0.07) 0.19       4.5      63.10
     1       16.53   10.44    2.662  0.40( 0.08) 0.19       5.8      64.10
     1       16.53   10.45    2.661  0.40( 0.08) 0.19       5.8      66.10
     1       16.64   11.02    2.582  0.40( 0.08) 0.19       6.1      62.10
     1       16.17   13.60    2.288  0.40( 0.08) 0.19       7.0      61.00
     2        6.13    8.55    2.985  0.40( 0.07) 0.19       2.3      68.10

 RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
 CONFLUENCE FORMULA USED FOR  2 STREAMS.

 ** PEAK FLOW RATE TABLE **
  STREAM       Q      Tc   Intensity   Fp(Fm)      Ap      Ae     HEADWATER
  NUMBER     (CFS)  (MIN.) (INCH/HR) (INCH/HR)          (ACRES)    NODE
     1       20.02    7.41    3.240  0.40( 0.07) 0.19       5.9      65.10
     2       20.10    7.47    3.225  0.40( 0.07) 0.19       5.9      67.10
     3       21.09    8.32    3.031  0.40( 0.07) 0.19       6.7      63.10
     4       21.32    8.55    2.985  0.40( 0.07) 0.19       6.9      68.10
     5       21.98   10.44    2.662  0.40( 0.08) 0.19       8.2      64.10
     6       21.98   10.45    2.661  0.40( 0.08) 0.19       8.2      66.10
     7       21.92   11.02    2.582  0.40( 0.08) 0.19       8.4      62.10
     8       20.83   13.60    2.288  0.40( 0.08) 0.19       9.3      61.00

 COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
 PEAK FLOW RATE(CFS) =      21.98    Tc(MIN.) =    10.45
 EFFECTIVE AREA(ACRES) =       8.18   AREA-AVERAGED Fm(INCH/HR) =  0.08
 AREA-AVERAGED Fp(INCH/HR) =  0.40  AREA-AVERAGED Ap =  0.19
 TOTAL AREA(ACRES) =       9.3
 LONGEST FLOWPATH FROM NODE     61.00 TO NODE     69.00 =    1512.00 FEET.

****************************************************************************
 FLOW PROCESS FROM NODE     69.00 TO NODE     70.00 IS CODE =  31
----------------------------------------------------------------------------
 >>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
 >>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<
============================================================================
 ELEVATION DATA: UPSTREAM(FEET) =   143.71  DOWNSTREAM(FEET) =   142.45
 FLOW LENGTH(FEET) =   180.00   MANNING'S N =  0.013
 DEPTH OF FLOW IN  27.0 INCH PIPE IS  19.9 INCHES
 PIPE-FLOW VELOCITY(FEET/SEC.) =   6.99
 ESTIMATED PIPE DIAMETER(INCH) =  27.00    NUMBER OF PIPES =   1
 PIPE-FLOW(CFS) =      21.98
 PIPE TRAVEL TIME(MIN.) =   0.43    Tc(MIN.) =   10.88
 LONGEST FLOWPATH FROM NODE     61.00 TO NODE     70.00 =    1692.00 FEET.

****************************************************************************
 FLOW PROCESS FROM NODE     70.00 TO NODE     70.00 IS CODE =   1
----------------------------------------------------------------------------
 >>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
============================================================================
 TOTAL NUMBER OF STREAMS =  2
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM  1 ARE:
 TIME OF CONCENTRATION(MIN.) =   10.88
 RAINFALL INTENSITY(INCH/HR) =   2.60
 AREA-AVERAGED Fm(INCH/HR) =  0.08
 AREA-AVERAGED Fp(INCH/HR) =  0.40
 AREA-AVERAGED Ap =  0.19
 EFFECTIVE STREAM AREA(ACRES) =       8.18
 TOTAL STREAM AREA(ACRES) =       9.30
 PEAK FLOW RATE(CFS) AT CONFLUENCE =      21.98

****************************************************************************
 FLOW PROCESS FROM NODE     68.00 TO NODE     70.00 IS CODE =  21
----------------------------------------------------------------------------
 >>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
 >>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<
============================================================================
 INITIAL SUBAREA FLOW-LENGTH(FEET) =   300.00
 ELEVATION DATA: UPSTREAM(FEET) =    150.40  DOWNSTREAM(FEET) =    146.65

 Tc = K*[(LENGTH** 3.00)/(ELEVATION CHANGE)]**0.20
 SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) =    7.151
```

```
*  10 YEAR RAINFALL INTENSITY(INCH/HR) =  3.307
SUBAREA Tc AND LOSS RATE DATA(AMC  II):
 DEVELOPMENT TYPE/       SCS SOIL   AREA      Fp         Ap      SCS   Tc
     LAND USE             GROUP   (ACRES)  (INCH/HR)  (DECIMAL)  CN  (MIN.)
COMMERCIAL                  A        0.36      0.40      0.100    32    7.15
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) =  0.40
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap =  0.100
SUBAREA RUNOFF(CFS) =       1.06
TOTAL AREA(ACRES) =       0.36   PEAK FLOW RATE(CFS) =       1.06

****************************************************************************
 FLOW PROCESS FROM NODE      70.00 TO NODE      70.00 IS CODE =   1
----------------------------------------------------------------------------
 >>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
 >>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<
============================================================================
 TOTAL NUMBER OF STREAMS =  2
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM  2 ARE:
 TIME OF CONCENTRATION(MIN.) =     7.15
 RAINFALL INTENSITY(INCH/HR) =   3.31
 AREA-AVERAGED Fm(INCH/HR) =  0.04
 AREA-AVERAGED Fp(INCH/HR) =  0.40
 AREA-AVERAGED Ap =  0.10
 EFFECTIVE STREAM AREA(ACRES) =       0.36
 TOTAL STREAM AREA(ACRES) =       0.36
 PEAK FLOW RATE(CFS) AT CONFLUENCE =       1.06

 ** CONFLUENCE DATA **
 STREAM       Q       Tc   Intensity   Fp(Fm)      Ap      Ae      HEADWATER
 NUMBER     (CFS)   (MIN.) (INCH/HR) (INCH/HR)           (ACRES)     NODE
    1       20.02    7.85    3.136  0.40( 0.07) 0.19        5.9       65.10
    1       20.10    7.91    3.122  0.40( 0.07) 0.19        5.9       67.10
    1       21.09    8.76    2.945  0.40( 0.07) 0.19        6.7       63.10
    1       21.32    8.98    2.902  0.40( 0.07) 0.19        6.9       68.10
    1       21.98   10.87    2.602  0.40( 0.08) 0.19        8.2       64.10
    1       21.98   10.88    2.601  0.40( 0.08) 0.19        8.2       66.10
    1       21.92   11.44    2.526  0.40( 0.08) 0.19        8.4       62.10
    1       20.83   14.04    2.247  0.40( 0.08) 0.19        9.3       61.00
    2        1.06    7.15    3.307  0.40( 0.04) 0.10        0.4       68.00

 RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
 CONFLUENCE FORMULA USED FOR  2 STREAMS.

 ** PEAK FLOW RATE TABLE **
 STREAM       Q       Tc   Intensity   Fp(Fm)      Ap      Ae      HEADWATER
 NUMBER     (CFS)   (MIN.) (INCH/HR) (INCH/HR)           (ACRES)     NODE
    1       20.32    7.15    3.307  0.40( 0.07) 0.18        5.7       68.00
    2       21.02    7.85    3.136  0.40( 0.07) 0.18        6.2       65.10
    3       21.10    7.91    3.122  0.40( 0.07) 0.18        6.3       67.10
    4       22.03    8.76    2.945  0.40( 0.07) 0.18        7.1       63.10
    5       22.24    8.98    2.902  0.40( 0.07) 0.18        7.3       68.10
    6       22.81   10.87    2.602  0.40( 0.07) 0.18        8.5       64.10
    7       22.81   10.88    2.601  0.40( 0.07) 0.18        8.5       66.10
    8       22.72   11.44    2.526  0.40( 0.07) 0.19        8.8       62.10
    9       21.54   14.04    2.247  0.40( 0.07) 0.19        9.7       61.00

 COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
 PEAK FLOW RATE(CFS) =      22.81    Tc(MIN.) =    10.88
 EFFECTIVE AREA(ACRES) =       8.54   AREA-AVERAGED Fm(INCH/HR) =  0.07
 AREA-AVERAGED Fp(INCH/HR) =  0.40  AREA-AVERAGED Ap =  0.18
 TOTAL AREA(ACRES) =       9.7
 LONGEST FLOWPATH FROM NODE     61.00 TO NODE     70.00 =    1692.00 FEET.

****************************************************************************
 FLOW PROCESS FROM NODE      70.00 TO NODE      71.00 IS CODE =  31
----------------------------------------------------------------------------
 >>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
 >>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<
============================================================================
 ELEVATION DATA: UPSTREAM(FEET) =   142.45  DOWNSTREAM(FEET) =   142.36
 FLOW LENGTH(FEET) =    12.00   MANNING'S N =  0.013
 DEPTH OF FLOW IN  27.0 INCH PIPE IS  20.0 INCHES
 PIPE-FLOW VELOCITY(FEET/SEC.) =   7.24
 ESTIMATED PIPE DIAMETER(INCH) =  27.00    NUMBER OF PIPES =   1
 PIPE-FLOW(CFS) =      22.81
 PIPE TRAVEL TIME(MIN.) =   0.03    Tc(MIN.) =   10.90
 LONGEST FLOWPATH FROM NODE     61.00 TO NODE     71.00 =    1704.00 FEET.

****************************************************************************
 FLOW PROCESS FROM NODE      71.00 TO NODE      71.00 IS CODE =   1
----------------------------------------------------------------------------
 >>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
============================================================================
 TOTAL NUMBER OF STREAMS =  2
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM  1 ARE:
 TIME OF CONCENTRATION(MIN.) =   10.90
 RAINFALL INTENSITY(INCH/HR) =   2.60
 AREA-AVERAGED Fm(INCH/HR) =  0.07
 AREA-AVERAGED Fp(INCH/HR) =  0.40
 AREA-AVERAGED Ap =  0.18
 EFFECTIVE STREAM AREA(ACRES) =       8.54
 TOTAL STREAM AREA(ACRES) =       9.66
 PEAK FLOW RATE(CFS) AT CONFLUENCE =      22.81

****************************************************************************
 FLOW PROCESS FROM NODE      70.10 TO NODE      71.00 IS CODE =  21
----------------------------------------------------------------------------
 >>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
 >>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<
============================================================================
 INITIAL SUBAREA FLOW-LENGTH(FEET) =   232.00
 ELEVATION DATA: UPSTREAM(FEET) =    149.50  DOWNSTREAM(FEET) =    146.60

 Tc = K*[(LENGTH** 3.00)/(ELEVATION CHANGE)]**0.20
 SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) =    6.452
*  10 YEAR RAINFALL INTENSITY(INCH/HR) =  3.508
 SUBAREA Tc AND LOSS RATE DATA(AMC  II):
  DEVELOPMENT TYPE/       SCS SOIL   AREA      Fp         Ap      SCS   Tc
      LAND USE             GROUP   (ACRES)  (INCH/HR)  (DECIMAL)  CN  (MIN.)
 COMMERCIAL                  A        0.77      0.40      0.100    32    6.45
 SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) =  0.40
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap =  0.100
```

```
 SUBAREA RUNOFF(CFS) =      2.40
 TOTAL AREA(ACRES) =      0.77   PEAK FLOW RATE(CFS) =      2.40

****************************************************************************
 FLOW PROCESS FROM NODE     71.00 TO NODE     71.00 IS CODE =   1
----------------------------------------------------------------------------
 >>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
 >>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<
============================================================================
 TOTAL NUMBER OF STREAMS =  2
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM  2 ARE:
 TIME OF CONCENTRATION(MIN.) =    6.45
 RAINFALL INTENSITY(INCH/HR) =   3.51
 AREA-AVERAGED Fm(INCH/HR) =  0.04
 AREA-AVERAGED Fp(INCH/HR) =  0.40
 AREA-AVERAGED Ap =  0.10
 EFFECTIVE STREAM AREA(ACRES) =       0.77
 TOTAL STREAM AREA(ACRES) =       0.77
 PEAK FLOW RATE(CFS) AT CONFLUENCE =       2.40

 ** CONFLUENCE DATA **
  STREAM       Q      Tc   Intensity   Fp(Fm)      Ap      Ae     HEADWATER
  NUMBER     (CFS)  (MIN.) (INCH/HR) (INCH/HR)          (ACRES)     NODE
     1       20.32    7.18    3.300  0.40( 0.07) 0.18       5.7      68.00
     1       21.02    7.87    3.129  0.40( 0.07) 0.18       6.2      65.10
     1       21.10    7.94    3.115  0.40( 0.07) 0.18       6.3      67.10
     1       22.03    8.78    2.939  0.40( 0.07) 0.18       7.1      63.10
     1       22.24    9.01    2.897  0.40( 0.07) 0.18       7.3      68.10
     1       22.81   10.90    2.598  0.40( 0.07) 0.18       8.5      64.10
     1       22.81   10.90    2.597  0.40( 0.07) 0.18       8.5      66.10
     1       22.72   11.47    2.522  0.40( 0.07) 0.19       8.8      62.10
     1       21.54   14.06    2.244  0.40( 0.07) 0.19       9.7      61.00
     2        2.40    6.45    3.508  0.40( 0.04) 0.10       0.8      70.10

 RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
 CONFLUENCE FORMULA USED FOR  2 STREAMS.

 ** PEAK FLOW RATE TABLE **
  STREAM       Q      Tc   Intensity   Fp(Fm)      Ap      Ae     HEADWATER
  NUMBER     (CFS)  (MIN.) (INCH/HR) (INCH/HR)          (ACRES)     NODE
     1       21.85    6.45    3.508  0.40( 0.07) 0.17       5.9      70.10
     2       22.58    7.18    3.300  0.40( 0.07) 0.17       6.5      68.00
     3       23.16    7.87    3.129  0.40( 0.07) 0.17       7.0      65.10
     4       23.23    7.94    3.115  0.40( 0.07) 0.17       7.1      67.10
     5       24.04    8.78    2.939  0.40( 0.07) 0.17       7.9      63.10
     6       24.22    9.01    2.897  0.40( 0.07) 0.17       8.1      68.10
     7       24.58   10.90    2.598  0.40( 0.07) 0.18       9.3      64.10
     8       24.58   10.90    2.597  0.40( 0.07) 0.18       9.3      66.10
     9       24.44   11.47    2.522  0.40( 0.07) 0.18       9.6      62.10
    10       23.07   14.06    2.244  0.40( 0.07) 0.18      10.4      61.00

 COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
 PEAK FLOW RATE(CFS) =      24.58    Tc(MIN.) =    10.90
 EFFECTIVE AREA(ACRES) =       9.30   AREA-AVERAGED Fm(INCH/HR) =  0.07
 AREA-AVERAGED Fp(INCH/HR) =  0.40  AREA-AVERAGED Ap =  0.18
 TOTAL AREA(ACRES) =       10.4
 LONGEST FLOWPATH FROM NODE     61.00 TO NODE     71.00 =    1704.00 FEET.

****************************************************************************
 FLOW PROCESS FROM NODE     71.00 TO NODE     72.00 IS CODE =  31
----------------------------------------------------------------------------
 >>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
 >>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<
============================================================================
 ELEVATION DATA: UPSTREAM(FEET) =   142.36  DOWNSTREAM(FEET) =   141.50
 FLOW LENGTH(FEET) =    55.00   MANNING'S N =  0.013
 DEPTH OF FLOW IN  24.0 INCH PIPE IS  18.1 INCHES
 PIPE-FLOW VELOCITY(FEET/SEC.) =   9.69
 ESTIMATED PIPE DIAMETER(INCH) =  24.00    NUMBER OF PIPES =   1
 PIPE-FLOW(CFS) =      24.58
 PIPE TRAVEL TIME(MIN.) =   0.09    Tc(MIN.) =   10.99
 LONGEST FLOWPATH FROM NODE     61.00 TO NODE     72.00 =    1759.00 FEET.

****************************************************************************
 FLOW PROCESS FROM NODE     72.00 TO NODE     72.00 IS CODE =   1
----------------------------------------------------------------------------
 >>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
============================================================================
 TOTAL NUMBER OF STREAMS =  2
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM  1 ARE:
 TIME OF CONCENTRATION(MIN.) =   10.99
 RAINFALL INTENSITY(INCH/HR) =   2.58
 AREA-AVERAGED Fm(INCH/HR) =  0.07
 AREA-AVERAGED Fp(INCH/HR) =  0.40
 AREA-AVERAGED Ap =  0.18
 EFFECTIVE STREAM AREA(ACRES) =       9.30
 TOTAL STREAM AREA(ACRES) =      10.43
 PEAK FLOW RATE(CFS) AT CONFLUENCE =      24.58

****************************************************************************
 FLOW PROCESS FROM NODE     71.10 TO NODE     71.20 IS CODE =  21
----------------------------------------------------------------------------
 >>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
 >>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<
============================================================================
 INITIAL SUBAREA FLOW-LENGTH(FEET) =   275.00
 ELEVATION DATA: UPSTREAM(FEET) =    150.10  DOWNSTREAM(FEET) =    149.80

 Tc = K*[(LENGTH** 3.00)/(ELEVATION CHANGE)]**0.20
 SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) =   11.247
 *  10 YEAR RAINFALL INTENSITY(INCH/HR) =  2.551
 SUBAREA Tc AND LOSS RATE DATA(AMC  II):
  DEVELOPMENT TYPE/       SCS SOIL   AREA      Fp         Ap      SCS   Tc
      LAND USE             GROUP   (ACRES)  (INCH/HR)  (DECIMAL)  CN  (MIN.)
 COMMERCIAL                  A        0.89      0.40       0.100    32   11.25
 SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) =  0.40
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap =  0.100
 SUBAREA RUNOFF(CFS) =      2.01
 TOTAL AREA(ACRES) =      0.89   PEAK FLOW RATE(CFS) =      2.01

****************************************************************************
 FLOW PROCESS FROM NODE     71.20 TO NODE     71.30 IS CODE =  51
```

```
----------------------------------------------------------------------------
 >>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<<
 >>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<<
============================================================================
 ELEVATION DATA: UPSTREAM(FEET) =    149.80  DOWNSTREAM(FEET) =    147.60
 CHANNEL LENGTH THRU SUBAREA(FEET) =   993.00   CHANNEL SLOPE =  0.0022
 CHANNEL BASE(FEET) =   10.00   "Z" FACTOR =  10.000
 MANNING'S FACTOR = 0.015   MAXIMUM DEPTH(FEET) =   1.00
 *  10 YEAR RAINFALL INTENSITY(INCH/HR) =  1.810
 SUBAREA LOSS RATE DATA(AMC  II):
  DEVELOPMENT TYPE/       SCS SOIL   AREA      Fp         Ap      SCS
      LAND USE             GROUP   (ACRES)  (INCH/HR)  (DECIMAL)  CN
 COMMERCIAL                  A        6.14      0.40      0.100    32
 SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) =  0.40
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap =  0.100
 TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) =       7.02
 TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) =   1.80
 AVERAGE FLOW DEPTH(FEET) =   0.30   TRAVEL TIME(MIN.) =   9.22
 Tc(MIN.) =   20.46
 SUBAREA AREA(ACRES) =     6.14      SUBAREA RUNOFF(CFS) =    9.78
 EFFECTIVE AREA(ACRES) =     7.03    AREA-AVERAGED Fm(INCH/HR) =   0.04
 AREA-AVERAGED Fp(INCH/HR) =   0.40  AREA-AVERAGED Ap =   0.10
 TOTAL AREA(ACRES) =       7.0       PEAK FLOW RATE(CFS) =      11.20

 END OF SUBAREA CHANNEL FLOW HYDRAULICS:
 DEPTH(FEET) =  0.39   FLOW VELOCITY(FEET/SEC.) =   2.09
 LONGEST FLOWPATH FROM NODE     71.10 TO NODE     71.30 =    1268.00 FEET.

****************************************************************************
 FLOW PROCESS FROM NODE     71.30 TO NODE     72.00 IS CODE =  31
----------------------------------------------------------------------------
 >>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
 >>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<
============================================================================
 ELEVATION DATA: UPSTREAM(FEET) =   147.60  DOWNSTREAM(FEET) =   141.50
 FLOW LENGTH(FEET) =    97.00   MANNING'S N =  0.013
 ESTIMATED PIPE DIAMETER(INCH) INCREASED TO 18.000
 DEPTH OF FLOW IN  18.0 INCH PIPE IS   8.4 INCHES
 PIPE-FLOW VELOCITY(FEET/SEC.) =  13.76
 ESTIMATED PIPE DIAMETER(INCH) =  18.00    NUMBER OF PIPES =   1
 PIPE-FLOW(CFS) =      11.20
 PIPE TRAVEL TIME(MIN.) =   0.12    Tc(MIN.) =   20.58
 LONGEST FLOWPATH FROM NODE     71.10 TO NODE     72.00 =    1365.00 FEET.

****************************************************************************
 FLOW PROCESS FROM NODE     72.00 TO NODE     72.00 IS CODE =   1
----------------------------------------------------------------------------
 >>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
 >>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<
============================================================================
 TOTAL NUMBER OF STREAMS =  2
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM  2 ARE:
 TIME OF CONCENTRATION(MIN.) =   20.58
 RAINFALL INTENSITY(INCH/HR) =   1.80
 AREA-AVERAGED Fm(INCH/HR) =  0.04
 AREA-AVERAGED Fp(INCH/HR) =  0.40
 AREA-AVERAGED Ap =  0.10
 EFFECTIVE STREAM AREA(ACRES) =       7.03
 TOTAL STREAM AREA(ACRES) =       7.03
 PEAK FLOW RATE(CFS) AT CONFLUENCE =      11.20

 ** CONFLUENCE DATA **
  STREAM       Q      Tc    Intensity   Fp(Fm)      Ap      Ae      HEADWATER
  NUMBER     (CFS)  (MIN.)  (INCH/HR)  (INCH/HR)           (ACRES)    NODE
     1       21.85    6.55     3.478  0.40( 0.07) 0.17       5.9      70.10
     1       22.58    7.27     3.275  0.40( 0.07) 0.17       6.5      68.00
     1       23.16    7.97     3.108  0.40( 0.07) 0.17       7.0      65.10
     1       23.23    8.03     3.094  0.40( 0.07) 0.17       7.1      67.10
     1       24.04    8.88     2.921  0.40( 0.07) 0.17       7.9      63.10
     1       24.22    9.10     2.880  0.40( 0.07) 0.17       8.1      68.10
     1       24.58   10.99     2.585  0.40( 0.07) 0.18       9.3      64.10
     1       24.58   11.00     2.584  0.40( 0.07) 0.18       9.3      66.10
     1       24.44   11.57     2.511  0.40( 0.07) 0.18       9.6      62.10
     1       23.07   14.16     2.236  0.40( 0.07) 0.18      10.4      61.00
     2       11.20   20.58     1.805  0.40( 0.04) 0.10       7.0      71.10

 RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
 CONFLUENCE FORMULA USED FOR  2 STREAMS.

 ** PEAK FLOW RATE TABLE **
  STREAM       Q      Tc    Intensity   Fp(Fm)      Ap      Ae      HEADWATER
  NUMBER     (CFS)  (MIN.)  (INCH/HR)  (INCH/HR)           (ACRES)    NODE
     1       28.79    6.55     3.478  0.40( 0.06) 0.15       8.2      70.10
     2       29.84    7.27     3.275  0.40( 0.06) 0.15       9.0      68.00
     3       30.71    7.97     3.108  0.40( 0.06) 0.15       9.7      65.10
     4       30.79    8.03     3.094  0.40( 0.06) 0.15       9.8      67.10
     5       31.93    8.88     2.921  0.40( 0.06) 0.15      10.9      63.10
     6       32.20    9.10     2.880  0.40( 0.06) 0.15      11.2      68.10
     7       33.21   10.99     2.585  0.40( 0.06) 0.16      13.1      64.10
     8       33.21   11.00     2.584  0.40( 0.06) 0.16      13.1      66.10
     9       33.26   11.57     2.511  0.40( 0.06) 0.16      13.5      62.10
    10       32.66   14.16     2.236  0.40( 0.06) 0.15      15.3      61.00
    11       29.68   20.58     1.805  0.40( 0.06) 0.15      17.5      71.10

 COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
 PEAK FLOW RATE(CFS) =      33.26    Tc(MIN.) =    11.57
 EFFECTIVE AREA(ACRES) =      13.51   AREA-AVERAGED Fm(INCH/HR) =  0.06
 AREA-AVERAGED Fp(INCH/HR) =  0.40  AREA-AVERAGED Ap =  0.16
 TOTAL AREA(ACRES) =      17.5
 LONGEST FLOWPATH FROM NODE     61.00 TO NODE     72.00 =    1759.00 FEET.

****************************************************************************
 FLOW PROCESS FROM NODE     72.00 TO NODE     73.00 IS CODE =  31
----------------------------------------------------------------------------
 >>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
 >>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<
============================================================================
 ELEVATION DATA: UPSTREAM(FEET) =   141.50  DOWNSTREAM(FEET) =   140.82
 FLOW LENGTH(FEET) =   287.00   MANNING'S N =  0.013
 DEPTH OF FLOW IN  39.0 INCH PIPE IS  28.2 INCHES
 PIPE-FLOW VELOCITY(FEET/SEC.) =   5.18
 ESTIMATED PIPE DIAMETER(INCH) =  39.00    NUMBER OF PIPES =   1
```

```
 PIPE-FLOW(CFS) =      33.26
 PIPE TRAVEL TIME(MIN.) =   0.92    Tc(MIN.) =   12.49
 LONGEST FLOWPATH FROM NODE     61.00 TO NODE     73.00 =    2046.00 FEET.

****************************************************************************
 FLOW PROCESS FROM NODE     73.00 TO NODE     73.00 IS CODE =   1
----------------------------------------------------------------------------
 >>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
============================================================================
 TOTAL NUMBER OF STREAMS =  2
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM  1 ARE:
 TIME OF CONCENTRATION(MIN.) =   12.49
 RAINFALL INTENSITY(INCH/HR) =   2.40
 AREA-AVERAGED Fm(INCH/HR) =  0.06
 AREA-AVERAGED Fp(INCH/HR) =  0.40
 AREA-AVERAGED Ap =  0.16
 EFFECTIVE STREAM AREA(ACRES) =      13.51
 TOTAL STREAM AREA(ACRES) =      17.46
 PEAK FLOW RATE(CFS) AT CONFLUENCE =      33.26

****************************************************************************
 FLOW PROCESS FROM NODE     72.10 TO NODE     73.00 IS CODE =  21
----------------------------------------------------------------------------
 >>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
 >>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<
============================================================================
 INITIAL SUBAREA FLOW-LENGTH(FEET) =   274.00
 ELEVATION DATA: UPSTREAM(FEET) =    147.21  DOWNSTREAM(FEET) =    146.38

 Tc = K*[(LENGTH** 3.00)/(ELEVATION CHANGE)]**0.20
 SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) =    9.156
 *  10 YEAR RAINFALL INTENSITY(INCH/HR) =  2.870
 SUBAREA Tc AND LOSS RATE DATA(AMC  II):
  DEVELOPMENT TYPE/       SCS SOIL   AREA      Fp         Ap      SCS   Tc
      LAND USE             GROUP   (ACRES)  (INCH/HR)  (DECIMAL)  CN  (MIN.)
 COMMERCIAL                  A        0.42      0.40      0.100    32    9.16
 SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) =  0.40
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap =  0.100
 SUBAREA RUNOFF(CFS) =      1.07
 TOTAL AREA(ACRES) =      0.42   PEAK FLOW RATE(CFS) =      1.07

****************************************************************************
 FLOW PROCESS FROM NODE     73.00 TO NODE     73.00 IS CODE =   1
----------------------------------------------------------------------------
 >>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
 >>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<
============================================================================
 TOTAL NUMBER OF STREAMS =  2
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM  2 ARE:
 TIME OF CONCENTRATION(MIN.) =    9.16
 RAINFALL INTENSITY(INCH/HR) =   2.87
 AREA-AVERAGED Fm(INCH/HR) =  0.04
 AREA-AVERAGED Fp(INCH/HR) =  0.40
 AREA-AVERAGED Ap =  0.10
 EFFECTIVE STREAM AREA(ACRES) =       0.42
 TOTAL STREAM AREA(ACRES) =       0.42
 PEAK FLOW RATE(CFS) AT CONFLUENCE =       1.07

** CONFLUENCE DATA **
 STREAM       Q      Tc    Intensity   Fp(Fm)      Ap      Ae      HEADWATER
 NUMBER     (CFS)  (MIN.)  (INCH/HR)  (INCH/HR)           (ACRES)    NODE
    1       28.79    7.51     3.214  0.40( 0.06) 0.15       8.2      70.10
    1       29.84    8.24     3.049  0.40( 0.06) 0.15       9.0      68.00
    1       30.71    8.93     2.911  0.40( 0.06) 0.15       9.7      65.10
    1       30.79    8.99     2.900  0.40( 0.06) 0.15       9.8      67.10
    1       31.93    9.81     2.759  0.40( 0.06) 0.15      10.9      63.10
    1       32.20   10.03     2.724  0.40( 0.06) 0.15      11.2      68.10
    1       33.21   11.92     2.468  0.40( 0.06) 0.16      13.1      64.10
    1       33.21   11.92     2.467  0.40( 0.06) 0.16      13.1      66.10
    1       33.26   12.49     2.402  0.40( 0.06) 0.16      13.5      62.10
    1       32.66   15.09     2.156  0.40( 0.06) 0.15      15.3      61.00
    1       29.68   21.54     1.758  0.40( 0.06) 0.15      17.5      71.10
    2        1.07    9.16     2.870  0.40( 0.04) 0.10       0.4      72.10

 RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
 CONFLUENCE FORMULA USED FOR  2 STREAMS.

** PEAK FLOW RATE TABLE **
 STREAM       Q      Tc    Intensity   Fp(Fm)      Ap      Ae      HEADWATER
 NUMBER     (CFS)  (MIN.)  (INCH/HR)  (INCH/HR)           (ACRES)    NODE
    1       29.78    7.51     3.214  0.40( 0.06) 0.15       8.5      70.10
    2       30.87    8.24     3.049  0.40( 0.06) 0.15       9.4      68.00
    3       31.77    8.93     2.911  0.40( 0.06) 0.15      10.2      65.10
    4       31.86    8.99     2.900  0.40( 0.06) 0.15      10.2      67.10
    5       32.09    9.16     2.870  0.40( 0.06) 0.15      10.5      72.10
    6       32.96    9.81     2.759  0.40( 0.06) 0.15      11.3      63.10
    7       33.21   10.03     2.724  0.40( 0.06) 0.15      11.6      68.10
    8       34.13   11.92     2.468  0.40( 0.06) 0.15      13.5      64.10
    9       34.13   11.92     2.467  0.40( 0.06) 0.15      13.5      66.10
   10       34.15   12.49     2.402  0.40( 0.06) 0.15      13.9      62.10
   11       33.46   15.09     2.156  0.40( 0.06) 0.15      15.7      61.00
   12       30.33   21.54     1.758  0.40( 0.06) 0.15      17.9      71.10

 COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
 PEAK FLOW RATE(CFS) =      34.15    Tc(MIN.) =    12.49
 EFFECTIVE AREA(ACRES) =      13.93   AREA-AVERAGED Fm(INCH/HR) =  0.06
 AREA-AVERAGED Fp(INCH/HR) =  0.40  AREA-AVERAGED Ap =  0.15
 TOTAL AREA(ACRES) =      17.9
 LONGEST FLOWPATH FROM NODE     61.00 TO NODE     73.00 =    2046.00 FEET.

****************************************************************************
 FLOW PROCESS FROM NODE     73.00 TO NODE     95.00 IS CODE =  31
----------------------------------------------------------------------------
 >>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
 >>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<
============================================================================
 ELEVATION DATA: UPSTREAM(FEET) =   140.82  DOWNSTREAM(FEET) =   137.00
 FLOW LENGTH(FEET) =   115.00   MANNING'S N =  0.013
 DEPTH OF FLOW IN  24.0 INCH PIPE IS  17.4 INCHES
 PIPE-FLOW VELOCITY(FEET/SEC.) =  14.03
 ESTIMATED PIPE DIAMETER(INCH) =  24.00    NUMBER OF PIPES =   1
 PIPE-FLOW(CFS) =      34.15
```

```
 PIPE TRAVEL TIME(MIN.) =   0.14    Tc(MIN.) =   12.63
 LONGEST FLOWPATH FROM NODE     61.00 TO NODE     95.00 =    2161.00 FEET.

****************************************************************************
 FLOW PROCESS FROM NODE     95.00 TO NODE     95.00 IS CODE =   1
----------------------------------------------------------------------------
 >>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
============================================================================
 TOTAL NUMBER OF STREAMS =  2
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM  1 ARE:
 TIME OF CONCENTRATION(MIN.) =   12.63
 RAINFALL INTENSITY(INCH/HR) =   2.39
 AREA-AVERAGED Fm(INCH/HR) =  0.06
 AREA-AVERAGED Fp(INCH/HR) =  0.40
 AREA-AVERAGED Ap =  0.15
 EFFECTIVE STREAM AREA(ACRES) =      13.93
 TOTAL STREAM AREA(ACRES) =      17.88
 PEAK FLOW RATE(CFS) AT CONFLUENCE =      34.15

****************************************************************************
 FLOW PROCESS FROM NODE     91.00 TO NODE     92.00 IS CODE =  21
----------------------------------------------------------------------------
 >>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
 >>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<
============================================================================
 INITIAL SUBAREA FLOW-LENGTH(FEET) =   300.00
 ELEVATION DATA: UPSTREAM(FEET) =    154.00  DOWNSTREAM(FEET) =    151.50

 Tc = K*[(LENGTH** 3.00)/(ELEVATION CHANGE)]**0.20
 SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) =    8.265
 *  10 YEAR RAINFALL INTENSITY(INCH/HR) =  3.044
 SUBAREA Tc AND LOSS RATE DATA(AMC  II):
  DEVELOPMENT TYPE/       SCS SOIL   AREA      Fp         Ap      SCS   Tc
      LAND USE             GROUP   (ACRES)  (INCH/HR)  (DECIMAL)  CN  (MIN.)
 APARTMENTS                  A       1.27      0.40      0.200    32    8.26
 SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) =  0.40
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap =  0.200
 SUBAREA RUNOFF(CFS) =      3.39
 TOTAL AREA(ACRES) =      1.27   PEAK FLOW RATE(CFS) =      3.39

****************************************************************************
 FLOW PROCESS FROM NODE     92.00 TO NODE     93.00 IS CODE =  51
----------------------------------------------------------------------------
 >>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<<
 >>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<<
============================================================================
 ELEVATION DATA: UPSTREAM(FEET) =    151.50  DOWNSTREAM(FEET) =    149.10
 CHANNEL LENGTH THRU SUBAREA(FEET) =   256.00   CHANNEL SLOPE =  0.0094
 CHANNEL BASE(FEET) =   10.00   "Z" FACTOR =  10.000
 MANNING'S FACTOR = 0.015   MAXIMUM DEPTH(FEET) =   1.00
 *  10 YEAR RAINFALL INTENSITY(INCH/HR) =  2.766
 SUBAREA LOSS RATE DATA(AMC  II):
  DEVELOPMENT TYPE/       SCS SOIL   AREA      Fp         Ap      SCS
      LAND USE             GROUP   (ACRES)  (INCH/HR)  (DECIMAL)  CN
 APARTMENTS                  A       2.19      0.40      0.200    32
 SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) =  0.40
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap =  0.200
 TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) =      6.03
 TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) =   2.84
 AVERAGE FLOW DEPTH(FEET) =   0.18   TRAVEL TIME(MIN.) =   1.50
 Tc(MIN.) =    9.77
 SUBAREA AREA(ACRES) =     2.19      SUBAREA RUNOFF(CFS) =    5.29
 EFFECTIVE AREA(ACRES) =     3.46    AREA-AVERAGED Fm(INCH/HR) =   0.08
 AREA-AVERAGED Fp(INCH/HR) =   0.40  AREA-AVERAGED Ap =   0.20
 TOTAL AREA(ACRES) =      3.5        PEAK FLOW RATE(CFS) =      8.36

 END OF SUBAREA CHANNEL FLOW HYDRAULICS:
 DEPTH(FEET) =  0.22   FLOW VELOCITY(FEET/SEC.) =   3.14
 LONGEST FLOWPATH FROM NODE     91.00 TO NODE     93.00 =     556.00 FEET.

****************************************************************************
 FLOW PROCESS FROM NODE     93.00 TO NODE     94.00 IS CODE =  51
----------------------------------------------------------------------------
 >>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<<
 >>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<<
============================================================================
 ELEVATION DATA: UPSTREAM(FEET) =    149.10  DOWNSTREAM(FEET) =    148.00
 CHANNEL LENGTH THRU SUBAREA(FEET) =   180.00   CHANNEL SLOPE =  0.0061
 CHANNEL BASE(FEET) =   10.00   "Z" FACTOR =  10.000
 MANNING'S FACTOR = 0.015   MAXIMUM DEPTH(FEET) =   1.00
 *  10 YEAR RAINFALL INTENSITY(INCH/HR) =  2.610
 SUBAREA LOSS RATE DATA(AMC  II):
  DEVELOPMENT TYPE/       SCS SOIL   AREA      Fp         Ap      SCS
      LAND USE             GROUP   (ACRES)  (INCH/HR)  (DECIMAL)  CN
 APARTMENTS                  A       1.70      0.40      0.200    32
 SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) =  0.40
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap =  0.200
 TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) =      10.30
 TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) =   2.88
 AVERAGE FLOW DEPTH(FEET) =   0.28   TRAVEL TIME(MIN.) =   1.04
 Tc(MIN.) =   10.81
 SUBAREA AREA(ACRES) =     1.70      SUBAREA RUNOFF(CFS) =    3.87
 EFFECTIVE AREA(ACRES) =     5.16    AREA-AVERAGED Fm(INCH/HR) =   0.08
 AREA-AVERAGED Fp(INCH/HR) =   0.40  AREA-AVERAGED Ap =   0.20
 TOTAL AREA(ACRES) =      5.2        PEAK FLOW RATE(CFS) =      11.75

 END OF SUBAREA CHANNEL FLOW HYDRAULICS:
 DEPTH(FEET) =  0.30   FLOW VELOCITY(FEET/SEC.) =   3.01
 LONGEST FLOWPATH FROM NODE     91.00 TO NODE     94.00 =     736.00 FEET.

****************************************************************************
 FLOW PROCESS FROM NODE     94.00 TO NODE     95.00 IS CODE =  31
----------------------------------------------------------------------------
 >>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
 >>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<
============================================================================
 ELEVATION DATA: UPSTREAM(FEET) =   148.00  DOWNSTREAM(FEET) =   137.00
 FLOW LENGTH(FEET) =   130.00   MANNING'S N =  0.013
 ESTIMATED PIPE DIAMETER(INCH) INCREASED TO 18.000
 DEPTH OF FLOW IN  18.0 INCH PIPE IS    8.0 INCHES
 PIPE-FLOW VELOCITY(FEET/SEC.) =  15.54
 ESTIMATED PIPE DIAMETER(INCH) =  18.00    NUMBER OF PIPES =   1
```

```
 PIPE-FLOW(CFS) =      11.75
 PIPE TRAVEL TIME(MIN.) =   0.14    Tc(MIN.) =   10.95
 LONGEST FLOWPATH FROM NODE     91.00 TO NODE     95.00 =     866.00 FEET.

****************************************************************************
 FLOW PROCESS FROM NODE     95.00 TO NODE     95.00 IS CODE =   1
----------------------------------------------------------------------------
 >>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
 >>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<
============================================================================
 TOTAL NUMBER OF STREAMS =  2
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM  2 ARE:
 TIME OF CONCENTRATION(MIN.) =   10.95
 RAINFALL INTENSITY(INCH/HR) =   2.59
 AREA-AVERAGED Fm(INCH/HR) =  0.08
 AREA-AVERAGED Fp(INCH/HR) =  0.40
 AREA-AVERAGED Ap =  0.20
 EFFECTIVE STREAM AREA(ACRES) =       5.16
 TOTAL STREAM AREA(ACRES) =       5.16
 PEAK FLOW RATE(CFS) AT CONFLUENCE =      11.75

 ** CONFLUENCE DATA **
 STREAM       Q      Tc    Intensity   Fp(Fm)      Ap      Ae      HEADWATER
 NUMBER     (CFS)  (MIN.) (INCH/HR) (INCH/HR)          (ACRES)     NODE
    1       29.78    7.65    3.181  0.40( 0.06) 0.15       8.5       70.10
    1       30.87    8.38    3.020  0.40( 0.06) 0.15       9.4       68.00
    1       31.77    9.07    2.886  0.40( 0.06) 0.15      10.2       65.10
    1       31.86    9.13    2.875  0.40( 0.06) 0.15      10.2       67.10
    1       32.09    9.29    2.846  0.40( 0.06) 0.15      10.5       72.10
    1       32.96    9.95    2.737  0.40( 0.06) 0.15      11.3       63.10
    1       33.21   10.17    2.703  0.40( 0.06) 0.15      11.6       68.10
    1       34.13   12.05    2.452  0.40( 0.06) 0.15      13.5       64.10
    1       34.13   12.06    2.451  0.40( 0.06) 0.15      13.5       66.10
    1       34.15   12.63    2.387  0.40( 0.06) 0.15      13.9       62.10
    1       33.46   15.22    2.145  0.40( 0.06) 0.15      15.7       61.00
    1       30.33   21.68    1.751  0.40( 0.06) 0.15      17.9       71.10
    2       11.75   10.95    2.591  0.40( 0.08) 0.20       5.2       91.00

 RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
 CONFLUENCE FORMULA USED FOR  2 STREAMS.

 ** PEAK FLOW RATE TABLE **
 STREAM       Q      Tc    Intensity   Fp(Fm)      Ap      Ae      HEADWATER
 NUMBER     (CFS)  (MIN.) (INCH/HR) (INCH/HR)          (ACRES)     NODE
    1       39.92    7.65    3.181  0.40( 0.07) 0.16      12.1       70.10
    2       41.39    8.38    3.020  0.40( 0.07) 0.16      13.3       68.00
    3       42.64    9.07    2.886  0.40( 0.07) 0.16      14.4       65.10
    4       42.76    9.13    2.875  0.40( 0.07) 0.16      14.5       67.10
    5       43.07    9.29    2.846  0.40( 0.07) 0.16      14.8       72.10
    6       44.25    9.95    2.737  0.40( 0.07) 0.17      16.0       63.10
    7       44.61   10.17    2.703  0.40( 0.07) 0.17      16.4       68.10
    8       45.34   10.95    2.591  0.40( 0.07) 0.17      17.5       91.00
    9       45.23   12.05    2.452  0.40( 0.07) 0.17      18.6       64.10
   10       45.23   12.06    2.451  0.40( 0.07) 0.17      18.6       66.10
   11       44.95   12.63    2.387  0.40( 0.07) 0.17      19.1       62.10
   12       43.13   15.22    2.145  0.40( 0.07) 0.16      20.8       61.00
   13       38.15   21.68    1.751  0.40( 0.06) 0.16      23.0       71.10

 COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
 PEAK FLOW RATE(CFS) =      45.34    Tc(MIN.) =   10.95
 EFFECTIVE AREA(ACRES) =      17.54   AREA-AVERAGED Fm(INCH/HR) =  0.07
 AREA-AVERAGED Fp(INCH/HR) =  0.40  AREA-AVERAGED Ap =  0.17
 TOTAL AREA(ACRES) =      23.0
 LONGEST FLOWPATH FROM NODE     61.00 TO NODE     95.00 =    2161.00 FEET.
============================================================================
 END OF STUDY SUMMARY:
 TOTAL AREA(ACRES)      =       23.0  TC(MIN.) =     10.95
 EFFECTIVE AREA(ACRES) =     17.54  AREA-AVERAGED Fm(INCH/HR)=  0.07
 AREA-AVERAGED Fp(INCH/HR) =  0.40  AREA-AVERAGED Ap = 0.167
 PEAK FLOW RATE(CFS)    =      45.34

 ** PEAK FLOW RATE TABLE **
 STREAM       Q      Tc    Intensity   Fp(Fm)      Ap      Ae      HEADWATER
 NUMBER     (CFS)  (MIN.) (INCH/HR) (INCH/HR)          (ACRES)     NODE
    1       39.92    7.65    3.181  0.40( 0.07) 0.16      12.1       70.10
    2       41.39    8.38    3.020  0.40( 0.07) 0.16      13.3       68.00
    3       42.64    9.07    2.886  0.40( 0.07) 0.16      14.4       65.10
    4       42.76    9.13    2.875  0.40( 0.07) 0.16      14.5       67.10
    5       43.07    9.29    2.846  0.40( 0.07) 0.16      14.8       72.10
    6       44.25    9.95    2.737  0.40( 0.07) 0.17      16.0       63.10
    7       44.61   10.17    2.703  0.40( 0.07) 0.17      16.4       68.10
    8       45.34   10.95    2.591  0.40( 0.07) 0.17      17.5       91.00
    9       45.23   12.05    2.452  0.40( 0.07) 0.17      18.6       64.10
   10       45.23   12.06    2.451  0.40( 0.07) 0.17      18.6       66.10
   11       44.95   12.63    2.387  0.40( 0.07) 0.17      19.1       62.10
   12       43.13   15.22    2.145  0.40( 0.07) 0.16      20.8       61.00
   13       38.15   21.68    1.751  0.40( 0.06) 0.16      23.0       71.10
============================================================================
============================================================================
 END OF RATIONAL METHOD ANALYSIS
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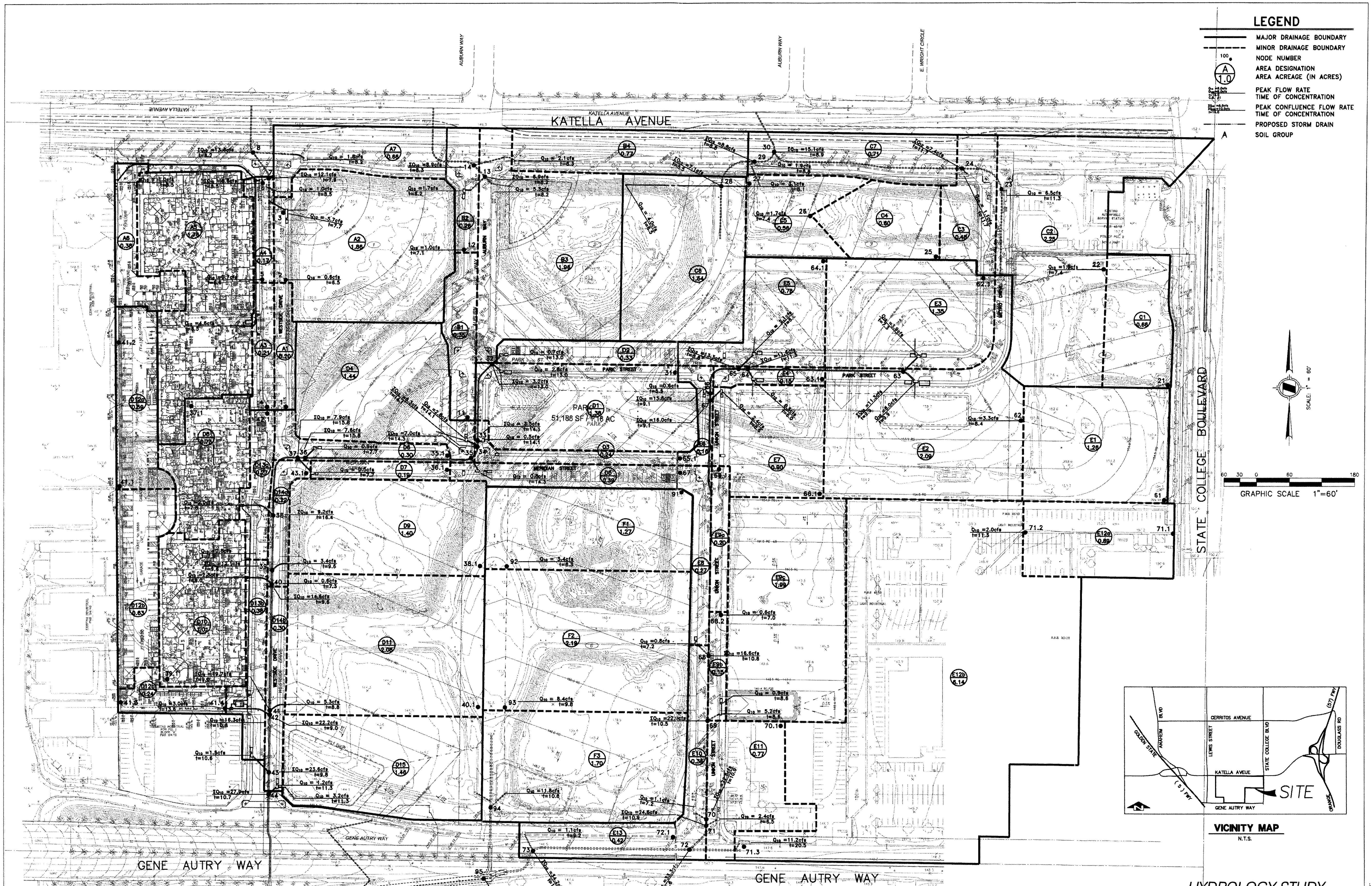
LEGEND
MAJOR DRAINAGE BOUNDARY
MINOR DRAINAGE BOUNDARY
NODE NUMBER
AREA DESIGNATION
AREA ACREAGE (IN ACRES)
PEAK FLOW RATE
TIME OF CONCENTRATION
PEAK CONFLUENCE FLOW RATE
TIME OF CONCENTRATION
PROPOSED STORM DRAIN
SOIL GROUP
KATELLA AVENUE
AUBURN WAY
E. WRIGHT CIRCLE
PARK STREET
MERIDIAN STREET
UNION STREET
WESTSIDE DRIVE
STATE COLLEGE BOULEVARD
GENE AUTRY WAY
PARK
51,188 SF / 1.16 AC
GRAPHIC SCALE 1"=60'
SCALE: 1" = 60'
VICINITY MAP
N.T.S.
CERRITOS AVENUE
KATELLA AVENUE
LEWIS STREET
STATE COLLEGE BLVD
DOUGLASS RD
ANAHEIM BLVD
GOLDEN STATE
(5) FWY
(57) FWY
ORANGE
GENE AUTRY WAY
SITE

PREPARED BY:

HUNSAKER & ASSOCIATES
IRVINE, INC.
PLANNING ■ ENGINEERING ■ SURVEYING
Three Hughes • Irvine, CA 92618 • PH: (949) 583-1010 • FX: (949) 583-0759

PREPARED FOR:

PT METRO, LLC
25 Enterprise, 3rd FLOOR
Aliso Viejo, CA 92656
(949) 349-8155

HYDROLOGY STUDY FOR A-TOWN TRACTS 17703 & 16859 PROPOSED CONDITION

WO: 3916-8

PLOTTED BY: Tu Trinh DATE: Apr. 15, 2015 03:29:33 PM FILE: F:\0108\Engineering\SY_HYDROLOGY\A-TOWN Metro\2015-03-10\HYDRO MAP FOR 17703.dwg

APR 2 3 2015